

BUILDER SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT TIME OF CONSTRUCTION.

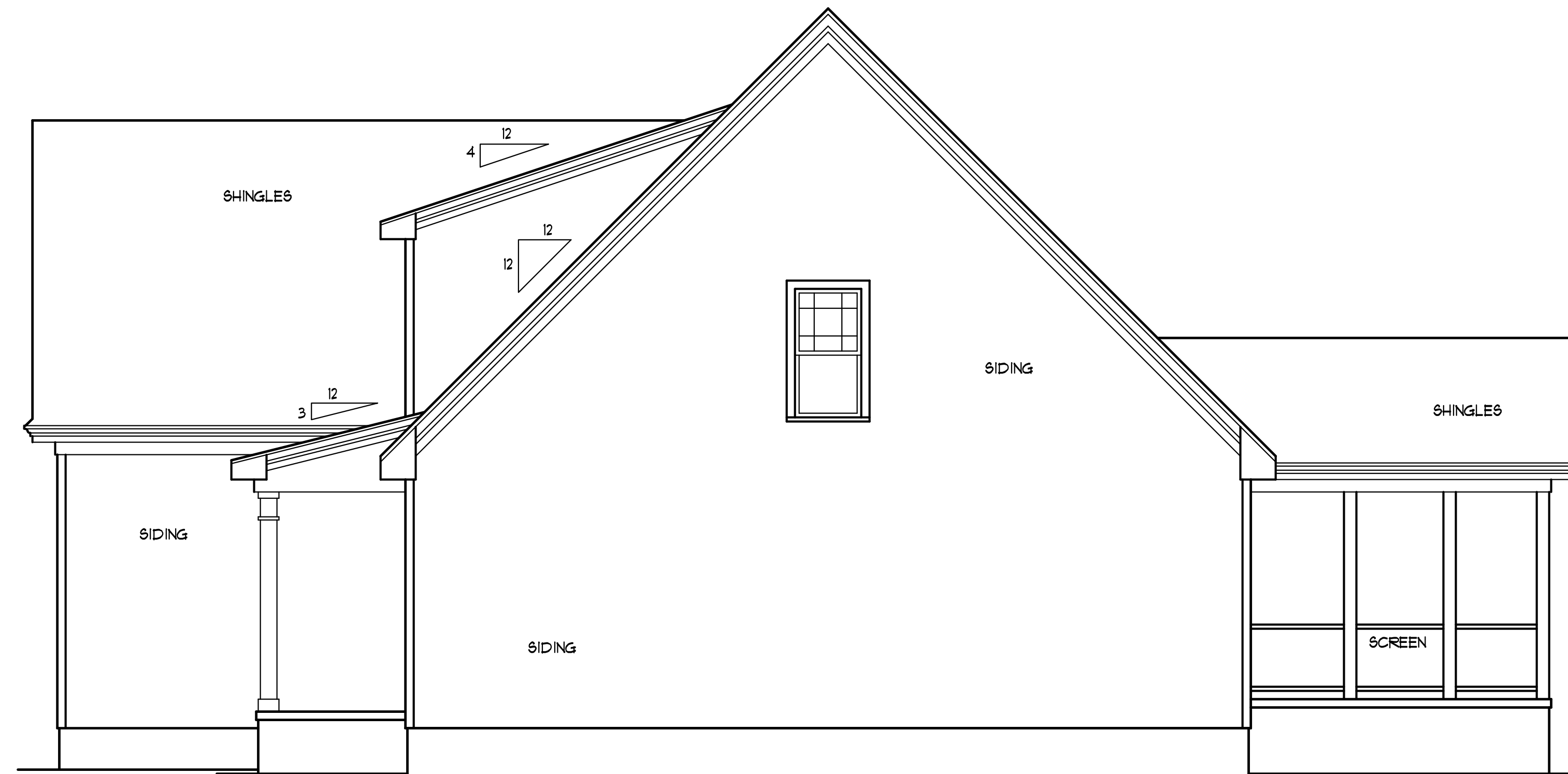
ALL CONSTRUCTION SHALL CONFORM TO THE 2018 EDITION OF THE NC STATE BUILDING CODE.

CODES GOVERN OVER DRAWINGS.

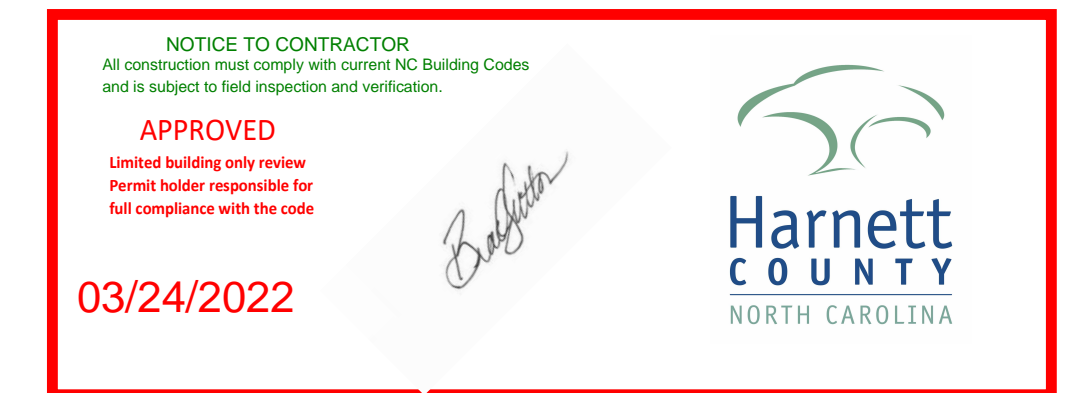
DIMENSIONS GOVERN OVER SCALE.

VERIFY ALL MECHANICAL REQUIREMENTS BEFORE FRAMING.

YUNCANNON DESIGNS DOES NOT ASSUME LIABILITY FOR ANY DEVIATION OF OR CONSTRUCTION METHODS OF THESE PLANS.



RIGHT SIDE ELEV
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"

MILTON BUILT HOMES

YUNCANNON DESIGNS
CUSTOM HOME PLANS
FLOQUAT-VARENA, NC • (919) 421-1314

DRAWN BY : WRY
CHK'D BY : WRY
DATE: 01/25/22
REVISIONS :

SHEET
A-1

Plan No. 1971-15.REV

BUILDER SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT TIME OF CONSTRUCTION.

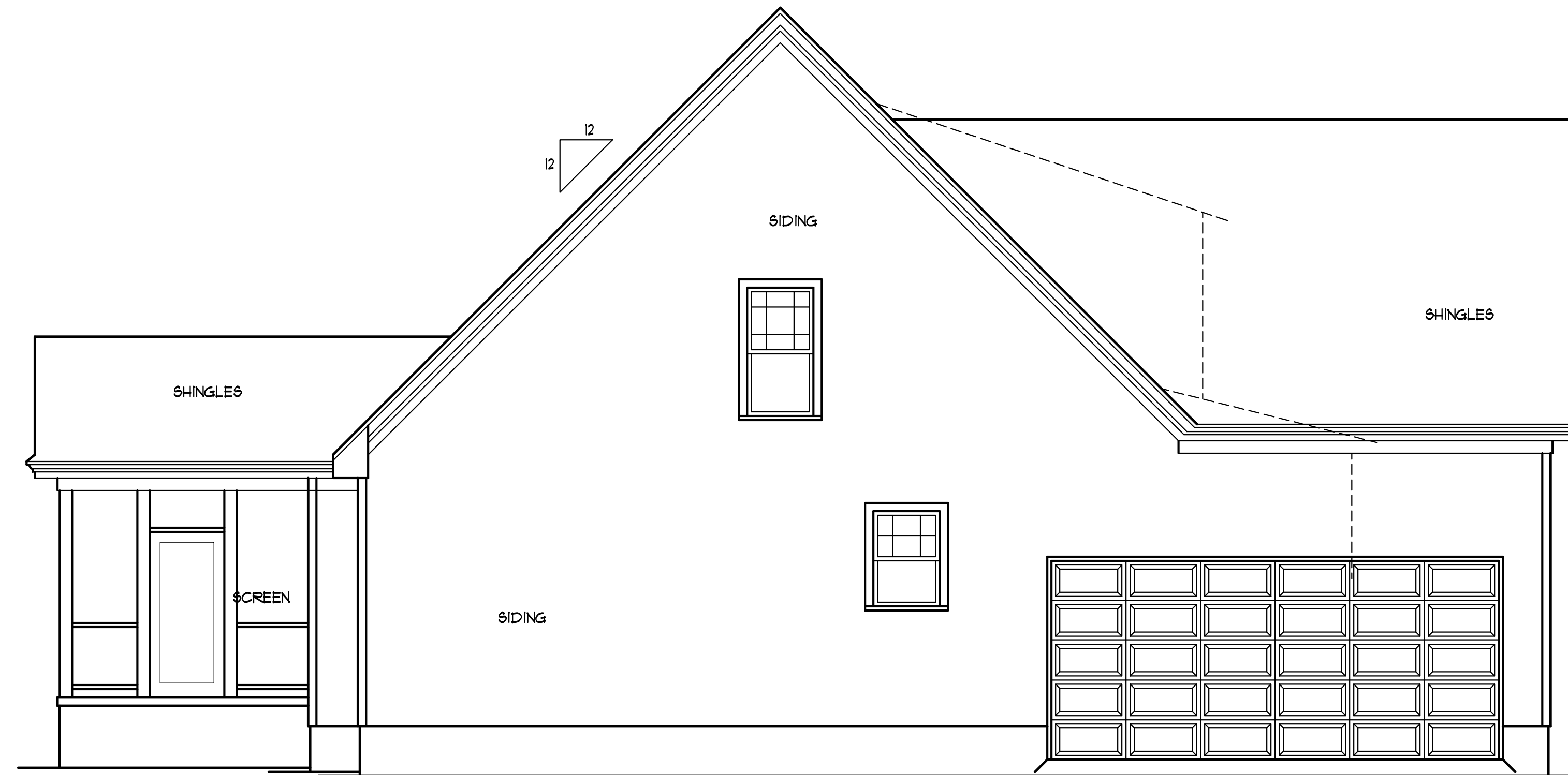
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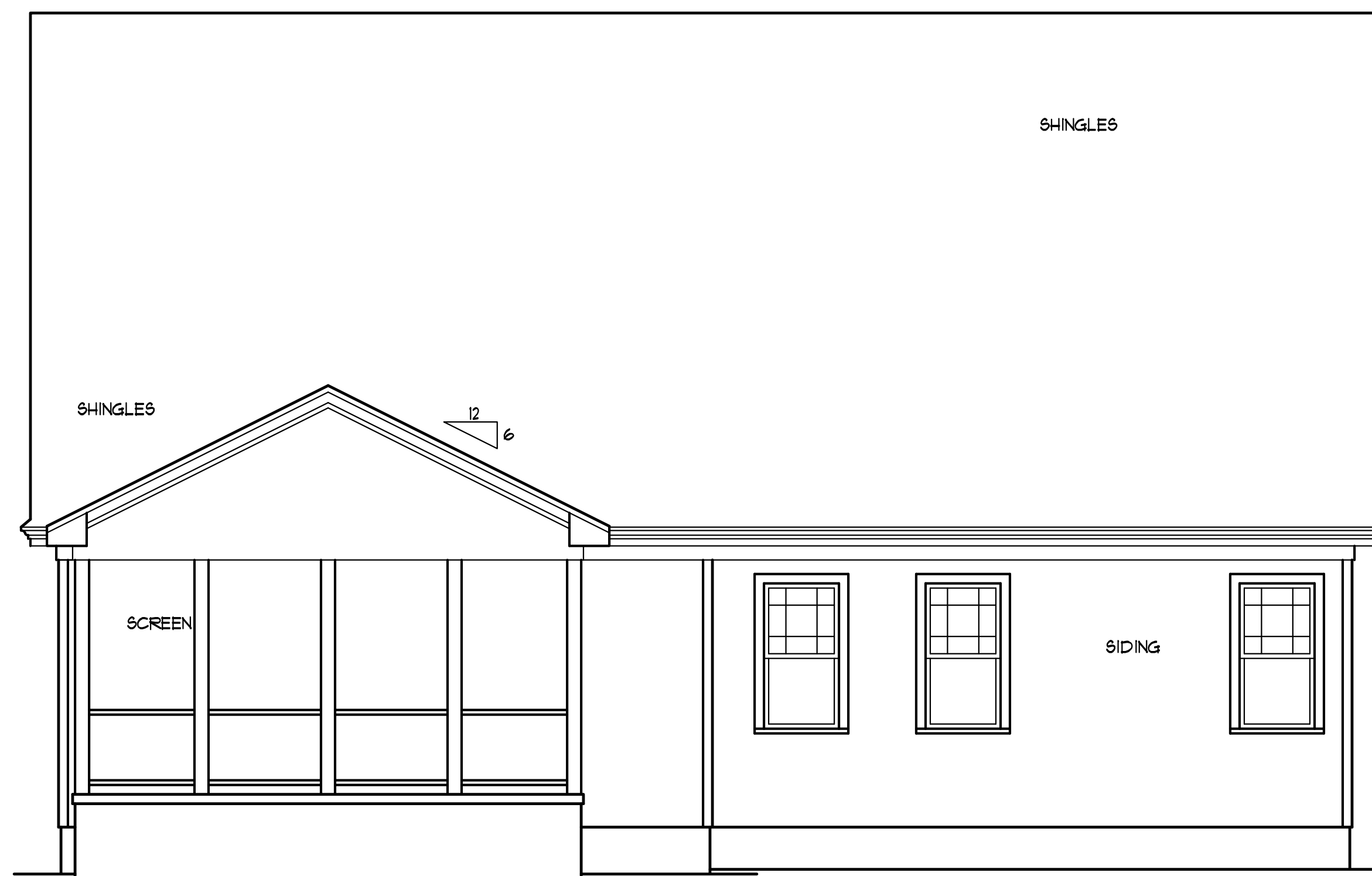
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LEFT SIDE ELEV
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REAR ELEVATION
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MILTON BUILT HOMES

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FOUNDATION NOTES

- SPECIFIC NOTES ON THE PLANS TAKE PRECEDENCE OVER THE FOLLOWING GENERAL NOTES. NOTED DIMENSIONS GOVERN OVER SCALE. PLAN DESIGNER AND CONTRACTOR ARE SOLELY RESPONSIBLE FOR DIMENSIONAL ACCURACY. ALL CONSTRUCTION TO COMPLY WITH NC STATE BUILDING CODE, VOL. VII WITH CURRENT REVISIONS.
- STRUCTURAL CONCRETE TO BE FC-3000 F91, PREPARED AND PLACED IN ACCORDANCE WITH ACI STANDARD 318.
- FOOTINGS TO BEAR ON UNDISTURBED EARTH, A MIN. OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE LOCAL INSPECTOR.
- FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLEY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
- FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. PROVIDE 3" MIN. FOOTING PROJECTION FROM FACE OF MASONRY.
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN VOLUME VII, SECTION R-304.3 OF THE NC STATE CODE.
- PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.
- PROVIDE FOUNDATION WATERPROOFING AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.
- PROVIDE PERIMETER INSULATION WITH BASEMENT SLABS.
- CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEER.
- CRAWL SPACE TO BE GRADED LEVEL AND CLEAR OF ALL DEBRIS.

