

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 150_1910_A KB Home 150.1910.A

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I48063641 thru I48063656

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



September 24,2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



ł	10-9-6		20-0-0		27	7-7-8	28-0-0) 37-2-10			-		
	10-9-6	j	9-2-10	•	7	′-7-8	0-4-8	ç	-2-10			10-9-6	
LOADING TCLL (roof) Snow (Pf/P TCDL	(psf)) 20.0 (g) 16.5/15.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC BC	0.57 0.50		DEFL. Vert(LL) Vert(CT)	in -0.09 -0.21	(loc) 2-19 2-19	l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 197/144
BCLL BCDL	0.0 * 10.0	Code IRC2015/TF	PI2014	Matr	ix-S		HOI2(CT)	0.02	15	n/a	n/a	Weight: 349 lb	FT = 20%
LUMBER- TOP CHOF BOT CHOF WEBS	RD 2x6 SP No.2 RD 2x6 SP No.2 2x4 SP No.3 *E 6-15: 2x4 SP No	xcept* 5.2 or 2x4 SPF No.2				BR TC BC	ACING- OP CHORD	Structura except 2-0-0 oc Rigid cei	al wood purlins iling dire	sheathin (10-0-0 r ectly appl	g directly ap nax.): 6-7. ied or 6-0-0	plied or 5-6-13 oc purlin oc bracing.	S,
REACTION	IS. (size) 11=0 Max Horz 2=14 Max Uplift 11=- Max Grav 11=5	0-3-8, 15=(0-3-8 + H10A \$ I9(LC 16) 98(LC 17), 15=-20(LC 16 585(LC 55), 15=2557(LC	Simpson Stron), 2=-90(LC 16 3), 2=964(LC 5	g-Tie) (req.) 4)	0-4-0), 2=	=0-3-	-8	2 Rows	at 1/3 p	ts	6-15	5, 8-15	
FORCES. TOP CHOF	(lb) - Max. Comp./N RD 2-3=-1433/152 10-11=-609/16	lax. Ten All forces 250 2, 3-5=-1156/143, 5-6=-35	(lb) or less exc 59/160, 6-7=0/7	ept when sł 16, 7-8=0/8	nown. 384, 8-10=	=-329	9/202,						
BOT CHOF WEBS	10-11=-609/168 30T CHORD 2-19=-193/1214, 17-19=-62/712, 15-17=-31/340, 13-15=-312/133, 11-13=-59/483 NEBS 3-19=-410/190, 5-19=-21/613, 5-17=-795/187, 6-17=-45/862, 7-15=-777/123, 8-15=-804/188, 8-13=-19/621, 10-13=-424/190, 6-15=-1374/130												
NOTES- 1) Unbalan 2) Wind: AS gable en 28-0-0, E Lumber 3) TCLL: A roof snov governs. 4) Unbalan 5) This trus non-con 6) Provide 7) This trus 8) * This trus 8) * This trus 8) * This trus 9) H10A Si for uplift 10) One H2 connec 11) Graphia	ced roof live loads has SCE 7-10; Vult=120n Id zone and C-C Exte Exterior(2) 28-0-0 to 5 DOL=1.60 plate grip SCE 7-10; Pr=20.0 p w: Lumber DOL=1.15 . Rain surcharge app ced snow loads have is has been designed current with other live adequate drainage to is has been designed taken the bottom ch mpson Strong-Tie co only and does not co 2.5A Simpson Strong tion is for uplift only a cal purlin representat	ave been considered for t hph Vasd=95mph; TCDL= prior(2) -0-10-8 to 3-11-2, 34-9-7, Interior(1) 34-9-7 DOL=1.60 sf (roof live load: Lumber 5 Plate DOL=1.15); Catego blied to all exposed surface a been considered for this b for greater of min roof live b loads. c) prevent water ponding. a for a 10.0 psf bottom ch- ded for a live load of 20.0pr ord and any other member ord and any other member ord and any other member onsider lateral forces. -Tie connectors recommended to and does not consider later ion does not depict the si	his design. =6.0psf; BCDL= Interior(1) 3-11 to 48-10-8 zone DOL=1.15 Pla ory II; Exp B; F res with slopes design. re load of 12.0 ord live load nc of on the bottor pers, with BCDL o connect truss ended to conne eral forces. ze or the orient	=6.0psf; h=2 -2 to 20-0-(e;C-C for m te DOL=1.1 Partially Exp less than 0 psf or 1.00 nconcurren n chord in a = 10.0psf. s to bearing ct truss to b ration of the	25ft; Cat. I 0, Exterior embers a 5); Pg=15 .; Ct=1.10 .500/12 in times flat t with any ill areas w walls due bearing wa	II; Ex r(2) 2 and fo 5.0 p: 0, Lu: n acc roof y othere e to L alls d ong th	p B; Enclosed; N 20-0-0 to 26-9-7, prces & MWFRS sf (ground snow) =50-0-0; Min. fla ordance with IBC load of 11.6 psf ar live loads. e a rectangle 3-6 JPLIFT at jt(s) 15 lue to UPLIFT at he top and/or bo	MWFRS (Interior(1 for reacti troof snc 2 1608.3. on overh -0 tall by 5. This cc i jt(s) 11 a ttom choi	envelop 1) 26-9- ions sho 5 psf (fla bw load 4. angs 2-0-0 w onnectic and 2. T	oe) 7 to own; at ride on is 'his	Summer Street	SEAL 044925 MGINEE September 24,2	

