

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 21050070-A

Cedar 2 A

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: I46242483 thru I46242491

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 21,2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Cedar 2 A	
21050070-A	A	Roof Special	7	1	Job Reference (optional)	146242483

Run: 8.5 S 0 May 17 2021 Print: 8.500 S May 17 2021 MiTek Industries, Inc. Fri May 21 12:04:02

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NOTES

WEBS

TCDL

BCLL

BCDL

WEBS

WEBS

Unbalanced roof live loads have been considered for 1) this design.

> 🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITeK decomectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



S minin May 21,2021

Job	Truss	Truss Type	Qty	Ply	Cedar 2 A	
21050070-A	AA	Common	10	1	Job Reference (optional)	146242484

1)

2)

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED WITHS KRETERENCE PAGE with-74/3 fev. on parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type Qty Ply Cedar 2		Cedar 2 A		
21050070-A	AA1	Common Structural Gable	1	1	Job Reference (optional)	146242485

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:05 Page: 1

					ID	ZtGoWmR7le	SGT3rTX	gJ2hAykgjD	-RfC?PsB70	Hq3NSgP	qnL8w3ı	uITXbGł	WrCDoi7J4z	JC?f
	-0-10-8 0-10-8	<u>5-10-3</u> 5-10-3		<u>11-4-13</u> 5-6-11		<u>16-11-8</u> 5-6-11		22-6-3 5-6-11		<u>28-0-</u> 5-6-1	<u>13</u> 1	+	<u>33-11-0</u> 5-10-3	34-9-8 0-10-8
12-2-3 12-0-13 0-9-2	1 2	3	2x4 v 4	8 ¹²	5	29 1	4x5= 7 8 17	¹⁰ 16 ¹⁵ 30	89	31 14		2x4 ø 10	11	12 13
	3x6 II						3x8=							3х6 и
1	⊢	8- 8-	7-8 7-8		16 8	-11-8 -4-0	18-1- 1-2-	12 4	25-3-8 7-1-12			33- 8-7	11-0 7-8	
(X, Y): [2:0-3-15,0-0-	-1], [12:0-	3-15,0-0-1]												
(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	Spaci Plate Lumbo Rep S Code	i ng Grip DOL er DOL Stress Incr	2-0-0 1.15 1.15 YES IRC201	5/TPI2014	CSI TC BC WB Mat	rix-MSH	0.38 0.55 0.36	DEFL Vert(LL) Vert(CT) Horz(CT	in -0.16) -0.22) 0.02	(loc) 17-19 17-19 2	I/defl >620 >447 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 20	GRIP 244/190 D6 lb FT = 20%
2x4 SP No.2 2x4 SP No.3 *Exce 2x4 SP No.3 *Exce 2x4 SPF No.2(flat) Left 2x4 SP No.3 2-6-0 Structural wood sh 6-0-0 oc purlins. Rigid ceiling direct bracing. T-Brace: Fasten (2X) T and of web with 10d (0 o.c.,with 3in minim Brace must cover (size) 2=18-3-1 17=18-3 Max Horiz 2=-235 (Max Uplift 2=-4 (LC	ept* 17-7:: - 2-6-0, R - 2-6-0, R - 2x4 SP 9-17, 5- Il braces: .131*x3") um end d : 90% of v 8, 12=0-3 :-8, 19=18 [LC 11), 2 LC 11), 12=	2x4 SP No.2 ight 2x4 SP lirectly applie l or 10-0-0 or F No.2 - 7-1 -17, 5-19 