

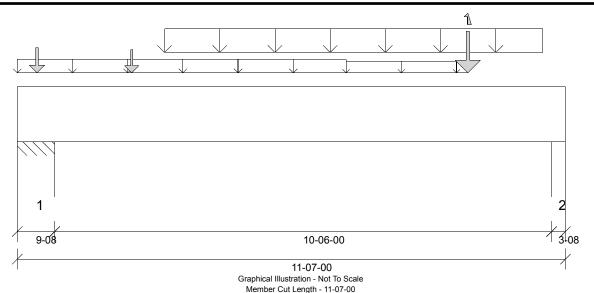
Member Type: Beam | Level: 2nd Floor Designed by Single Member Design Engine

Member: 2 - 1 3/4" x 14" (2.0E 3100) LVL

Label: 1BM1-2-i1248

Page: 1 of 7 Date: 03/03/2022 09:28:01

Status: Design Passed



MemberPitch - 0/12

**Design Information:** 

Building Code: IRC2015 Floor Dead Load: 10.0 lb/ft² Roof Dead Load: 10.0 lb/ft² Ground Snow Load: 20.0 lb/ft²
Design Methodology: ASD Floor Live Load: 40.0 lb/ft² Roof Live Load: 20.0 lb/ft²

Unbraced Length Top: 0-00 Bottom: 1-08-04

#### **Design Results:**

	<u>Location</u>	<u>Design</u>	<u>Control</u>	Result	<u>LDF</u>	Load Combination
Critical Moment (Pos)	7-01-05	8328.20 lb ft	28945.56 lb ft	Passed - 29%	1.00	D + L
Critical Moment (Neg)	8-08	-250.91 lb ft	28945.56 lb ft	Passed - 1%	1.00	D + L
Critical Shear	10-01-08	3609.82 lb	9310.00 lb	Passed - 39%	1.00	D + L
Live Load Deflection	6-02-03	0-01	0-12 (L/360)	Passed - L/999	-	L
Total Load Deflection	6-02-03	0-02	1-00 (L/240)	Passed - L/999	-	D + L
Max. Reaction			Supported Mtl Supporting Mtl			
	8-08	3466.16 lb	24937.62 lb 29093.89 lb	Passed - 14%	1.00	D + L
	11-04-08	3652.68 lb	9187.48 lb 10718.73 lb	Passed - 40%	1.00	D + L

#### Design Notes:

Point

Point

\* Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code

specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

Loading:										
				Maximum Load Magnitudes						
<u>Type</u>	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	Snow			
Self Weight	0-00	11-07-00	Self Weight	14 lb/ft	-	-	-			
Uniform	0-00	6-11-06	FC1 Floor Decking	18 lb/ft	72 lb/ft	-	-			
Uniform	3-01-06	11-01-05	Smoothed Load	77 lb/ft	308 lb/ft	-	-			
Uniform	6-11-06	9-06-04	FC1 Floor Decking	8 lb/ft	32 lb/ft	-	-			
Point	9-06-04	9-06-04	1BM3-2(i1345)	338.00 lb	1267.00/-55.00 lb	-	-			

## **Support Information:**

			_	<u>Maximum Analysis Reactions</u>					
Support	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	Snow		
1	0-00	9-08	W35(i37)	763.00 lb	2691.00/-10.00 lb	-	-		
2	11-03-08	11-07-00	W40(i40)	810.00 lb	2854.00/-45.00 lb	-	-		

154.00 lb

135.00 lb

616.00 lb

538.00 lb

#### **Errors, Warnings & Notes:**

5-00

2-05-00

\* The dead loads used in the design of this member were applied to the structure as projected dead loads.

5-00

2-05-00

- \* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

FJ16(i1259)

FJ16(i1261)



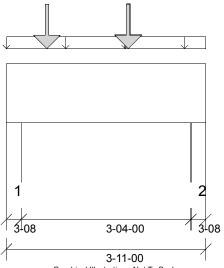
Member Type: Beam | Level: 2nd Floor Designed by Single Member Design Engine

Member: 2 - 1 3/4" x 14" (2.0E 3100) LVL

Label: 1BM2-2-i1247

Page: 2 of 7 Date: 03/03/2022 09:28:03

Status: Design Passed



Graphical Illustration - Not To Scale Member Cut Length - 3-11-00 MemberPitch - 0/12