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





L	10-9	-6	20-0-0	27-7-8	28 ₀ -0	37-2-10			48-0-0	
	10-9		9-2-10	7-7-8	0-4-8	9-2-10			10-9-6	
Plate Utisets (X	, r) [6:0-3-0,	U-3-4j, [7:U-3-8,U-3-0j, [15:U-1 T	1-12,0-2-4], [18:0-2-4,	0-2-0]	1					
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLI	20.0 6.5/15.0 10.0 0.0 *	SPACING- 2- Plate Grip DOL 1 Lumber DOL 1 Rep Stress Incr 1	-0-0 CS I 1.15 TC 1.15 BC YES WB	0.59 0.51 0.64	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.35 18-20 -0.64 18-20 0.09 11	l/defl >941 >512 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code IRC2015/TPI20	014 Mat	trix-S	Attic	0.04 15-16	422	360	Weight: 373 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x6 SP No.2 2x6 SP DSS *E	xcept*		B Tr	RACING- OP CHORD	Structural wood except	sheathing	g directly app	lied or 3-6-12 oc purlin	IS,
WEBS	2x4 SP No.3 *E 6-18,7-15: 2x4 \$	xcept* SP No.2 or 2x4 SPF No.2		B Vi J(OT CHORD /EBS OINTS	Rigid ceiling dir 1 Row at midpt 1 Brace at Jt(s):	ectly applie	ed or 10-0-0 5-18, 8-15	oc bracing. , 21-22	
	Max Horz 2=-1 Max Uplift 2=-1 Max Grav 2=18	49(LC 17) 44(LC 16), 11=-18(LC 16), 10 344(LC 57), 11=1708(LC 3), 1	6=-146(LC 17) 16=910(LC 58)							
TOP CHORD	- Max. Comp./M 2-3=-3464/269 8-10=-2875/20	1ax. Ten All forces 250 (lb) 9, 3-5=-3230/266, 5-6=-2402/ 64. 10-11=-3101/262	or less except when s 285, 6-7=-2081/299,	shown. 7-8=-2274/297	7,					
BOT CHORD	2-20=-296/30 11-13=-127/2	08, 18-20=-166/2563, 16-18= 706	-39/2078, 15-16=-0/1	956, 13-15=-4	6/2317,					
WEBS	3-20=-359/180 15-22=-74/582	6, 5-20=-24/658, 5-18=-883/1 2, 7-22=-49/627, 8-15=-691/2	93, 18-21=-44/817, 6 03, 8-13=-38/532, 10	-21=-15/814, -13=-396/191						
NOTES- 1) Unbalanced r 2) Wind: ASCE gable end zo 28-0-0, Exter Lumber DOL. 3) TCLL: ASCE roof snow: Lu governs. Rai 4) Unbalanced s 5) This truss has non-concurre 6) Provide adeq 7) This truss has non-concurre 6) Provide adeq 7) This truss has non-concurre 9) Ceiling dead 10) Bottom chor 11) One H2.5A This connec 21) Graphical pi 13) ATTIC SPA	roof live loads h: 7-10; Vult=120n ne and C-C Exti ior(2) 28-0-0 to 2 =1.60 plate grip 7-10; Pr=20.0 p umber DOL=1.11 in surcharge app snow loads have s been designed nt with other live juate drainage to s been designed as been designed in the bottom ch load (5.0 psf) or rd live load (40.0 Simpson Strong ction is for uplist CE SHOWN IS	ave been considered for this of nph Vasd=95mph; TCDL=6.0 erior(2) -0-10-8 to 3-11-2, Inte 34-9-7, Interior(1) 34-9-7 to 4 DOL=1.60 stf (roof live load: Lumber DO 5 Plate DOL=1.15); Category blied to all exposed surfaces to a been considered for this des d for greater of min roof live lo be loads. b prevent water ponding. d for a 10.0 psf bottom chord l ed for a live load of 20.0psf or ord and any other members, n member(s). 