ALLOWABLE PIER HEIGHTS		
SIZE	HOLLOW	SOLID
8"x16"	2'-8"	5'-0"
12"x16"	4'-0"	8'-0"
16"x16"	5'-4"	12'-0"
24"x24"	8'-0"	
FOOTINGS 30"x30"x12" MINIMUM UNLESS NOTED OTHERWISE		

NOTE

- USE 2X10'S AT 16" O.C. #2 S.P.F. MINIMUM SPECIFIED (UON)
- JOISTS DIRECTION DESIGNATED BY
- DOUBLE ALL JOIST PARALLEL TO WALLS ABOVE.
- GIRDERS TO BE (3) 2X10'S #2 S.P.F. MINIMUM (UON)
- MINIMUM CRAWL SPACE ACCESS TO BE 22" HEIGHT X 36" WIDTH.
- FOUNDATION WALL AND PIER CAPS TO BE MINIMUM 8" SOLID.
- FOOTING UNDER MASONRY FIREPLACE TO BE 12" THICK AND EXTEND 12" BEYOND.

CRAWL SPACE VENTILATION

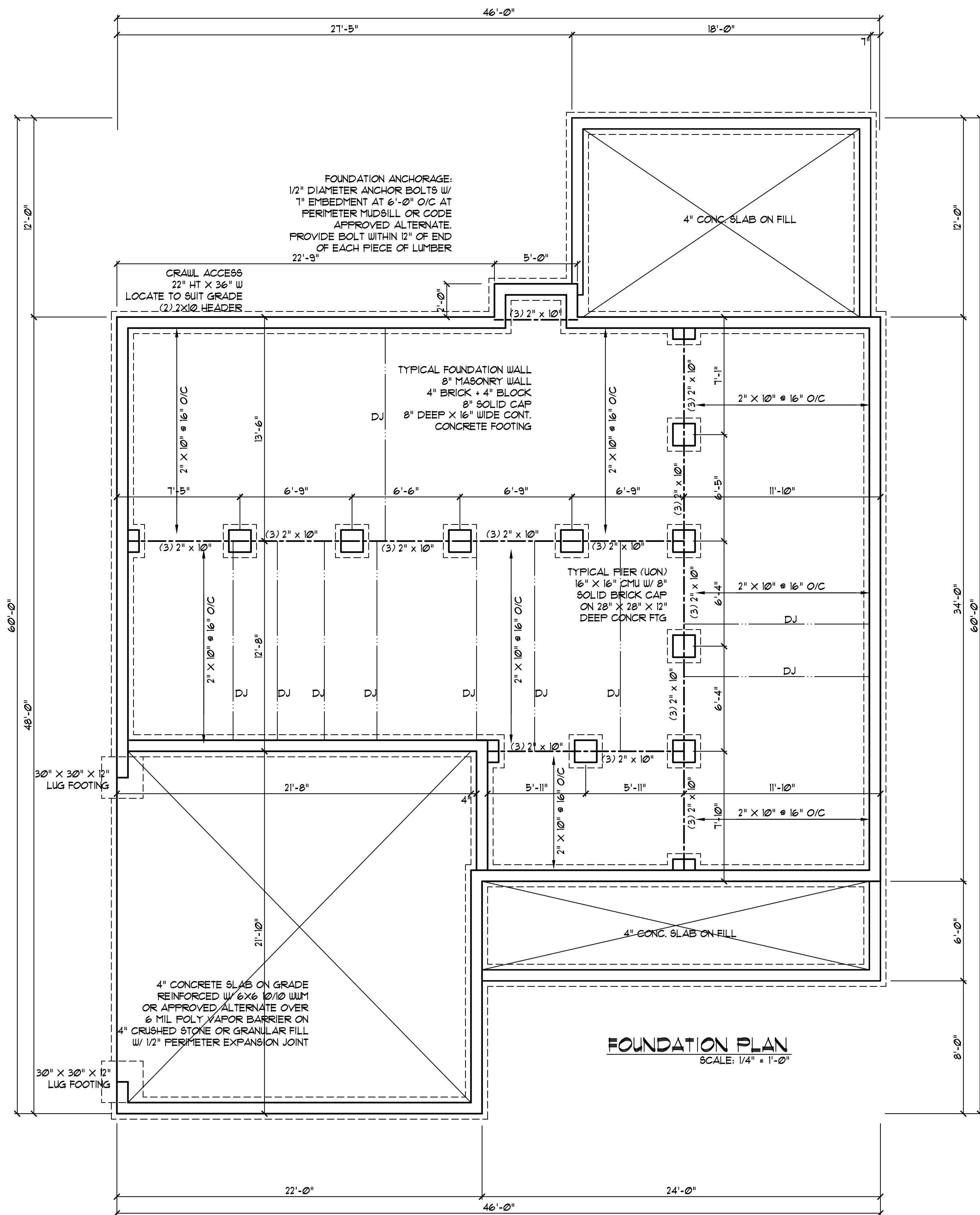
PROVIDE AT LEAST 10 SQ. FT. NET FREE VENTILATION AREA FOR EACH 150 SQ. FT. OF CRAWL SPACE

CRAWL SPACE AREA:
1394 / 150 = 9.29 SQ. FT. REQ'D.

REDUCE REQUIRED AREA TO 10 SQ. FT. NET FREE VENTILATION AREA FOR EACH 1500 SQ. FT. OF CRAWL SPACE WITH APPROVED VAPOR BARRIER

PROVIDE (1) VENT WITHIN 3'-0" OF EACH CORNER

REFER TO MANUFACTURER SPECIFICATIONS FOR ACTUAL VENTS USED TO DETERMINE NUMBER OF VENTS REQUIRED



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

MILTON BUILT HOMES

VINCANNON DESIGNS
CUSTOM HOME PLANS
FISHEAT-VARINA, NC • (919) 421-1314

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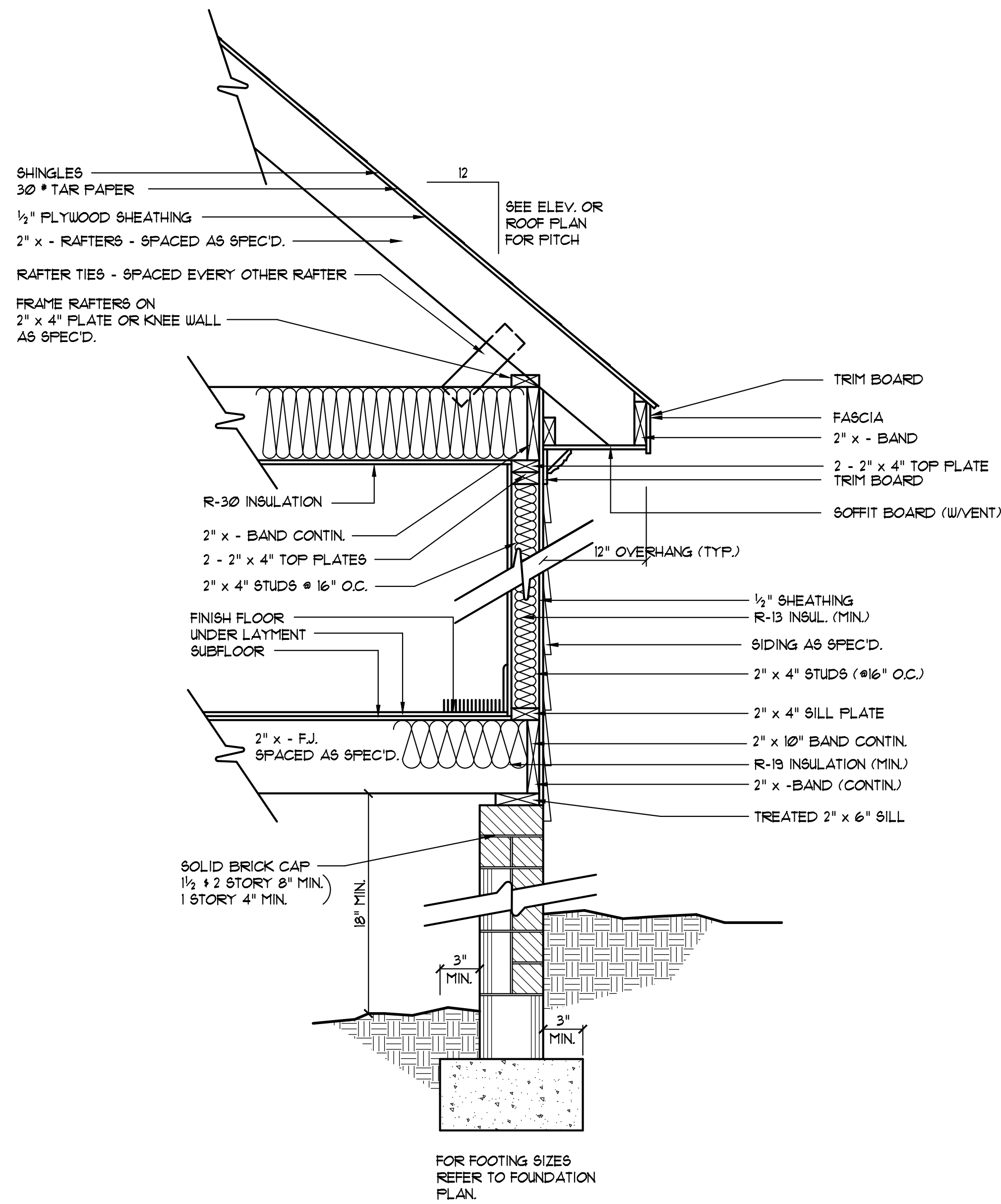
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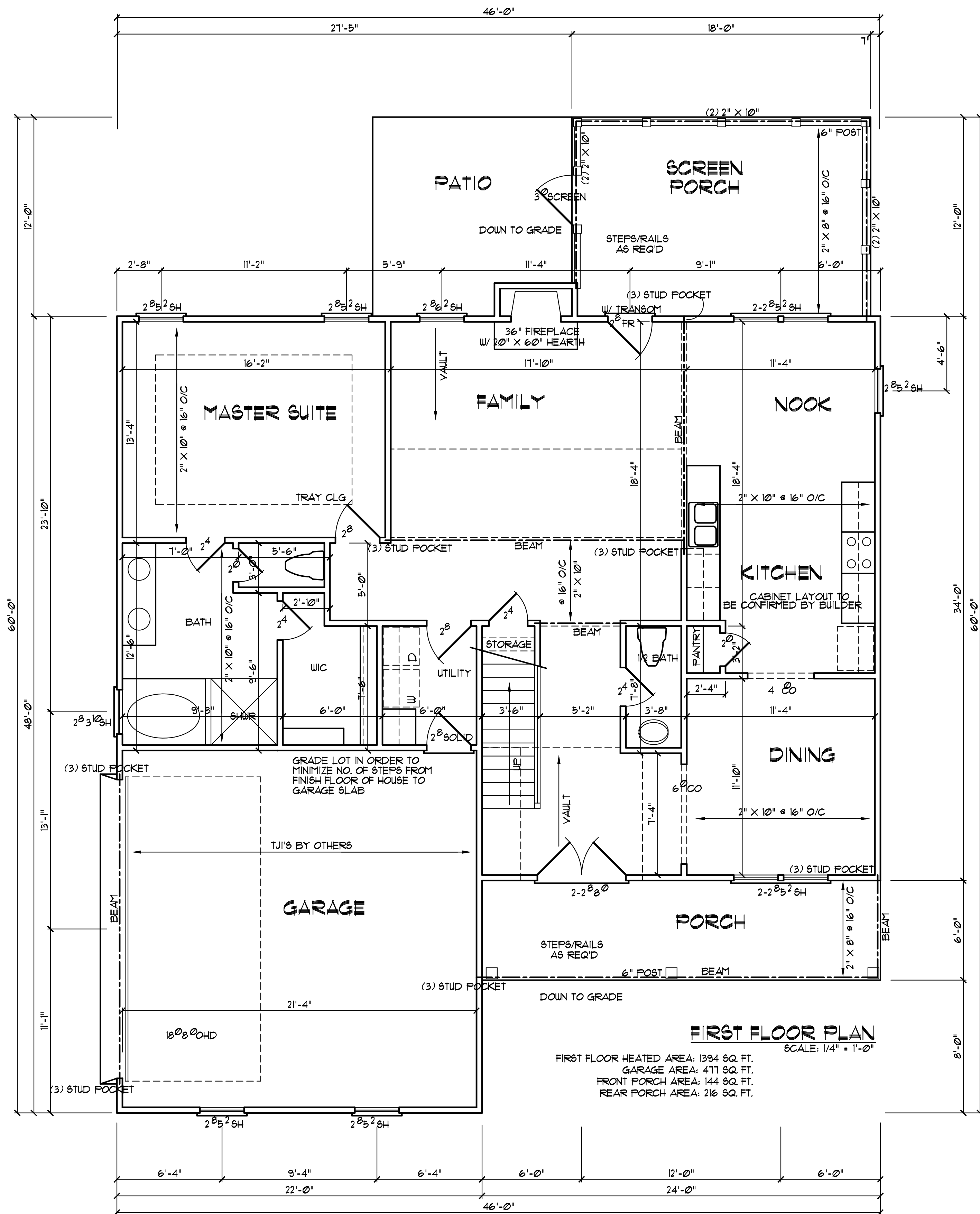
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VERIFY ALL MECHANICAL REQUIREMENTS BEFORE FRAMING.

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EXTERIOR WALL SECTION
SCALE: NTS



FIRST FLOOR PLAN
SCALE: 1/4" = 1'-0"
FIRST FLOOR HEATED AREA: 1394 SQ. FT.
GARAGE AREA: 411 SQ. FT.
FRONT PORCH AREA: 144 SQ. FT.
REAR PORCH AREA: 216 SQ. FT.

- FRAMING NOTES**
- 1) ALL FRAMING LUMBER SFF #7 GRADE (NLGA) OR BETTER, UNLESS NOTED.
 - 2) JOIST SIZE AND SPACING BASED ON CODE MINIMUMS. BUILDER SHOULD CONSIDER STIFFENING FLOOR SYSTEM IN LONG SPAN AREAS, TO REDUCE FLOOR FLEXIBILITY.
 - 3) PROVIDE DOUBLE JOIST UNDER WALLS PARALLEL TO JOIST DIRECTION.
 - 4) PROVIDE SOLID BLOCKING BETWEEN ALL POINT LOADS AND TOP OF GIRDER, PIER OR FOUNDATION WALL.
 - 5) JOISTS DIRECTION DESIGNATED BY

- NOTE**
- 1) 3'-0" CEILING HEIGHT
 - 2) FRAME WINDOWS AT 3'-0" ABOVE FINISHED FLOOR
 - 3) DOUBLE ALL JOISTS PARALLEL TO WALLS ABOVE.
 - 4) USE TEMPERED GLASS AT TUB & SHUR. AREAS 4 FT. HIGH, LESS THAN 8" ABOVE FINISHED FLOOR WITH MORE THAN 3 SQ. FT. OF GLASS AND LOCUS WITHIN 7' OF SWINGING DOORS.
 - 5) USE 2X10'S AS HEADERS (MIN) LON. ON STRUCTURAL SHEETS
 - 6) ATTIC ACCESS 22"X30" MINIMUM
 - 7) SILL AND PLATES TO BE 8" OR MORE ABOVE GARAGE OR BE TREATED MATERIAL.
 - 8) SIDING TO BE MINIMUM 6" ABOVE GRADE.
 - 9) DOOR FROM GARAGE TO HOUSE TO BE 30 MINUTE FIRE RATED. ADD 1/2" SHEETROCK ON ALL WALLS & CEILING COPPER TO GARAGE & HEATED SPACE.
 - 10) MINIMUM 8" SIDE CLEARANCE FROM CENTER LINE OF WATER CLOSET.
 - 11) PROVIDE TREATED BAND AT CONCRETE SLAB.

