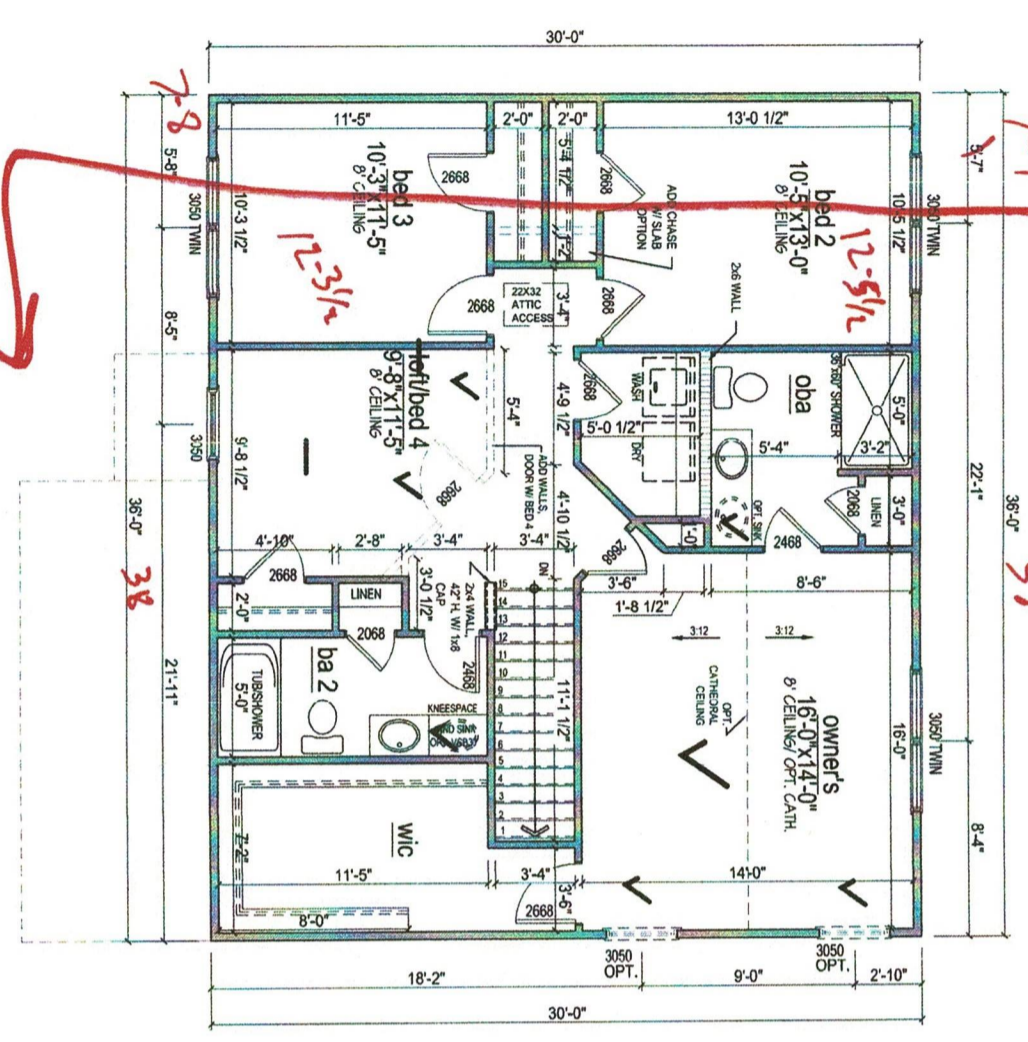


Add 2!



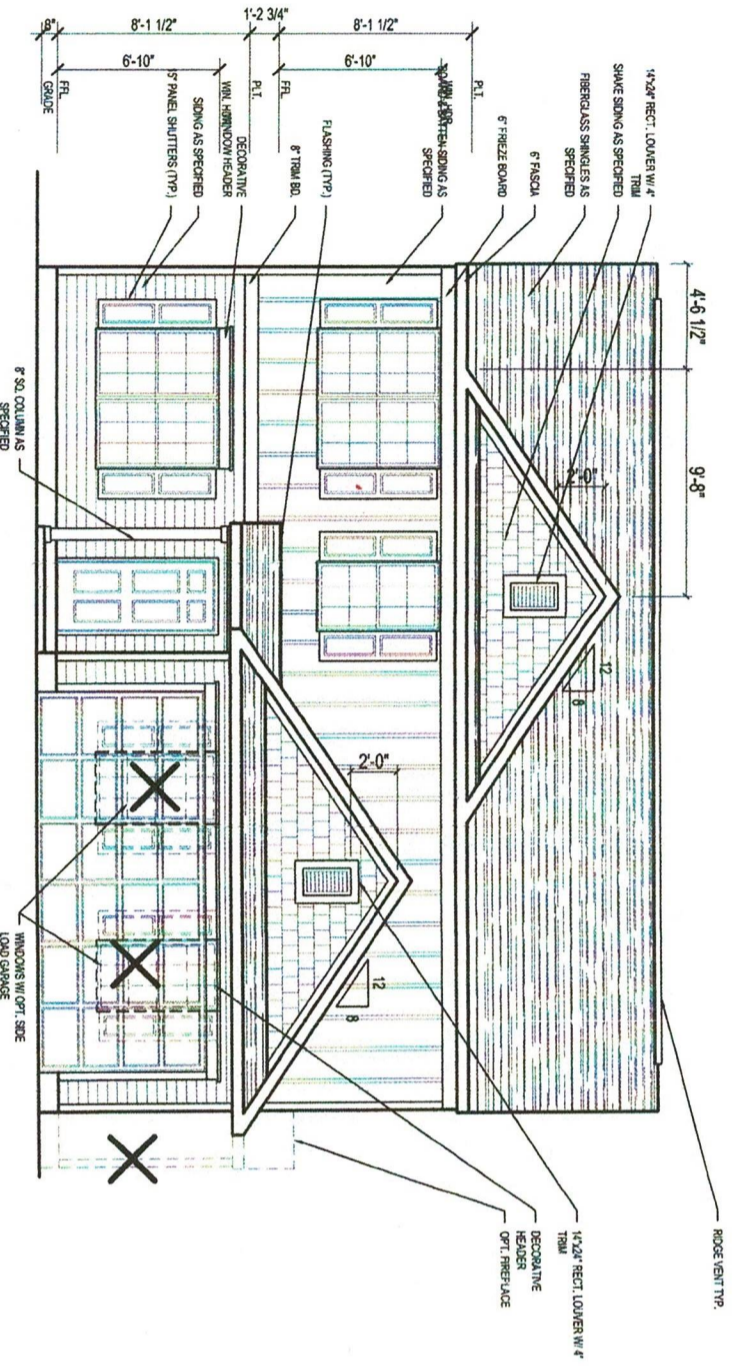
Second Floor 'A'
 1/8" = 1'-0" @ 11x17
 1/4" = 1'-0" @ 22x34

BP-5 (WWW.DOVESOURCE.COM) - GENERAL INFORMATION
 ALL CONTRACTORS SHALL REVIEW AND VERIFY ALL DIMENSIONS BEFORE BEGINNING ANY WORK.
 ALL CONTRACTORS CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE STATE AND LOCAL BUILDING CODES.
 BUILDERS PLAN SOURCE, INC. (WWW.BUILDERSPLAN.COM) IS NOT RESPONSIBLE FOR FINISHED RESOURCES, INCLUDING, BUT NOT LIMITED TO, THE CONSTRUCTION DETAILS THAT ARE NOT SPECIFICALLY SHOWN IN THESE PLANS SHALL BE CONSTRUCTION DETAILS IN ACCORDANCE WITH THE BUILDING CODES AND SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
 ALL STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO, SLAB BEAMS, BEAMS, JOISTS, VAULTS, PARTITIONS, GIRDERS, FLOOR JOISTS, EXTERIOR WALLS, ETC. SHALL BE SHOWN AND SPECIFIED IN ACCORDANCE WITH CURRENT STATE AND LOCAL BUILDING CODES. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE ACCURACY OF ALL DIMENSIONS AND PROVIDE A WRITTEN SERVICE ONLY AND SHALL NOT BE HELD LIABLE FOR STRUCTURAL COMPONENTS FOUND WITHIN PLANS.
 SOME FINISHED MATERIALS SUCH AS FLOOR COVERINGS, WALL COVERINGS, AND ANY RELATED ITEMS MAY NOT BE FOUND IN THESE PLANS. THESE ITEMS ARE TO BE DETERMINED BY THE CONTRACTOR.
 ALL EXTERIOR WALLS ARE TO BE REASURED AT 3/4" UNLESS NOTED OTHERWISE. EXTERIOR WALLS SHALL BE REASURED AT 3/4" UNLESS NOTED OTHERWISE.
 THIS RESERVE IS NOT DESIGNED FOR A SEVERE WEATHER CONDITION. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE ACCURACY OF ALL DIMENSIONS AND PROVIDE A WRITTEN SERVICE ONLY AND SHALL NOT BE HELD LIABLE FOR STRUCTURAL COMPONENTS FOUND WITHIN PLANS.
 LOCAL BUILDING CODES.