21-22 0 psf) and additional bottom c p-Tie connectors recommender only and does not consider la tion does not depict the size of DESIGNED AS UNINHABITA	design. psf; BCDL=6.0psf; h= rrior(1) 3-11-2 to 20-0 8-10-8 zone;C-C for r PL=1.15 Plate DOL=1. II; Exp B; Partially Ex with slopes less than r sign. bad of 12.0 psf or 1.00 live load nonconcurre n the bottom chord in with BCDL = 10.0psf. hord dead load (0.0 p ed to connect truss to iteral forces. or the orientation of th ABLE.	25ft; Cat. II; E -0, Exterior(2) nembers and f 15); Pg=15.0 p.; Ct=1.10, Li 0.500/12 in ac 0 times flat roo nt with any ott all areas wher sf) applied onl bearing walls e purlin along	xp B; Enclosed; I 20-0-0 to 26-9-7, forces & MWFRS psf (ground snow u=50-0-0; Min. fla cordance with IBr f load of 11.6 psf her live loads. re a rectangle 3-6 ly to room. 16-18, due to UPLIFT a the top and/or bo	MWFRS (envelop Interior(1) 26-9- for reactions shi troof snow load C 1608.3.4. on overhangs 6-0 tall by 2-0-0 w , 15-16 t jt(s) 2, 11, and httom chord.	be) 7 to own; at ride 16.	State State State	SEAL 044925 MGINEE September 24,2	
WARNING Design valid a truss system building desig is always req fabrication, st Safety Inform	Or Verify design para for use only with Mil m. Before use, the b gn. Bracing indicate uired for stability and torage, delivery, ere mation available from	meters and READ NOTES ON THIS A Tek® connectors. This design is base uilding designer must verify the applied d is to prevent buckling of individual t d to prevent collapse with possible pe ction and bracing of trusses and truss m Truss Plate Institute, 2670 Crain H	ND INCLUDED MITEK REF d only upon parameters shu cability of design parameter russ web and/or chord men resonal injury and property of s systems, see AN dighway, Suite 203 Waldorf	ERENCE PAGE N own, and is for an s and properly inc hbers only. Additin lamage. For gene SI/TP11 Quality C MD 20601	III-7473 rev. 5/19/2020 individual building cor corporate this design in onal temporary and pe real guidance regardin Criteria, DSB-89 and	BEFORE USE. mponent, not nto the overall ermanent bracing ig the BCSI Building Com	ponent		A MITEK BIS Soundside Road Edenton, NC 27932	Affiliate