Bottom: 1-05-00

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Building Code: IRC2015 Floor Dead Load: 10.0 lb/ft<sup>2</sup> Roof Dead Load: 10.0 lb/ft<sup>2</sup> Ground Snow Load: 20.0 lb/ft<sup>2</sup> 40.0 lb/ft<sup>2</sup> Design Methodology: ASD Floor Live Load: Roof Live Load: 20.0 lb/ft2 Unbraced Length Top: 0-00

#### **Design Results:**

<del></del>	Location	<u>Design</u>	<u>Control</u>	Result	<u>LDF</u>	Load Combination
Critical Moment (Pos)	2-04-12	769.78 lb ft	28945.56 lb ft	Passed - 3%	1.00	D + L
Critical Shear	1-05-08	855.61 lb	9310.00 lb	Passed - 9%	1.00	D + L
Live Load Deflection	1-11-10	0-00	0-12 (L/360)	Passed - L/999	-	L
Total Load Deflection	1-11-10	0-00	1-00 (L/240)	Passed - L/999	-	D + L
Max. Reaction			Supported Mtl Supporting Mtl			
	2-08	898.47 lb	9187.52 lb 10718.78 lb	Passed - 10%	1.00	D + L
	3-08-08	614.22 lb	9187.63 lb 10718.90 lb	Passed - 7%	1.00	D + L

#### **Design Notes:**

\* Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

#### Loading:

				<u>Maximum Load Magnitudes</u>				
<u>Type</u>	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	<u>Snow</u>	
Self Weight	0-00	3-11-00	Self Weight	14 lb/ft	-	-	-	
Uniform	-0-00	3-11-00	FC1 Floor Decking	3 lb/ft	12 lb/ft	-	-	
Point	9-09	9-09	FJ18(i1352)	139.00 lb	556.00 lb	-	-	
Point	2-04-12	2-04-12	FJ18(i1365)	140.00 lb	561.00 lb	-	-	

## **Support Information:**

			_	<u>Maximum Analysis Reactions</u>					
Support	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	Snow		
1	0-00	3-08	W40(i40)	202.00 lb	696.00 lb	-	-		
2	3-07-08	3-11-00	W41(i41)	145.00 lb	469.00 lb	-	-		

- \* The dead loads used in the design of this member were applied to the structure as projected dead loads.
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- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



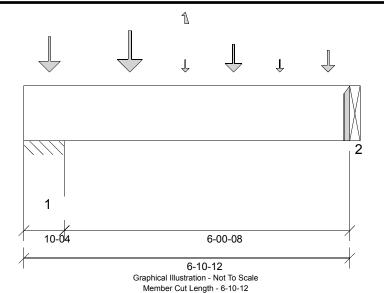
Member Type: Beam | Level: 2nd Floor Designed by Single Member Design Engine

Member: 2 - 1 3/4" x 14" (2.0E 3100) LVL

Label: 1BM3-2-i1345

Page: 3 of 7 Date: 03/03/2022 09:28:03

Status: Design Passed



MemberPitch - 0/12

**Design Information:** 

Building Code: IRC2015 Floor Dead Load: 10.0 lb/ft² Roof Dead Load: 10.0 lb/ft² Ground Snow Load: 20.0 lb/ft²

Design Methodology: ASD Floor Live Load: 40.0 lb/ft² Roof Live Load: 20.0 lb/ft²

Roof Live Load: 20.0 lb/ft²

Unbraced Length Top: 0-00 Bottom: 1-01-08

## **Design Results:**

	Location	<u>Design</u>	<u>Control</u>	Result	<u>LDF</u>	Load Combination
Critical Moment (Pos)	4-05-04	2640.55 lb ft	28945.56 lb ft	Passed - 9%	1.00	D + L
Critical Moment (Neg)	9-04	-292.35 lb ft	28945.56 lb ft	Passed - 1%	1.00	D + L
Critical Shear	2-00-04	1078.97 lb	9310.00 lb	Passed - 12%	1.00	D + L
Live Load Deflection	3-09-04	0-00	0-12 (L/360)	Passed - L/999	-	L
Total Load Deflection	3-09-05	0-00	1-00 (L/240)	Passed - L/999	-	D + L
Max. Reaction			Supported Mtl Supporting Mtl			
	9-04	3030.81 lb	26840.36 lb 31313.75 lb	Passed - 11%	1.00	D + L
	6-10-12	1609.32 lb	1609.32 lb 0.00 lb	Passed - 100%	1.00	D + L

