

# RENAISSANCE

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APPLATION, OR DISPLAY OF THE PLANS IS STRICTLY

INCHERIED, SEE NUM HOME SALES CONSULTANT FOR

WEAVER HOMES CAROLINA COLLECTION MAGNOLIA-II

DATE: FEBRUARY 19, 2021

REV.:

SCALE: 1/4" = 1'-0"

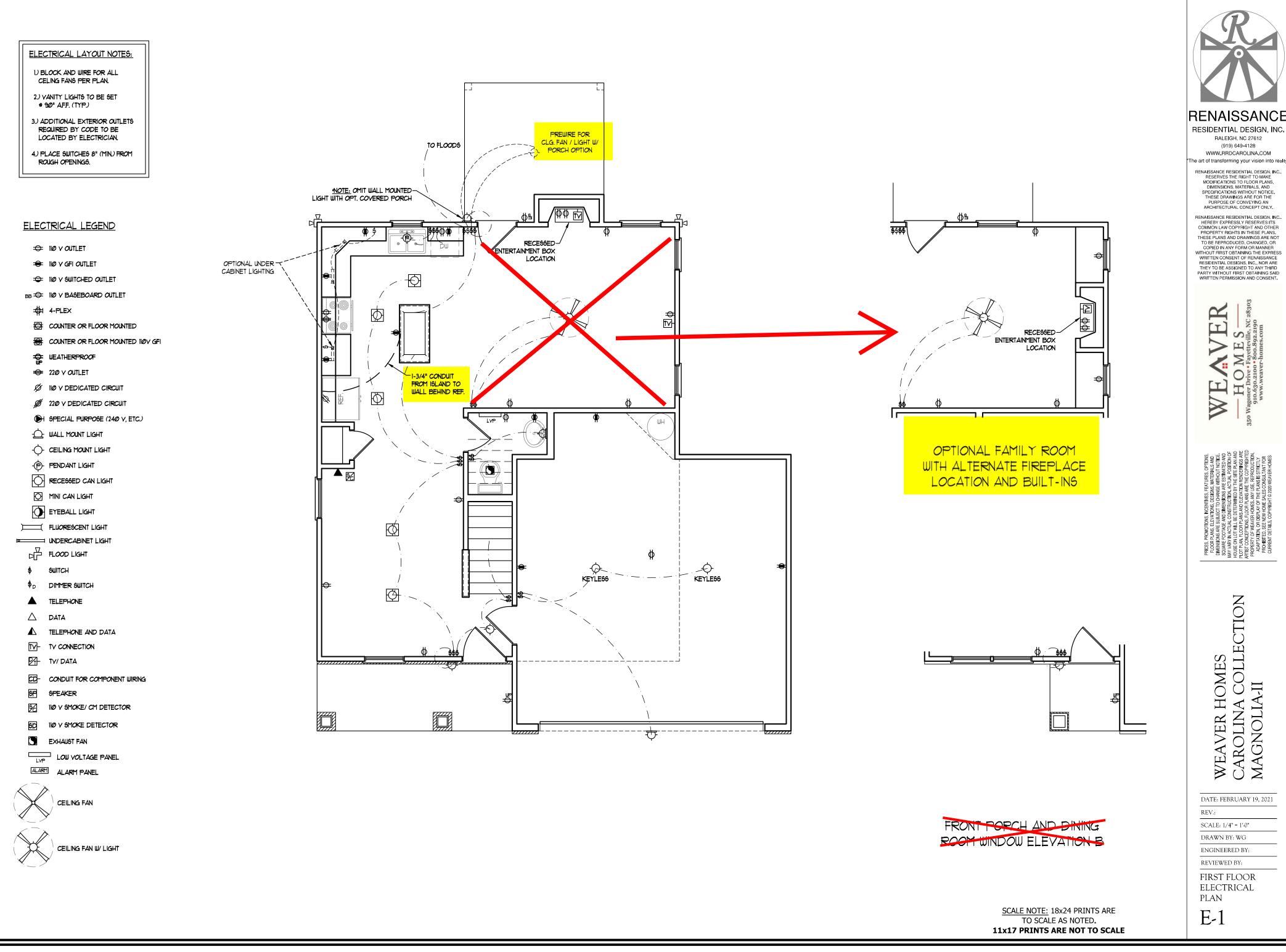
DRAWN BY: WG
ENGINEERED BY:

REVIEWED BY:

SECOND FLOOR PLAN

A-5

SCALE NOTE: 18x24 PRINTS ARE
TO SCALE AS NOTED.
11x17 PRINTS ARE NOT TO SCALE

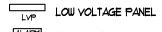




- I.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET 9 90" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

## ELECTRICAL LEGEND

- →
   IIØ ∨ OUTLET
- € 110 Y GFI OUTLET
- → 110 Y SWITCHED OUTLET
- BB IIØ Y BASEBOARD OUTLET
- 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110Y GF1
- ₩EATHERPROOF
- **⇒** 22*0* ∨ *0*UTLET
- Ø 110 Y DEDICATED CIRCUIT
- # 220 V DEDICATED CIRCUIT
- ●H SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- -P- PENDANT LIGHT
- RECESSED CAN LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- \_\_\_\_\_\_ FLUORESCENT LIGHT
- undercabinet light
- FLOOD LIGHT
- \$ SWITCH
- \$D DIMMER SWITCH
- ▲ TELEPHONE
  △ DATA
- ▲ TELEPHONE AND DATA
- TY- TY CONNECTION
- TV/ DATA
- CD- CONDUIT FOR COMPONENT WIRING
- SPEAKER
- 110 Y SMOKE/ CO DETECTOR
- 6D IIØ V SMOKE DETECTOR
- EXHAUST FAN