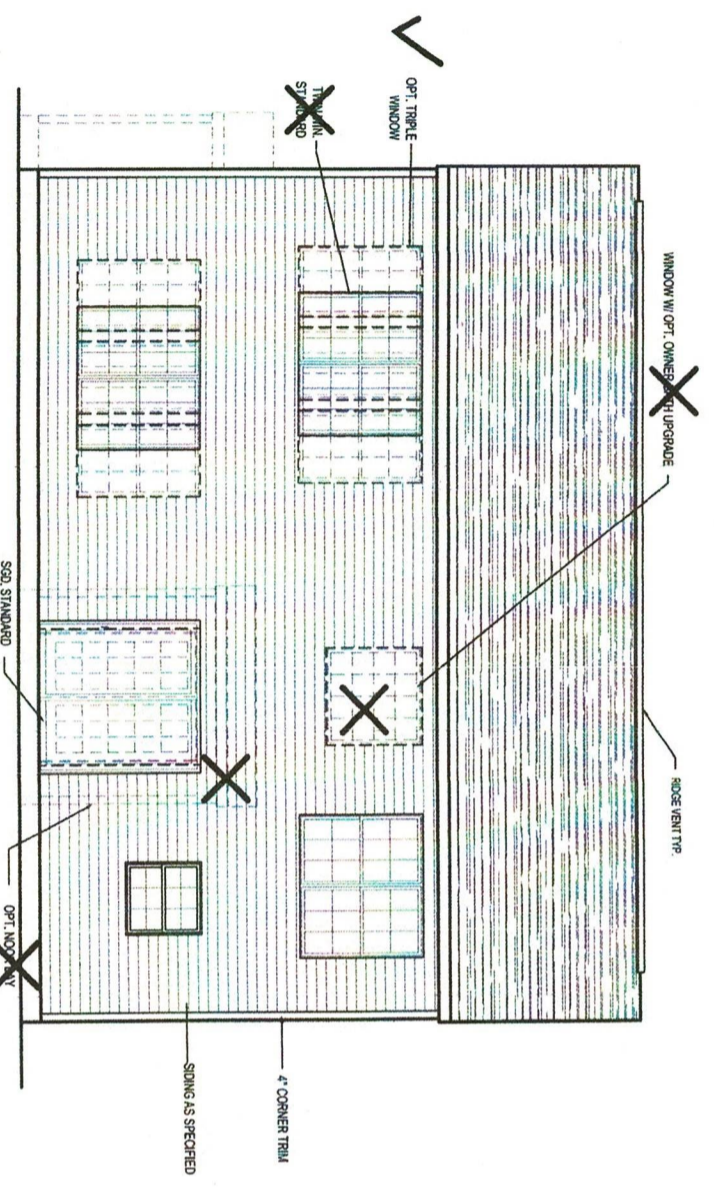
- COPYRIGHT INFORMATION**
 Copyright 2021 by Builders Plan Source, Inc. All rights reserved. No part of this drawing may be reproduced, stored in a retrieval system, or transmitted in any form or by any means, electronic, mechanical, photocopying, recording, or by any information storage and retrieval system, without the prior written permission of Builders Plan Source, Inc. This plan was prepared through a contract agreement with the client and is the property of Builders Plan Source, Inc. It is to be used for the project and location specified. No other use, reproduction, or distribution is permitted without the prior written consent of Builders Plan Source, Inc. according to the provisions of the contract.
- PLAN NOTES**
- REFER TO PLANS AND ELEVATIONS FOR WALL PLATE HEIGHTS. COVER NOTES CONTAIN STAIR CONST. INFORMATION.
 - SEE ELEVATIONS FOR WINDOW AND DOOR HEADER HEIGHTS.
 - ALL EXTERIOR WALLS ARE BRWMA AT 4" U.L.O. W/ STUD SPACING AT 16" O.C.
 - ALL INTERIOR WALLS ARE DRWMA AT 3 1/2" U.L.O. ALL LOAD BEARING WALLS ARE DRWMA AT 4" O.C. W/ DEL. TOP PLATE U.L.O.
 - APPLY 1/2" GYP. BD. ON ALL GARAGE WALLS AND 5/8" TYPE X GYP. BD. ON GARAGE CEILING.
 - VERIFY LOCATION OF HVAC CONDENSOR WITH FIELD MANAGER.
 - (2) HOSE BIBS SHALL BE INSTALLED. LOCATION TO BE DETERMINED BY PLUMBING CONTRACTOR.

SHEET # 4.20A	PLAN NAME BROOKE	PLAN # RH
	DRAWING: SECOND FLOOR PLAN ELEVATION A	
DRAWN BY: JIT	APPROVED BY: SAVED: AMCRBDE	
ORIG. DATE: 2021-04-09	REVISIONS:	
BUILDER: DOVE HOMES LLC	ADDRESS: BUILDERS PLAN SOURCE, INC. PO BOX 836 KING, NORTH CAROLINA 27021	
	PHONE: 336-985-0363	
	FAX: 336-985-0884	





Front Elevation 'A'
 1/8" = 1'-0" @ 11x17
 1/4" = 1'-0" @ 22x34



Rear Elevation 'A'
 1/8" = 1'-0" @ 11x17
 1/4" = 1'-0" @ 22x34

EPIC (www.epicplans.com) - GENERAL INFORMATION
 ALL CONSTRUCTION SHALL REFER AND VERIFY ALL SPECIFICATIONS BEFORE BEGINNING ANY WORK.
 ALL CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE STATE AND LOCAL BUILDING CODES.
 BUILDING MATERIALS SHALL BE APPROVED BY THE LOCAL BUILDING DEPARTMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT.
 ALL FINISHED MATERIALS SHALL AS FLOOR COVERINGS, WALL COVERINGS, AND ANY RELATED FINISHES SHALL NOT BE FOUND IN THESE PLANS. THESE ITEMS ARE TO BE DETERMINED BY THE BUILDER.
 ALL INTERIOR WALLS ARE TO BE VENEERED AT 3 1/2" UNLESS NOTED OTHERWISE. EXTERIOR WALLS ARE TO BE VENEERED FOR A SPECIFIC LONGITUDE LOCATION. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE LOCATION OF THE BUILDER'S PROPERTY AND TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT.
 THIS RESERVE IS NOT DESIGNED FOR A SPECIFIC LONGITUDE LOCATION. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE LOCATION OF THE BUILDER'S PROPERTY AND TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT.
 ALL FINISHED MATERIALS SHALL AS FLOOR COVERINGS, WALL COVERINGS, AND ANY RELATED FINISHES SHALL NOT BE FOUND IN THESE PLANS. THESE ITEMS ARE TO BE DETERMINED BY THE BUILDER.
 ALL INTERIOR WALLS ARE TO BE VENEERED AT 3 1/2" UNLESS NOTED OTHERWISE. EXTERIOR WALLS ARE TO BE VENEERED FOR A SPECIFIC LONGITUDE LOCATION. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE LOCATION OF THE BUILDER'S PROPERTY AND TO OBTAIN ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL BUILDING DEPARTMENT.

5.10A SHEET #	PLAN NAME BROOKE	PLAN # RH
	DRAWING: FRONT AND REAR ELEVATION A	
DRAWN BY: JLT	APPROVED BY: SAVED: ALCARBIDE	ORG. DATE: 2021-04-09
BUILDER: DOVE HOMES LLC		ADDRESS: BUILDERS PLANSOURCE, INC. PO BOX 836 KING, NORTH CAROLINA 27021
PHONE: 336-985-0863		FAX: 336-985-0884



DESIGN SPECIFICATIONS:

Construction Type: Commercial Residential

- Applicable Building Codes:
- 2018 North Carolina Residential Building Code
 - ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

Design Loads:

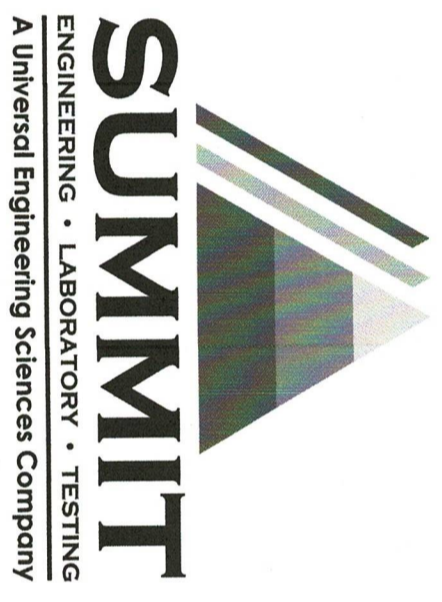
- Roof Live Loads
 - Conventional 2x 20 PSF
 - Truss 20 PSF
 - Attic Truss 60 PSF
- Roof Dead Loads
 - Conventional 2x 10 PSF
 - Truss 20 PSF
- Snow 1.0
- Importance Factor 1.0
- Floor Live Loads
 - Typ. Dwelling 40 PSF
 - Sleeping Areas 30 PSF
 - Decks 40 PSF
 - Passenger Garage 50 PSF
- Floor Dead Loads
 - Conventional 2x 10 PSF
 - 1-Joist 15 PSF
 - Floor Truss 15 PSF
- Ultimate Wind Speed (3 sec. gust) 130 MPH
- Exposure B
- Importance Factor 1.0
- Wind Base Shear
 - Vx =
 - Vy =