to narrow ee nails, 6in listance. web length. -8, 16=0-3-6 8-3-8, 20=18 8-3-5 (LC -17 (LC 14)	No.3 ed or c 3) 7, dge 4) 3, 5) 3, 5) 3, 6) 3-11)	OTES Unbalance this design Wind: ASC Vasd=103r Cat. II; Exp Exterior (2; vertical left forces & M DOL=1.60 TCLL: ASC DOL=1.15 snow); Pf= Plate DOL Ct=1.10 This truss load of 12. overhangs All plates a * This truss on the bott 3-06-00 tal chord and	d roof li F 7-10; mph; TC B; Enc J 20ne; and rig WFRS plate gr 2E 7-10 Plate D 13.9 ps =1.15); mas bee 0 psf or non-co re 3x5 l s has be b om cho l by 2-0	ve loads hav Vult=130mp DL=6.0psf; dosed; MWF cantilever lef ht exposed; (for reactions ip DOL=1.33; Pr=20.0 psi OL=1.15); P f (flat roof sn Category II; n designed f 2.00 times fit mcurrent with MT20 unless ten designed of in all area 0-00 wide wit	re been of BCDL=6 RS (env t and rig C-C for n shown; f (roof liv g=20.0 p ow: Lurr Exp B; F for great at roof liv o ther liv o there liv t o there viv f for a liv s otherevi s where	considered cond gust) .0psf; h=2 elope) and ht expose nembers a Lumber e load: Lu ssf (ground ber DOL= ully Exp.; er of min r bad of 13.9 ve loads. se indicate e load of 2 a rectangl veen the b DL = 10.0	d for 25ft; d C-C d; end and mber d t:1.15 oof live 9 psf on ed. 20.0psf le wottom lnsf					
Max Grav 2=506 (l 16=126 19=481 (lb) - Maximum Co Tension 1-2=0/35, 2-4=-40(5-7=-176/190, 7-9	(LC 14), 2 LC 29), 12 (LC 20), 1 (LC 20), 7 (LC 25), 2 mpressio D/133, 4-5 =-176/187	20=-4 (LC 13) 2=693 (LC 2 17=1169 (LC 20=506 (LC n/Maximum 5=-340/170, 7, 9-10=-608	,, 3) 7) 2), 226), 29) 8) 5/206, 9)	chord and One RT7A truss to be 12. This cc consider la This truss i Internation R802.10.2 See Stand	any oth MiTek aring wa nnectio teral for s desig al Resic and ref ard Indu	er members, connectors r alls due to Ul n is for uplift rces. ned in accorr dential Code erenced star ustry Piggyba	with BC ecomme PLIFT at only and dance w sections ndard AN ack Trus	EL = 10.0 inded to ca i jt(s) 2, 17 d does not ith the 201 i R502.11. ISI/TPI 1. s Connect	ipsf. onnect /, and 15 1 and ion		ann an the	Ľ	ORTH S 04	EAL 4925
	(X, Y): [2:0-3-15,0-0 (psf) 20.0 13.9/20.0 13.9/20.0 13.9/20.0 13.9/20.0 10.0 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *Exce 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *Exce 2x4 SP No.2 2x4 SP No.3 *Exce 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *Exce 2x4 SP No.2 2x4 SP	-0-10-8 0-10-1 0-10-8 0-10-8 0-10-8 0-10-1 0-10-8 0-10-8 0-10-8 0-10-8 0-10-1 0-10-8 0-10-1 0-10-8 0-10-1 0-10-8 0-10-1 0-10	$\begin{array}{c} -0-10-8 & 5-10-3 \\ 0-10-8 & 5-10-3 \\ \hline 0-10-9 & 5-10-3 \\ \hline $	$\begin{array}{c} \begin{array}{c} -0.10-8 & 5.10-3 \\ 0.10-8 & 5.10-3 \end{array}$	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	$\begin{array}{c} 0.10-8 & 5-10-3 \\ 0.10-8 & 5-10-3 & 5-6-11 \\ \hline \\ 0.10-10-8 & 5-10-3 & 5-6-11 \\ \hline \\ 0.10-10-10-10-10-10-10 & 10-20 & 0 \\ \hline \\ 0.10-10-10-10-10-10-10 & 0.15 & 0.15 \\ 0.10-10-10-10-10-10 & 0.15 & 0.15 \\ 0.10-10-10-10-10-10-10-10 & 0.15 \\ 0.10-10-10-10-10-10-10-10-10-10-10-10-10-1$	0-10-8 5-10-3 114-13 16-11-8 0-10-8 5-10-3 5-6-11 5-6-11 1 0-10-8 5-0-11 5-6-11 1 0-10-8 5-0-11 5-6-11 1 0-10-8 5-0-11 5-6-11 1 0-10-8 5-0-11 5-6-11 1 0-10-8 5-0-11 5-0-11 1 0-10-8 5-0-11 5-0-11 1 0-10-1 19 28 29 3 1 19 28 29 1 3 3-0 19 28 29 1 3 3-7-8 16-11-8 5-0 1 1 1 19 28 29 1 3 3 1	0-10-8 5-10-3 114-13 16-11-8 0-10-8 5-10-3 5-6-11 5-6-11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ID:2IG6WIRR/INSCITATEXQL2MAYQED -0-10-8 5-10-3 114-13 16-11-8 224-32 0-10-8 5-10-3 5-6-11 5-6-11 5-6-11 10 20 10 20 10 4:5 = 11 2 20 18 17 16 10 20 10 20 18 17 16 10 20 18 17 16 10 20 18 17 16 10 20 10 20 18 17 16 10 20 18 17 16 10 20 18 17 16 10 20 10 20 10 20 10 20 10 20 10 20 10	10-21GoWmR7#63GTarTxgJzhv/egiD-REC7#6570 -0-10-8 5-10-3 114-13 16-11-8 22-6-3 -0-10-8 5-10-3 5-6-11 5-6-11 5-6-11 -0-10-8 5-6-11 5-6-11 4-55 -0-10-8 5-6-11 5-6-11 4-55 -0-10-8 5-6-11 5-6-11 4-55 -0-10-8 -0-10-8 -0-10-8 -0-10-8 -0-10-9 -0-10-9 -0-10-9 -0-10-9 -0-10-9 -0-10-9 -0-10-9 -0-10-9 -0-10-9 -0-10-9 -0-10-9 -0-10-9	10 - 10 - 10 - 10 - 10 - 10 - 10 - 10 -	$\frac{10,10,4}{0,10,4} = 5-10-3$ $\frac{114,413}{5-611} = \frac{116+1.8}{5-6-11} = \frac{22.6-3}{5-6-11} = \frac{22.6-13}{5-6-11} = \frac{22.6-11}{5-6-11} = \frac{22.6-1}{5-6-11} = \frac{22.6-1}{5-6-1} = \frac{22.6-1}{5-6-1}$	$ \begin{array}{c} 10.20 \text{ dm} \text{RPRBGT3Tb} (20,4) \text{RCTPBT0HeSMB2Prul AsAUTOGP } \\ \begin{array}{c} 0.1013 \\ 0.1019 \\ 0.1019 \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \begin{array}{c} 0.1019 \\ 0.1019 \\ \hline \end{array} \\ \hline \bigg $ \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \\ \hline \Biggl \Biggl \Biggl \Biggl \Biggl \\ \hline \end{array} \\ \hline \Biggl \Biggl \\ \hline \end{array} \hline \Biggl \Biggl \Biggl \\ \hline \bigg \hline \Biggl \\ \hline \end{array} \\ \hline \end{array} \\ \hline \end{array} \hline \end{array} \\ \hline \Biggl \Biggl \Biggl \\ \Biggl \Biggl \Biggl \\ \hline \Biggl \Biggl \Biggl \\ \Biggl \Biggl \Biggl \\ \hline \Biggl \hline \Biggl \hline \Biggl \\ \Biggl \Biggl \Biggl \Biggl \Biggl \\ \Biggl \Biggl \Biggl \Biggl \Biggl \Biggl \Biggl	$\begin{array}{c c c c c c c c c c c c c c c c c c c $