		48-0-	0					
		48-0-	0					
Plate Offsets (X,Y) [12:0-3-	0,0-4-0], [16:0-3-0,0-4-0], [32:0-4-0,0-4-8], [38:0-4-0,0-4-8], [44:0	-4-0,0-4-8]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.05 WB 0.18 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (lo 0.00 2 0.00 2 0.01 2	c) l/defl 27 n/r 27 n/r 26 n/a	L/d 120 120 n/a	PLATES MT20 Weight: 435 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2		B	RACING-	Structural we	ood sheathir	ng directly ap	oplied or 6-0-0 oc purlins	, except
OTHERS 2x4 SP No.3		B V	OT CHORD VEBS	2-0-0 oc pur Rigid ceiling 1 Row at mid	directly app directly app	iax.): 12-16. lied or 10-0- 16-36, 1 17-35	0 oc bracing. 5-37, 14-38, 13-39, 12-4	0, 11-41,

REACTIONS. All bearings 48-0-0.

(lb) - Max Horz 2=149(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 37, 38, 39, 41, 42, 43, 44, 45, 46, 47, 48, 35, 34, 33, 32, 31, 30, 29, 28, 2

Max Grav All reactions 250 lb or less at joint(s) 26, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 35, 34, 33, 32, 31, 30, 29, 2 except 48=310(LC 54), 28=310(LC 55)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 10-11=-91/258, 11-12=-103/293, 12-13=-92/282, 13-14=-91/282, 14-15=-91/282,

15-16=-92/282, 16-17=-103/296, 17-18=-91/261

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 4-0-0, Exterior(2) 4-0-0 to 20-0-0, Corner(3) 20-0-0 to 24-9-10, Exterior(2) 24-9-10 to 28-0-0, Corner(3) 28-0-0 to 32-9-10, Exterior(2) 32-9-10 to 48-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 5) Unbalanced snow loads have been considered for this design.

- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.

10) Gable studs spaced at 2-0-0 oc.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

13) n/a

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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ł	<u> </u>	6	<u>20-0-0</u> 9-2-10		28-0-0		37-0-0	37-2-	10 0	47-8-8 10-5-14	<u>48</u> 70-0 0-3-8
Plate Offse	ts (X,Y) [6:0-3-0,	0-3-4], [7:0-3-8,0-:	3-0], [15:0-1-12,0-2-4],	[17:0-2-0,0-2-0	0]				-		
LOADING TCLL (roof) Snow (Pf/P TCDL BCLL BCDL	(psf)) 20.0 g) 16.5/15.0 10.0 0.0 * 10.0	SPACING Plate Grip Lumber DO Rep Stress Code IRC	- 2-0-0 DOL 1.15 DL 1.15 s Incr YES 2015/TPI2014	CSI. TC 0 BC 0 WB 0 Matrix-S	0.59 0.50 0.88 S	DEFL. Vert(LL) Vert(CT) Horz(CT) Attic	in (loc) -0.35 17-19 -0.65 17-19 0.08 11 -0.18 15-17	l/defl >999 >676 n/a 568	L/d 240 180 n/a 360	PLATES MT20 Weight: 374 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHOF BOT CHOF WEBS REACTION	RD 2x6 SP No.2 RD 2x6 SP DSS *E 15-17: 2x6 SP N 2x4 SP No.3 *E 6-17,7-15: 2x4 s IS. (size) 13= Max Horz 2=-1 Max Uplift 13=- Max Grav 13=	xcept* No.2 xcept* SP No.2 or 2x4 SF 0-3-8, 2=0-3-8, 11 49(LC 17) 192(LC 17), 2=-1: 1264(LC 58), 2=18	PF No.2 =0-3-0 20(LC 16), 11=-74(LC 875(LC 3), 11=1306(L0	16) C 3)	BR TO BC WE JO	ACING- P CHORD DT CHORD EBS INTS	Structural wood 2-0-0 oc purlins Rigid ceiling dird 1 Row at midpt 1 Brace at Jt(s):	sheathing (4-5-8 max ectly applie	directly ap (.): 6-7. d or 10-0-(5-17, 8-1	plied or 3-6-4 oc purlir) oc bracing. 5, 20-21	is, except
FORCES. TOP CHOP BOT CHOP WEBS	(lb) - Max. Comp./N 2-3=-3531/24: 8-10=-2080/24 2-19=-251/300 3-19=-357/180 15-21=-43/605	Max. Ten All forc 2, 3-5=-3297/248, 61, 10-11=-2234/2 67, 17-19=-119/26 6, 5-19=-31/656, 5 9, 7-21=-18/655, 8	es 250 (lb) or less exc 5-6=-2471/265, 6-7=-2 259 524, 15-17=0/2112, 13- 5-17=-877/199, 17-20= 3-15=-40/382, 8-13=-85	ept when show 2141/282, 7-8= -15=-75/1943, ⁻ -23/862, 6-20= 53/174, 10-13=	/n. -2391/275, 11-13=-160 0/858, -430/197)/1940					