7. Component and Cladding (in PSF)

MEAN ROOF HT.	UP TO 30'	30'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

8. Seismic

- Site Class D
- Design Category C
- Importance Factor 1.0
- Seismic Use Group 1
- Spectral Response Acceleration
 - Sms = %g
 - Sml = %g
- Seismic Base Shear
 - Vx =
 - Vy =
- Basic Structural System (check one)
 - Bearing Wall
 - Building Frame
 - Moment Frame
 - Dual w/ Special Moment Frame
 - Dual w/ Intermediate R/C or Special Steel
 - Inverted Pendulum
- Arch/Mech Components Anchored No
- Lateral Design Control: Seismic Wind 2000psf
- Assumed Soil Bearing Capacity 2000psf



STRUCTURAL PLANS PREPARED FOR:

FARAH SHEA WAY

OWNER:
John Dove
2516 Brook Crossing Circle
Raleigh, NC 27606

DESIGNER:
Builders Plansource, Inc.
PO Box 836
King, North Carolina 27021

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory and Testing, INC. before construction begins.

PLAN ABBREVIATIONS:

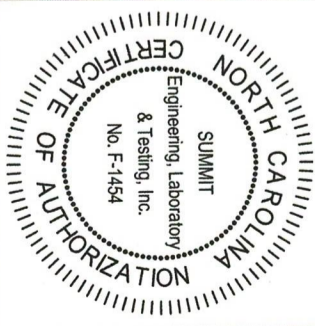
AB	Anchor Bolt	OC	On Center
ACI	American Concrete Institute	PCF	Pounds per Cubic Foot
ASCE	American Society of Civil Engineers	PCI	Pounds per Cubic Inch
AFA	American Fiberboard Association	PSF	Pounds per Square Foot
AFF	Above Finished Floor	PSI	Pounds per Square Inch
AISC	American Institute for Steel Construction	PT	Pressure Treated
APA	American Plywood Association	SC	Stud Column
AWS	American Welding Society	SER	Structural Engineer of Record
CJ	Ceiling Joist	SJ	Single Joist
CLR	Clear	SPF	Spruce Pine Fir
DBL	Double	SST	Simpson Strong Tie
DJ	Double Joist	ST	Single Truss
DSP	Double Stud Pocket	STD	Standard
EA	Each	SYP	Southern Yellow Pine
EE	Each End	TJ	Triple Joist
EOS	Edge of Slab	TOF	Top of Footing
EW	Each Way	TSP	Triple Stud Pocket
HDG	Hot Dipped Galvanized	TYP	Typical
NDS	Notion Design Spec. for Wood	UNO	Unless Noted Otherwise
NTS	Not to Scale	WWF	Welded Wire Fabric

SHEET LIST:

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
CS2	Specifications Continued
CS3	Revision Log
S1.0m	Monolithic Slab Foundation
S1.0s	Stem Wall Foundation
S1.0c	Crawl Space Foundation
S1.0b	Basement Foundation
S2.0	Basement Framing Plan
S3.0	First Floor Framing Plan
S4.0	Second Floor Framing Plan
S5.0	Roof Framing Plan
S6.0	Basement Bracing Plan
S7.0	First Floor Bracing Plan
S8.0	Second Floor Bracing Plan

REVISION LIST:

Revision No.	Date	Project No.	Description



PROJECT
Farah Shea Way-RH

CLIENT
John Dove
8626 Macedonia Lake Dr
Cary, NC 27578

CURRENT DRAWING
DATE: 06/6/2022
SCALE: 1/8"=1'-0"
PROJECT #: 2672.10416
DRAWN BY: JV
CHECKED BY: HKM

ORIGINAL INFORMATION
PROJECT # 2672.10416
DATE 06/11/2021

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET
CS1



STRUCTURAL MEMBERS ONLY

GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, INC. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code. All structural and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions.

FOUNDATIONS:

- Foundations shall be constructed in accordance with chapter 4 of the 2018 NC Residential Code (Special consideration shall be given to chapter 45 in wind zones 130 mph and above.)
- Footings sizes are based on a presumptive soil bearing capacity of 2000 PSF. The contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction.
- Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC. The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bear in the middle third of the pier. Pilasters to be bonded to the perimeter foundation wall.
- Crawl space to be graded level and clear of all debris.
- Proved foundation waterproofing and drain with positive slope to outlet as required by site conditions.
- Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC.

STRUCTURAL STEEL:

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- All steel shall have a minimum yield stress (F_y) of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shop and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.

CONCRETE:

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'_c) at 28 days of 3000 psi, unless noted otherwise on the plan.
- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
 - Footings: 5%
 - Exterior Slabs: 5%
- No admixtures shall be added to any structural concrete without written permission of the SER.
- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab Construction".
- The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless noted otherwise.
- Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished. Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
- All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely supported during the concrete pour.

CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- Fibermesh shall comply with ASTM C1116, any local building code requirements, and shall meet or exceed the current industry standard.
- Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement

with a class B tension splice.

- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

- Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP) #2.
- LVL or PSL engineered wood shall have the following minimum design values:
 - E = 1,900,000 psi
 - F_b = 2600 psi
 - F_v = 285 psi
 - F_c = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPFA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPFA standard C-2.
- Nails shall be common wire nails unless otherwise noted.
- Log screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for log screws shall be in accordance with NDS specifications.
- All beams shall have full bearing on supporting framing members unless otherwise noted.
- Exterior and load bearing stud walls are to be 2x4 SYP #2 @ 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Individual studs forming a column shall be attached with one 10d nail @ 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer.
- Multi-ply beams shall have each ply attached with (3) 12d nails @ 12" O.C.
- Fitch beams, 4-ply beams and 3-ply side loaded beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 24" O.C. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each end of the beam.

WOOD TRUSSES:

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses. SUMMIT shall be notified by the truss manufacturer/fabricator or the client of any discrepancies between the truss/joint layouts and the sealed structural plans prior to the start of construction.
- The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- All girder truss to girder truss connections, truss to top plate connections and uplift connections are the responsibility of the wood truss manufacturer/fabricator.
- The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- The truss manufacturer shall provide adequate bracing

Information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.

- Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

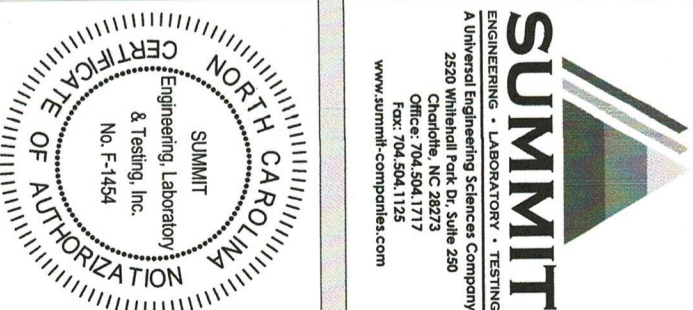
- Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- All structurally required wood sheathing shall bear the mark of the APA.
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6" O.C. at panel edges and at 12" O.C. in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6" O.C. at panel edges and at 12" O.C. in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.



PROJECT
Farah Shea Way-RH
Coversheet
CLIENT
John Dove
8626 Macedonia Lake Dr
Cary, NC 27578

CURRENT DRAWING
DATE: 06/6/2022
SCALE: 1/8"=1'-0"
PROJECT #: 2672.T0416
DRAWN BY: JV
CHECKED BY: HKM

ORIGINAL INFORMATION
PROJECT # DATE
2672.T0416 06/1/2021

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

CS2



STRUCTURAL MEMBERS ONLY

FOUNDATION NOTES:

1. FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NC RESIDENTIAL BUILDING CODE.
2. STRUCTURAL CONCRETE TO BE $f_c = 3000$ PSI, PREPARED AND PLACED IN ACCORDANCE WITH ACI STANDARD 318.
3. FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.
4. FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
5. ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3".
6. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROTECTION FROM THE FACE OF MASONRY.
7. MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION R904.1 OF THE 2018 NC RESIDENTIAL BUILDING CODE.
8. PLASTER TO BE BONDED TO PERIMETER FOUNDATION WALL.
9. PROVIDE FOUNDATION WATERPROOFING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.
10. PROVIDE PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NC RESIDENTIAL BUILDING CODE.
11. CORREL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEERS.
12. CRAWL SPACE TO BE GRADED LEVEL & CLEARED OF ALL DEBRIS.
13. FOUNDATION ANCHORAGE SHALL BE A MIN. OF 1/2" DIA. ANCHOR BOLTS AND SHALL EXTEND A MIN. OF 7" INTO MASONRY OR CONCRETE. BOLTS SHALL BE 6'-0" O.C. AND WITH IN 12" OF ALL PLATE SPLICES. MIN. (2) ANCHOR BOLTS PER PLATE SECTION.
14. ALL PIERS TO BE 16"x16" MASONRY AND ALL PLASTERERS TO BE 8"x16" MASONRY, TYPICAL (UNO).
15. WALL FOOTINGS TO BE CONTINUOUS CONCRETE. SIZES PER STRUCTURAL PLAN.
16. A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER, OR HIS QUALIFIED REPRESENTATIVE, IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION. SUMMIT MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
17. ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REINFORCE GARAGE PORTAL WALLS PER FIGURE R602.10.4.3 OF THE 2018 NCRC.