#### **Design Notes:**

\* Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code

specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

# <u>Loading:</u>

				<u>Maximum Load Magnitudes</u>				
<u>Type</u>	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	<u>Snow</u>	
Self Weight	0-00	6-10-12	Self Weight	14 lb/ft	-	-	-	
Point	6-08	6-08	-	253.00 lb	1010.00 lb	-	-	
Point	2-02-14	2-02-14	-	309.00 lb	1238.00 lb	-	-	
Point	4-05-04	4-05-04	FJ18(i1350)	177.00 lb	710.00 lb	-	-	
Point	6-05-04	6-05-04	FJ18(i1321)	116.00 lb	462.00 lb	-	-	
Point	3-05-00	3-05-00	FJ20(i1299)	-	103.00/-75.00 lb	-	-	
Point	5-05-00	5-05-00	FJ4(i1341)	24.00 lb	95.00 lb	-	-	

#### **Support Information:**

				<u>Maximum Analysis Reactions</u>					
Support	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	Snow		
1	0-00	10-04	W39(i39)	646.00 lb	2406.00/-43.00 lb	-	-		
2	6 10 12	6-10-12	1DM1 2(i1249)	338 00 lb	1267 00/ 55 00 lb				

Nailing Requirements

## **Connector Information:**

				tanning i to quin on to the			
Support	<u>Manufacturer</u>	<u>Model</u>	<u>Top</u>	<u>Face</u>	<u>Member</u>	<u>IVIIII Ətal</u> I Anath	Other Information
2	Simpson	HHUS410	-	-	-	N/A	Connector manually specified by the user.

- \* The dead loads used in the design of this member were applied to the structure as projected dead loads.
- \* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

<sup>-</sup> This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.



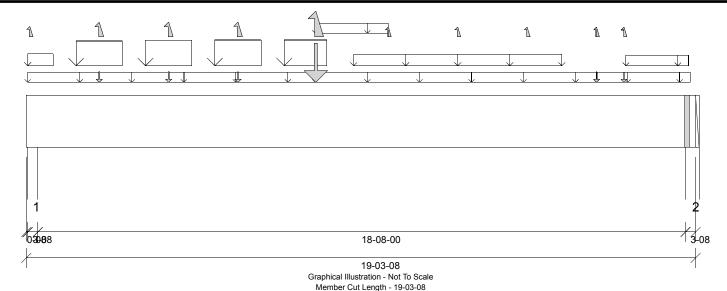
Member Type: Beam | Level: 2nd Floor Designed by Single Member Design Engine

Member: 3 - 1 3/4" x 18" (2.0E 3100) LVL

Label: 1BM4-3-i1202

Page: 4 of 7 Date: 03/03/2022 09:28:04

Status: Design Passed



MemberPitch - 0/12

#### **Design Information:**

Building Code: IRC2015 Floor Dead Load: 10.0 lb/ft<sup>2</sup> Roof Dead Load: 10.0 lb/ft<sup>2</sup> Ground Snow Load: 20.0 lb/ft<sup>2</sup> Design Methodology: ASD Floor Live Load: 40.0 lb/ft<sup>2</sup> Roof Live Load: 20.0 lb/ft2

Unbraced Length Top: 0-08

Bottom: 1-10-08

**Design Results:** 

	<b>Location</b>	<u>Design</u>	<u>Control</u>	<u>Result</u>	<u>LDF</u>	<b>Load Combination</b>
Critical Moment (Pos)	8-04-14	49196.21 lb ft	79170.20 lb ft	Passed - 62%	1.15	D + Lr
Critical Moment (Neg)	8-01-04	-8586.35 lb ft	110149.85 lb ft	Passed - 8%	1.60	0.6D + 0.6W
Critical Shear	1-10-00	8287.82 lb	20648.25 lb	Passed - 40%	1.15	D + Lr
Live Load Deflection	9-03-00	0-04	0-12 (L/360)	Passed - L/828	-	0.75(L + Lr + 0.6W)
Total Load Deflection	9-02-09	0-09	1-00 (L/240)	Passed - L/403	-	D + 0.75(L + Lr + 0.6W)
Max. Reaction			Supported Mtl Suppo	orting Mtl		
	3-00	8942.48 lb	13781.29 lb 1607	8.17 lb Passed - 65%	1.60	D + 0.75(L + Lr + 0.6W)
	3-00	-1995.99 lb	0.00 lb	-	1.60	0.6D + 0.6W
	19-01-00	5293.45 lb	13781.40 lb 1607	8.30 lb Passed - 38%	1.15	D + 0.75(L + Lr)
	19-01-00	-587.04 lb	0.00 lb	-	1.60	0.6D + 0.6W