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Job	Truss	Truss Type	Qty	Ply	_ Cedar 2 A	
21050070-A	AA2	Common	2	1	Job Reference (optional)	146242486

TCDL

BCLL

BCDL

WEBS

SLIDER

WEBS

WEBS

NOTES

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May 21,2021

Job	Truss	Truss Type	Qty	Ply	Cedar 2 A	
21050070-A	AA3	Common Structural Gable	1	1	Job Reference (optional)	146242487

Loading

TCDL

BCLL

BCDL

WEBS

OTHERS

SLIDER

BRACING

WEBS

WEBS

JOINTS

LUMBER

TCLL (roof)

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Job	Truss	Truss Type	Qty	Ply	·Cedar 2 A	
21050070-A	AA3	Common Structural Gable	1	1	Job Reference (optional)	146242487

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) N/A
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 58 lb uplift at joint 35.
- One RT7A MiTek connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 36, 24, 32, 33, 37, and 39. This connection is for uplift only and does not consider lateral forces.
- 12) One RT16A MiTek connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 29. This connection is for uplift only and does not consider lateral forces.
- 13) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 15) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply Cedar 2 A		
21050070-A	AE	Common Supported Gable	1	1	Job Reference (optional)	146242488

Run: 8.5 S 0 May 17 2021 Print: 8.500 S May 17 2021 MiTek Industries, Inc. Fri May 21 12:04:08 ID:6ZIBtiWzpxMW0GiKuUURSIznq5n-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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Scale = 1:72.2 Plate Offsets (X, Y): [30:0-3-0,0-3-0]