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER, U.N.O.

NOTE: BEAM POCKETS MAY BE SUBSTITUTED FOR 8"x16" CMU PIERS AT GIRDER ENDS. BEAM POCKETS SHALL HAVE A MIN. OF 4" SOLID WAS. BEARING.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS

100% CRAWL SPACE TO BE COVERED W/ 6 MIL. VAPOR BARRIER

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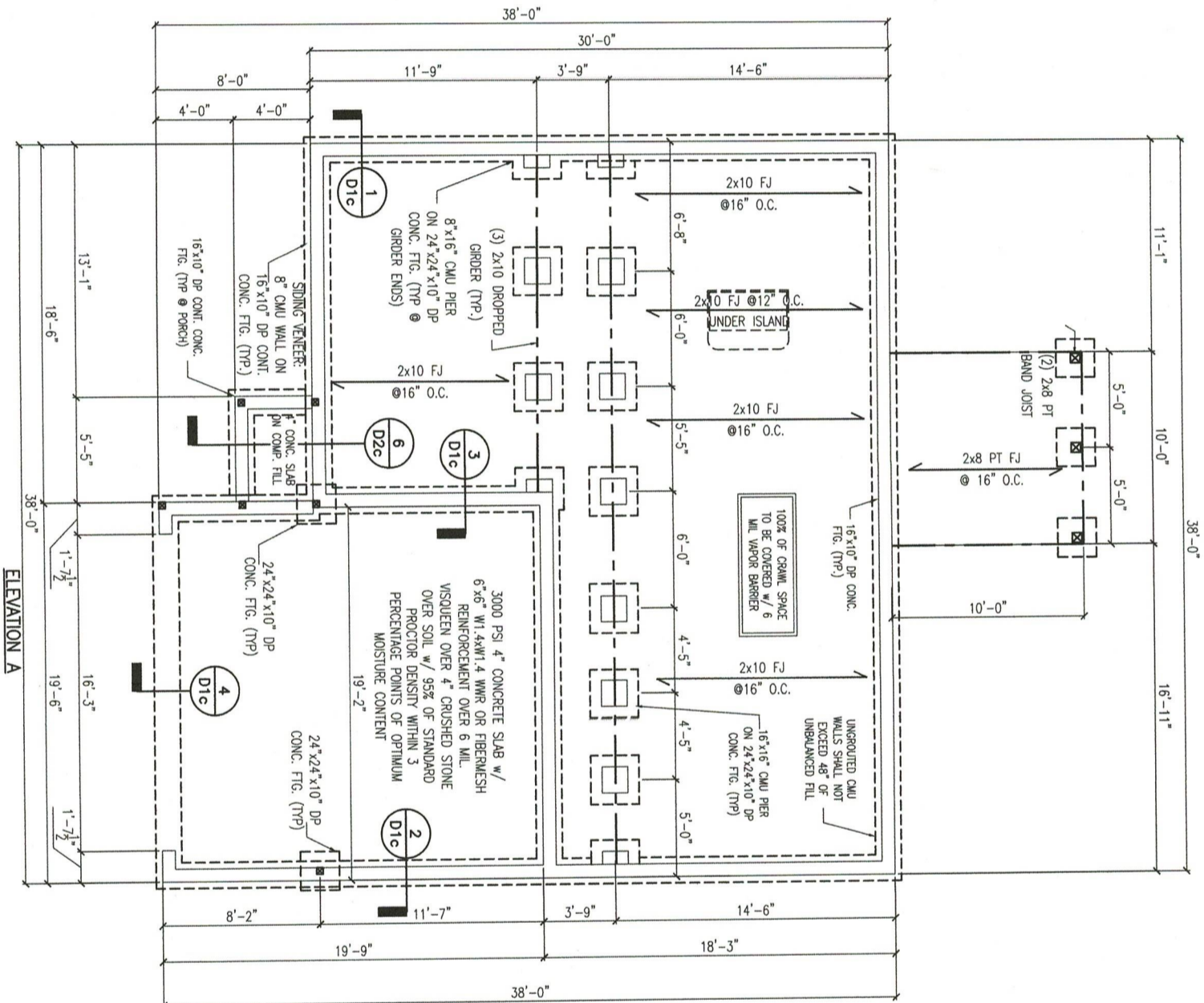
STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

CRAWL SPACE FOUNDATION

SCALE: 1/8"=1'-1"



36"x24" MIN. CRAWL SPACE ACCESS DOOR LOCATED BY BUILDER. PROVIDE MIN. (2) 2x10 HEADER OVER DOOR W/ MIN 4" BEARING EE

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLD-DOWNS. ADDITIONAL INFORMATION PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

NOTE: FOUNDATION ANCHORAGE HAS BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.

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CERTIFICATE OF AUTHORIZATION
NORTH CAROLINA
SUMMIT
Engineering, Laboratory
& Testing, Inc.
No. F-1454

PROJECT
Farah Shea Way-RH
Crawl Space Fnd.
CLIENT
John Dove
8626 Macedonia Lake Dr
Cary, NC 27578

CURRENT DRAWING
DATE: 06/6/2022
SCALE: 1/8"=1'-0"
PROJECT #: 2672.T0416
DRAWN BY: JV
CHECKED BY: HKM

ORIGINAL INFORMATION
PROJECT # DATE
2672.T0416 06/1/2021

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SEAL
053883
HENRY KUEHL
ENGINEER
NORTH CAROLINA
REGISTERED PROFESSIONAL ENGINEER
SUMMIT ENGINEERING, LABORATORY & TESTING, INC.
06/10/2022

SHEET
S1.0C

STRUCTURAL MEMBERS ONLY

HEADER SCHEDULE		
TAG	SIZE	JACKS (E)
A	(2) 2x6	(1)
B	(2) 2x8	(2)
C	(2) 2x10	(2)
D	(2) 2x12	(2)
E	(2) 9-1/4" LSL/LVL	(3)
F	(3) 2x6	(1)
G	(3) 2x8	(2)
H	(3) 2x10	(2)
I	(3) 2x12	(3)

NOTES:
 1. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.
 2. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE.
 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE UNLESS NOTED OTHERWISE.
 4. KING STUDS SHALL BE FRAMED PER TABLE R802.3(5) SUBNOTE d UNLESS NOTED OTHERWISE.

ALL HEADERS WHERE BRICK IS USED, TO BE:
 UNTEL (UN.O.)

UNTEL SCHEDULE:

STEEL ANGLES TO HAVE MIN. 4" BEARING ONTO BRICK AT EACH END.

- ① L3x3x1/4"
- ② L5x3x1/4"
- ③ L5x3-1/2x5/16"
- ④ L5x3-1/2x5/16" ROLLED OR EQUAL ARCHED COMPONENT.

NOTE: JOIST & BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: SHADED WALLS INDICATED LOAD BEARING WALLS.

TWO STORY WALL NOTE (BALLROOM FRAMING):
 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. w/ GROSS BRACING @ 6'-0" O.C. VERTICAL.

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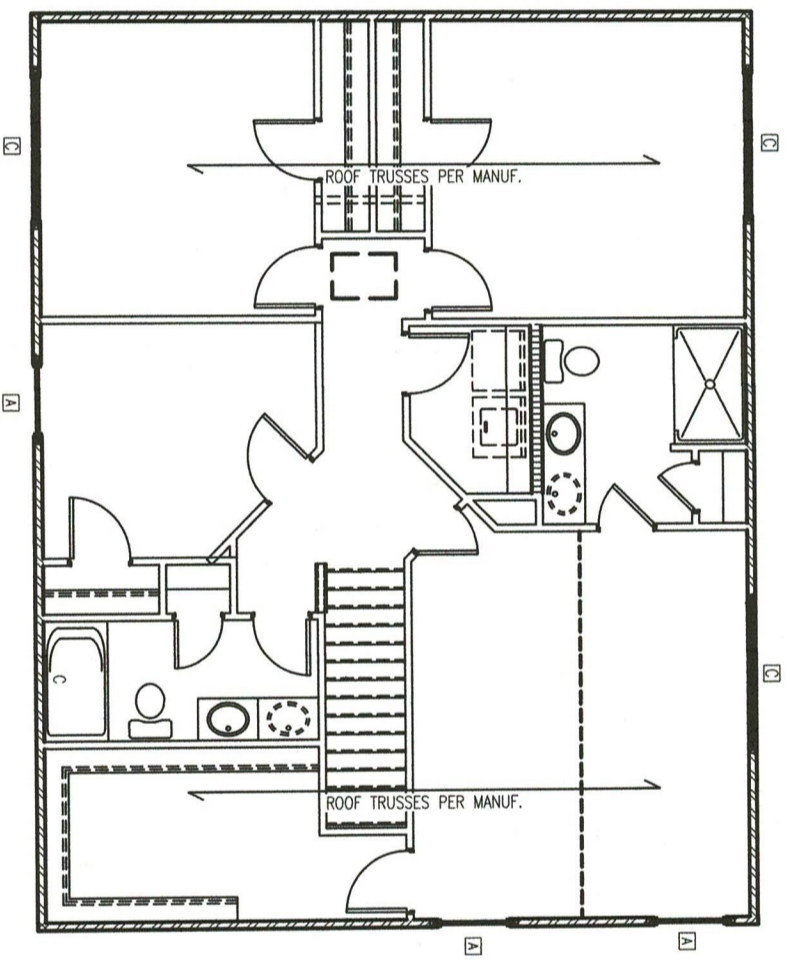
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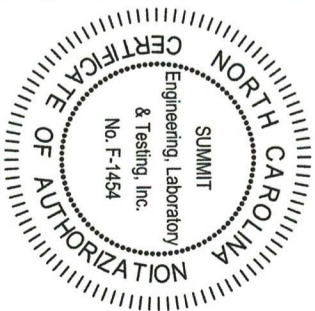
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR FRAMING PLAN