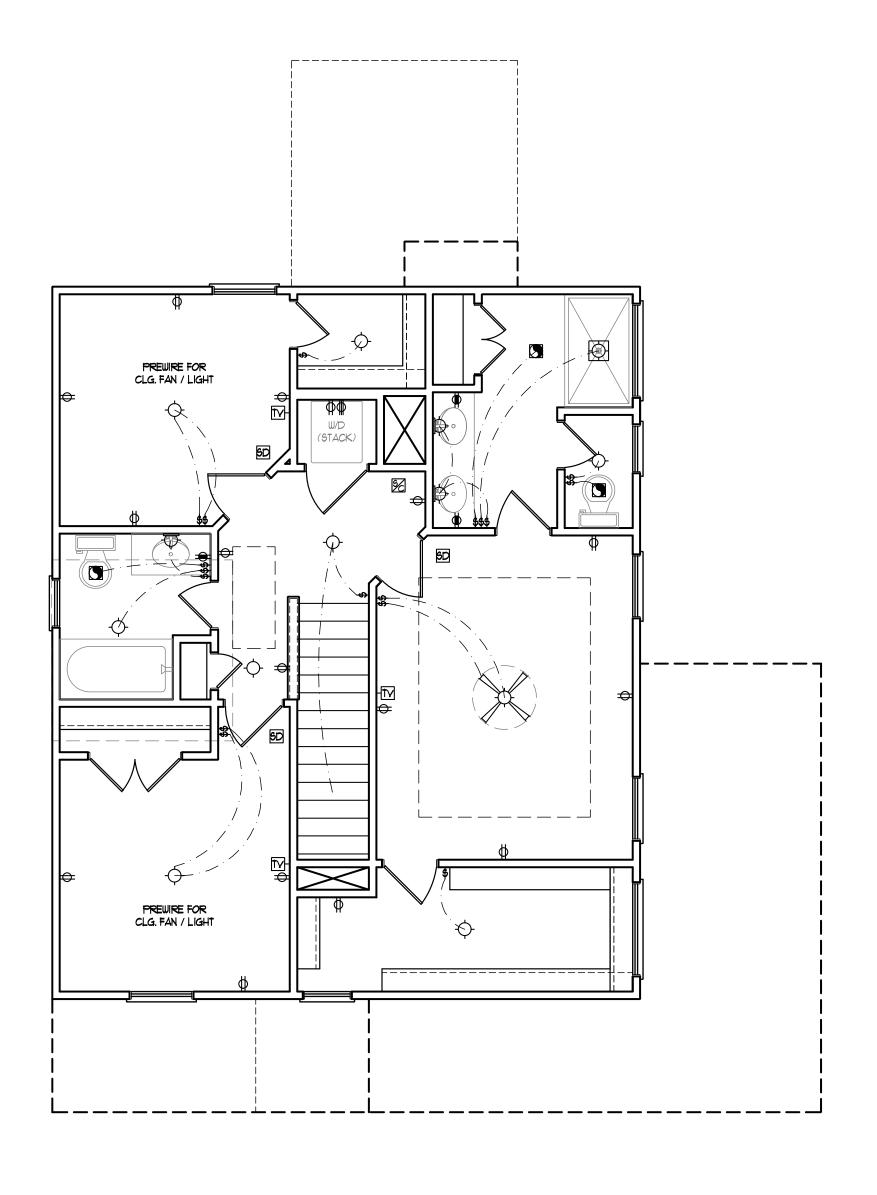




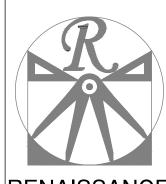
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WEAVER HOMES CAROLINA COLLECTIO MAGNOLIA-II

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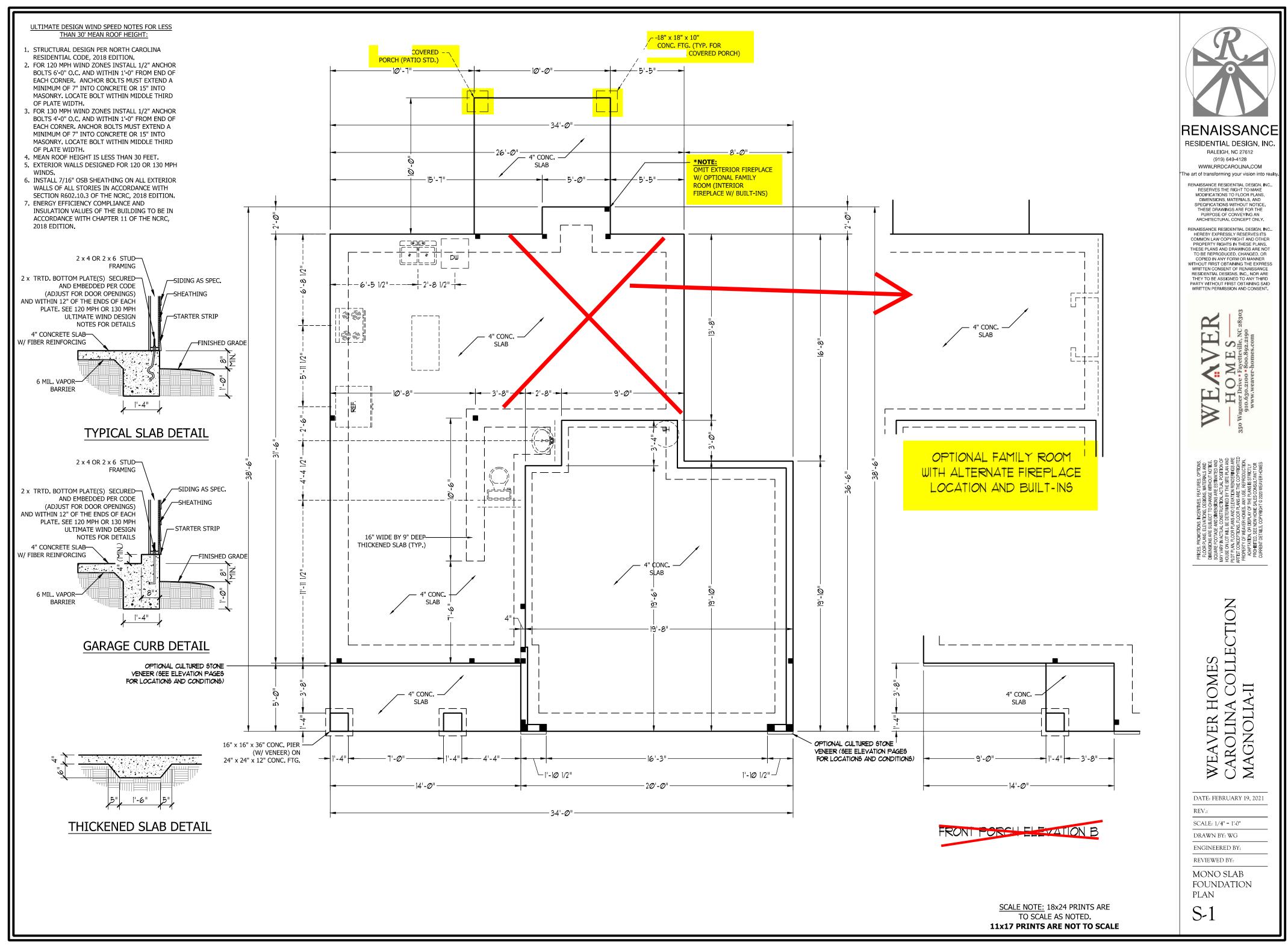
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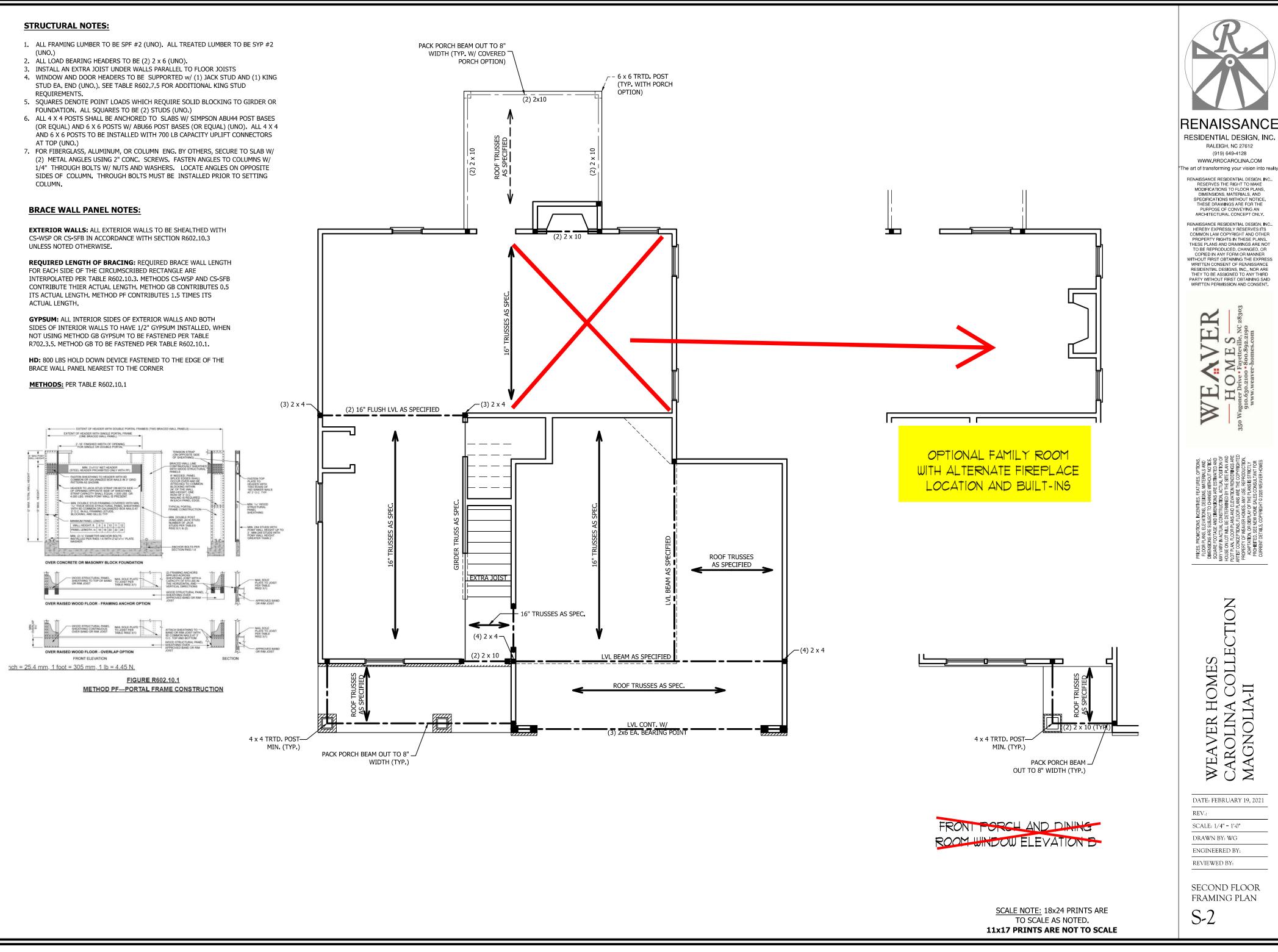
ENGINEERED BY:

REVIEWED BY:

SECOND FLOOR ELCTRICAL PLAN

E-2





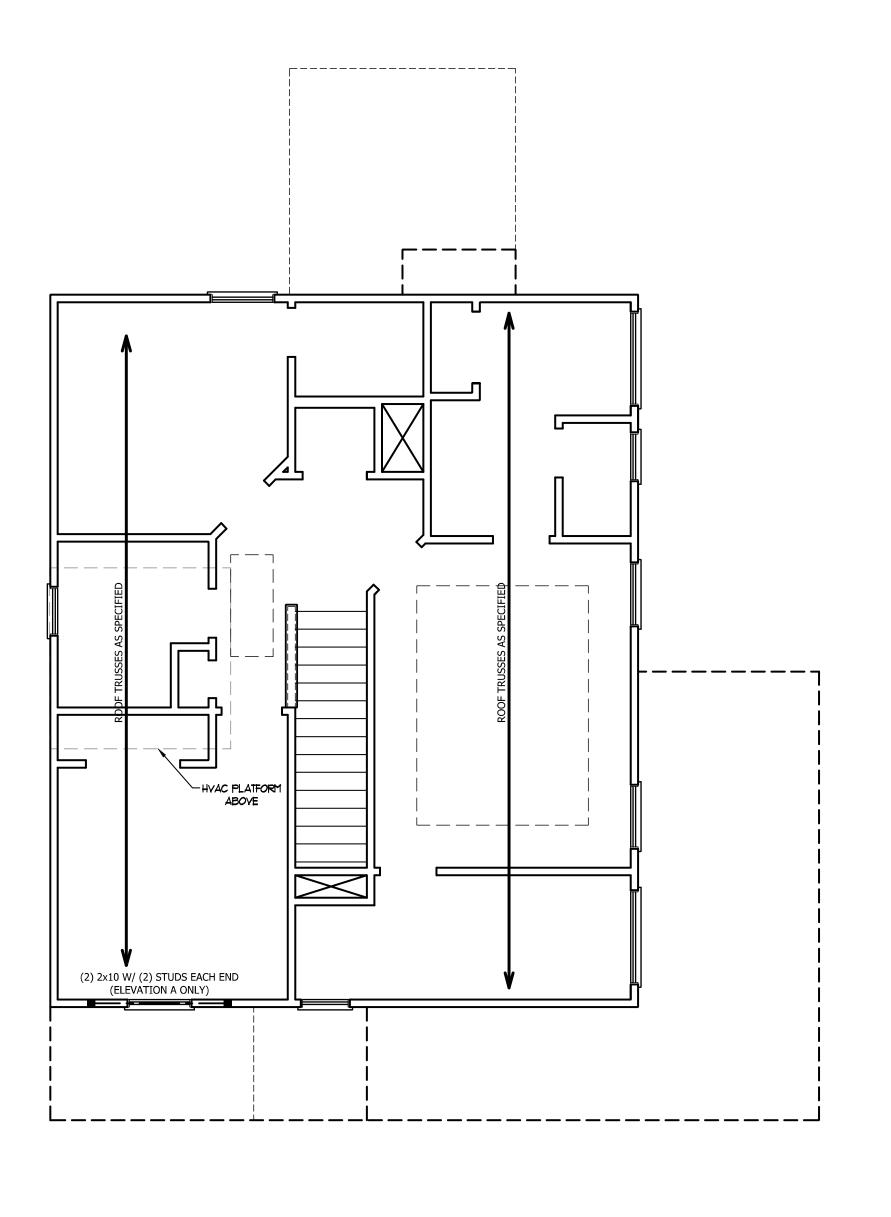


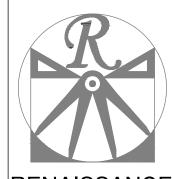
TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

AT EXCITEIO O	THE RELIES IN EA	CIERCON WINE			
HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHI (PER TABLE R602.3(5)				
()	16	24			
UP TO 3'	1	1			
4'	2	1			
8'	3	2			
12'	5	3			
16'	6	4			
	HEADER SPAN (FEET)  UP TO 3' 4' 8' 12'	HEADER SPAN (FEET)  16  UP TO 3' 4' 2 8' 12' 5			

## STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



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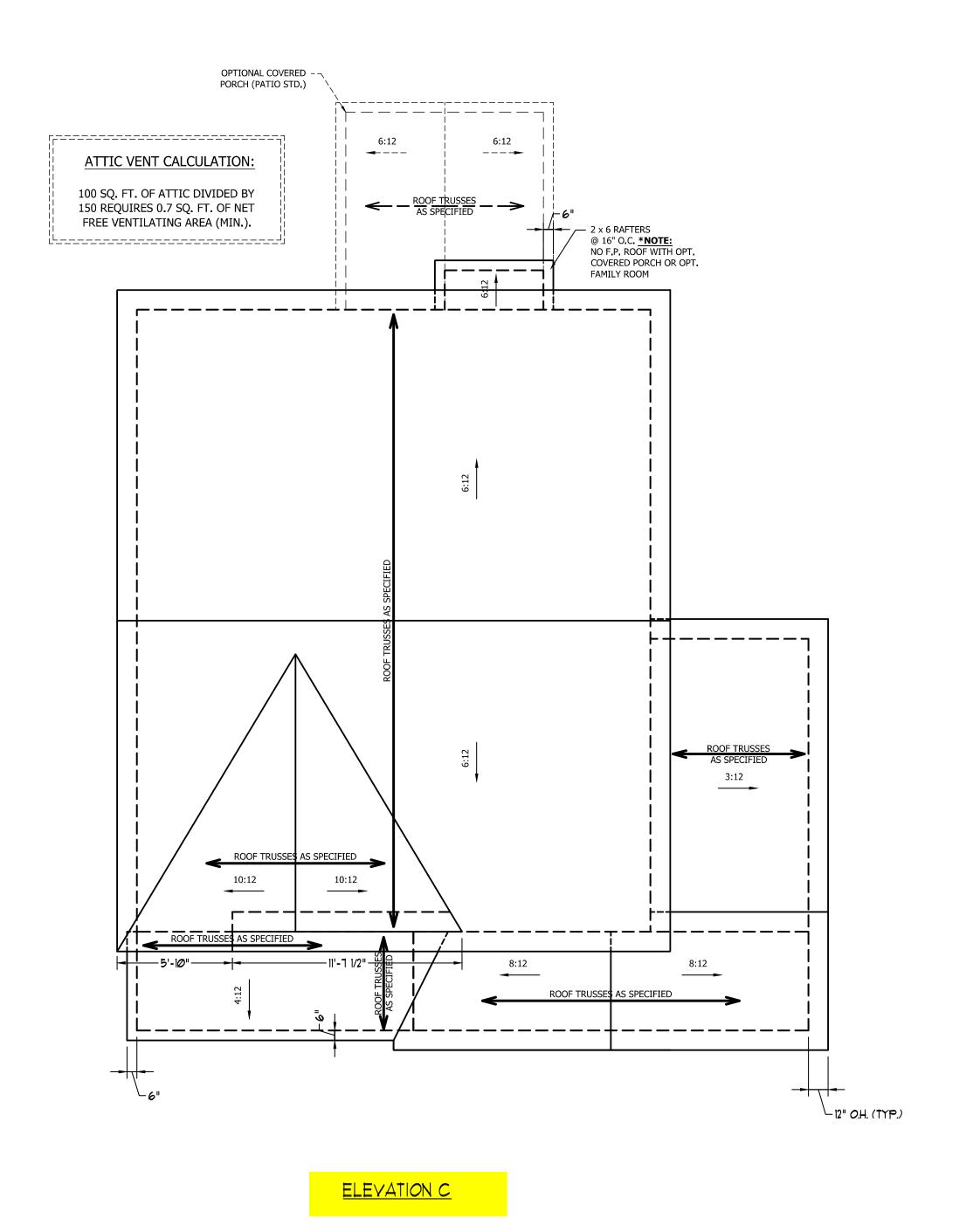
DRAWN BY: WG
ENGINEERED BY:

REVIEWED BY:

ATTIC FLOOR FRAMING PLAN

S-3

SCALE NOTE: 18x24 PRINTS ARE
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11x17 PRINTS ARE NOT TO SCALE



## **ATTIC VENT CALCULATION:**

1250 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 8.3 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

### **STRUCTURAL NOTES:**

- ALL FRAMING LUMBER TO BE #2 SPF (UNO).
   HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0".
- 2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8-0".
  FASTEN MEMBERS WITH THREE ROWS OF 12d
  NAILS @ 16" O.C. (TYP.)
  3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/
- 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES. 4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH
- MIN. OF (6) 12d TOE NAILS.

  5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.

NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A



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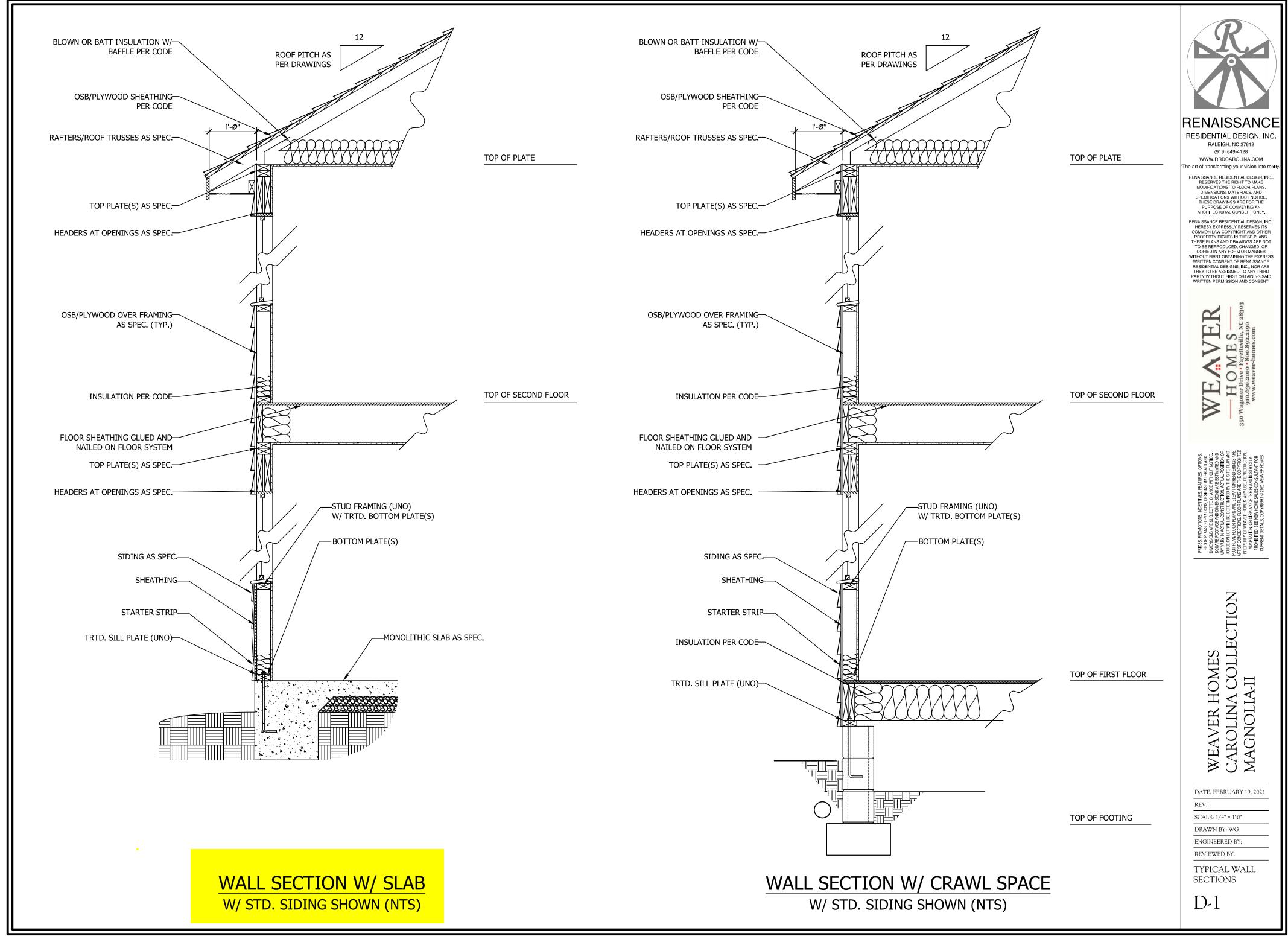
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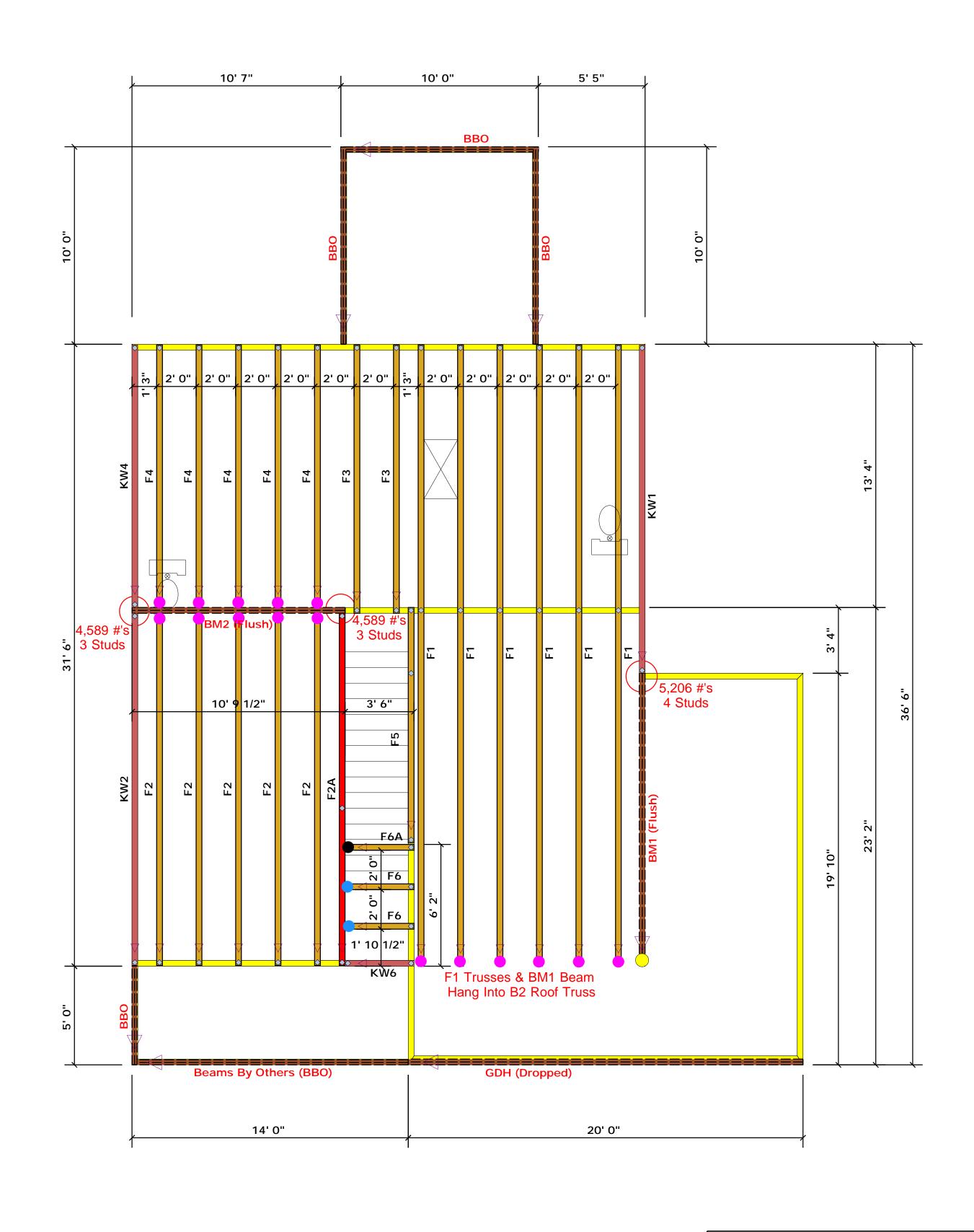
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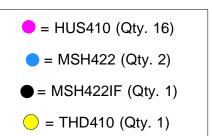
ROOF PLAN ELEVATION - C

S-4

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		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH (Dropped)	20' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
BM1 (Flush)	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM2 (Flush)	11' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF

Truss Placement Plan SCALE: NTS

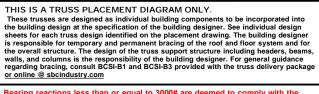
▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

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END REJ	36.		ETPOPENCE CIT 410	88. 88. 88.		END 8,000 50 ° 10	989 989
1700	1		2550	1		3400	1
3400	2		5100	2		6800	2
5100	3		7650	3		10200	3
6800	4	1	10200	4		13600	4
8500	5	1	12750	5		17000	5
10200	á	1	15300	6			
11900	7						
13600	8						
15300	9						

					_
	BUILDER	Weaver Development	CITY / CO.	Harnett Co. / Harnett	THIS I
il S	JOB NAME	Hayes Farm lot 3	ADDRESS	Hayes Farm Lot 3	is respo the ove walls, a regardi
(S) PL	PLAN	Magnolia I I "C"	MODEL	Floor	or onlin  Bearing  prescri
	SEAL DATE	Seal Date	DATE REV.	/ /	( derive foundar than 30 be reta
	QUOTE #	Quote #	DRAWN BY	Christine Shivy	specifie retaine
_	JOB#	J0821-5141	SALES REP.	Lenny Norris	Sig



Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for any reaction that exceed 15000#.