NOTES- 1) Unbalan 2) Wind: A: gable er 28-0-0, I Lumber 3) TCLL: A roof sno governs 4) Unbalan 5) This trus non-con 6) Provide 7) All plate: 8) This trus 9) * This trus 9) * This trus 10) Ceiling 11) Bottom 12) One H: This cc 13) Graphi 14) ATTIC	ced roof live loads h SCE 7-10; Vult=120r Id zone and C-C Exte Exterior(2) 28-0-0 to DOL=1.60 plate grip SCE 7-10; Pr=20.0 p w: Lumber DOL=1.19 . Rain surcharge ap ced snow loads have is has been designed is has been designed to hord live load (AU) to hord live load (5.0 psf) of chord live load (40.0 2.5A Simpson Strong innection is for uplift cal purlin represental SPACE SHOWN IS	ave been consider nph Vasd=95mph; erior(2) -0-10-8 to 34-9-7, Interior(1) DOL=1.60 ssf (roof live load: 1 5 Plate DOL=1.15 blied to all expose a been considered d for greater of mir e loads. b prevent water po ss otherwise indic d for a 10.0 psf bo ed for a live load c for a and any other on member(s). 20- 0 psf) and addition 5-Tie connectors rr only and does not tion does not depi DESIGNED AS U	red for this design. ; TCDL=6.0psf; BCDL= 3-11-2, Interior(1) 3-1 34-9-7 to 48-10-8 zon Lumber DOL=1.15 Pla); Category II; Exp B; F d surfaces with slopes I for this design. n roof live load of 12.0 onding. ated. ttom chord live load no of 20.0psf on the bottor members, with BCDL -21 hal bottom chord dead ecommended to conne consider lateral forces of the size or the orien NINHABITABLE.	=6.0psf; h=25ft; -2 to 20-0-0, E e;C-C for memili- te DOL=1.15); Partially Exp.; C less than 0.500 psf or 1.00 time enconcurrent win n chord in all a = 10.0psf. load (0.0 psf) a ct truss to beau tation of the pu	; Cat. II; Ex xterior(2) 2 bers and fo Pg=15.0 ps Ct=1.10, Lu: 0/12 in acc es flat roof ith any other reas where upplied only ring walls d rlin along th	p B; Enclosed; N 20-0-0 to 26-9-7, rrces & MWFRS sf (ground snow) =50-0-0; Min. fla ordance with IBC load of 11.6 psf er live loads. e a rectangle 3-6 to room. 15-17 lue to UPLIFT at he top and/or bo	//WFRS (envelop Interior(1) 26-9- for reactions sho troof snow load C 1608.3.4. on overhangs -0 tall by 2-0-0 w jt(s) 13, 2, and 1 ttom chord.	be) 7 to own; at vide 11.	Contraction of the second seco	SEAL 044925 MGINEE September 24,	
WAI Design a truss building is alwa fabrica Safety	RNING - Verify design para valid for use only with Mil system. Before use, the b g design. Bracing indicate ys required for stability am tion, storage, delivery, ere Information available fro	meters and READ NOT Fek® connectors. This uilding designer must v d is to prevent buckling d to prevent collapse w ction and bracing of tru om Truss Plate Institute	ES ON THIS AND INCLUDE! design is based only upon pr verify the applicability of desi g of individual truss web and/ ith possible personal injury a ssees and truss systems, see e, 2670 Crain Highway, Suite	D MITEK REFEREN arameters shown, a gn parameters and or chord members nd property damag ANSI/TP 203 Waldorf, MD 2	NCE PAGE MII and is for an in properly inco only. Additior ge. For genera 11 Quality Cri 20601	-7473 rev. 5/19/2020 ndividual building con rporate this design in hal temporary and pe al guidance regarding iteria, DSB-89 and F	BEFORE USE. nponent, not to the overall irmanent bracing g the BCSI Building Comp	ponent		TREASING BY A MITE 818 Soundside Road Edenton, NC 27932	CO k Affiliate