MILTON BUILT HOMES

VINCANNON DESIGNS
 CUSTOM HOME PLANS
 FUGIAT-VARINA, NC • (919) 471-1314

DRAWN BY : WRY
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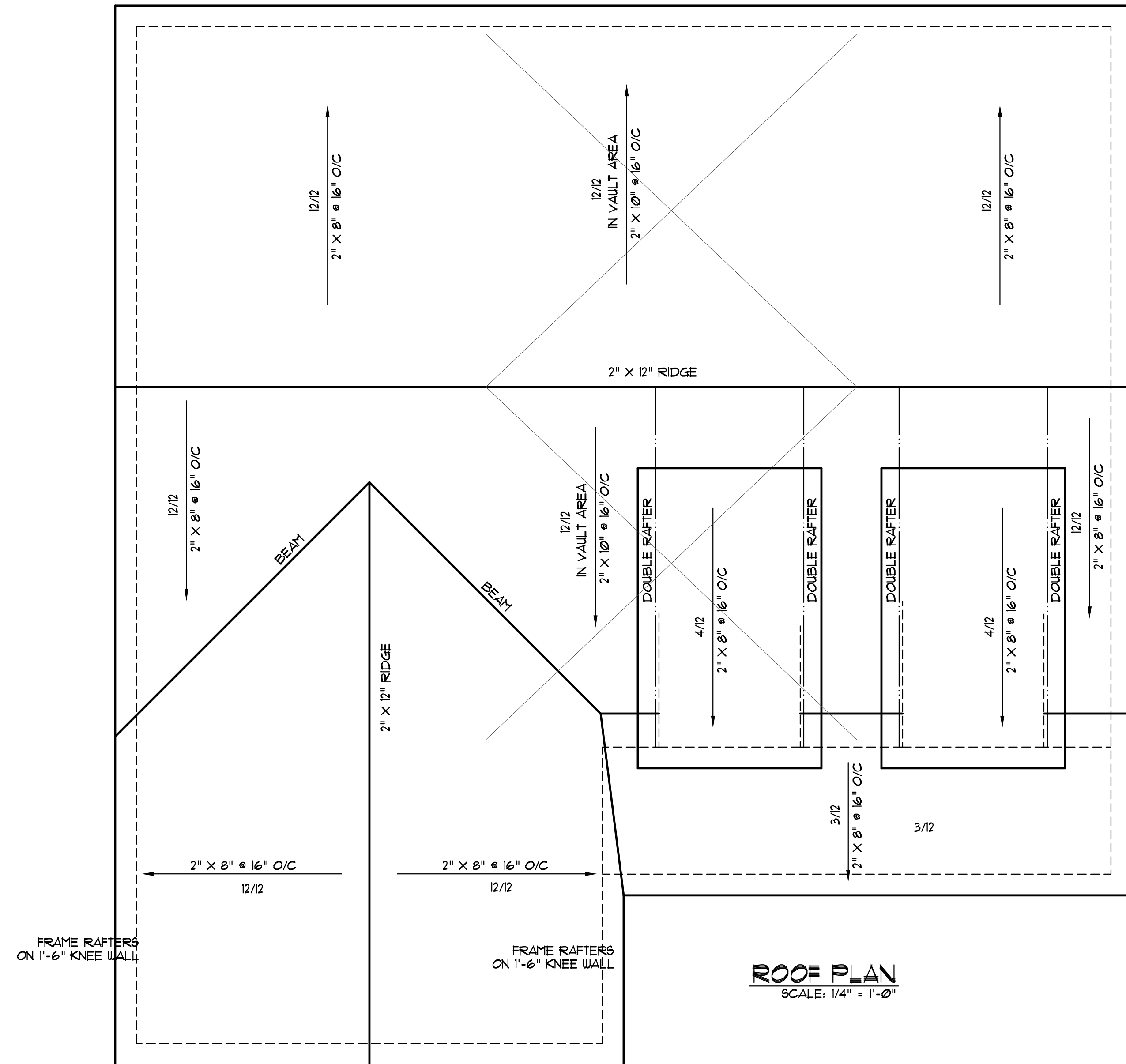
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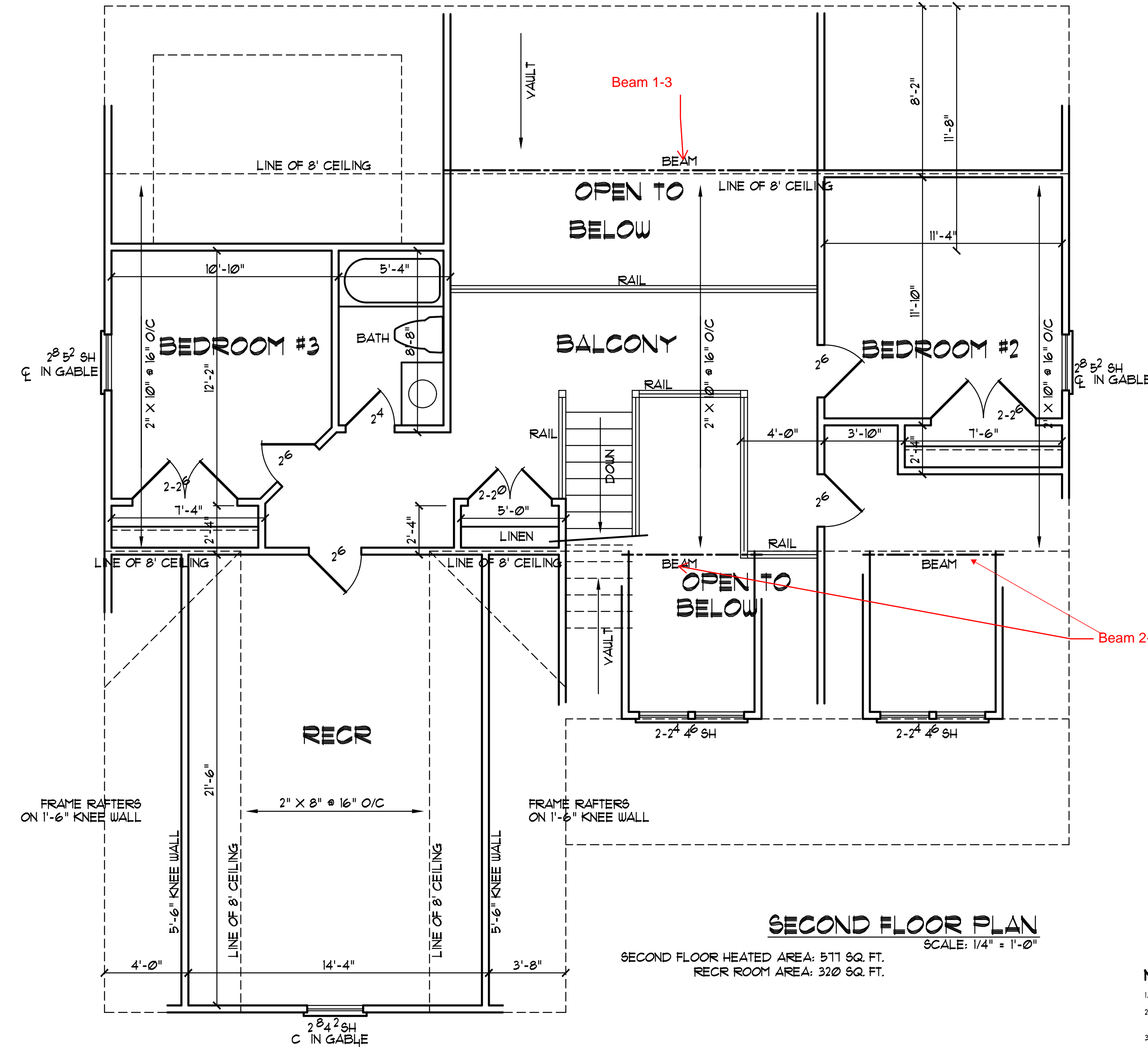


ROOF PLAN
SCALE: 1/4" = 1'-0"

ATTIC SPACE VENTILATION

MAIN ROOF
ATTIC AREA = 2231 SQ. FT.
REQUIRED AREA = $A/300 = 7.44$ SQ. FT.

NOTE:
ALL EAVES TO HAVE 2" CONTINUOUS SOFFIT VENT
ALLOW 1" AIR SPACE ABOVE INSULATION FOR AIR FLOW



SECOND FLOOR PLAN
SCALE: 1/4" = 1'-0"

SECOND FLOOR HEATED AREA: 511 SQ. FT.
RECR ROOM AREA: 320 SQ. FT.

NOTE

- 1) 8'-0" CEILING HEIGHT
- 2) FRAME WINDOWS AT 8'-0" ABOVE FINISHED FLOOR
- 3) DOUBLE ALL JOISTS PARALLEL TO WALLS ABOVE.
- 4) USE TEMPERED GLASS AT TUB & SHUR. AREAS 4' WITH UNITS LESS THAN 8" ABOVE FINISHED FLOOR WITH MORE THAN 9 SQ. FT. OF GLASS AND UNITS WITHIN 7' OF SLUNGING DOORS.
- 5) USE 2X10'S AS HEADERS (MINI W/O.N. ON STRUCTURAL SHEETS)
- 6) ATTIC ACCESS 22X30" MINIMUM.
- 7) SILL AND FLATES TO BE 6" OR MORE ABOVE GRADE OR BE TREATED MATERIAL.
- 8) SIDING TO BE MINIMUM 6" ABOVE GARAGE.
- 9) DOOR FROM GARAGE TO HOME TO BE 20 MINUTE FIRE RATED. ADD 1/2" SHEETROCK ON ALL WALLS & CEILING COMMON TO GARAGE & HEATED SPACE.
- 10) MINIMUM 6" RISE CLEARANCE FROM CENTER LINE OF WATER CLOSET.
- 11) PROVIDE TREATED BAND AT CONCRETE SLAB.

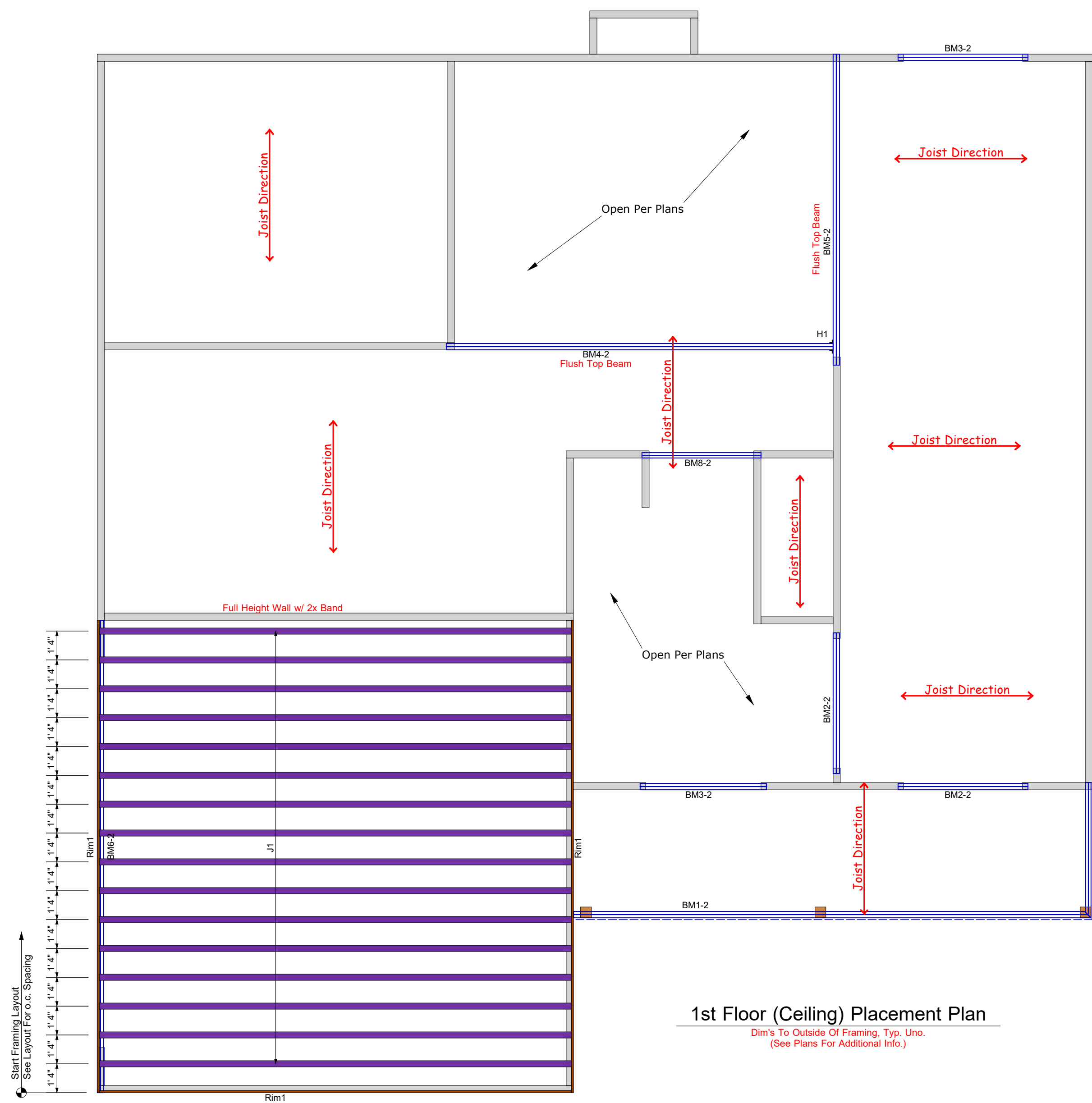
MILTON BUILT HOMES

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PlotID	Length	Product	Piles	Net Qty
J1	22' 0"	BLI 55 16'	1	16
BM1-2	24' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	2
BM2-2	8' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	4
BM3-2	8' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	4
BM4-2	18' 0"	onCENTER® LVL 2.0E 1 3/4" x 11 7/8"	2	2
BM5-2	16' 0"	onCENTER® LVL 2.0E 1 3/4" x 11 7/8"	2	2
BM6-2	22' 0"	onCENTER® LVL 2.0E 1 3/4" x 18"	2	2
BM7-2	8' 0"	2x10 SPF No.2	2	2
BM8-2	6' 0"	2x10 SPF No.2	2	2
Rim1	12' 0"	onCENTER® Rim SE 1 1/8" x 16"	1	6