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Christine Shivy

Christine Shivy

TRUSSES & BEAMS

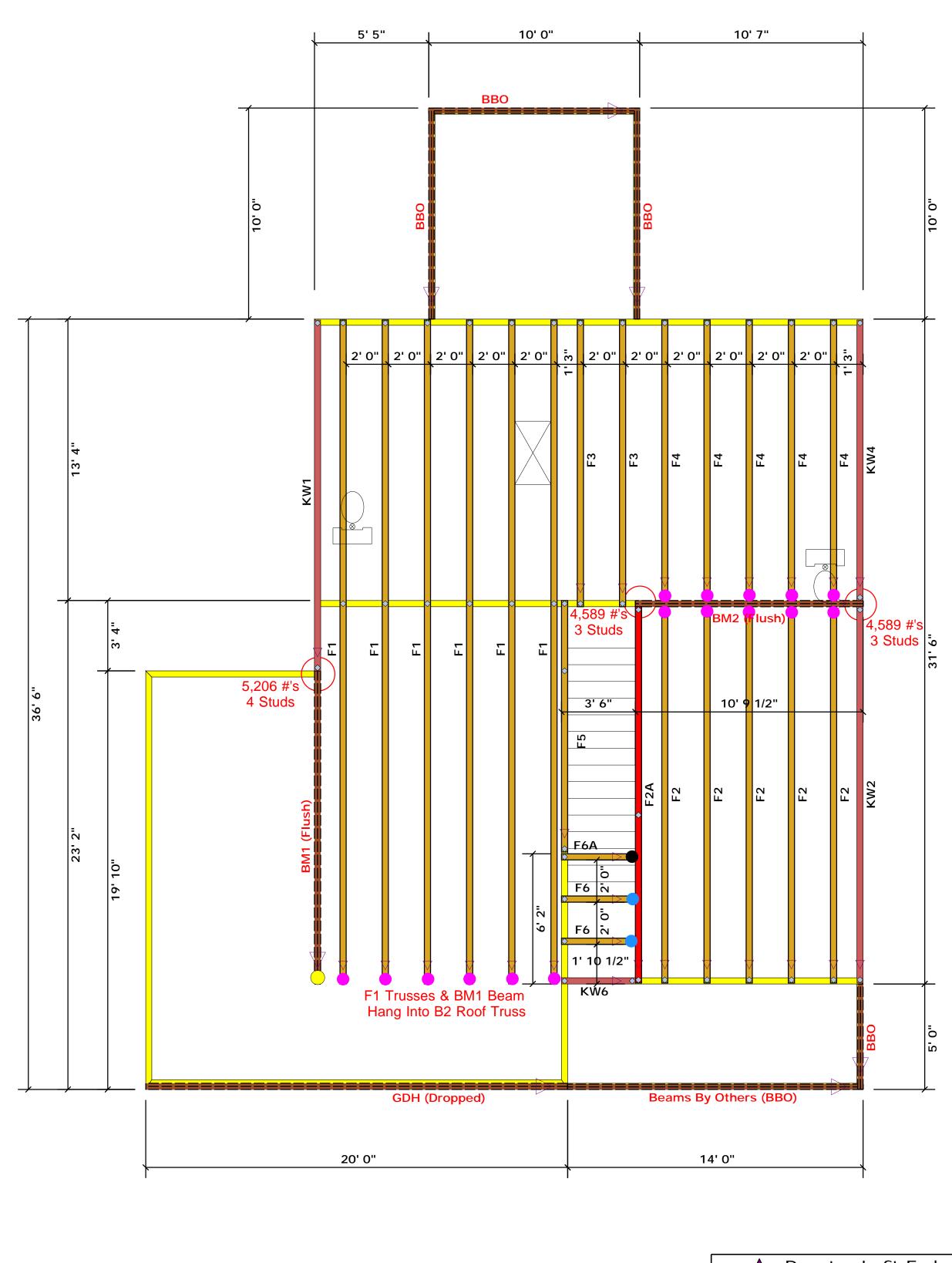
Reilly Road Industrial Park
Fayetteville, N.C. 28309

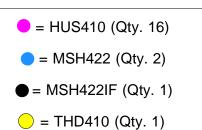
Phone: (910) 864-8787

Fax: (910) 864-4444

соттесн

**ROOF & FLOOR** 





# Truss Placement Plan SCALE: NTS

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH (Dropped)	20' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
BM1 (Flush)	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
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All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

LOA	D C	HART FO	RJ	4CK 5	STUD	5
	m.	ASED ON TABLES	R502	510 4.0	m	
NUMB		E JACK STUDG &				
		PEAGER/6	CROES			
NDEFACTOR	(C) NY HEADEN	CUL ATO	NEQ 15 STUDS FOR CODEN FEMBER		ND 43ACTOON (01.10)	REQ'D STUDS FOR (4) NY HEADER
1700	1	2550	1		3400	1
3400	2	5100	2		6600	2
5100	3	7650	3		10200	3
6800	4	10200	4		13600	4
8500	5	12750	5		17000	5
10200	á	15300	6			
11900	7					
13600	8					
15300	9					

	BUILDER	Weaver Development	CITY / CO.	Harnett Co. / Harnett	THIS Thes the bu
	JOB NAME	. Hayes Farm Lot 3	ADDRESS	Hayes Farm Lot 3	is res the ov walls, regard
(3) (4)	PLAN	Magnolia I I "C"	MODEL	Floor	Beari presc
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	QUOTE #	Quote #	DRAWN BY	Christine Shivy	speci retain
_	JOB #	J0821-5141	SALES REP.	Lenny Norris	Si

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater

**Christine Shivy** 

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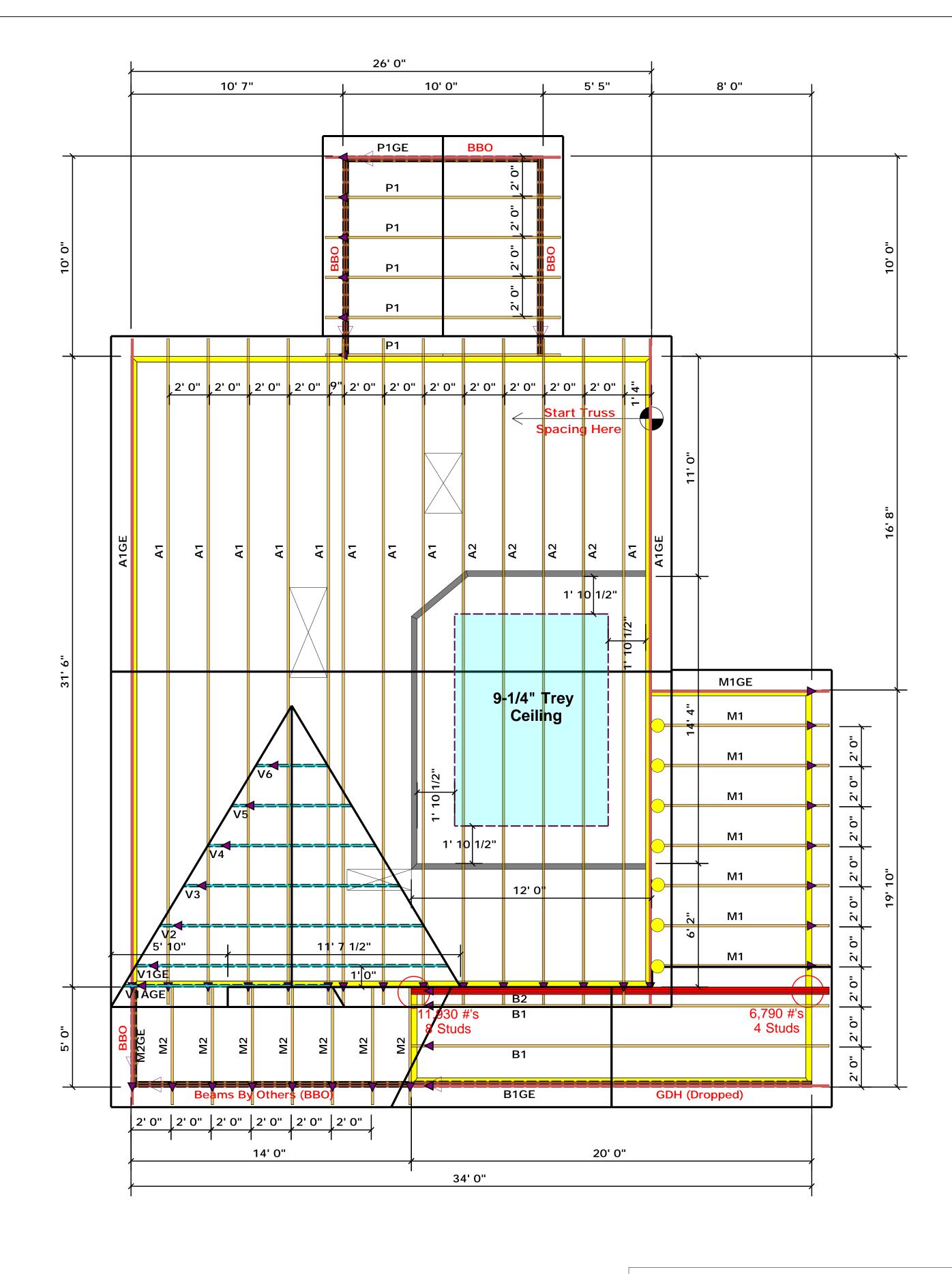
Christine Shivy

ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787

соттесн

Fax: (910) 864-4444



= JUS24 (Qty. 7)

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

# Truss Placement Plan SCALE: NTS

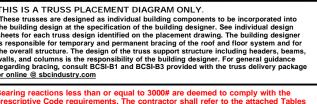
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-- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

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NI.A	Maje ()	e jac	PEADER!			with an extension	
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1700	1		2550	1		3400	1
3400	2		5100	2		6800	2
5100	3		7650	3		10200	3
6800	4		10200	4		13600	4
8500	5		12750	5		17000	5
10200	á		15300	6			
11900	7						
13600	8						
15300	9						

BUILDER	Weaver Development Co. Inc.	CITY / CO.	Spring Lake / Harnett	THIS These the buil
JOB NAME	Hayes Farm Lot 3	ADDRESS	Hayes Farm Lot 3	is respo the ove walls, a regardi
PLAN	Magnolia I I "C"	MODEL	Roof	or onling Bearing prescri
SEAL DATE	Seal Date	DATE REV.	/ /	( derive founda than 30 be reta
QUOTE #	Quote #	DRAWN BY	Christine Shivy	specific retaine
JOB#	J0821-5140	SALES REP.	Lenny Norris	Sig



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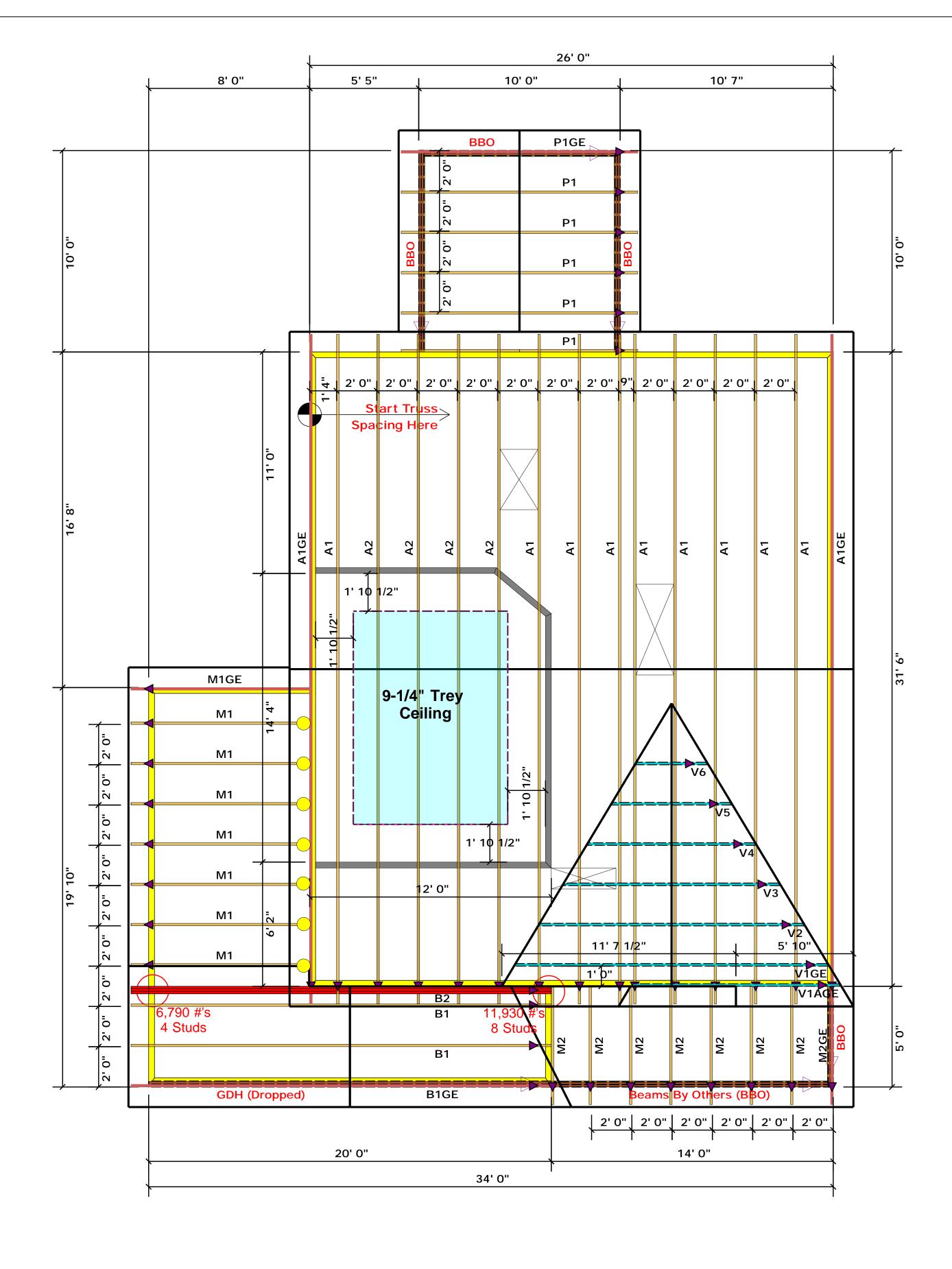
Christine Shivy

**Christine Shivy** 

ROOF & FLOOR TRUSSES & BEAMS Reilly Road Industrial Park

соттесн

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444



= JUS24 (Qty. 7)

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

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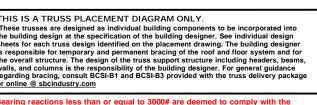
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LO	AD C	HART	FO	RЈ	ACK .	STUD	5
		ASEB ON I					
NLA	MMC & CO	HJAGKST FE	tues a Aderya			A EMB OF	
OND REACTION	60, pistudo Pok (r) RIV HEADEN		CIL 410	SQUESTUDS FOR CORNER CORN - EMBER		END NEACTION (UP TO)	REQUESTUDS FOR
1700	1	2	550	1	1	3400	1
3400	2	5	100	2		6800	2
5100	3	7	650	3		10200	3
0086	4	10	200	4		13600	4
8500	5	12	2750	5		17000	5
10200	6	15	5300	6			
11900	7						
13600	8						
15300	9						

BUILDER	Weaver Development Co. Inc.	CITY / CO.	Spring Lake / Harnett	THIS IS These to the build sheets for
JOB NAME	Hayes Farm Lot 3	ADDRESS	Hayes Farm Lot 3	is respo the over walls, ar regardin
PLAN	Magnolia I I "C"	MODEL	Roof	or online Bearing prescrip
SEAL DATE	Seal Date	DATE REV.	/ /	( derive foundat than 30 be retai
QUOTE #	Quote #	DRAWN BY	Christine Shivy	specifie retained
JOB#	J0821-5140	SALES REP.	Lenny Norris	Sign



Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Christine Shivy

**Christine Shivy** 

ROOF & FLOOR TRUSSES & BEAMS Reilly Road Industrial Park

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444



Client: Project: Address: Weaver Homes Magnolia-II Elev. C Magnolia-II Elev. C Date: 7/23/2021

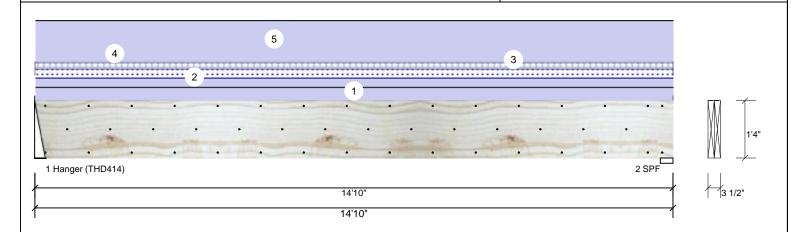
Input by: Christine Shivy Job Name: Magnolia-II Elev. C Page 1 of 1