SCALE: 1/8"=1'-0"



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PROJECT
Farah Shea Way-RH
Second Floor Framing Plan
 CLIENT
John Dove
8626 Macedonia Lake Dr
Cary, NC 27578

CURRENT DRAWING
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 PROJECT #: 2672.T0416
 DRAWN BY: JV
 CHECKED BY: HKM

ORIGINAL INFORMATION
 PROJECT # DATE
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REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

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NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (UNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (UNO)

MAX. GIRDER TRUSS REACTION (LBS)			
NO	TRUSS	SP #2 TOP PLATE	2x6 WALL
1	OF PLYS	2x4 WALL	2x6 WALL
2		5134	7013
3		7702	10519
4		10289	14025
WITH TRUSS, SIP #2 TOP PLATE			
2		7045	8933
3		9622	12439
4		12189	15945

GIRDER TRUSS PLYS SHOWN ARE FOR ILLUSTRATION ONLY. PLEASE REFER TO TRUSS LAYOUT DRAWINGS PROVIDED BY TRUSS MANUF. FOR ACTUAL NUMBER OF PLYS REQ'D.

TRUSS UPLIFT CONNECTOR SCHEDULE			
MAX. UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO RFD
800 LBS	H2.5A	PER WALL SHEATHING & FASTENERS	DT1ZZ
1200 LBS	(2) H2.5A	CS16 (END = 11')	DT1ZZ
1450 LBS	H1S20	CS16 (END = 11')	DT1ZZ
2000 LBS	(2) H1S20	(2) CS16 (END = 11')	DT1ZZ
2900 LBS	(2) H1S20	(2) CS16 (END = 11')	HT14
3885 LBS	LG13-S1S2.5	MS1S2	HT14

- ALL PRODUCTS LISTED ARE SAMPSON STRONG-TEC EQUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.
- UPLIFT VALUES LISTED ARE FOR SIP #2 GRADE MEMBERS.
- REFER TO TRUSS LAYOUT PER MANUF. FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTIONS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE.
- CONTACT SUMMIT FOR REQUIRED CONNECTIONS WHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION R802.11.1.1, WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R802.3.5 OF THE 2018 NRC. REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

REFER TO DETAIL 5/03F FOR DORMER, RETURN OR SHED ROOF FRAMING REQUIREMENTS. (TYP FOR ROOFS PROTRUDING MAXIMUM 24" FROM STRUCTURE)

□

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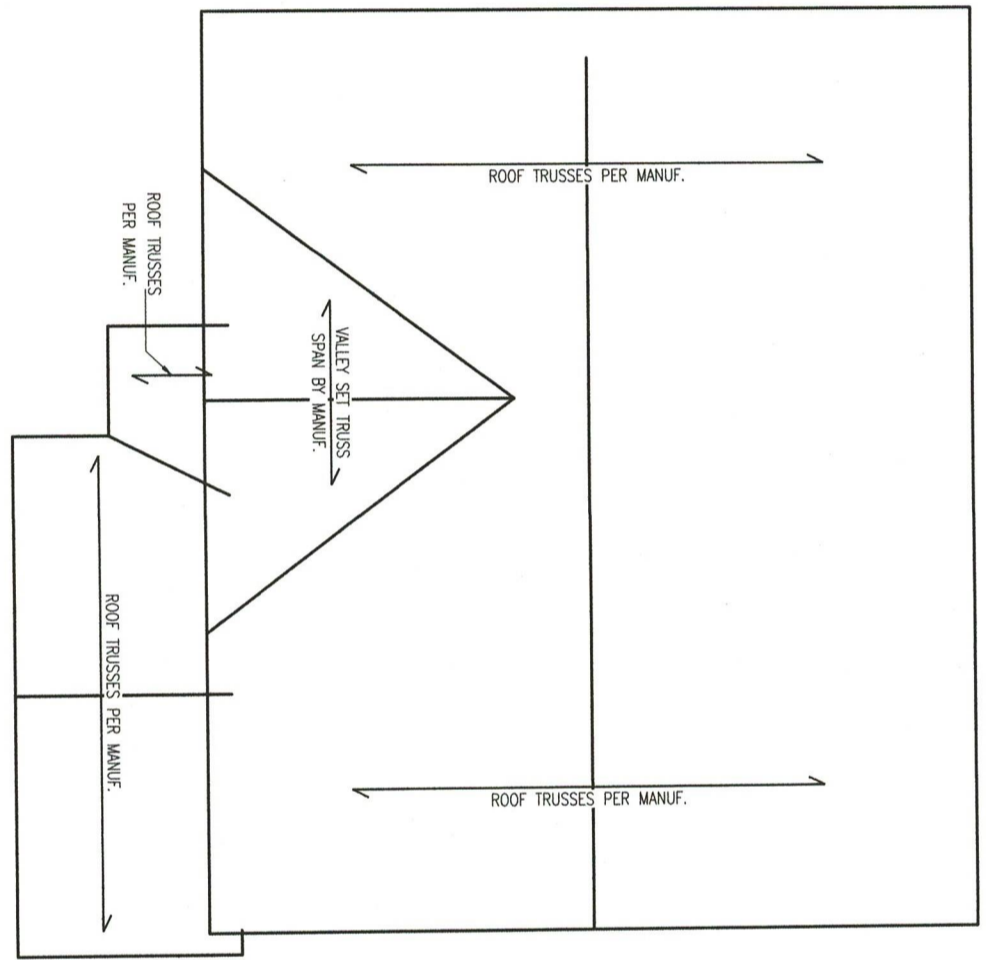
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STRUCTURAL ANALYSIS BASED ON 2018 NRC.

ROOF FRAMING PLAN

SCALE: 1/8"=1'



ELEVATION A

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Engineering, Laboratory & Testing, Inc.
No. F-1454

PROJECT
Farah Shea Way-RH
Roof Framing Plan

CLIENT
John Dove
8626 Macedonia Lake Dr
Cary, NC 27578

CURRENT DRAWING
DATE: 06/6/2022
SCALE: 1/8"=1'-0"
PROJECT #: 2672.10416
DRAWN BY: JV
CHECKED BY: HKM

ORIGINAL INFORMATION
PROJECT # DATE
2672.10416 06/11/2021
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SEAN MCCLAREN
ENGINEER
053883
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06/11/2022

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REQUIRED BRACED WALL PANEL CONNECTIONS				
METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION	
			● PANEL EDGES	● INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS ● 6" O.C.	6d COMMON NAILS ● 12" O.C.
GB	GRPSUM BOARD	1/2"	5d COOLER NAILS** ● 7" O.C.	5d COOLER NAILS** ● 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS ● 6" O.C.	6d COMMON NAILS ● 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1

BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH AMENDED PERMANENT RULES.
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND SPEEDS OF 130 MPH.
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GRPSUM BOARD (UNO).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- CORNERS AND BRACED WALL LINE INTERSECTIONS SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.3(3)
- A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH CORNER OF EACH ELEVATION VIEW OF THE HOUSE OR EACH END OF THE CIRCUMSCRIBED RECTANGLES.
- THE EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- ADEQUATE CONTINUOUS LOAD PATHS FOR TRANSFER OF BRACING LOADS AND UPLIFT LOADS SHALL COMPLY WITH SECTION R602.10.4
- MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.4.3
- BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.4.
- BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.5.
- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10.4.6
- BALLOON FRAMED WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10.4.8 WITH A MAXIMUM LENGTH OF 20 FEET.
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1 (UNO)
- ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
- ABBREVIATIONS:

GB = GRPSUM BOARD
 CS-XXX = CONT. SHEATHED
 PF = PORTAL FRAMED
 WSP = WOOD STRUCTURAL PANEL
 ENG = ENGINEERED SOLUTION

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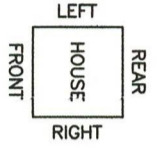
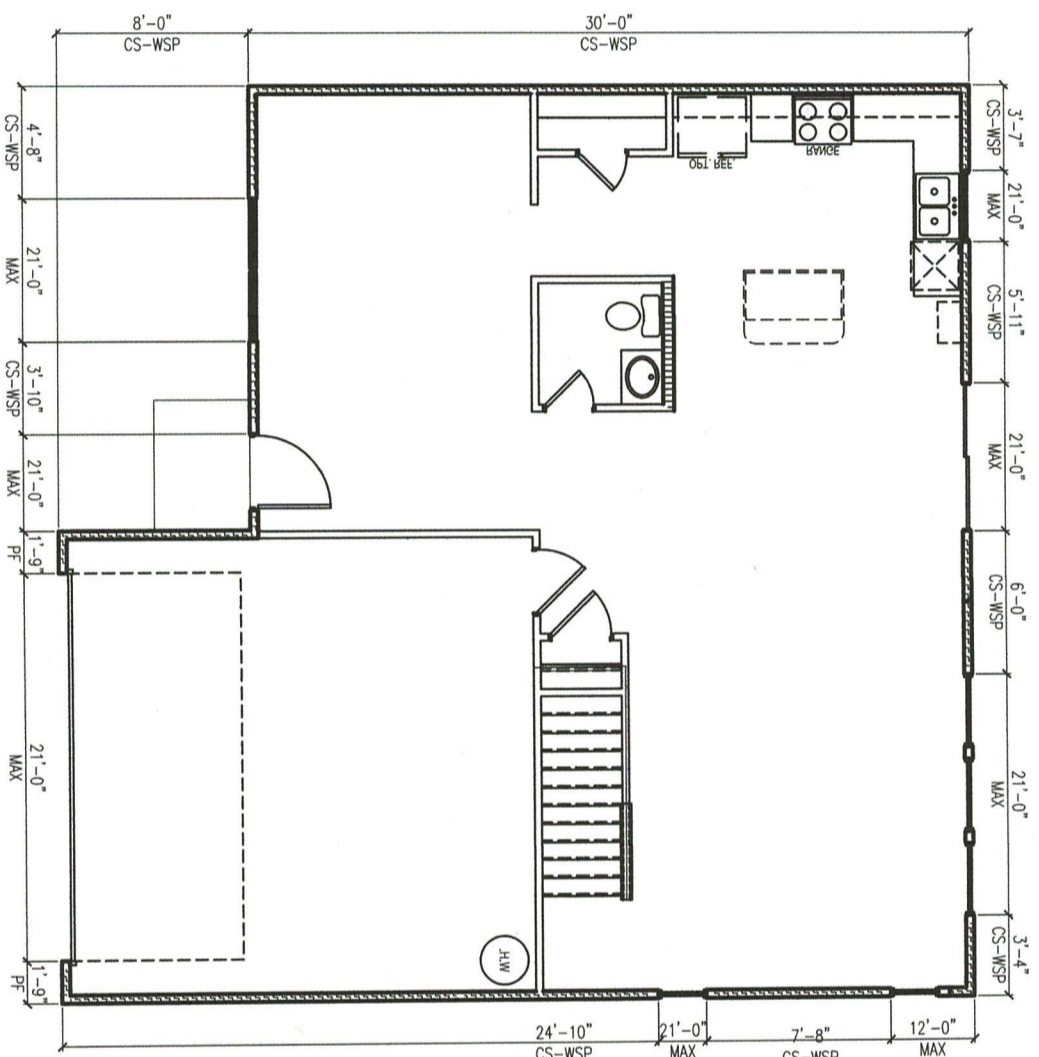
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR BRACING PLAN

SCALE: 1/8"=1'-0"

FIRST FLOOR BRACING (FT)			
CONTINUOUS SHEATHING METHOD - HOUSE			
	REQUIRED	PROVIDED	
FRONT	11.8	13.7	
RIGHT	11.8	32.5	
REAR	11.8	45.8	
LEFT	11.8	38.0	



NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5.

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

ALL ELEVATIONS

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 Engineering, Laboratory & Testing, Inc.
 No. F-1454
 CERTIFICATE OF AUTHORIZATION

PROJECT
Farah Shea Way-RH
First Floor Bracing Plan
 CLIENT
John Dove
8626 Macedonia Lake Dr
Cary, NC 27578

CURRENT DRAWING
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 PROJECT #: 2672.10416
 DRAWN BY: JV
 CHECKED BY: HKM

ORIGINAL INFORMATION
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 2672.10416 06/1/2021

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

NORTH CAROLINA
 ENGINEER
 HENRY KUEHR-SUMCLAREN
 053883
 SEAL
 06/10/2022

SHEET
S7.0

STRUCTURAL MEMBERS ONLY

REQUIRED BRACED WALL PANEL CONNECTIONS

METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION	
			● PANEL EDGES	● INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS ● 6" O.C.	6d COMMON NAILS ● 12" O.C.
GB	GIPSUM BOARD	1/2"	5d COOLER NAILS** ● 7" O.C.	5d COOLER NAILS** ● 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS ● 6" O.C.	6d COMMON NAILS ● 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R802.10.1	PER FIGURE R802.10.1

**OR EQUIVALENT PER TABLE R702.3.5

BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R802.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH AMENDED PERMANENT RULES.
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND SPEEDS OF 130 MPH.
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R802.10.1
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GIPSUM BOARD (UNO).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- CORNERS AND BRACED WALL LINE INTERSECTIONS SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R802.10.3(5)
- A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH CORNER OF EACH ELEVATION VIEW OF THE HOUSE OR EACH END OF THE CIRCUMSCRIBED RECTANGLES.
- THE EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- ADEQUATE CONTINUOUS LOAD PATHS FOR TRANSFER OF BRACING LOADS AND UPLIFT LOADS SHALL COMPLY WITH SECTION R802.10.4
- MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R802.10.4.3
- BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R802.10.4.4
- BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R802.10.4.5
- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R802.10.4.6
- BALLOON FRAMED WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R802.10.4.8 WITH A MAXIMUM LENGTH OF 20 FEET.
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R802.10.1 (UNO)
- ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
- ABBREVIATIONS:

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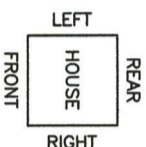
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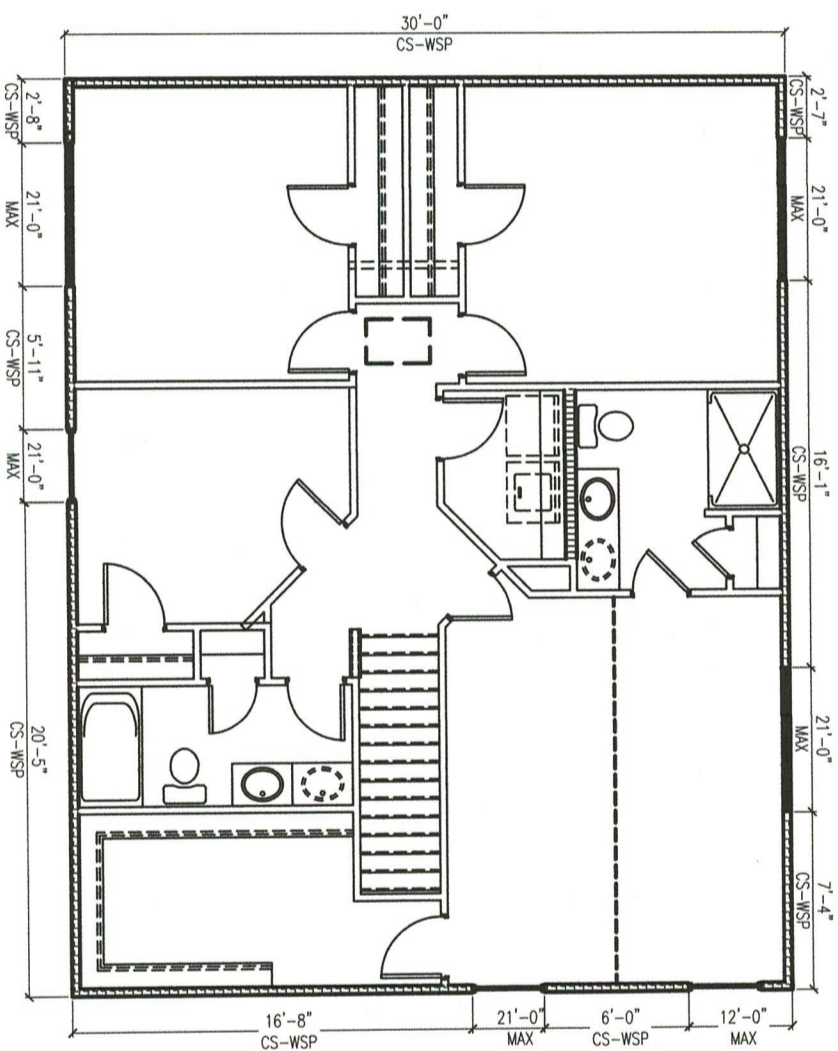
SECOND FLOOR BRACING PLAN