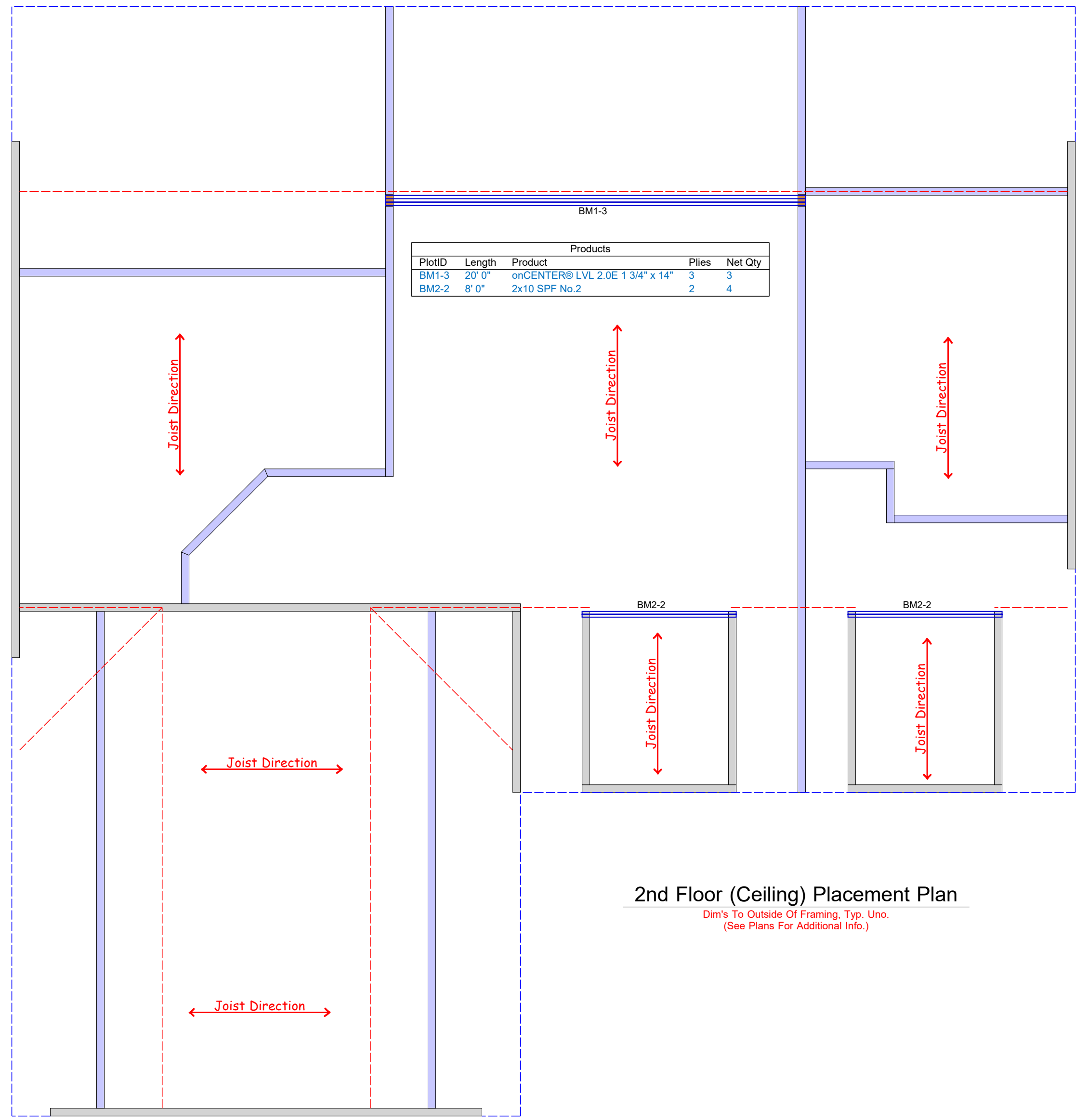
- = Point Load From Above
- = Post / Stud Column Below
- BBO = Beam By Others
- ▬ = Load Bearing Wall
- ▬ = Non-Load Bearing Wall

Framer Notes:
 *First Floor I-Joists Painted - Green (Verde)
 (Primer piso I-vigas pintadas de verde.)
 *Second Floor I-Joists Painted - Blue (Azul)
 (Vigas en l del segundo piso pintadas de azul.)
 *Third Floor I-Joists Painted - Red (Rojo)
 (Tercer piso I-vigas pintadas de rojo.)

HVAC Hole Notes:
 ■ = Red End (Lado Rojo)
 *Field Trim Non Red End To Keep Holes Aligned
 (Cortar el lado sin marca rojo para alinear los hoyos.)

PlotID	Qty	Manuf	Product
H1	1	USP	HD412
H1*	1	Simpson	HU412*

USP Hangers
 Simpson Hangers
 Hanger Conversion List



3941 USHwy 421 North
 Wilmington, NC 28401
 (910) 386-4300

DRAWN BY:
 JJC

DATE:
 03-17-2022

SCALE:
 1/4" = 1'-0"

SALESPERSON:
 Kemper / Cunningham

BUILDER NAME:
 Milton Built Homes

PROJECT NAME:
 1006 Butler Drive

LEVEL NAME:
 EWP Framing
 Layouts (Ceiling)

PAGE: 1



3941 USHwy 421 North
Wilmington, NC 28401
(910) 386-4300

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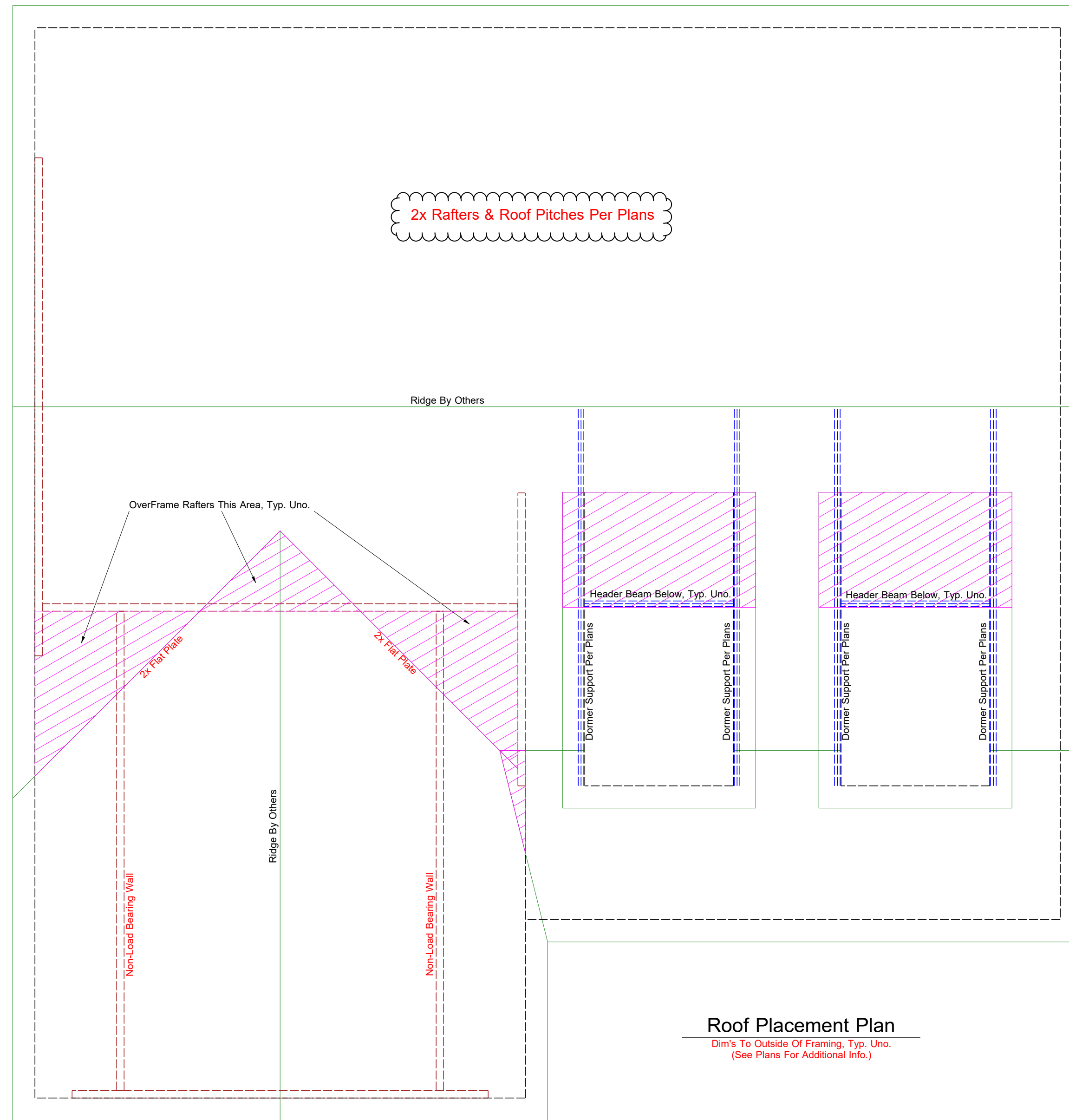
1006 Butler Drive

BUILDER NAME:

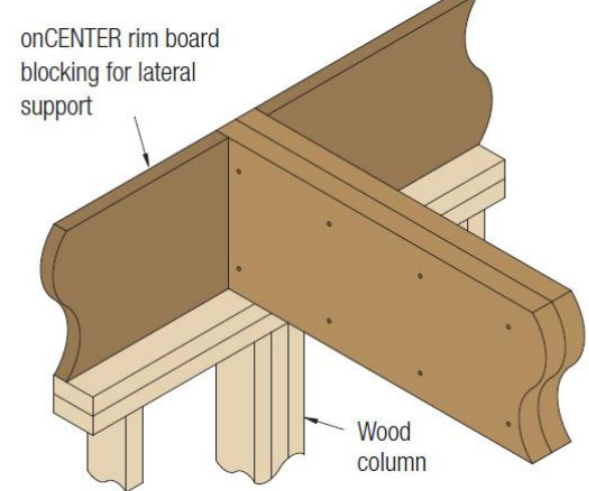
PROJECT NAME:

LEVEL NAME:
Roof Framing Layout

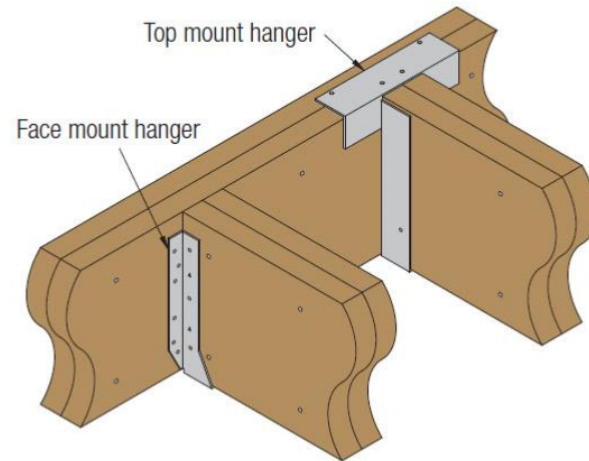
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B1 BEARING ON WALL

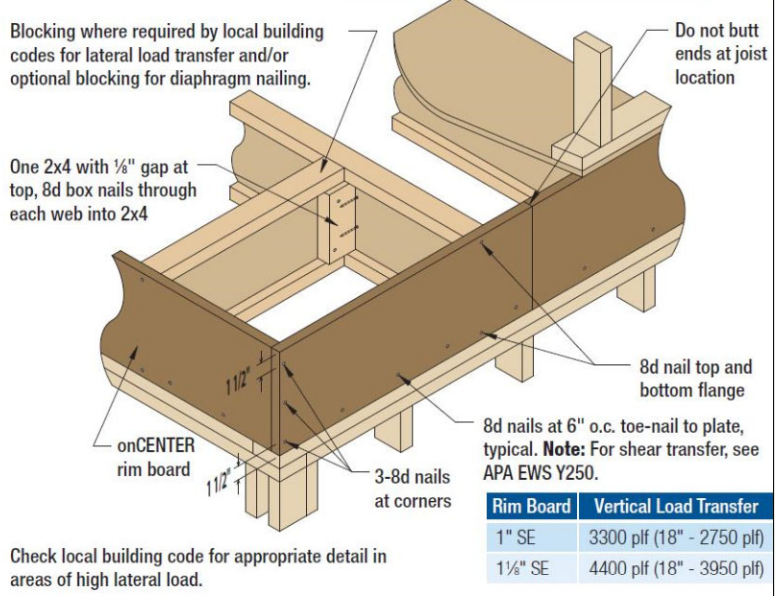


B3 BEAM-TO-BEAM CONNECTION

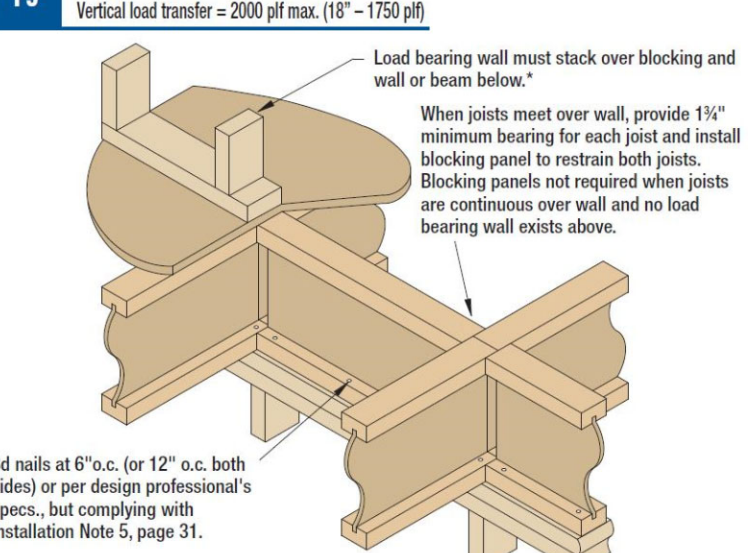


Install hangers per hanger manufacturer's instructions.

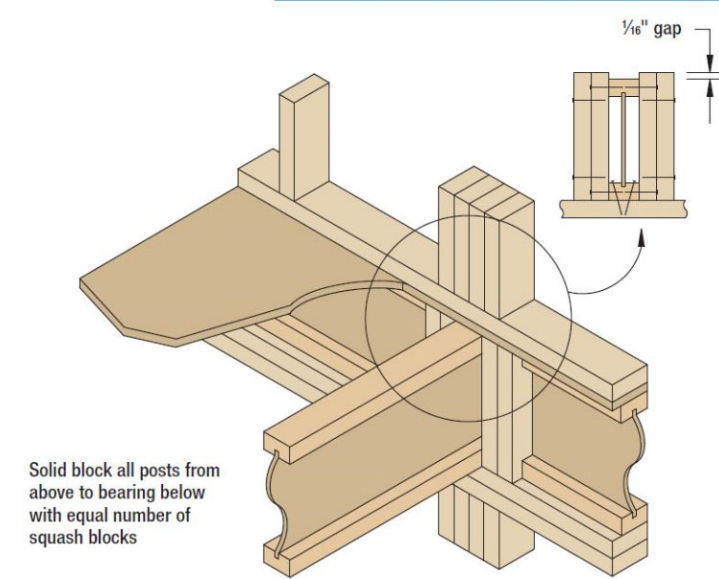
onCENTER® RIM BOARD CLOSURE F5



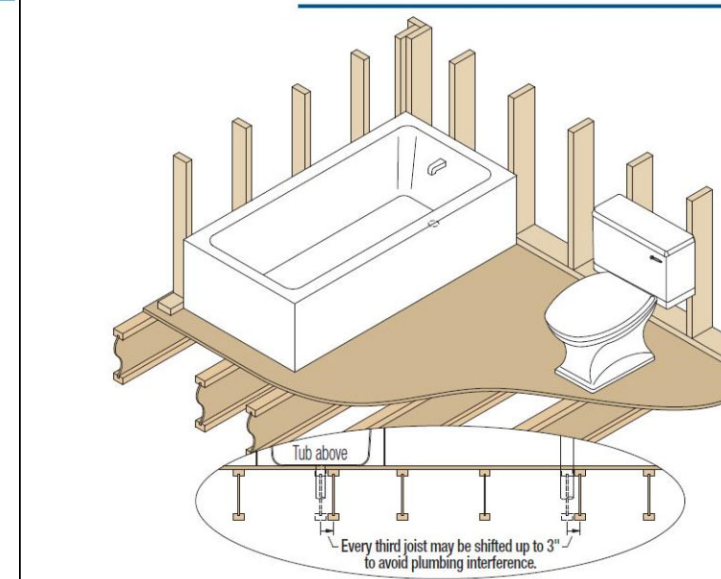
F9 BLOCKING PANEL INTERIOR



SQUASH BLOCKS AT CONCENTRATED LOADS F19



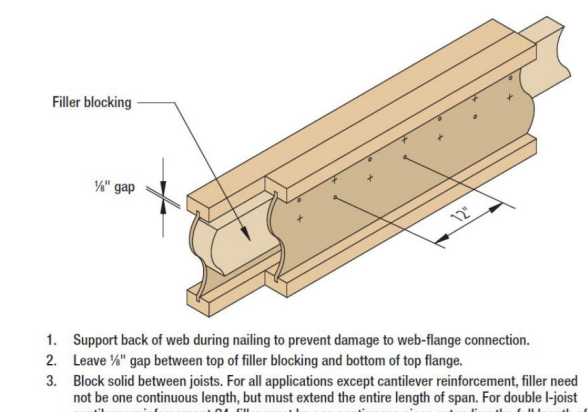
JOIST SPACING BELOW PLUMBING FIXTURES P2



F11 DOUBLE JOIST CONSTRUCTION WITH FILLER

Note: Filler blocks and blocking between joists can be omitted when double joists are loaded evenly from above in the top of each joist, such as when a parallel bearing wall is directly centered over the double joist.