		1						
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11-4 1.15 1.15 YES IRC2015/TPI2014	CSI TC 0.12 BC 0.19 WB 0.12 Matrix-MSH	DEFL in Vert(LL) -0.01 Vert(CT) -0.03 Horz(CT) 0.02	(loc) I/defl I 30-31 >999 2 31-33 >999 1 22 n/a r	L/d PLATES GRIP MT20 244/190 m/a Weight: 267 lb FT = 20%	
LUMBER TOP CHORD BOT CHORD OTHERS SLIDER BRACING TOP CHORD BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *Ext SP No.2, 0-0,0-0, Left 2x4 SP No.3 2-6-0 Structural wood s 6-0-0 oc purlins. Rigid ceiling direct bracing. T-Brace:	ept* 31-12,33-11,30-1 0-0,0-0:2x4 SPF No.2(1-1-7, Right 2x4 SP heathing directly appli ty applied or 10-0-0 o 2x4 SPF No.2 - 11- 10-34, 13-30, 14-2(3:2x4 (flat) No.3 ed or FORCES ic TOP CHORD -33, 9	$\begin{array}{llllllllllllllllllllllllllllllllllll$	2=328 (LC 2), 25=149 (LC 2), 27=176 (LC 26), 9=308 (LC 26), 35=84 (LC 25), 37=157 (LC 25), 39=165 (LC 25), 1=360 (LC 27), on/Maximum =-336/63, -7=-301/0, 0-11=-305/62, 1/111,	 Wind: ASCE 7 Vasd=103mph Cat. II; Exp B; Exterior (2) zo vertical left ann forces & MWF DOL=1.60 plat Truss designe only. For stud see Standard or consult qua TCLL: ASCE 7 DOL=1.15 Pla snow); Pf=13.9 Plate DOL=1.1 	-10; Vult=130mph (3-second gust) h; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Enclosed; MWFRS (envelope) and C-C ne; cantilever left and right exposed ; end d right exposed;C-C for members and RS for reactions shown; Lumber te grip DOL=1.33 ed for wind loads in the plane of the truss s exposed to wind (normal to the face), Industry Gable End Details as applicable, lified building designer as per ANSI/TPI 1. 7-10; Pr=20.0 psf (roof live load: Lumber te DOL=1.15); Pg=20.0 psf (ground 9 psf (flat roof snow: Lumber DOL=1.15 15); Category II; Exp B; Fully Exp.;	
REACTIONS	Fasten (2X) T ar of web with 10d (o.c.,with 3in mini Brace must cov (size) 2=14-3 25=13 35=14 38=14 41=14 Max Horiz 2=227 Max Uplift 24=-10 26=-33 28=-43 34=-27 38=-25 40=-13	I l braces to narrow e 0.131"x3") nails, 6in num end distance. er 90% of web length. -8, 22=13-3-8, 24=13- 3-8, 26=13-3-8, 27=13 3-8, 29=13-3-8, 34=14 3-8, 36=14-3-8, 37=14 3-8, 36=14-3-8, 40=14 2 (LC 12), 41=227 (LC 1 2 (LC 14), 25=-5 (LC 2 (LC 14), 25=-5 (LC 2 (LC 14), 25=-5 (LC 2 (LC 13), 35=-36 (LC 2 (LC 13), 35=-26 (LC 2 9 (LC 13))	dge 3-8, BOT CHORD 3-3-8, 1-3-8, 1-3-8, 1-3-8, 12) 14), WEBS 14), 13), 13), 13), NOTES	11-1227/1111, 12-1327 13-14=-306/62, 14-16=-270 16-17=-285/0, 