SCALE: 1/8"=1'-0"



SECOND FLOOR BRACING (FT)

CONTINUOUS SHEATHING METHOD - HOUSE	
REQUIRED	PROVIDED
FRONT	29.0
RIGHT	22.8
REAR	26.0
LEFT	30.0



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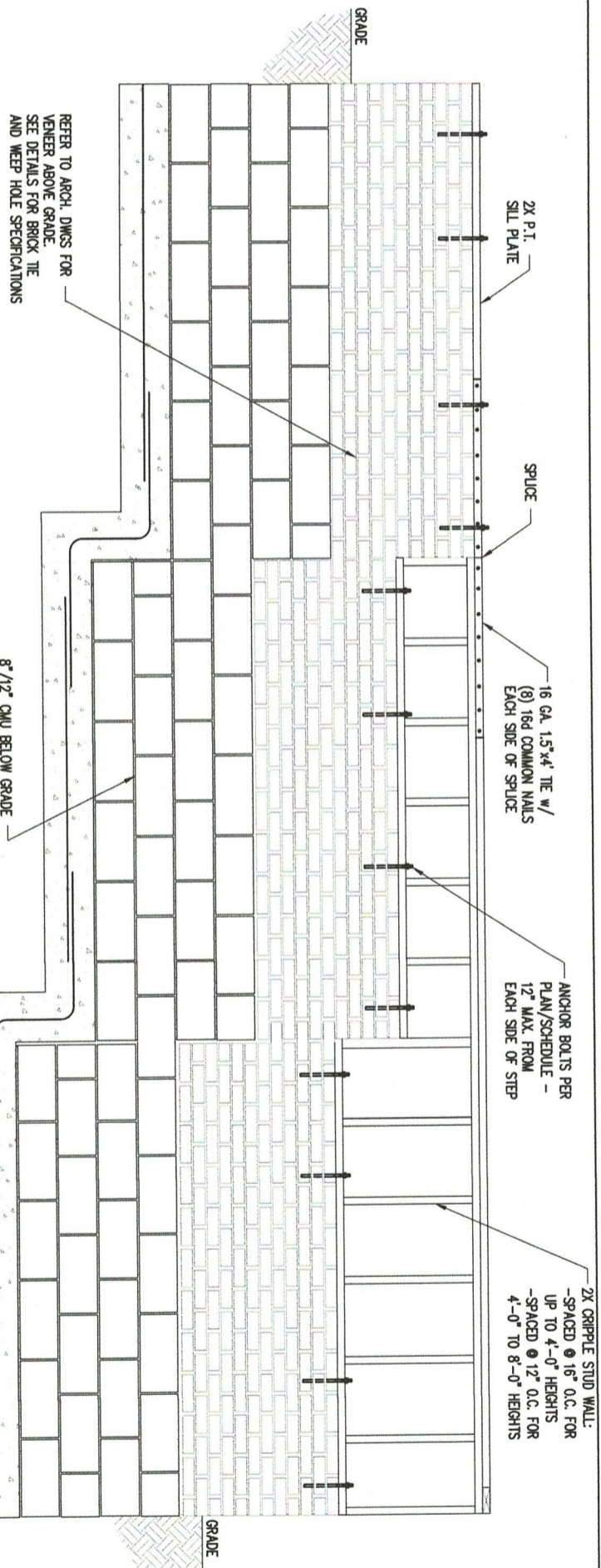
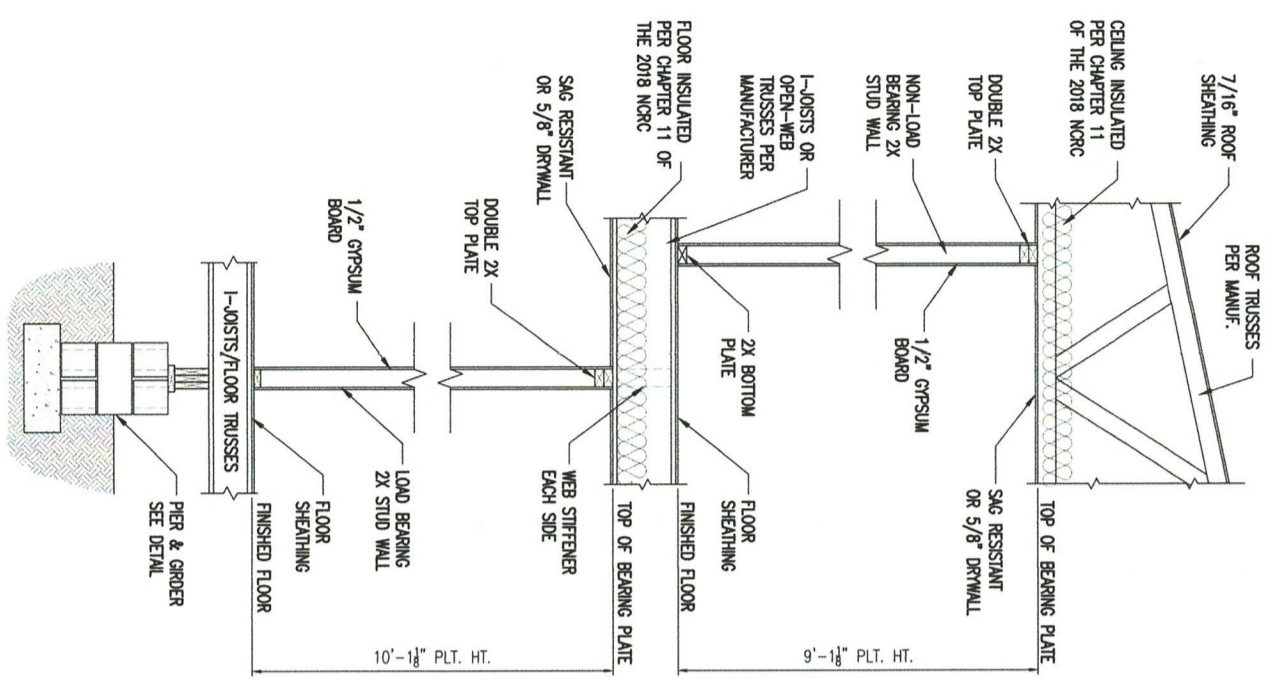
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 DATE 06/1/2021

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

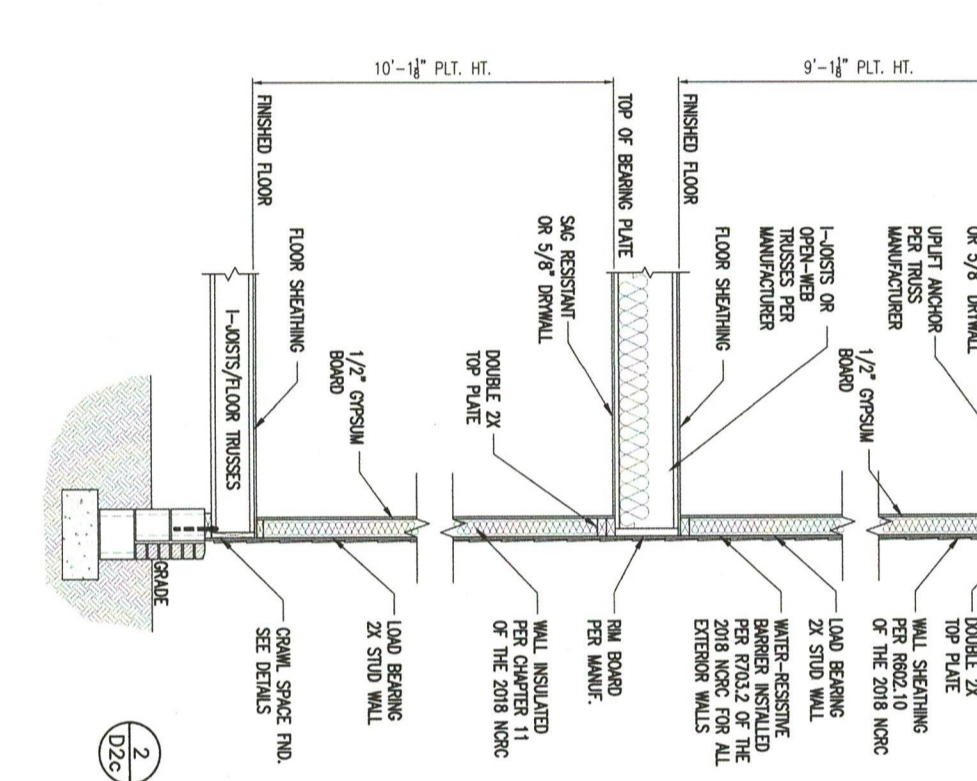
STRUCTURAL MEMBERS ONLY

SHEET
S8.0

1 TYP. INTERIOR LOAD BEARING WALL SECTION
 D2c 3/4" = 1'-0"



3 TYP. STEPPED FOUNDATION WALL DETAIL
 D2c N.T.S.



2 TYP. EXTERIOR LOAD BEARING WALL SECTION
 D2c 3/4" = 1'-0"

-SIMILAR W/ BRICK AND STONE
 -BRICK TIES SPACED @ 24" O.C. HORIZ. & 16" O.C. VERT.
 -MIN. 3/16" WEEP HOLES @ 33" O.C.