Project #:

1.750" X 16.000" **Kerto-S LVL** BM<sub>1</sub>

2-Ply - PASSED

Level: Level



Member Inform	nation			Reactions UNPATTERNED Ib (Uplift)					
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	296	4522	577	0	0
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015	2	298	4548	580	0	0
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	360	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F			Bearing	S				
				Bearing	Length	Cap. Rea	ct D/L lb	Total Ld. Case	Ld. Comb.
				1 - Hanger	3.000"	57% 4	522 / 655	5177 L	D+0.75(L+S)
Analysis Results	S			2 - SPF	3.500"	100% 4	548 / 658	5206 L	D+0.75(L+S)

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	15931 ft-lb	7'4 3/4"	31109 ft-lb	0.512 (51%)	D	Uniform
Unbraced	18237 ft-lb	7'4 3/4"	18282 ft-lb	0.998 (100%)	D+0.75(L+S)	L
Shear	3723 lb	1'6 1/8"	10752 lb	0.346 (35%)	D	Uniform
LL Defl inch	0.041 (L/4231)	7'4 13/16"	0.361 (L/480)	0.110 (11%)	0.75(L+S)	L
TL Defl inch	0.324 (L/535)	7'4 13/16"	0.481 (L/360)	0.670 (67%)	D+0.75(L+S)	L

#### **Design Notes**

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 6'4 7/8" o.c.
- 7 Bottom braced at bearings.

8 Lateral	slenderness ratio based on	single ply width.								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Wall
2	Uniform			Near Face	78 PLF	0 PLF	78 PLF	0 PLF	0 PLF	M1
3	Uniform			Far Face	15 PLF	40 PLF	0 PLF	0 PLF	0 PLF	Floor Load
4	Uniform			Тор	130 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Load
5	Uniform			Тор	251 PLF	0 PLF	0 PLF	0 PLF	0 PLF	A1GE
	Self Weight				12 PLF					

#### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Indicating & Installation

  I. VIL beams must not be cut or drilled

  Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  1. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 1/8/2023



Client: Project: Address:

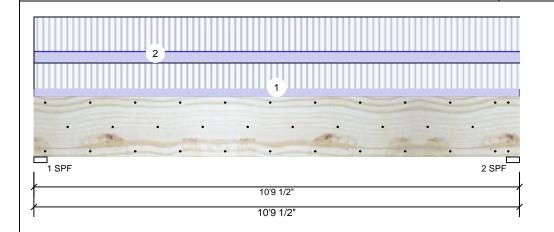
Weaver Homes Magnolia-II Elev. C Magnolia-II Elev. C Date: 7/23/2021 Input by: Christine Shivy

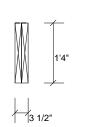
Job Name: Magnolia-II Elev. C

Project #:

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED BM<sub>2</sub>

Level: Level





D+I

Page 1 of 1

#### Member Information Reactions UNPATTERNED Ib (Uplift) Brg Туре: Girder Application: Floor Snow Wind Const Live Dead Plies: 2 Design Method: ASD 3389 1200 0 0 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 3389 1200 0 0 0 2 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. D+L 1 - SPF 3.500" 1200 / 3389 4589 L

2 - SPF 3.500"

88%

1200 / 3389

4589 I

#### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	11397 ft-lb	5'4 3/4"	34565 ft-lb	0.330 (33%)	D+L	L
Unbraced	11397 ft-lb	5'4 3/4"	11746 ft-lb	0.970 (97%)	D+L	L
Shear	4386 lb	1'6 5/8"	11947 lb	0.367 (37%)	D+L	L
LL Defl inch	0.085 (L/1457)	5'4 3/4"	0.259 (L/480)	0.330 (33%)	L	L
TL Defl inch	0.115 (L/1076)	5'4 3/4"	0.345 (L/360)	0.330 (33%)	D+L	L

#### **Design Notes**

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6. Lateral slenderness ratio based on single ply width

0 Laterar	Sicrideffiess fallo based off									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	89 PLF	267 PLF	0 PLF	0 PLF	0 PLF	F4
2	Uniform			Near Face	121 PLF	361 PLF	0 PLF	0 PLF	0 PLF	F2
	Self Weight				12 PLF					

#### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
  approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info



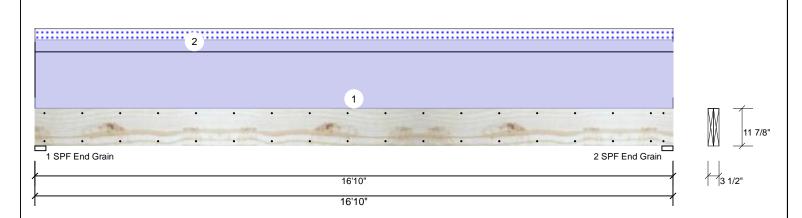
Client: Project: Address: Weaver Homes Magnolia-II Elev. C Magnolia-II Elev. C Date: 7/23/2021

Input by: Christine Shivy Job Name: Magnolia-II Elev. C Page 1 of 1

Project #:

**Kerto-S LVL** 2-Ply - PASSED 1.750" X 11.875" **GDH** 

Level: Level



Member Int	formation						Reactions UNPATTERNED lb (Uplift)							
Туре:	Girder		Applicat	ion: F	loor		Brg	Live	Dead	Snow	Wind	Const		
Plies:	2		Design I	Method: A	ASD		1	0	2098	337	0	0		
Moisture Cond	dition: Dry		Building	Code: I	BC/IRC 2015		2	0	2098	337	0	0		
Deflection LL:	480		Load Sh	naring: 1	No									
Deflection TL:	360		Deck:	1	Not Checked									
Importance:	Normal													
Temperature:	Temp <= 1	00°F												
							Bearings	S						
							Bearing	Length	Cap. F	React D/L lb	Total Ld. Case	Ld. Comb.		
							1 - SPF	3.500"	23%	2098 / 337	2434 L	D+S		
							End							
Analysis Re	sults						Grain							
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - SPF	3.500"	23%	2098 / 337	2434 L	D+S		
Moment	8354 ft-lb	8'5"	17919 ft-lb	0.466 (47%	6) D	Uniform	End Grain							
Unbraced	9694 ft-lb	8'5"	9704 ft-lb	0.999 (100%)	D+S	L								

Uniform

1

### TL Defl inch **Design Notes**

Shear

1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".

8'5 1/16" 0.409 (L/480) 0.170 (17%) S

8'5 1/16" 0.546 (L/360) 0.930 (93%) D+S

1'2 5/8" 7980 lb

0.225 (22%) D

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 9'6 3/4" o.c.
- 6 Bottom braced at bearings.

1794 lb

0.506 (L/388)

LL Defl inch 0.070 (L/2809)

7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Loads
2	Uniform			Тор	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	2'-0" Gable End
	Self Weight				9 PLF					

#### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- Indiang & Installation

  LVL beams must not be cut or drilled

  Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  Damaged Beams must not be used

  Design assumes top edge is laterally restrained

  Provide lateral support at bearing points to avoid lateral displacement and rotation

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This design is valid until 1/8/2023

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