17-18=-289/(19-20=-312/5, 20-22=-319/8 2-40=-93/312, 37-38=-93/3 36-37=-93/312, 35-36=-93/3 31-33=-93/312, 29-31=-93/3 28-29=-92/311, 25-26=-92/3 24-25=-92/311, 22-24=-92/3 24-25=-92/311, 22-24=-92/3 24-25=-92/311, 22-24=-92/3 24-25=-92/311, 22-24=-92/3 24-25=-92/311, 22-24=-92/3 24-25=-92/311, 23-24=-92/3 24-25=-92/311, 23-24=-92/3 24-25=-92/311, 23-24=-92/3 24-25=-92/311, 23-24=-92/3 24-25=-92/311, 23-24=-92/3 24-25=-92/311, 23-24=-92/3 24-25=-92/311, 23-24=-92/3 24-25=-92/311, 23-24=-92/3 24-25=-92/311, 25-26=-92/3 24-25=-92/311, 25-26=-92/3 24-25=-92/312, 25-21=-92/3 24-25=-92/312, 25-21=-92/3 24-25=-92/32, 25-21=-92/32 24-25=-92/32, 25-21=-92/32 24-25=-92/32, 25-21=-92/32 24-25=-92/32, 25-21=-92/32 24-25=-92/32, 25-21=-92/32 24-25=-92/32, 25-21=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25=-92/32 24-25	Ct=1.10 5) This truss has load of 12.0 ps overhangs nor 6) All plates are 2 7) Gable studs sp	osf (flat roof snow: Lumber DOL=1.15); Category II; Exp B; Fully Exp.; een designed for greater of min roof live or 2.00 times flat roof load of 13.9 psf on concurrent with other live loads. 4 MT20 unless otherwise indicated. ced at 2-0-0 oc. HCARO HCARO SEAL 044925		
			1) Unbalance this design	ed roof live loads have been o n.	onsidered for	7	May 21 2021	

Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



May 21,2021

Job	Truss	Truss Type	Qty	Ply	Cedar 2 A	
21050070-A	AE	Common Supported Gable	1	1	Job Reference (optional)	146242488

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) One RT7A MiTek connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 40, 34, 35, 36, 37, 38, 39, 29, 28, 27, 26, 25, and 24. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard

Run: 8.5 S 0 May 17 2021 Print: 8.500 S May 17 2021 MiTek Industries, Inc. Fri May 21 12:04:08 ID:6ZIBtiWzpxMW0GiKuUURSIznq5n-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 2

ENGINEERING BY AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	Cedar 2 A	
21050070-A	с	Common	2	1	Job Reference (optional)	146242489

Run: 8.5 S 0 May 17 2021 Print: 8.500 S May 17 2021 MiTek Industries, Inc. Fri May 21 12:04:08 ID:N1d3mpVuuUCPC_JduxPSxRykgj7-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





BOT CHORD

FORCES

Scale = 1:47.1

Loading

TCDL

BCLL

BCDL

WEBS

SLIDER

BRACING

LUMBER

TCLL (roof)

Snow (Pf/Pg)

1) Unbalanced roof live loads have been considered for this design.