#### **Design Notes:**

\* Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

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				Maximum Load Magnitudes					
<u>Type</u>	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	Snow		
Self Weight	0-00	19-03-08	Self Weight	28 lb/ft	-	-	-		
Uniform	0-08	8-04-00	FC1 Floor Decking	-	29 lb/ft	-	-		
Uniform	0-08	9-04	W61(i338)	-	-	106 lb/ft	44 lb/ft		
Uniform	1-05-04	2-09-04	W61(i338)	512 lb/ft	-	520 lb/ft	218 lb/ft		
Uniform	3-05-04	4-09-04	W61(i338)	524 lb/ft	-	543 lb/ft	228 lb/ft		
Uniform	5-05-04	6-09-04	W61(i338)	525 lb/ft	-	546 lb/ft	229 lb/ft		
Uniform	7-05-04	8-08-00	W61(i338)	552 lb/ft	-	557 lb/ft	234 lb/ft		
Uniform	8-04-00	19-01-12	FC1 Floor Decking	-	24 lb/ft	-	-		
Uniform	8-04-00	10-05-04	FC1 Floor Decking	-	9 lb/ft	-	-		
Uniform	9-05-04	15-05-04	Smoothed Load	30 lb/ft	124 lb/ft	-	-		
Uniform	17-03-06	19-01-02	FC1 Floor Decking	-	83 lb/ft	-	-		
Point	2-01-04	2-01-04	-	98.00 lb	-	86.00/-4.00 lb	82.00 lb		
Point	4-01-04	4-01-04	-	98.00 lb	-	86.00/-4.00 lb	82.00 lb		
Point	6-01-04	6-01-04	-	105.00 lb	-	102.00/-5.00 lb	97.00 lb		
Point	8-04-03	8-04-03	-	3307.00 lb	122.00 lb	3235.00/-3.70 lb	1255.00 lb		
Point	10-05-04	10-05-04	FJ8(i1256)	-	-5.00 lb	-	-		
Point	12-05-04	12-05-04	FJ8(i1258)	-	-9.00 lb	-	-		
Point	14-05-04	14-05-04	FJ8(i1373)	-	-9.00 lb	-	-		
Point	16-05-04	16-05-04	FJ8(i1262)	-	174.00/-6.00 lb	-	-		
Point	17-02-12	17-02-12	FJ8(i1367)	-	111.00/-2.00 lb	-	-		
Point	1-04	1-04	W61(i338)	-	-	-	-		

## **Support Information:**

			_	Maximum Analysis Reactions					
Support	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	Snow		
1	0-08	4-00	W23(i25)	4668.00 lb	648.00/-9.00 lb	4244.00/-12.00 lb	1836.00 lb		
2	19-00-00	19-03-08	W39(i39)	2757.00 lb	1179.00/-22.00 lb	2174.00/-4.00 lb	898.00 lb		

CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.

<sup>-</sup> Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.

<sup>-</sup> This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.



Member Type: Beam | Level: 2nd Floor Designed by Single Member Design Engine

Member: 3 - 1 3/4" x 18" (2.0E 3100) LVL

Label: 1BM4-3-i1202

Page: 5 of 7 Date: 03/03/2022 09:28:04

Status: Design Passed

- \* The dead loads used in the design of this member were applied to the structure as projected dead loads.
- The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



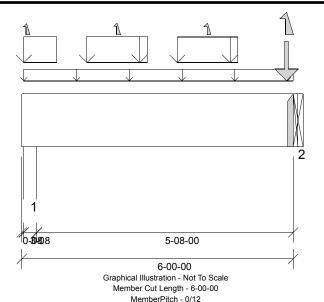
Member Type: Beam | Level: 2nd Floor Designed by Single Member Design Engine