Joist	Depth	Regular Filler Blocking Detail F10	Full-depth Filler Blocking Details C1, E1, F14 & D1
BU 150	11 1/2"	2x6 @ 1" OSG/Passwd	2x6 @ 1" OSG/Passwd
	14"	2x6 @ 1" OSG/Passwd	2x12 @ 1" OSG/Passwd
	18"	2x6 @ 1" OSG/Passwd	2x12 @ 1" OSG/Passwd
BU 40	11 1/2"	2x6 @ 1" OSG/Passwd	2x6 @ 1" OSG/Passwd
	14"	2x6 @ 1" OSG/Passwd	2x12 @ 1" OSG/Passwd
	18"	2x6 @ 1" OSG/Passwd	2x12 @ 1" OSG/Passwd
BU 40, 90	11 1/2"	2x6	2x6
	14"	2x6	2x12
	18"	2x6	2x12
BU 90, 90, 90	11 1/2"	2x6	2x6
	14"	2x6	2x12
	18"	2x6	2x12



- Support back of web during nailing to prevent damage to web-flange connectors.
- Leave 1" gap between top of filler blocking and bottom of the flange.
- Block solid between joists. For all applications except ceiling reinforcement, filler need not be one continuous length, but must extend the entire length of span. For double joist ceiling reinforcement CA filler must be one continuous piece extending the full length of the reinforcement.
- Place joist together and nail from each side with 2 min of 10d common nails (16d common for BU 150, BU 40, and BU 90) at 12" o.c. Other nails on opposite side 6".

onCENTER® 2.0E LVL Multiple Ply Fastening

Fastener Type	LVL Depth	Fastener Spacing	2 Ply 10' W/O	3 Ply 10' W/O	4 Ply 17' W/O	Notes
1/2" HD bolts (0.511" x 3.5") or corner (0.162" x 3.5")	7 1/2" - 11 1/2"	2 (diagonal)	12"			These minimum requirements are adequate only when all loads are evenly applied to the surface of all plies. If loads are applied to side faces of joists, see designer's specifications.
	14" - 18"	3	12"			
	24"	4	12"			Top and bottom rows of connectors should be 2" from edge.
	7 1/2" - 18"	2 (diagonal)	24"			Fastening for depths less than 7-1/2" requires special consideration. Contact Builder. Fasteners must meet full cross-section of shank, but must not be over-driven, over-tightened, or cross-threaded.
	24"	3	24"			
	7 1/2" - 18"	2 (diagonal)	24"			Bolt heads must be 1/2" to 1/16" larger than bolt diameter. Bolt must extend through full thickness of member end at least 1/2" beyond. Use a washer under head and nut.
	24"	3	24"			
USP WS or Simpson SDS Screws	7 1/2" - 18"	2 (diagonal)	24"			Spacings closer than those indicated may be acceptable, but require evaluation. Please contact Builder.
	24"	3 (diagonal)	24"			
Simpson Strong-Tie Fastener/Trussak Screws	7 1/2" - 18"	2 (diagonal)	24"			Install screws per manufacturer's guidelines.
	24"	3 (diagonal)	24"			

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- PROVIDE FOUNDATION WATERPROOFING AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.
- PROVIDE PERIMETER INSULATION WITH BASEMENT SLABS.
- CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEER.
- CRAILL SPACE TO BE GRADED LEVEL AND CLEAR OF ALL DEBRIS.

ALLOWABLE PIER HEIGHTS		
SIZE	HOLLOW	SOLID
8"x16"	2'-8"	5'-0"
12"x16"	4'-0"	8'-0"
16"x16"	5'-4"	12'-0"
24"x24"	8'-0"	

FOOTINGS 30"x30"x12" MINIMUM UNLESS NOTED OTHERWISE OTHERWISE

NOTE

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- GIRDERS TO BE (3)2X10'S #2 S.P.F. MINIMUM (UON)
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- FOOTING UNDER MASONRY FIREPLACE TO BE 12" THICK AND EXTEND 12" BEYOND.

CRAWL SPACE VENTILATION

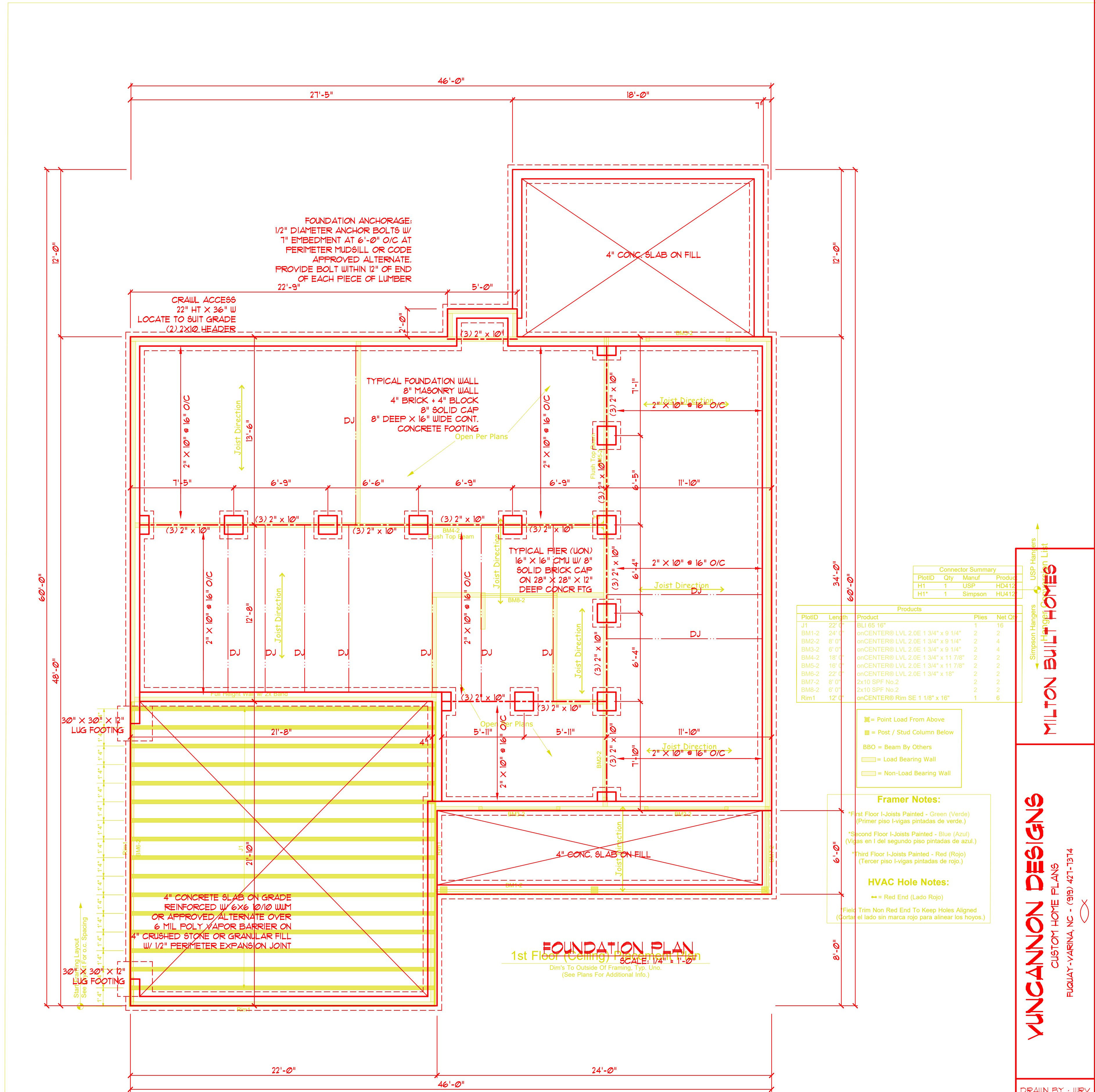
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CRAWL SPACE AREA:
1394 / 150 = 9.29 SQ. FT. REQ'D.

REDUCE REQUIRED AREA TO 10 SQ. FT. NET FREE VENTILATION AREA FOR EACH 1500 SQ. FT. OF CRAWL SPACE WITH APPROVED VAPOR BARRIER

PROVIDE (1) VENT WITHIN 3'-0" OF EACH CORNER

REFER TO MANUFACTURER SPECIFICATIONS FOR ACTUAL VENTS USED TO DETERMINE NUMBER OF VENTS REQUIRED



Connector Summary				
PlotID	Qty	Manuf	Product	
H1	1	USP	HD412	
H1	1	Simpson	HU412	

Products				
PlotID	Length	Product	Plies	Net Qty
J1	22'-0"	2x10 16'	1	10
BM1-2	24'-0"	8" CENCLERS LVL 2.0E 1 3/4" x 9 1/4"	2	2
BM2-2	8'-0"	8" CENCLERS LVL 2.0E 1 3/4" x 9 1/4"	2	4
BM3-2	6'-0"	8" CENCLERS LVL 2.0E 1 3/4" x 9 1/4"	2	4
BM4-2	18'-0"	8" CENCLERS LVL 2.0E 1 3/4" x 11 7/8"	2	2
BM5-2	18'-0"	8" CENCLERS LVL 2.0E 1 3/4" x 11 7/8"	2	2
BM6-2	22'-0"	8" CENCLERS LVL 2.0E 1 3/4" x 19"	2	2
BM7-2	8'-0"	2x10 SPF No.2	2	2
BM8-2	6'-0"	2x10 SPF No.2	2	2
Rim1	12'-0"	8" CENCLERS Rim SE 1 1/8" x 10"	1	6

- Point Load From Above
- Post / Stud Column Below
- BBO = Beam By Others
- Load Bearing Wall
- Non-Load Bearing Wall

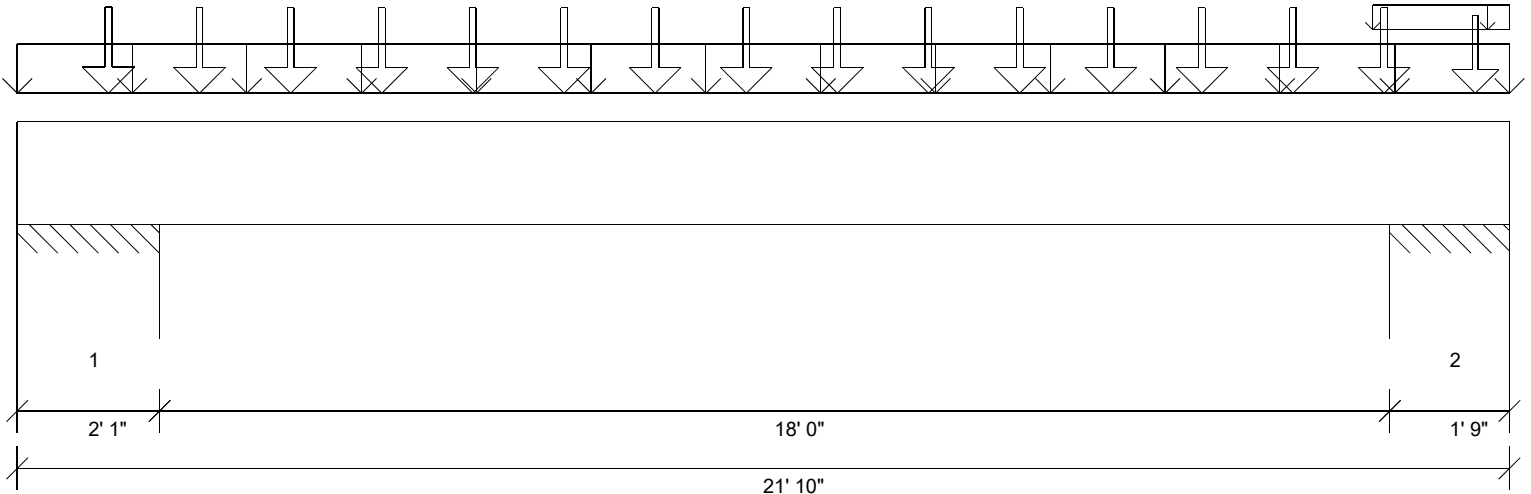
- Framer Notes:**
- 1st Floor Joists Painted - Green (Verde) (Primer piso l-vigas pintadas de verde.)
 - 2nd Floor Joists Painted - Blue (Azul) (Vigas en l del segundo piso pintadas de azul.)
 - 3rd Floor Joists Painted - Red (Rojo) (Tercer piso l-vigas pintadas de rojo.)
- HVAC Hole Notes:**
- Red End (Lado Rojo)
 - Field Trim Non Red End To Keep Holes Aligned (Cortar el lado sin marca rojo para alinear los hoyos.)