2-8=-219/605, 6-8=-221/605

6-7=0/35

4-8=0/389

- Wind: ASCE 7-10; Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=13.9 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10

Contraction of the WWWWWWW SEAL 4925 S 11111111 May 21,2021



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED WITHS KRETERENCE PAGE with-74/3 fev. on parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Cedar 2 A	
21050070-A	CE	Common Supported Gable	1	1	Job Reference (optional)	146242490

Run: 8.5 S 0 May 17 2021 Print: 8.500 S May 17 2021 MiTek Industries, Inc. Fri May 21 12:04:09 ID:sDBS_9WWfoKGp7tpRexhUfykgj6-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Plate Offsets (X, Y): [10:Edge,0-4-3]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(r 2 13.9/2 1	psf) 20.0 20.0 0.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC20	15/TPI2014	CSI TC BC WB Matrix-MSH	0.06 0.03 0.05	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 10	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 96 lb	GRIP 244/190 FT = 205	%	
LUMBER TOP CHORD BOT CHORD OTHERS SLIDER BRACING TOP CHORD	RD 2x4 SP No.2 RD 2x4 SP No.2 2x4 SP No.3 *Except* 15-6:2x4 SP No.2 Left 2x4 SP No.3 2-2-15, Right 2x4 SP No.3 2-2-15 RD Structural wood sheathing directly applied or 6-0-0 oc purlins.				VEBS 6 3 9 IOTES) Unbalanced r this design. 2) Wind: ASCE Vasd=103mp Cat. II; Exp B	6-15=-97/36, 5-16=-134/82, 4-17=-129/84, 11) This truss is d. 3-18=-143/93, 7-14=-133/82, 8-13=-129/84, 11) This truss is d. 9-12=-145/94 11) This truss is d. roof live loads have been considered for 12) See Standard perform the loads have been considered for 13) This truss is d. 11) This truss is d. 14) This truss is d. 12) See Standard 12) See Standard 13) The loads have been considered for 12) See Standard 14) This truss is d. 13) This truss is d. 15) Total Formation of the loads have been considered for 13) This truss is d. 14) This truss is d. 14) This truss is d. 15) Total Formation of the loads have been considered for 14) This truss is d. 15) Total Formation of the loads have been considered for 12) See Standard 16) Total Formation of the loads have been considered for 12) See Standard 17) Total Formation of the loads have been considered for 14) This truss is d. 18) Total Formation of the loads have been considered for 15) Total Formation of the loads have been consult qualifiered for the loads have been considered for 16) Total Formation of the loads have been considered for 10) Total Formation of the loads have been considered fo							signed in accordance with the 2015 esidential Code sections R502.11.1 and referenced standard ANSI/TPI 1. Industry Piggyback Truss Connection section to base truss as applicable, or ed building designer. Standard			
BOT CHORD	Rigid ceiling d bracing. (size) 2=1 12= 14= 16= 18= 23= Max Horiz 2=- Max Uplift 2=- 13= 16= 18= Max Grav 2=1	lirectly : 15-11-0 =15-11- =15-11- =15-11- =15-11- =15-11- =15-11- =15-11- =15-11- =15-11- =15-(LC =-28 (LC =-31 (LC =-32	applied or 10-0-0 oc , 10=15-11-0, 0, 13=15-11-0, 0, 15=15-11-0, 0, 19=15-11-0, 0 C 11), 19=-116 (LC 1 9), 12=-47 (LC 14), C 14), 14=-31 (LC 14 C 13), 17=-27 (LC 13 C 13), 19=-24 (LC 2) C 20, 10=144 (LC 2)	3 11) 4 1), 3), 50)	Exterior (2) z: vertical left at forces & MW DOL=1.60 pli Truss design only. For stu see Standard or consult qu TCLL: ASCE DOL=1.15 Pl snow); Pf=13 Plate DOL=1 Ct=1.10 i) This truss has load of 12.0 pt	one; cantilever left nd right exposed;C FRS for reactions s ate grip DOL=1.33 and for wind loads i ds exposed to wind I Industry Gable Er alified building des 7-10; Pr=20.0 psf ate DOL=1.15); Pg .9 psf (flat roof snc .15); Category II; E s been designed fo ssf or 2.00 times fi	and rig -C for n shown; in the p d (norm nd Deta igner as (roof liv g=20.0 p bw: Lum Exp B; F or great	the exposed; nembers and Lumber ane of the tru al to the face ls as applica s per ANSI/TI s f (ground ber DOL=1.1 ully Exp.; er of min roof pad of 13.9 p	end Jss), ble, PI 1. ber 5 5				WTH CA	ROJ		
FORCES TOP CHORD BOT CHORD	12=174 (LC 26), 13=164 (LC 26), 14=173 (LC 26), 15=137 (LC 28), 16=175 (LC 25), 17=163 (LC 25), 18=181 (LC 25), 19=149 (LC 26), 23=142 (LC 2) (lb) - Maximum Compression/Maximum Tension 1-2=0/35, 2-3=-50/23, 3-4=-91/64, 4-5=-82/80, 5-6=-132/138, 6-7=-132/138, 7-8=-81/80, 8-9=-60/26, 9-10=-42/23, 10-11=0/35 2-18=-56/93, 17-18=-56/93, 16-17=-56/93, 15-16=-56/93, 14-15=-56/93, 13-14=-56/93, 12-13=-56/93, 10-12=-56/93			66), 88), 55), 6 26), 7 8 26), 7 8 9 3, 93,	 overhangs nc overhangs nc All plates are Gable require Gable studs s Gable studs s (able studs s) * This truss h on the botton 3-06-00 tall b chord and an One RT7A M truss to beari 18, 14, 13, ar does not con 	D psf or 2.00 times flat roof load of 13.9 psf on non-concurrent with other live loads. Ire 2x4 MT20 unless otherwise indicated. Iires continuous bottom chord bearing. Is spaced at 2-0-0 oc. Is has been designed for a live load of 20.0psf om chord in all areas where a rectangle I by 2-00-00 wide will fit between the bottom any other members. MiTek connectors recommended to connect aring walls due to UPLIFT at jt(s) 2, 16, 17, and 12. This connection is for uplift only and onsider lateral forces.							A Stanmannin			