Member: 1 - 1 3/4" x 14" (2.0E 3100) LVL

Label: 1BM5-i1251

Page: 6 of 7 Date: 03/03/2022 09:28:04

Status: Design Passed



**Design Information:** 

**Building Code:** IRC2015 Floor Dead Load: 10.0 lb/ft<sup>2</sup> Roof Dead Load: 10.0 lb/ft<sup>2</sup> Ground Snow Load: 20.0 lb/ft<sup>2</sup> Design Methodology: ASD Floor Live Load: Roof Live Load: 20.0 lb/ft2

40.0 lb/ft<sup>2</sup> Unbraced Length Top: 0-08 Bottom: 5-08-00

#### **Design Results:**

	<b>Location</b>	<u>Design</u>	Con	<u>Control</u>		<u>LDF</u>	<b>Load Combination</b>
Critical Moment (Pos)	3-09-02	1550.05 lb ft	16643.70 lb ft		Passed - 9%	1.15	D + Lr
Critical Moment (Neg)	4-01-04	-369.93 lb ft	23156.44 lb ft		Passed - 2%	1.60	0.6D + 0.6W
Critical Shear	4-10-00	1132.52 lb	5353.25 lb		Passed - 21%	1.15	D + Lr
Live Load Deflection	3-02-06	0-00	0-12 (L/360)		Passed - L/999	-	0.6W
Total Load Deflection	3-02-08	0-00	1-00 (L	1-00 (L/240)		-	D + 0.75(L + Lr + 0.6W)
Max. Reaction			Supported Mtl	Supporting Mtl			
	2-04	1069.14 lb	4593.73 lb	5359.36 lb	Passed - 23%	1.60	D + 0.75(L + Lr + 0.6W)
	2-04	-282.74 lb	0.00 lb	-		1.60	0.6D + 0.6W
	6-00-00	6391.77 lb	6391.77 lb	0.00 lb	Passed - 100%	1.15	D + Lr
	6-00-00	-761.87 lb	0.00 lb	_		1.60	0.6D + 0.6W

#### **Design Notes:**

#### Loading:

				Maximum Load Magnitudes						
<u>Type</u>	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	<u>Snow</u>			
Self Weight	0-00	6-00-00	Self Weight	7 lb/ft	-	-	-			
Uniform	0-08	6-00-00	FC1 Floor Decking	11 lb/ft	42 lb/ft	-	-			
Uniform	0-08	9-04	W60(i340)	211 lb/ft	-	182 lb/ft	63 lb/ft			
Uniform	1-05-04	2-09-04	W60(i340)	180 lb/ft	-	202 lb/ft	70 lb/ft			
Uniform	3-05-04	4-09-04	W60(i340)	176 lb/ft	-	185 lb/ft	65 lb/ft			
Point	1-04	1-04	W60(i340)	-	-	-	-			
Point	2-01-04	2-01-04	W60(i340)	-	-	-	-			
Point	4-01-04	4-01-04	W60(i340)	-	-	-	-			
Point	5-10-04	5-10-04	W60(i340)	3013.00 lb	-	2975.00/-3.00 lb	1165.00 lb			

## Support Information:

Support	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	Snow	
1	0-08	4-00	W34(i26)	440.00 lb	129.00 lb	389.00 lb	135.00 lb	
2	6-00-00	6-00-00	1BM4-3(i1202)	3307.00 lb	122.00 lb	3235.00/-3.00 lb	1255.00 lb	

Nailing Requirements

Maximum Analysis Reactions

# **Connector Information:**

Support	<u>Manufacturer</u>	Model	<u>Top</u>	<u>Face</u>	Member	I Anath	Other Information
2		IUS1.81/14	-	-	-	N/A	Connector manually specified by the user.

- CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- \* The dead loads used in the design of this member were applied to the structure as projected dead loads.
- \* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

<sup>-</sup> This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.



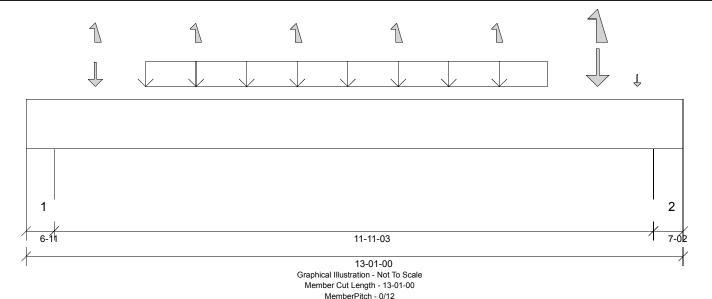
Member Type: Beam | Level: 3rd Floor Designed by Single Member Design Engine