Plan No. 1971-15.REV

MILTON BUILT HOMES
YUNCANNON DESIGNS
 CUSTOM HOME PLANS
 FLOUAT-VARINA, NC • (919) 421-1314

DRAWN BY: WRY
 CHK'D BY: WRY
 DATE: 01/25/22
 REVISIONS:

SHEET
A-3



Graphical Illustration - Not To Scale
 Member Cut Length - 21'- 10"
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 0'	Bottom:	18'		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	10'- 8"	31516.02 lb ft	45022.29 lb ft	Passed - 70%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	3'- 7"	6286.90 lb	11970.00 lb	Passed - 53%	1.00	D + L
Live Load Deflection	11'- 1"	0'- 3/8"	N/A (L/360)	Passed - L/606	-	0.75(L + Lr)
Total Load Deflection	11'- 1"	0'- 9/16"	N/A (L/240)	Passed - L/371	-	D + 0.75(L + Lr)
Max. Reaction			<u>Supported Mt</u> <u>Supporting Mt</u>			
	0'- 1 1/2"	97.64 lb	25520.83 lb 21437.50 lb	Passed - 0%	1.25	D + 0.75(L + Lr)
	0'- 1 1/2"	-213.68 lb	18375.00 lb -	Passed - 1%	0.90	D
	1'- 11 1/2"	8250.46 lb	18375.00 lb 21437.50 lb	Passed - 45%	1.00	D + L
	20'- 2 1/2"	8353.42 lb	18375.00 lb 21437.50 lb	Passed - 45%	1.00	D + L
	21'- 8 1/2"	157.58 lb	25520.83 lb 21437.50 lb	Passed - 1%	1.25	D + 0.75(L + Lr)
	21'- 8 1/2"	-276.19 lb	18375.00 lb -	Passed - 2%	0.90	D

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	21'- 10"	Self Weight	16 lb/ft	-	-	-
Uniform	0'	21'- 10"	User Load	165 lb/ft	-	220 lb/ft	-
Uniform	19'- 10"	21'- 10"	Rim1(i466)	45 lb/ft	-	-	-
Point	1'- 4"	1'- 4"	J1(i707)	218.00 lb	586.00 lb	-	-
Point	2'- 8"	2'- 8"	J1(i714)	209.00 lb	586.00 lb	-	-
Point	4'	4'	J1(i684)	209.00 lb	586.00 lb	-	-
Point	5'- 4"	5'- 4"	J1(i702)	209.00 lb	586.00 lb	-	-
Point	6'- 8"	6'- 8"	J1(i687)	209.00 lb	586.00 lb	-	-
Point	8'	8'	J1(i717)	209.00 lb	586.00 lb	-	-
Point	9'- 4"	9'- 4"	J1(i691)	209.00 lb	586.00 lb	-	-
Point	10'- 8"	10'- 8"	J1(i693)	209.00 lb	586.00 lb	-	-
Point	12'	12'	J1(i710)	209.00 lb	586.00 lb	-	-
Point	13'- 4"	13'- 4"	J1(i706)	209.00 lb	586.00 lb	-	-
Point	14'- 8"	14'- 8"	J1(i699)	209.00 lb	586.00 lb	-	-
Point	16'	16'	J1(i692)	209.00 lb	586.00 lb	-	-
Point	17'- 4"	17'- 4"	J1(i683)	209.00 lb	586.00 lb	-	-
Point	18'- 8"	18'- 8"	J1(i682)	209.00 lb	586.00 lb	-	-
Point	20'	20'	J1(i686)	209.00 lb	586.00 lb	-	-
Point	21'- 4"	21'- 4"	J1(i705)	183.00 lb	512.00 lb	-	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	2'- 1"	-	3652.00 lb	4790.00 lb	2687.00 lb	-
+++	0'- 1 1/2"	0'- 1 1/2"	E1(i6)	-	189.00 lb	226.00 lb	-
+++	1'- 11 1/2"	1'- 11 1/2"	E11(i29)	3652.00 lb	4601.00 lb	2461.00 lb	-
2	20'- 1"	21'- 10"	E10(i28)	3725.00 lb	5138.00 lb	2574.00 lb	-
==>	20'- 2 1/2"	20'- 2 1/2"	E10(i28)	3725.00 lb	4753.00 lb	2381.00 lb	-

- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.

- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.



Job: STR. 1006 Butler Drive. EWP
Member Type: Beam | Level: 2nd Floor
MiTek SAPPHIRE™ Supply Version 8.2.2.241.Update18
Designed by Single Member Design Engine

Label: BM6-2-i688

Page: 2 of 12
Date: 03/17/2022 11:53:22

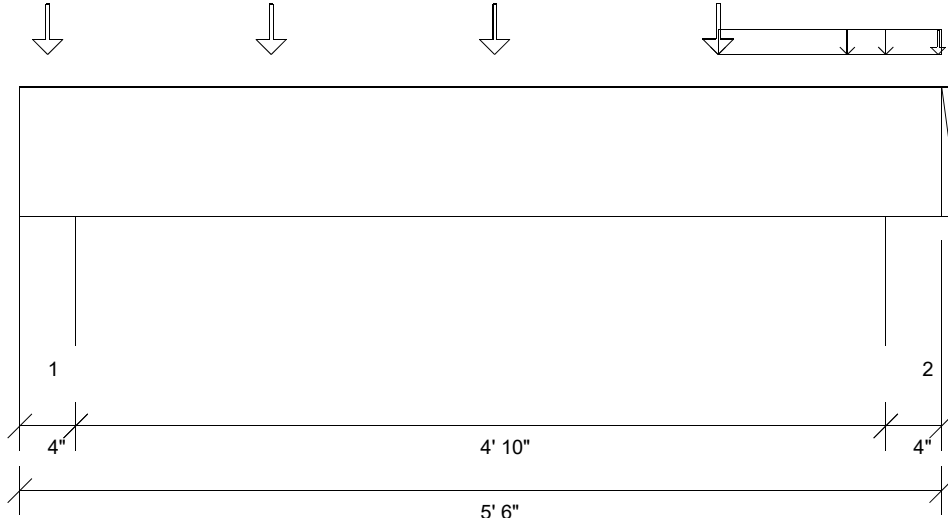
Member: 2 - onCENTER LVL 2.0E 1 3/4" x 18"

Status: Design Passed

==> 21'- 8 1/2" 21'- 8 1/2" E10(i28) - 385.00 lb 193.00 lb -

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
 Member Cut Length - 5'- 6"
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 0'	Bottom:	1'- 2 1/2"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination	
Critical Moment (Pos)	2'- 10"	454.89 lb ft	3429.65 lb ft	Passed - 13%	1.00	D + L	
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	4'- 4 3/4"	293.73 lb	2497.50 lb	Passed - 12%	1.00	D + L	
Live Load Deflection	2'- 9 1/4"	0'	N/A (L/360)	Passed - L/999	-	L	
Total Load Deflection	2'- 9 1/4"	0'	N/A (L/240)	Passed - L/999	-	D + L	
Max. Reaction	0'- 3"	467.26 lb	Supported Mt 5100.07 lb	Supported Mt 10500.14 lb	Passed - 9%	1.00	D + L
	5'- 3"	309.45 lb	5099.98 lb	10499.96 lb	Passed - 6%	1.00	D + L

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

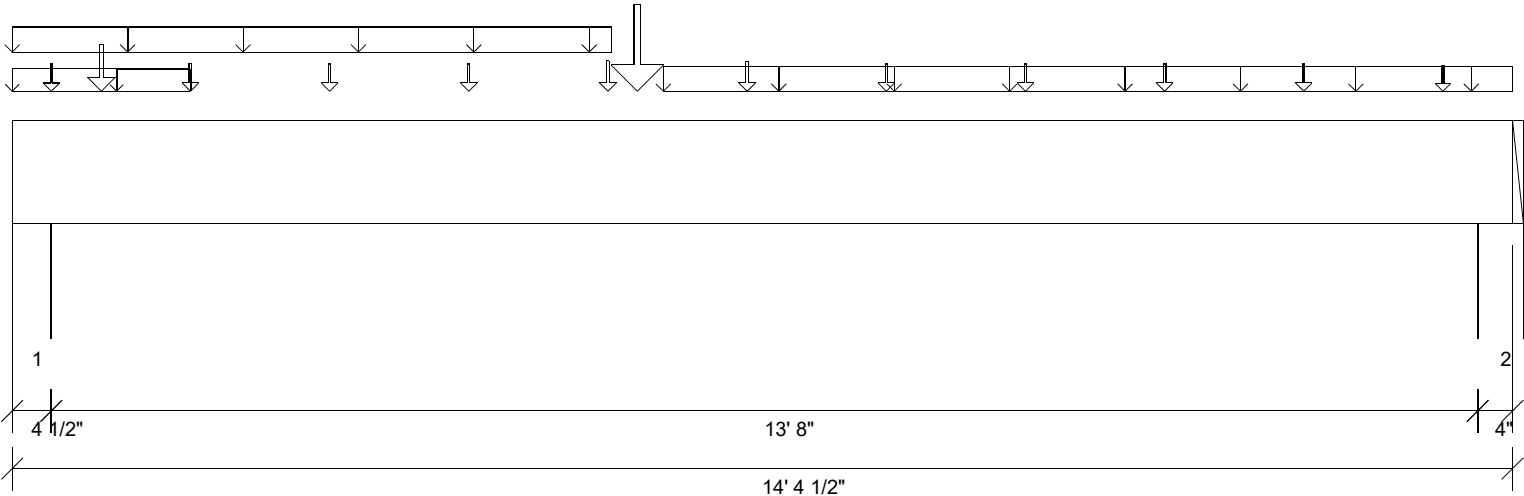
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	5'- 6"	Self Weight	6 lb/ft	-	-	-
Uniform	4'- 2"	5'- 2"	FC2 Floor Material	2 lb/ft	5 lb/ft	-	-
Uniform	5'- 2"	5'- 6"	FC2 Floor Material	2 lb/ft	8 lb/ft	-	-
Point	0'- 2"	0'- 2"	J7(i594)	42.00 lb	140.00 lb	-	-
Point	1'- 6"	1'- 6"	J7(i601)	42.00 lb	140.00 lb	-	-
Point	2'- 10"	2'- 10"	J7(i598)	42.00 lb	140.00 lb	-	-
Point	4'- 2"	4'- 2"	J7(i582)	43.00 lb	144.00 lb	-	-
Point	5'- 5 3/4"	5'- 5 3/4"	FC2 Floor Material	-	1.00 lb	-	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E24(i20)	122.00 lb	351.00 lb	-	-
2	5'- 2"	5'- 6"	E19(i15)	83.00 lb	221.00 lb	-	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
 Member Cut Length - 14' 4 1/2"
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 0'	Bottom:	1'- 2 1/2"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	5'- 11 7/8"	21062.97 lb ft	21278.23 lb ft	Passed - 99%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	1'- 4 3/8"	4891.59 lb	7896.87 lb	Passed - 62%	1.00	D + L
Live Load Deflection	6'- 11 9/16"	0'- 3/8"	N/A (L/360)	Passed - L/434	-	L
Total Load Deflection	6'- 11 5/8"	0'- 5/8"	N/A (L/240)	Passed - L/255	-	D + L
Max. Reaction	0'- 3 1/2"	6061.21 lb	Supported Mt/ 11812.29 lb	Passed - 51%	1.00	D + L
	14'- 1 1/2"	3856.91 lb	Supporting Mt/ 12249.88 lb	Passed - 37%	1.00	D + L

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

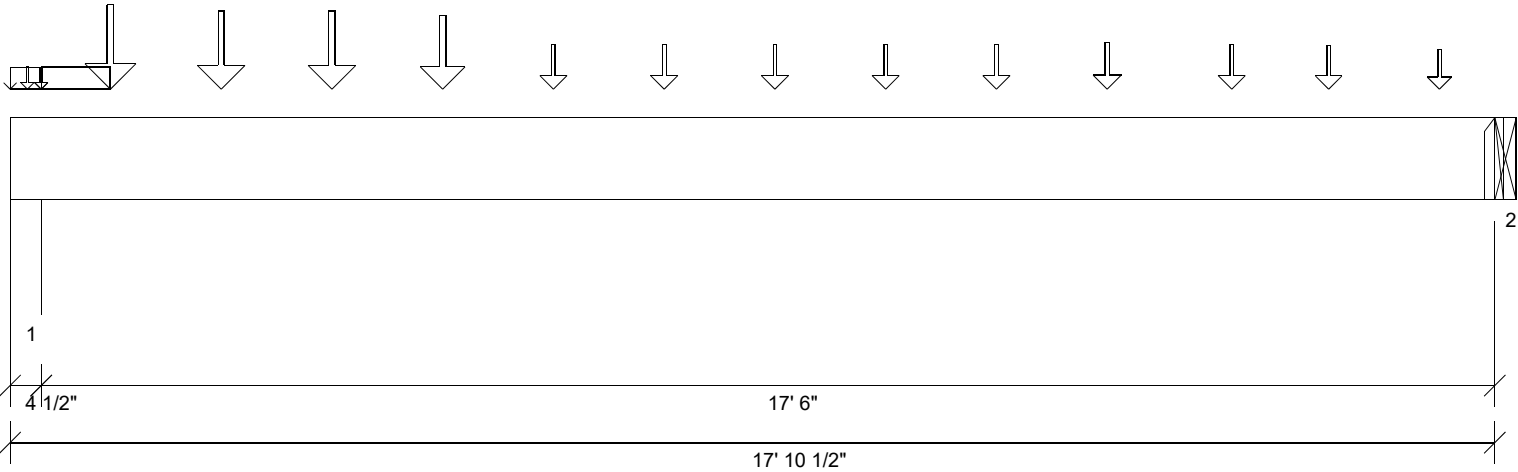
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	14'- 4 1/2"	Self Weight	11 lb/ft	-	-	-
Uniform	0'	5'- 8 7/8"	E34(i55)	45 lb/ft	-	-	-
Uniform	-0'	1'	FC2 Floor Material	5 lb/ft	15 lb/ft	-	-
Uniform	1'	1'- 8 1/2"	FC2 Floor Material	2 lb/ft	4 lb/ft	-	-
Uniform	6'- 2 7/8"	14'- 4 1/2"	E35(i77)	45 lb/ft	-	-	-
Point	0'- 4 1/2"	0'- 4 1/2"	J3(i653)	94.00 lb	235.00 lb	-	-
Point	0'- 10 1/4"	0'- 10 1/4"	BM4-2(i719)	409.00 lb	1031.00 lb	-	-
Point	1'- 8 1/2"	1'- 8 1/2"	J3(i624)	95.00 lb	238.00 lb	-	-
Point	3'- 1/2"	3'- 1/2"	J3(i656)	96.00 lb	241.00 lb	-	-
Point	4'- 4 1/2"	4'- 4 1/2"	J3(i656)	96.00 lb	241.00 lb	-	-
Point	5'- 8 1/2"	5'- 8 1/2"	J3(i625)	237.00 lb	262.00 lb	-	-
Point	5'- 11 7/8"	5'- 11 7/8"	PBO7(i76)	1432.00 lb	2341.00 lb	-	-
Point	7'- 1/2"	7'- 1/2"	J3(i615)	224.00 lb	241.00 lb	-	-
Point	8'- 4 1/2"	8'- 4 1/2"	J3(i656)	96.00 lb	241.00 lb	-	-
Point	9'- 8 1/2"	9'- 8 1/2"	J3(i640)	96.00 lb	241.00 lb	-	-
Point	11'- 1/2"	11'- 1/2"	J3(i640)	96.00 lb	241.00 lb	-	-
Point	12'- 4 1/2"	12'- 4 1/2"	J3(i640)	96.00 lb	241.00 lb	-	-
Point	13'- 8 1/2"	13'- 8 1/2"	J3(i593)	72.00 lb	181.00 lb	-	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4 1/2"	E17(i41)	2344.00 lb	3781.00 lb	-	-
2	14'- 1/2"	14'- 4 1/2"	E8(i40)	1581.00 lb	2212.00 lb	-	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
 Member Cut Length - 17'- 10 1/2"
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 0'	Bottom:	1'- 4 1/2"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	7'- 10 1/2"	7329.75 lb ft	21278.23 lb ft	Passed - 34%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	1'- 4 3/8"	1659.99 lb	7896.87 lb	Passed - 21%	1.00	D + L
Live Load Deflection	8'- 10"	0'- 5/16"	N/A (L/360)	Passed - L/703	-	L
Total Load Deflection	8'- 10 1/8"	0'- 7/16"	N/A (L/240)	Passed - L/507	-	D + L
Max. Reaction	0'- 3 1/2"	2184.83 lb	Supported Mt 11812.50 lb	Passed - 18%	1.00	D + L
	17'- 10 1/2"	1440.70 lb	Supporting Mt 13781.25 lb	Passed - 37%	1.00	D + L
			0.00 lb			

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	17'- 10 1/2"	Self Weight	11 lb/ft	-	-	-
Uniform	0'	0'- 4 1/2"	FC2 Floor Material	-	1 lb/ft	-	-
Uniform	0'- 4 1/2"	1'- 2 1/2"	FC2 Floor Material	2 lb/ft	6 lb/ft	-	-
Point	0'- 2 1/2"	0'- 2 1/2"	E37(i78)	13.00 lb	-	-	-
Point	0'- 4 1/2"	0'- 4 1/2"	FC2 Floor Material	-	1.00 lb	-	-
Point	1'- 2 1/2"	1'- 2 1/2"	J2(i703)	156.00 lb	333.00 lb	-	-
Point	2'- 6 1/2"	2'- 6 1/2"	J2(i718)	103.00 lb	342.00 lb	-	-
Point	3'- 10 1/2"	3'- 10 1/2"	J2(i708)	103.00 lb	342.00 lb	-	-
Point	5'- 2 1/2"	5'- 2 1/2"	J2(i712)	94.00 lb	313.00 lb	-	-
Point	6'- 6 1/2"	6'- 6 1/2"	J5(i661)	43.00 lb	142.00 lb	-	-
Point	7'- 10 1/2"	7'- 10 1/2"	J5(i661)	43.00 lb	142.00 lb	-	-
Point	9'- 2 1/2"	9'- 2 1/2"	J7(i594)	43.00 lb	142.00 lb	-	-
Point	10'- 6 1/2"	10'- 6 1/2"	J7(i594)	43.00 lb	142.00 lb	-	-
Point	11'- 10 1/2"	11'- 10 1/2"	J7(i598)	43.00 lb	142.00 lb	-	-
Point	13'- 2 1/2"	13'- 2 1/2"	J7(i582)	45.00 lb	151.00 lb	-	-
Point	14'- 8 1/2"	14'- 8 1/2"	J6(i606)	43.00 lb	142.00 lb	-	-
Point	15'- 10 1/2"	15'- 10 1/2"	J6(i648)	40.00 lb	133.00 lb	-	-
Point	17'- 2 1/2"	17'- 2 1/2"	J6(i674)	34.00 lb	112.00 lb	-	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4 1/2"	E22(i18)	632.00 lb	1553.00 lb	-	-
2	17'- 10 1/2"	17'- 10 1/2"	BM5-2(i711)	409.00 lb	1031.00 lb	-	-

Connector Information:

Support	Manufacturer	Model	Nailing Requirements			MII Seal Length	Other Information
			Top	Face	Member		
2	USP	HD412	-	16- 16d	8- 10d	N/A	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.

- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.

- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.



Job: STR. 1006 Butler Drive. EWP
Member Type: Beam | Level: 2nd Floor
MiTek SAPPHIRE™ Supply Version 8.2.2.241.Update18
Designed by Single Member Design Engine

Label: BM4-2-i719

Page: 6 of 12
Date: 03/17/2022 11:53:22

Member: 2 - onCENTER LVL 2.0E 1 3/4" x 11 7/8"

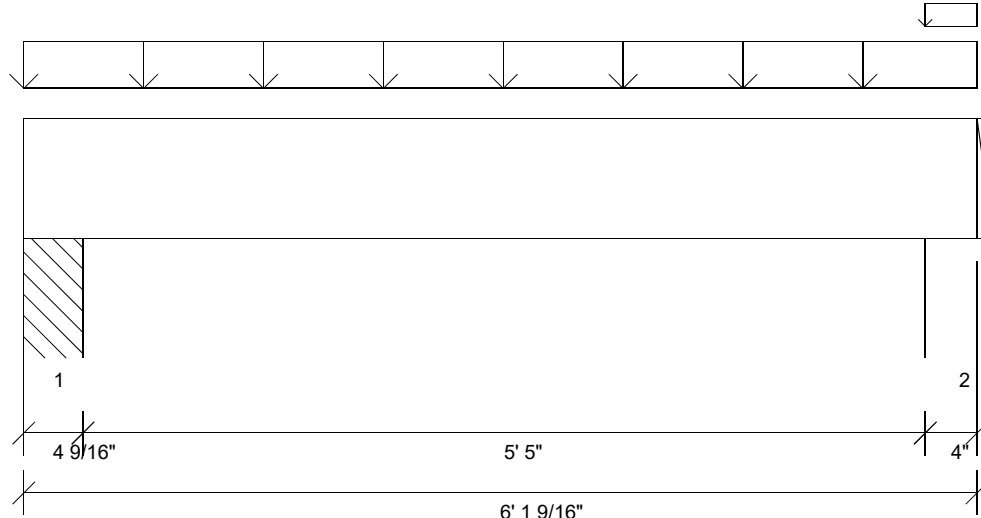
Status: Design Passed

* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.

* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.

- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.



Graphical Illustration - Not To Scale
 Member Cut Length - 6'- 3 1/16"
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 5'- 9 9/16"	Bottom:	5'- 9 9/16"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination	
Critical Moment (Pos)	3'- 1 1/16"	1123.01 lb ft	4287.06 lb ft	Passed - 26%	1.25	D + 0.75(L + Lr)	
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1 13/16"	564.51 lb	3121.88 lb	Passed - 18%	1.25	D + 0.75(L + Lr)	
Live Load Deflection	3'- 1 1/16"	0'	N/A (L/360)	Passed - L/999	-	0.75(L + Lr)	
Total Load Deflection	3'- 1 1/16"	0'	N/A (L/240)	Passed - L/999	-	D + 0.75(L + Lr)	
Max. Reaction	0'- 3 9/16"	899.56 lb	Supported Mt 5820.58 lb	Supported Mt 9929.23 lb	Passed - 15%	1.25	D + 0.75(L + Lr)
	5'- 10 9/16"	886.98 lb	5100.04 lb	10500.08 lb	Passed - 17%	1.25	D + 0.75(L + Lr)

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

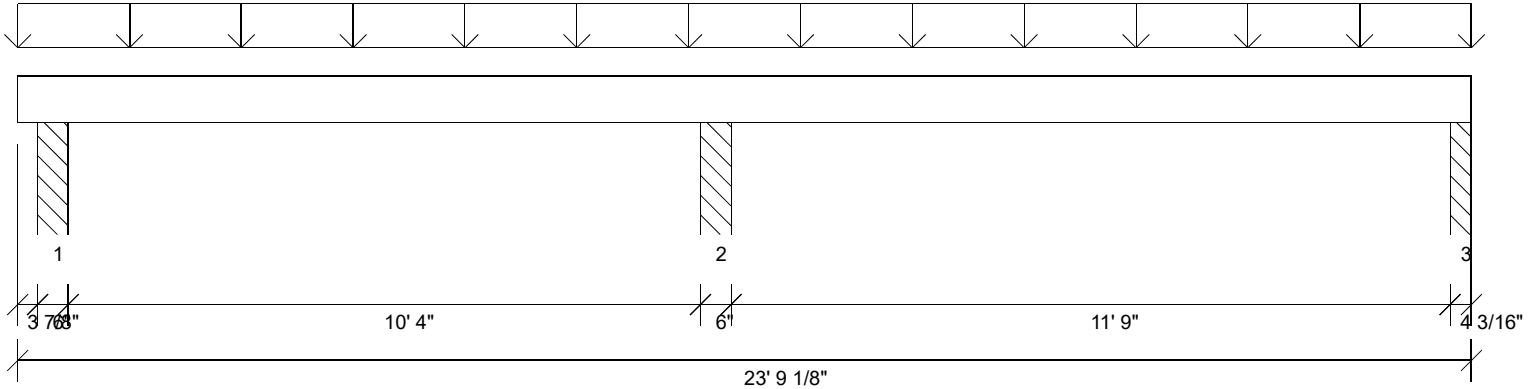
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	-0'	6'- 1 9/16"	Self Weight	6 lb/ft	-	-	-
Uniform	-0'	6'- 1 9/16"	User Load	150 lb/ft	60 lb/ft	120 lb/ft	-
Uniform	5'- 9 9/16"	6'- 1 9/16"	FC2 Floor Material	3 lb/ft	6 lb/ft	-	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	-0'	0'- 4 9/16"	PBO3(i27)	482.00 lb	185.00 lb	371.00 lb	-
2	5'- 9 9/16"	6'- 1 9/16"	E5(i38)	475.00 lb	184.00 lb	365.00 lb	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
 Member Cut Length - 23'- 10 7/8"
 MemberPitch - 0/12

Design Information:

Building Code: IRC 2018	Floor Dead Load: 12.0 lb/ft ²	Roof Dead Load: 15.0 lb/ft ²	Ground Snow Load: 0.0 lb/ft ²
Design Methodology: ASD	Floor Live Load: 40.0 lb/ft ²	Roof Live Load: 20.0 lb/ft ²	
	Unbraced Length Top: 23'- 10 7/8"	Bottom: 23'- 10 7/8"	

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	11'- 4 15/16"	4845.74 lb ft	12078.16 lb ft	Passed - 40%	1.25	D + 0.75(L + Lr)
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	12'- 5 3/16"	1873.70 lb	7689.06 lb	Passed - 24%	1.25	D + 0.75(L + Lr)
Live Load Deflection	17'- 9 11/16"	0'- 1/8"	N/A (L/360)	Passed - L/999	-	0.75(L + Lr)
Total Load Deflection	18'- 1/4"	0'- 3/16"	N/A (L/240)	Passed - L/830	-	D + 0.75(L + Lr)
Max. Reaction			<u>Supported Mt</u> <u>Supporting Mt</u>			
	0'- 6 7/8"	1435.38 lb	16734.29 lb 15224.92 lb	Passed - 9%	1.25	D + 0.75(L + Lr)
	11'- 4 15/16"	4205.50 lb	16734.31 lb 15224.94 lb	Passed - 28%	1.25	D + 0.75(L + Lr)
	23'- 5 15/16"	1527.94 lb	10983.25 lb 10617.15 lb	Passed - 14%	1.25	D + 0.75(L + Lr)

Design Notes:

- * The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- * Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	23'- 9 1/8"	Self Weight	8 lb/ft	-	-	-
Uniform	0'	23'- 9 1/8"	User Load	150 lb/ft	60 lb/ft	120 lb/ft	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'- 3 7/8"	0'- 9 7/8"	PBO1(i25)	710.00 lb	322.00/-53.00 lb	644.00/-107.00 lb	-
2	11'- 1 15/16"	11'- 7 15/16"	PBO2(i26)	2271.00 lb	860.00 lb	1720.00 lb	-
3	23'- 4 15/16"	23'- 9 1/8"	PBO3(i27)	783.00 lb	331.00/-34.00 lb	662.00/-68.00 lb	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



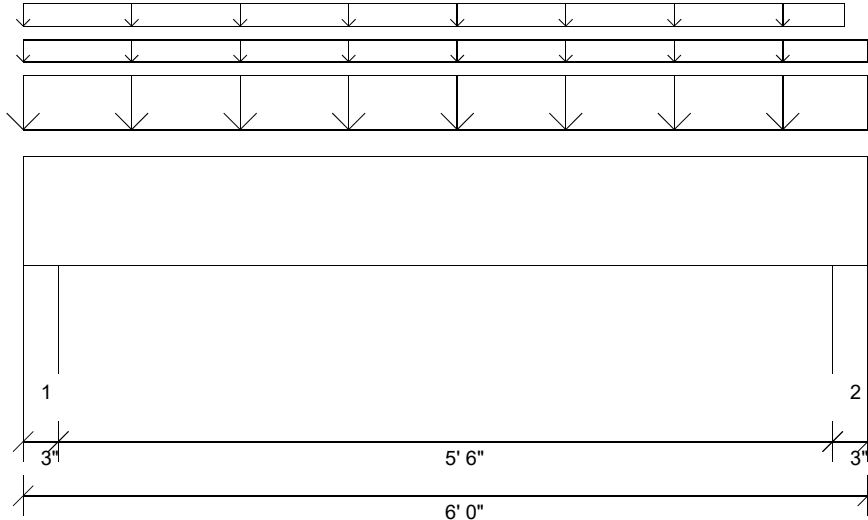
Job: STR. 1006 Butler Drive. EWP
 Member Type: Beam | Level: 2nd Floor
 MiTek SAPPiRE™ Supply Version 8.2.2.241.Update18
 Designed by Single Member Design Engine

Label: BM2-2-i581

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 Date: 03/17/2022 11:53:22

Member: 2 - onCENTER LVL 2.0E 1 3/4" x 9 1/4"

Status: Design Passed



Graphical Illustration - Not To Scale
 Member Cut Length - 6'
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 0'	Bottom:	5'- 6"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination	
Critical Moment (Pos)	3'	2638.05 lb ft	16650.34 lb ft	Passed - 16%	1.25	D + Lr	
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1/4"	1305.23 lb	7689.06 lb	Passed - 17%	1.25	D + Lr	
Live Load Deflection	3'	0'	N/A (L/360)	Passed - L/999	-	Lr	
Total Load Deflection	3'	0'- 1/16"	N/A (L/240)	Passed - L/999	-	D + Lr	
Max. Reaction	0'- 2"	1978.40 lb	<u>Supported Mt</u> 7875.03 lb	<u>Supporting Mt</u> 9187.54 lb	Passed - 25%	1.25	D + Lr
	5'- 10"	1970.62 lb	7875.03 lb	9187.54 lb	Passed - 25%	1.25	D + Lr

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

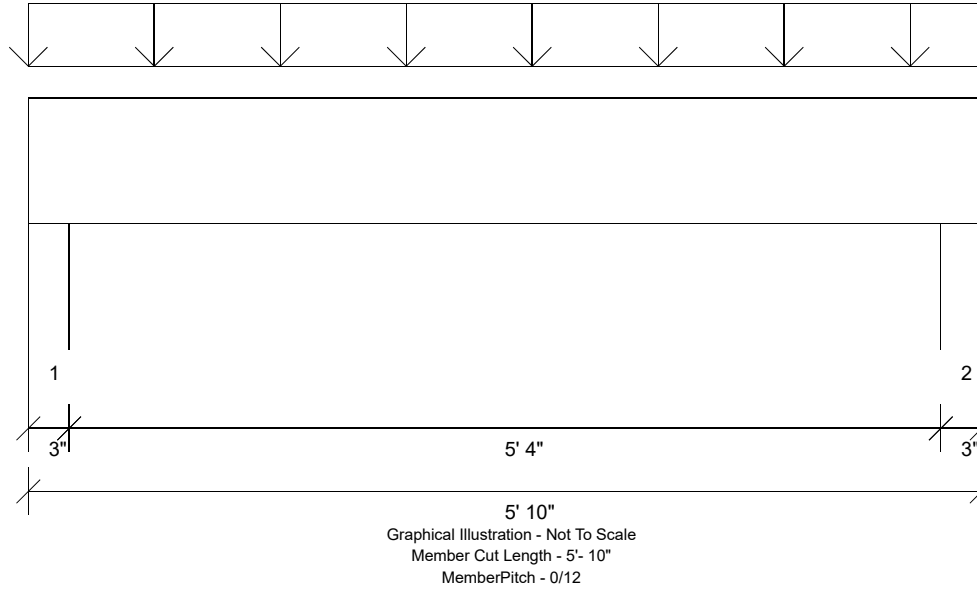
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	6'	Self Weight	8 lb/ft	-	-	-
Uniform	0'	6'	User Load	255 lb/ft	-	340 lb/ft	-
Uniform	0'	6'	Rim2(i672)	11 lb/ft	20 lb/ft	-	-
Uniform	0'	5'- 10"	Rim2(i672)	45 lb/ft	-	-	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 3"	E4(i39)	958.00 lb	60.00 lb	1020.00 lb	-
2	5'- 9"	6'	E5(i38)	951.00 lb	60.00 lb	1020.00 lb	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 5'- 4"	Bottom:	5'- 4"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination	
Critical Moment (Pos)	2'- 11"	2273.35 lb ft	16650.34 lb ft	Passed - 14%	1.25	D + Lr	
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1/4"	1144.00 lb	7689.06 lb	Passed - 15%	1.25	D + Lr	
Live Load Deflection	2'- 11"	0'	N/A (L/360)	Passed - L/999	-	Lr	
Total Load Deflection	2'- 11"	0'	N/A (L/240)	Passed - L/999	-	D + Lr	
Max. Reaction	0'- 2"	1760.01 lb	Supported Mt/ 7875.03 lb	9187.54 lb	Passed - 22%	1.25	D + Lr
	5'- 8"	1760.01 lb	7875.03 lb	9187.54 lb	Passed - 22%	1.25	D + Lr