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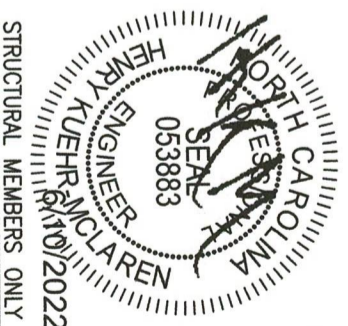
PROJECT
Farah Shea Way-RH
Crawlspace Details
 CLIENT
John Dove
 8626 Macedonia Lake Dr
 Cary, NC 27578

CURRENT DRAWING
 DATE: 06/6/2022
 SCALE: 1/8"=1'-0"
 PROJECT #: 2672.T0416
 DRAWN BY: JV
 CHECKED BY: HKM

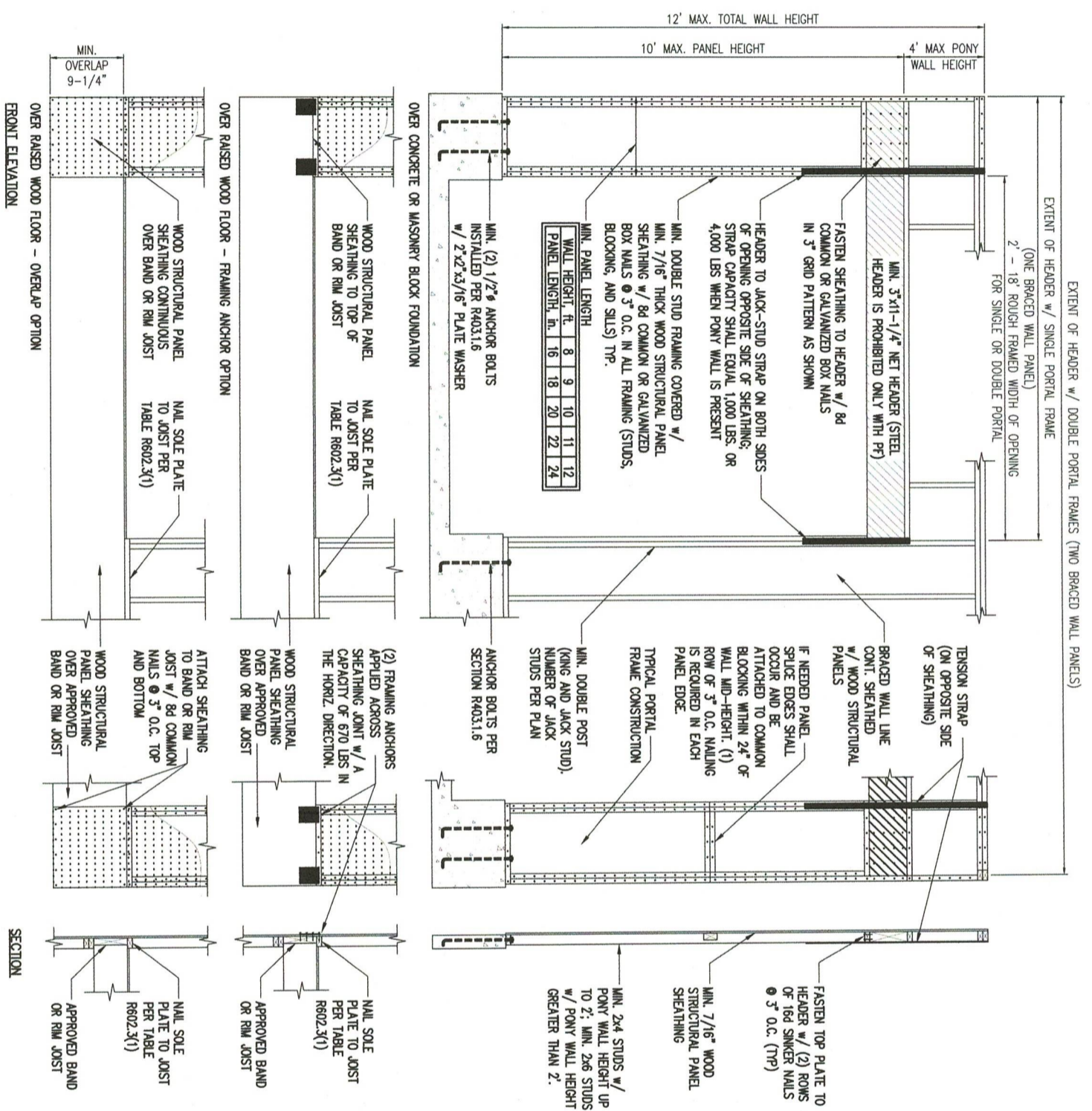
ORIGINAL INFORMATION
 PROJECT # DATE
 2672.T0416 06/1/2021

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D2c



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1 METHOD PF: PORTAL FRAME DETAIL
 D1f 3/8" = 1'-0"

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NORTH CAROLINA
 SUMMIT
 Engineering Laboratory
 & Testing, Inc.
 No. E-1454
 CERTIFICATE OF AUTHORIZATION

PROJECT
Farah Shea Way-RH
Framing Details
 CLIENT
John Dove
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Cary, NC 27578

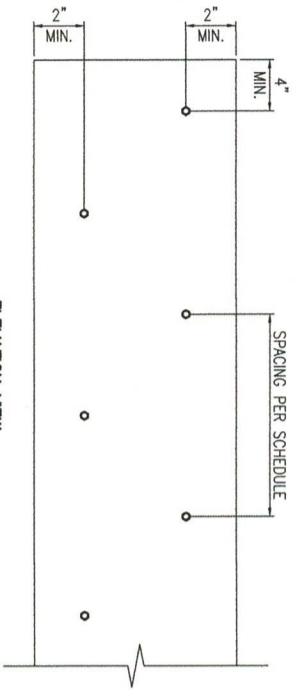
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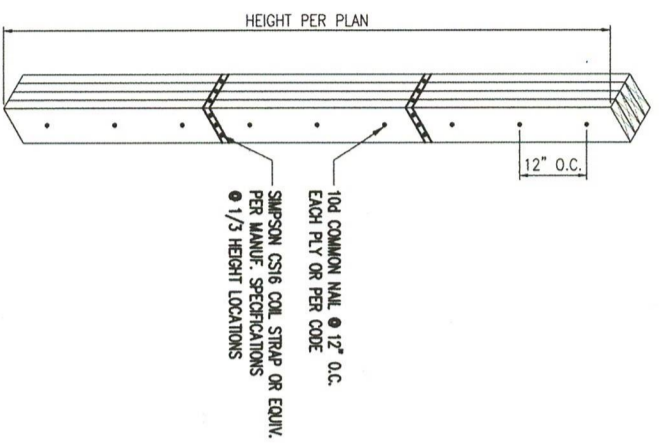
HENRY KUEHR-McCLAREN
 ENGINEER
 053883
 NORTH CAROLINA
 6/10/2022

SHEET
D1f

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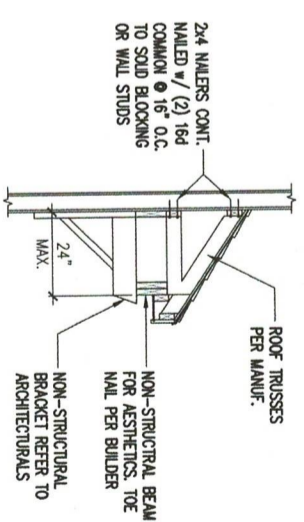
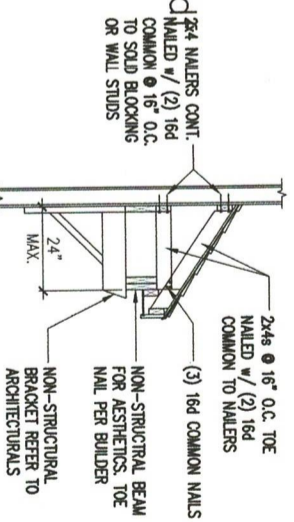


1 MULTI-PLY BEAM CONNECTION DETAIL
D3f N.T.S.



2 MULTI-PLY STUD CONNECTION DETAIL
D3f N.T.S. 4+ PILES

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Sheet: 1

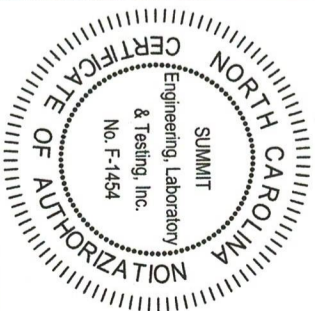


5 GABLE ROOF RETURN
D3f N.T.S.



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