Member: 2 - 1 3/4" x 11 7/8" (2.0E 3100) LVL

Label: 2BM6-2-i1243

Page: 7 of 7 Date: 03/03/2022 09:28:04

Status: Design Passed



## **Design Information:**

Building Code: IRC2015 Floor Dead Load: 10.0 lb/ft<sup>2</sup> Roof Dead Load: 10.0 lb/ft<sup>2</sup> Ground Snow Load: 20.0 lb/ft<sup>2</sup> Design Methodology: ASD 40.0 lb/ft<sup>2</sup> Roof Live Load: 20.0 lb/ft2 Floor Live Load:

#### **Design Results:**

	<b>Location</b>	<u>Design</u>	<u>Con</u>	<u>itrol</u>	Result	<u>LDF</u>	Load Combination
Critical Moment (Pos)	7-04-09	13052.28 lb ft	24437.95 lb ft		Passed - 53%	1.15	D + Lr
Critical Moment (Neg)	7-04-09	-4744.21 lb ft	34000.63 lb ft		Passed - 14%	1.60	0.6D + 0.6W
Critical Shear	11-06-00	5243.11 lb	9081.	41 lb	Passed - 58%	1.15	D + Lr
Live Load Deflection	6-07-04	0-04	0-12 (l	L/360)	Passed - L/587	-	0.6W
Total Load Deflection	6-07-00	0-06	1-00 (I	L/240)	Passed - L/385	-	D + 0.75(L + Lr + 0.6W)
Max. Reaction			Supported Mtl	Supporting Mtl			
	5-11	4313.74 lb	17586.81 lb	20517.95 lb	Passed - 25%	1.60	D + 0.75(L + Lr + 0.6W)
	5-11	-1489.57 lb	0.00 lb	-		1.60	0.6D + 0.6W
	12-06-14	5362.32 lb	18673.47 lb	21785.72 lb	Passed - 29%	1.60	D + 0.75(L + Lr + 0.6W)
	12-06-14	-1991.29 lb	0.00 lb	-		1.60	0.6D + 0.6W

Bottom: 1-10-08

## **Design Notes:**

\* Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

Unbraced Length Top: 1-10-08

#### Loading:

				<u>Maximum Load Magnitudes</u>					
<u>Type</u>	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	<u>Snow</u>		
Self Weight	0-00	13-01-00	Self Weight	12 lb/ft	-	-	-		
Uniform	2-04-09	10-04-09	Smoothed Load	327 lb/ft	-	321 lb/ft	135 lb/ft		
Point	1-04-09	1-04-09	A3(c01)	673.00 lb	-	679.00 lb	284.00 lb		
Point	3-04-09	3-04-09	A3(c02)	-	-	-	-		
Point	5-04-09	5-04-09	A4(c01)	-	-	-0.80 lb	-		
Point	7-04-09	7-04-09	A4(c02)	-	-	-0.70 lb	-		
Point	9-04-09	9-04-09	A4(c03)	-	-	-0.70 lb	-		
Point	11-04-09	11-04-09	A4(c04)	1048.00 lb	-	1428.00/-2.00 lb	599.00 lb		
Point	12-02-01	12-02-01	A5A(c01)	333 00 lb	_	_	_		

# **Support Information:**

			_	Maximum Analysis Reactions					
Support	<u>Start</u>	<u>End</u>	Source	<u>Dead</u>	Floor Live	Roof Live	Snow		
1	0-00	6-11	-	2157.00 lb	-	2085.00/-1.00 lb	874.00 lb		
++>	2-02	2-02	W49(i343)	1360.00 lb	-	1315.00/-1.00 lb	551.00 lb		
++>	6-00	6-00	W50(i333)	797.00 lb	-	770.00 lb	323.00 lb		
2	12-05-14	13-01-00	-	2673.00 lb	-	2590.00/-3.00 lb	1086.00 lb		
++>	12-06-10	12-06-10	W52(i339)	930.00 lb	-	901.00/-1.00 lb	378.00 lb		
++>	12-10-11	12-10-11	W53(i345)	1743.00 lb	-	1689.00/-2.00 lb	708.00 lb		

- CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- \* The dead loads used in the design of this member were applied to the structure as projected dead loads.
- \* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- \* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

<sup>-</sup> This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.