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

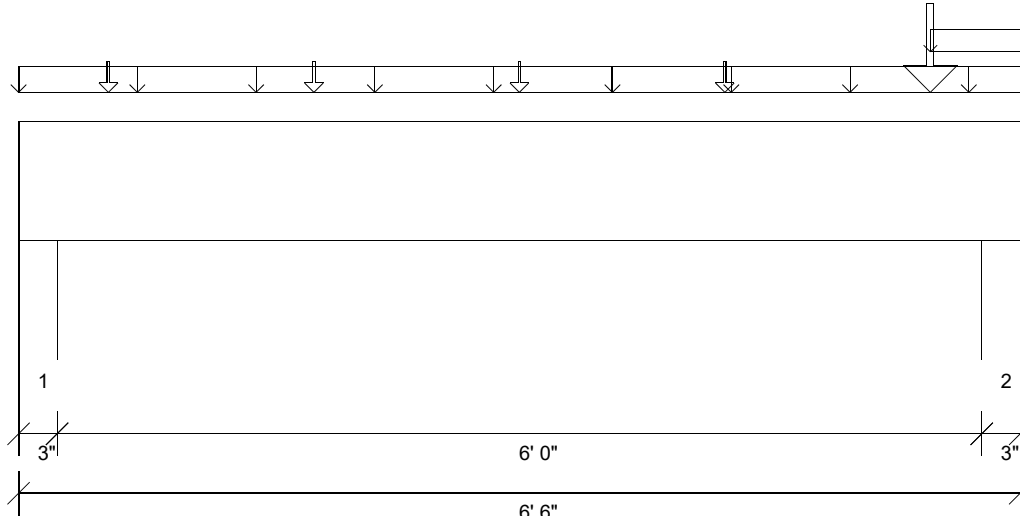
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	5'- 10"	Self Weight	8 lb/ft	-	-	-
Uniform	0'	5'- 10"	User Load	255 lb/ft	-	340 lb/ft	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 3"	E3(i4)	768.00 lb	-	992.00 lb	-
2	5'- 7"	5'- 10"	E4(i39)	768.00 lb	-	991.00 lb	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
 Member Cut Length - 6'- 6"
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 0'	Bottom:	6'		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination	
Critical Moment (Pos)	3'- 3"	2144.89 lb ft	13320.27 lb ft	Passed - 16%	1.00	D + L	
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1/4"	1027.80 lb	6151.25 lb	Passed - 17%	1.00	D + L	
Live Load Deflection	3'- 4 1/16"	0'	N/A (L/360)	Passed - L/999	-	L	
Total Load Deflection	3'- 4 3/16"	0'- 1/16"	N/A (L/240)	Passed - L/999	-	D + L	
Max. Reaction	0'- 2"	1317.32 lb	Supported Mt 7874.93 lb	9187.42 lb	Passed - 17%	1.00	D + L
	6'- 4"	3016.50 lb	7874.92 lb	9187.41 lb	Passed - 38%	1.00	D + L

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	6'- 6"	Self Weight	8 lb/ft	-	-	-
Uniform	0'	6'- 6"	Rim2(i676)	45 lb/ft	-	-	-
Uniform	5'- 11"	6'- 6"	Rim2(i676)	1 lb/ft	2 lb/ft	-	-
Point	0'- 7"	0'- 7"	J3(i658)	176.00 lb	238.00 lb	-	-
Point	1'- 11"	1'- 11"	J3(i588)	160.00 lb	238.00 lb	-	-
Point	3'- 3"	3'- 3"	J3(i633)	160.00 lb	238.00 lb	-	-
Point	4'- 7"	4'- 7"	J3(i618)	160.00 lb	238.00 lb	-	-
Point	5'- 11"	5'- 11"	J3(i590)	1208.00 lb	1169.00 lb	717.00 lb	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 3"	E16(i13)	659.00 lb	658.00 lb	48.00 lb	-
2	6'- 3"	6'- 6"	E17(i41)	1552.00 lb	1464.00 lb	669.00 lb	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



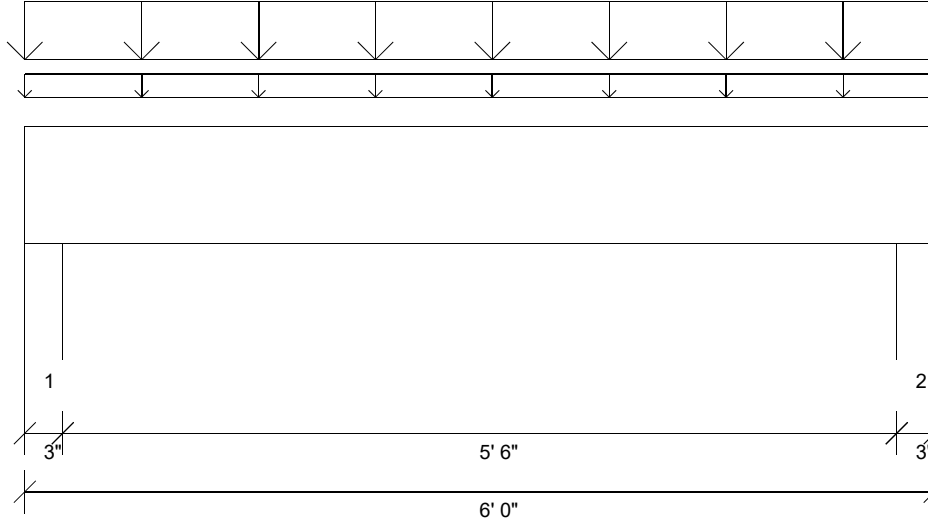
Job: STR. 1006 Butler Drive. EWP
 Member Type: Beam | Level: 2nd Floor
 MiTek SAPPHERE™ Supply Version 8.2.2.241.Update18
 Designed by Single Member Design Engine

Label: BM3-2-i622

Page: 12 of 12
 Date: 03/17/2022 11:53:23

Member: 2 - onCENTER LVL 2.0E 1 3/4" x 9 1/4"

Status: Design Passed



Graphical Illustration - Not To Scale
 Member Cut Length - 6'
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length Top:	0'	Bottom:	5'- 6"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	3'	2441.74 lb ft	16650.34 lb ft	Passed - 15%	1.25	D + Lr
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	1'- 1/4"	1208.14 lb	7689.06 lb	Passed - 16%	1.25	D + Lr
Live Load Deflection	3'	0'	N/A (L/360)	Passed - L/999	-	Lr
Total Load Deflection	3'	0'	N/A (L/240)	Passed - L/999	-	D + Lr
Max. Reaction	0'- 2"	1831.29 lb	<u>Supported Mt</u> 7874.93 lb	Passed - 23%	1.25	D + Lr
	5'- 10"	1831.29 lb	<u>Supporting Mt</u> 9187.41 lb	Passed - 23%	1.25	D + Lr

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

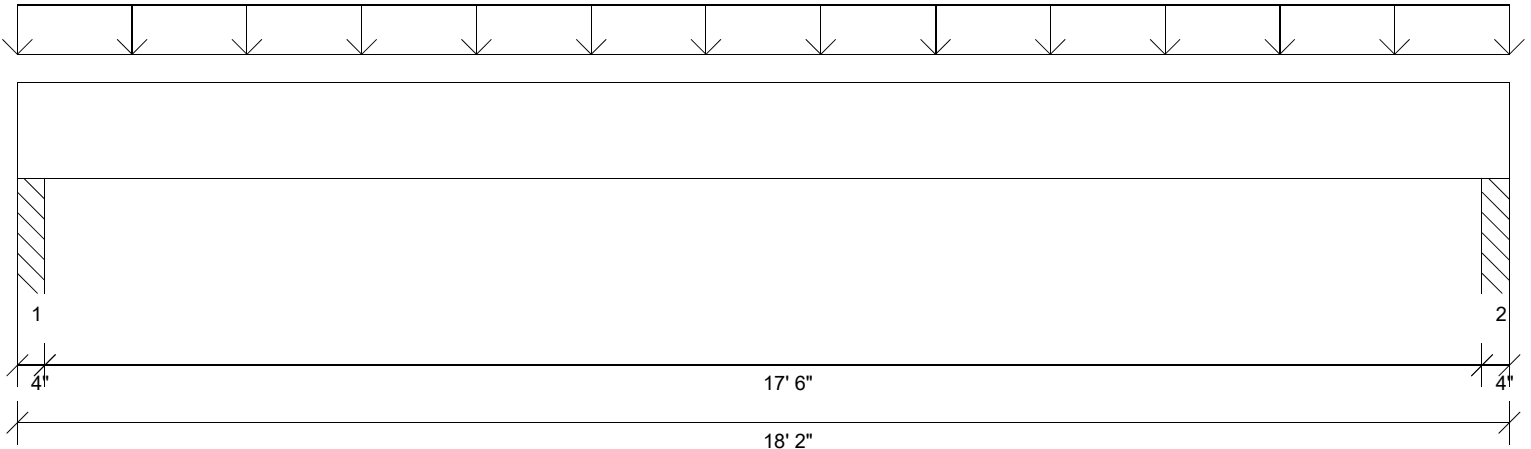
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	6'	Self Weight	8 lb/ft	-	-	-
Uniform	0'	6'	User Load	255 lb/ft	-	340 lb/ft	-
Uniform	-0'	6'	Rim2(i583)	7 lb/ft	10 lb/ft	-	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 3"	E8(i40)	811.00 lb	30.00 lb	1020.00 lb	-
2	5'- 9"	6'	E7(i2)	811.00 lb	30.00 lb	1020.00 lb	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
 Member Cut Length - 18'- 2"
 MemberPitch - 0/12

Design Information:

Building Code: IRC 2018	Floor Dead Load: 12.0 lb/ft ²	Roof Dead Load: 15.0 lb/ft ²	Ground Snow Load: 0.0 lb/ft ²
Design Methodology: ASD	Floor Live Load: 40.0 lb/ft ²	Roof Live Load: 20.0 lb/ft ²	
	Unbraced Length Top: 18'- 2"	Bottom: 18'- 2"	

Design Results:

	Location	Design	Control	Result	LDF	Load Combination	
Critical Moment (Pos)	9'- 1"	15949.45 lb ft	42377.24 lb ft	Passed - 38%	1.00	D + L	
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 6"	3102.64 lb	13965.00 lb	Passed - 22%	1.00	D + L	
Live Load Deflection	9'- 1"	0'- 1/4"	N/A (L/360)	Passed - L/902	-	L	
Total Load Deflection	9'- 1"	0'- 3/8"	N/A (L/240)	Passed - L/573	-	D + L	
Max. Reaction	0'- 3"	3716.35 lb	Supported Mt 15749.76 lb	Supported Mt 15224.77 lb	Passed - 24%	1.00	D + L
	17'- 11"	3716.35 lb	15749.85 lb	15224.86 lb	Passed - 24%	1.00	D + L

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

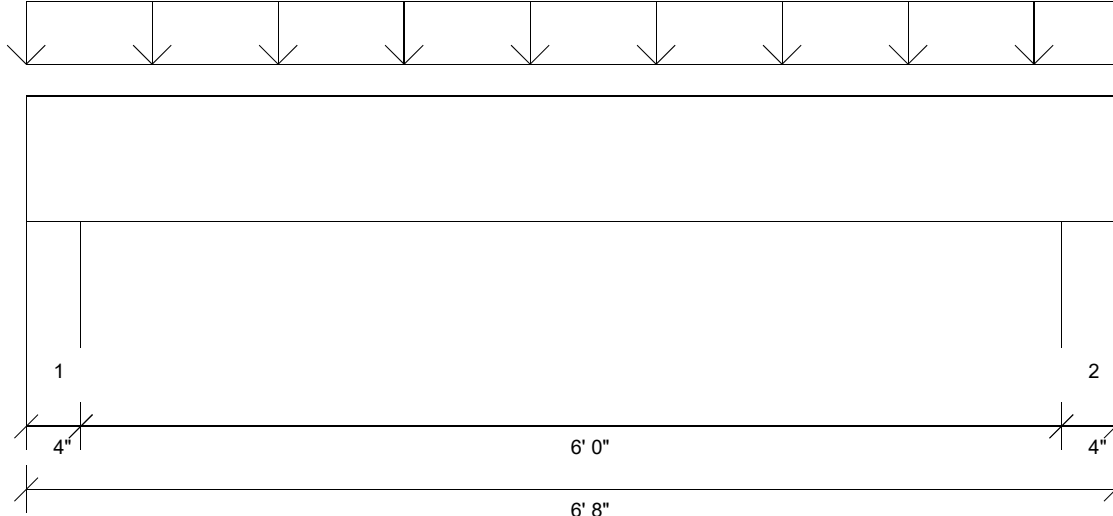
Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	18'- 2"	Self Weight	19 lb/ft	-	-	-
Uniform	-0'	18'- 2"	User Load	130 lb/ft	260 lb/ft	-	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	PBO6(i75)	1355.00 lb	2362.00 lb	-	-
2	17'- 10"	18'- 2"	PBO7(i76)	1355.00 lb	2362.00 lb	-	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
 Member Cut Length - 6'- 8"
 MemberPitch - 0/12

Design Information:

Building Code:	IRC 2018	Floor Dead Load:	12.0 lb/ft ²	Roof Dead Load:	15.0 lb/ft ²	Ground Snow Load:	0.0 lb/ft ²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft ²	Roof Live Load:	20.0 lb/ft ²		
		Unbraced Length	Top: 6'- 8"	Bottom:	6'		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination	
Critical Moment (Pos)	3'- 4"	2578.65 lb ft	3429.65 lb ft	Passed - 75%	1.00	D + L	
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1 1/4"	1217.24 lb	2497.50 lb	Passed - 49%	1.00	D + L	
Live Load Deflection	3'- 4"	0'- 1/16"	N/A (L/360)	Passed - L/999	-	0.75(L + Lr)	
Total Load Deflection	3'- 4"	0'- 1/16"	N/A (L/240)	Passed - L/950	-	D + 0.75(L + Lr)	
Max. Reaction			<u>Supported Mt</u>	<u>Supporting Mt</u>			
	0'- 3"	1820.15 lb	5099.98 lb	10499.96 lb	Passed - 36%	1.00	D + L
	6'- 5"	1820.15 lb	5099.98 lb	10499.96 lb	Passed - 36%	1.00	D + L

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

Type	Start	End	Source	Maximum Load Magnitudes			
				Dead	Floor Live	Roof Live	Snow
Self Weight	0'	6'- 8"	Self Weight	6 lb/ft	-	-	-
Uniform	0'	6'- 8"	User Load	280 lb/ft	260 lb/ft	200 lb/ft	-

Support Information:

Support	Start	End	Source	Maximum Analysis Reactions			
				Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E41(i60)	953.00 lb	867.00 lb	667.00 lb	-
2	6'- 4"	6'- 8"	E40(i59)	953.00 lb	867.00 lb	667.00 lb	-

Errors, Warnings & Notes:

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.