May 21,2021



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Job	Truss	Truss Type	Qty	Ply	Cedar 2 A	
21050070-A	DE	Common Supported Gable	1	1	Job Reference (optional)	146242491

Run: 8.5 S 0 May 17 2021 Print: 8.500 S May 17 2021 MiTek Industries, Inc. Fri May 21 12:04:10 ID:sDBS_9WWfoKGp7tpRexhUfykgj6-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f





Scale = 1:29.3

Plate Offsets (X,	Y):	[6:Edge,0-4-3]
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				_														
Loading		(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP				
TCLL (roof)		20.0	Plate Grip DOL	1.15		TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190				
Snow (Pf/Pg)	13.9	9/20.0	Lumber DOL	1.15		BC	0.02	Vert(CT)	n/a	-	n/a	999						
TCDL		10.0	Rep Stress Incr	YES		WB	0.03	Horz(CT)	0.00	6	n/a	n/a						
BCLL		0.0*	Code	IRC20	I5/TPI2014	Matrix-MP												
BCDL		10.0											Weight: 38 lb	FT = 20%				
LUMBER				2) Wind: ASCE	7-10; Vult=130mp	oh (3-seo	ond gust)										
TOP CHORD	2x4 SP No.2	2			Vasd=103mp	oh; TCDL=6.0psf; I	BCDL=6	.0psf; h=25ft	;									
BOT CHORD	2x4 SP No.2	2			Cat. II; Exp E	3; Enclosed; MWFI	RS (env	elope) and C	-C									
OTHERS	2x4 SP No.3	3			Exterior (2) z	one; cantilever lef	t and rig	ht exposed ;	end									
SLIDER	Left 2x4 SP	No.3 1	-7-11, Right 2x4 SP		vertical left a	nd right exposed;C	C-C for n	nembers and										
	No.3 1-7-1	11			forces & MW	FRS for reactions	Lumber											
BRACING					DOL=1.60 pl	ate grip DOL=1.33	3 											
TOP CHORD	Structural w	ood shea	athing directly applie	dor ³) Truss design	ied for wind loads	n the p	ane or the tro	uss N									
	6-0-0 oc pu	rlins.			see Standard	d Industry Gable E	nd Deta	ils as applica	ble.									
BOT CHORD	Rigid ceiling	g directly	applied or 6-0-0 oc		or consult qualified building designer as per ANSI/TPI 1.													
REACTIONS	(size) 2	=6-11-0	6=6-11-0 8=6-11-0	4) TCLL: ASCE	7-10; Pr=20.0 psf	f (roof liv	e load: Lumb	per									
	(0.20) 2	=6-11-0	10=6-11-0 11=6-11	-0	DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground													
	1	5=6-11-0)	σ,	snow); Pt=13	3.9 pst (flat root sn	ow: Lum	ber DOL=1.	15									
	Max Horiz 2	=-57 (LC	11), 11=-57 (LC 11))	Ct=1 10	. 15); Category II; I	Ехр В; Г	ully Exp.;										
	Max Uplift 2	:=-9 (LC §	9), 8=-42 (LC 14), 10	=-44 5) This trues ha	s heen designed f	for areat	er of min root	f live									
	. (I	LC 13), 1	1=-9 (LC 9)	0	load of 12 0	osf or 2 00 times fl	lat roof le	ad of 13.9 p	sfon									
	Max Grav 2	=118 (LC	2), 6=118 (LC 2), 8	=161	overhands n	on-concurrent with	other liv	/e loads.	0. 0									
	(1	LC 26), 9	=123 (LC 2), 10=165	5(LC 6) Gable require	es continuous bott	om chor	d bearing.										
	2	5), 11=1 <i>°</i>	18 (LC 2), 15=118 (L	.C 2) 7) Gable studs	spaced at 2-0-0 oc	C.	5										
FORCES	(lb) - Maxim	um Com	pression/Maximum	8) * This truss h	as been designed	l for a liv	e load of 20.	0psf				mm	1111,				
	Tension				on the bottor	n chord in all areas	s where	a rectangle					IN CA	Ro"				
I OP CHORD	1-2=0/35, 2-	-3=-1//42	2, 3-4=-74/70,		3-06-00 tall b	y 2-00-00 wide wi	ill fit betv	een the bott	om			1	'A'		11.			
	4-5=-74/70,	5-6=-13/	42, 6-7=0/35	0	chord and ar	y other members.			4			56	O'.FESS	10n.1	1-2			
BOICHORD	2-10=-22/51	1, 9-10=-2	22/51, 8-9=-22/51,	9) One RI/A N	III ek connectors re		nded to conr	nect		()	11	in the	1	NAS N			
WEBS	4-9=-81/0	3-10=-14	5/100 5-8=-147/101		This connect	ing wails due to Or	v and do	s not consid	hu o. 1er		2	×	inter ?	- and	Ne-			
NOTES	10 01/0,0	0 10 11	5,100,00 111,101		lateral forces	ion io ior apint only	y and do		101		-	:	SEA		: =			
NUIE3 1) Unbalance	ed roof live loa	de have	heen considered for	1	0) This truss is	designed in accord	dance w	ith the 2015			=	:	JLA		: =			
this design	n	us nave			International	Residential Code	sections	R502.11.1 a	and		Ξ		0449	25	÷ Ξ			
the deergi					R802.10.2 ai	nd referenced stan	ndard AN	ISI/TPI 1.			-		•					
				1	1) See Standar	d Industry Piggyba	ack Trus	s Connection	l .			-	·	~ · ·				
				Detail for Connection to base truss as applicable, or								5. ON NOWEER .						
					consult quali	sult qualified building designer.												
				L	LOAD CASE(S) Standard							TM SE IN						
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													Ma	. 04 000	•			



May 21,2021

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