H	10-9-6 10-9-6		20-0-0 9-2-10		27-8-8 7-8-8	8 <u>28</u> -0-0 0-3-8	34-7-1 6-7-1	37-2	- <u>10 40-0-0 </u> -9 2-9-6	42-4-4 2-4-4 5	<u>'-8-8</u> -4-4	<u>48</u> 70-0 0-3-8
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC BC WB Matrix-	0.57 0.68 0.89 S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.15 1 -0.27 0.08	(loc) l/def 8-20 >999 2-20 >999 13 n/a	L/d 240 180 a n/a	PLATE: MT20 Weight:	S 355 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS.	2x6 SP No.2 2x6 SP No.2 2x4 SP No.3 *Ex 6-16: 2x4 SP No (size) 2=0-3 Max Horz 2=14 Max Uplift 2=-10 Max Grav 2=17	ccept* .2 or 2x4 SPF No.2 3-8, 13=(0-3-8 + H10A Sir 9(LC 16))3(LC 16), 13=-51(LC 17) 05(LC 2), 13=2345(LC 2)	npson Strong-Tir , 11=-232(LC 56 , 11=21(LC 16)	e) (req. 0-3	BI TC B(3-11), 11=0-	RACING- DP CHORD DT CHORD EBS 3-0	Structural 2-0-0 oc p Rigid ceili 6-0-0 oc b 1 Row at p	wood sheati ourlins (5-2-5 ng directly a pracing: 11-1 midpt	ning directly ap max.): 6-7. oplied or 10-0-1 3. 5-18, 6-1	oplied or 3-8-3 or 0 oc bracing, E	c purlins	s, except
FORCES. (IIL TOP CHORD BOT CHORD WEBS	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. FOP CHORD 2-3=-3036/251, 3-5=-2785/257, 5-6=-2044/284, 6-7=-1540/275, 7-8=-1783/263, 8-10=-1337/174, 10-11=-76/935 3OT CHORD 2-20=-217/2637, 18-20=-86/2193, 16-18=0/1755, 14-16=-35/1406, 13-14=0/446, 11-13=-749/121 WEBS 3-20=-383/190, 5-20=-20/590, 6-18=-47/857, 5-18=-781/186, 7-16=0/475, 8-16=-22/330, 8-14=-758/106, 10-14=0/1137, 6-16=-446/97, 10-13=-2466/215											
NOTES- 1) Unbalancec 2) Wind: ASCI gable end z 28-0-0, Extt Lumber DO 3) TCLL: ASC roof snow: I governs. R 4) Unbalancec 5) This truss h non-concur 6) Provide add 7) This truss h 8) * This truss s will fit betwe 9) H10A Simp for uplift onl 10) One H2.5/ connectior 11) Graphical	d roof live loads ha E 7-10; Vult=120m cone and C-C Exte erior(2) 28-0-0 to 3 vL=1.60 plate grip I E 7-10; Pr=20.0 ps Lumber DOL=1.15 ain surcharge app d snow loads have has been designed rent with other live equate drainage to has been designed has been designed	ve been considered for the ph Vasd=95mph; TCDL= rior(2) -0-10-8 to 3-11-2, 1 4-9-7, Interior(1) 34-9-7 to DOL=1.60 5f (roof live load: Lumber / Plate DOL=1.15); Catego lied to all exposed surface been considered for this for greater of min roof live loads. prevent water ponding. for a 10.0 psf bottom cho d for a live load of 20.0ps ord and any other membe onnectors recommended to nsider lateral forces. Tie connectors recomme ind does not consider later on does not depict the size	ais design. 6.0psf; BCDL=6. Interior(1) 3-11-2 o 48-10-8 zone;C DOL=1.15 Plate ory II; Exp B; Par as with slopes less design. e load of 12.0 ps and live load nonco f on the bottom or rs, with BCDL = o connect truss to nded to connect ral forces. ze or the orientati	Opsf; h=25f to 20-0-0, l >-C for men DOL=1.15) tially Exp.; ss than 0.50 f or 1.00 tim concurrent w chord in all a 10.0psf. b bearing w truss to bea	it; Cat. II; E: Exterior(2) nbers and f ; Pg=15.0 p Ct=1.10, Lu 20/12 in acc nes flat roof with any oth areas wher alls due to aring walls urlin along	xp B; Enclosed; N 20-0-0 to 26-9-7, orces & MWFRS osf (ground snow) =50-0-0; Min. fla cordance with IBC i load of 11.6 psf er live loads. e a rectangle 3-6 UPLIFT at jt(s) 13 due to UPLIFT at the top and/or bo	//WFRS (e Interior(1) for reactio); Pf=16.5 t roof snov C 1608.3.4 on overhal -0 tall by 2 3. This con jt(s) 2 anc ttom chorc	envelope) 26-9-7 to ons shown; psf (flat v load	and a state of the	SEA 0449		

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TRENCO A MiTek Affiliate



10-9-	<u>6 20-0-0</u> -6 9-2-10	28-0-0	9-2-10		48-0-0	
Plate Offsets (X,Y) [11:0-1-	4,Edge]				1000	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2015/TPI2014	CSI. TC 0.60 BC 0.78 WB 0.37 Matrix-S	DEFL. in (loc) Vert(LL) -0.20 12-14 Vert(CT) -0.38 12-14 Horz(CT) 0.14 11	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 347 lb	GRIP 197/144
BCDL 10.0		Matin-0			Weight. 547 lb	11 = 2078
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 *E 6-14: 2x4 SP N REACTIONS. (size) 2=0 Max Horz 2=1 Max Uplift 2=-{ Max Grav 2=1	:xcept* o.2 or 2x4 SPF No.2 -3-8, 11=Mechanical 54(LC 20) 99(LC 16), 11=-83(LC 17) 974(LC 2), 11=1911(LC 2)	BRAC TOP (BOT (WEBS	ING- CHORD Structural wood 2-0-0 oc purlins CHORD Rigid ceiling dir CHORD 1 Row at midpt	d sheathing directly appl s (4-2-5 max.): 6-7. rectly applied or 10-0-0 o 5-16, 8-14,	ied or 3-2-6 oc purlins oc bracing. 6-14	, except
FORCES. (lb) - Max. Comp./f TOP CHORD 2-3=-3617/23 8-10=-3374/2 8-10=-3374/2 BOT CHORD 2-18=-215/31 WEBS 3-18=-375/19 8-14=-791/18 8-14=-791/18	Aax. Ten All forces 250 (lb) or less exc 4, 3-5=-3363/239, 5-6=-2632/272, 6-7=-2 47, 10-11=-3649/247 53, 16-18=-84/2715, 14-16=-6/2280, 12- 0, 5-18=-20/579, 6-16=-45/859, 5-16=-7 7, 8-12=-22/599, 10-12=-398/197	ept when shown. 2270/278, 7-8=-2594/271, 14=-72/2706, 11-12=-142/31 76/187, 7-14=0/819,	90			
 NOTES- 1) Unbalanced roof live loads h 2) Wind: ASCE 7-10; Vult=1200 gable end zone and C-C Ext 28-0-0, Exterior(2) 28-0-0 to shown; Lumber DOL=1.60 p 3) TCLL: ASCE 7-10; Pr=20.0 µ roof snow: Lumber DOL=1.1 governs. Rain surcharge ap 4) Unbalanced snow loads hav 5) This truss has been designe non-concurrent with other liv 6) Provide adequate drainage t 7) This truss has been designe 8) * This truss has been designe 8) * This truss has been designe 8) * This truss has been designe 10) Provide mechanical connect 11) One H2.5A Simpson Strong connection is for uplift only 12) Graphical purlin representation 	ave been considered for this design. nph Vasd=95mph; TCDL=6.0psf; BCDL= erior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 32-2-15, Interior(1) 32-2-15 to 47-11-4 z late grip DOL=1.60 ssf (roof live load: Lumber DOL=1.15 Plat 5 Plate DOL=1.15); Category II; Exp B; f plied to all exposed surfaces with slopes e been considered for this design. d for greater of min roof live load of 12.0 e loads. o prevent water ponding. d for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the botton ord and any other members, with BCDL truss connections. tion (by others) of truss to bearing plate g-Tie connectors recommended to conne and does not consider lateral forces. tion does not depict the size or the orien	=6.0psf; h=25ft; Cat. II; Exp B to 20-0-0, Exterior(2) 20-0-0 one;C-C for members and for the DOL=1.15); Pg=15.0 psf (g Partially Exp.; Ct=1.10, Lu=SC less than 0.500/12 in accord psf or 1.00 times flat roof loa onconcurrent with any other li m chord in all areas where a n = 10.0psf. capable of withstanding 100 tot truss to bearing walls due tation of the purlin along the f	; Enclosed; MWFRS (envelo to 24-2-15, Interior(1) 24-2-1 ces & MWFRS for reactions ground snow); Pf=16.5 psf (fl -0-0; Min. flat roof snow load ance with IBC 1608.3.4. d of 11.6 psf on overhangs ve loads. 'ectangle 3-6-0 tall by 2-0-0 v lb uplift at joint(s) 11. to UPLIFT at jt(s) 2. This op and/or bottom chord.	pe) I5 to	SEAL 044925 M. SE September 24,2	D21

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I.		48-0-	0					1
		48-0-	0					
Plate Offsets (X,Y) [13:0-3-0	0,0-4-0], [17:0-3-0,0-4-0], [34:0-4-0,0-4-8], [40:0-4-0,0-4-8], [46:0	-4-0,0-4-8]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.05 BC 0.02 WB 0.18 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l- -0.00 -0.00 0.01	loc) l/defl 1 n/r 1 n/r 28 n/a	L/d 120 120 n/a	PLATES MT20 Weight: 434 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 OTHERS 2x4 SP No.3		E T V	BRACING- TOP CHORD BOT CHORD VEBS	Structural w 2-0-0 oc pu Rigid ceiling 1 Row at m	vood sheath Irlins (6-0-0 g directly ap Iidpt	ing directly ap max.): 13-17. plied or 10-0- 17-38, 1 18-37	oplied or 6-0-0 oc purlins 0 oc bracing. 6-39, 15-40, 14-41, 13-4	s, except 42, 12-43,

REACTIONS. All bearings 48-0-0.

- (lb) Max Horz 2=154(LC 16)
 - Max Uplift All uplift 100 lb or less at joint(s) 2, 39, 40, 41, 43, 44, 45, 46, 47, 48, 49, 50, 51, 37, 36, 35, 34, 33, 32, 31, 30, 29

Max Grav All reactions 250 lb or less at joint(s) 2, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 12-13=-102/275, 13-14=-90/264, 14-15=-90/264, 15-16=-90/264, 16-17=-90/264, 17-18=-102/276

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-0-0, Exterior(2) 2-0-0 to 20-0-0, Corner(3) 20-0-0 to 23-0-0, Exterior(2) 23-0-0 to 28-0-0, Corner(3) 28-0-0 to 31-0-0, Exterior(2) 31-0-0 to 48-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 5) Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs
- non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) n/a
- 14) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 28.

Continued on page 2

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	-		-			
Job	Truss	Truss Type	Qty	Ply	KB Home 150.1910.A	
						148063647
						140003047
150_1910_A	A4E	PIGGYBACK BASE SUPPO	1	1		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.5	520 s Aug	27 2021 MiTek Industries, Inc. Thu Sep 23 16:31:08 2021	Page 2
		ID	VMD62rz	1yiHD_Oo	RtbnrlFztQ8K-tb0U2RmfzXKadPuz9lhniPyII2OTUWxio5e	5h?yahlX

NOTES-

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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TRENGINEERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-0-8, Exterior(2) 2-0-8 to 7-2-4, Corner(3) 7-2-4 to 10-4-0, Exterior(2) 10-4-0 to 15-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 6) All plates are 1.5x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.

8) Gable studs spaced at 2-0-0 oc.

- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



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Job	Truss	Truss Type	Qty	Ply	KB Home 150.1910.A	
						148063650
150_1910_A	BG	COMMON GIRDER	1	2		
				_	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Thu Sep 23 16:31:14 2021	Page 2
		ID:	VMD62rz	lyiHD Oql	RtbnrlFztQ8K-ilNIIUqQZN4kLKL7V0oBxqCBJTJ4u7sbA15Q	vfyahlR

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-43, 3-5=-43, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-1768(B) 8=-1772(B) 9=-1768(B) 10=-1768(B) 11=-1768(B) 12=-1768(B) 13=-1768(B)

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Edenton, NC 27932

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- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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REACTIONS. (size) 1=9-0-0, 3=9-0-0, 4=9-0-0 Max Horz 1=-57(LC 10) Max Uplift 1=-26(LC 14), 3=-34(LC 15) Max Grav 1=173(LC 2), 3=173(LC 2), 4=301(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-6-6, Exterior(2) 4-6-6 to 7-6-6, Interior(1) 7-6-6 to 8-7-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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	0-0-6	6-0-6						1	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.20 BC 0.11 WB 0.03 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 20 lb	GRIP 244/190 FT = 20%
LUMBER-		BR	ACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. 1=6-0-0, 3=6-0-0, 4=6-0-0 (size) Max Horz 1=-36(LC 10) Max Uplift 1=-16(LC 14), 3=-21(LC 15) Max Grav 1=109(LC 2), 3=109(LC 2), 4=189(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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2x4 🥢

2x4 📎

Rigid ceiling directly applied or 10-0-0 oc bracing.

3-0-12 0-<u>0</u>-6 0-0-6 3-0-6 Plate Offsets (X,Y)-- [2:0-2-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) l/defl L/d PLATES GRIP 20.0 TCLL (roof) Plate Grip DOL тс 244/190 1.15 0.03 Vert(LL) 999 MT20 n/a n/a Snow (Pf/Pg) 11.6/15.0 BC Lumber DOL 1.15 0.09 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a BCLL 0.0 * Code IRC2015/TPI2014 FT = 20% Matrix-P Weight: 8 lb BCDL 10.0 LUMBER-BRACING-TOP CHORD 2x4 SP No.3 TOP CHORD Structural wood sheathing directly applied or 3-0-12 oc purlins.

BOT CHORD

BOT CHORD 2x4 SP No.3

REACTIONS. 1=3-0-0, 3=3-0-0 (size) Max Horz 1=15(LC 13) Max Uplift 1=-4(LC 14), 3=-4(LC 15) Max Grav 1=84(LC 2), 3=84(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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