Rosado Residence- 6085 Cool Springs Road Broadway NC

Block Parged Crawlspace Foundatin

PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE



ROOF VENTILATION

2. Endoed attifyints space over unandhoned space may be verse, we continue safely even only.

SQUARE FOOTMED OF ROOF TO BE VENTED = 8,668 SQ.FT.

METHOD TS9% TO 85% OF VENTED ST PAR PARE SAFE SQ.FT.

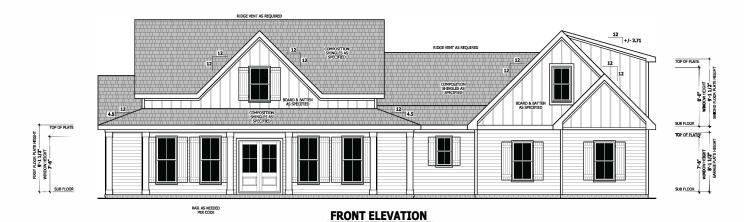
METHOD TS9% TO 85% OF VENTED ST PAR ROOFE SAFE = 24.65 SQ.FT.

WITH SW TO 105 NO VENTED ST PAR ROOFE SAFE SAFE SQ.FT.

WARD RETARDER ON WARM-BEHAVIOR ES DEC OF CELLING = 12.25 SQ.FT.

GUARD RAIL NOTES

AIR LEAKAGE



Front Door- 1/0 3/0 Fiberglass Painted Windows- MGM SH Vinyl- Clear- White Colunms- 8" PVC- White Shingles- 30 Year Arch- Charcoal Garage Door- 18'x 8' Recessed Panel- White Shake Siding- Alside Harbor Blue Horizontal Siding- Alside Platinum Gray Trim- White

SCALE 1/4" = 1'-0"



SQUARE FOOTAGE
HEATED
FIRST FLOOR 2258 SQ.FT.
TOTAL 2258 SQ.FT.
HEATED FUTURE PLAYROOM TOTAL UNHEATED FRONT PORCH REAR PORCH GARAGE TOTAL



REAR ELEVATION

FRONT & REAR ELEVATIONS 6 7

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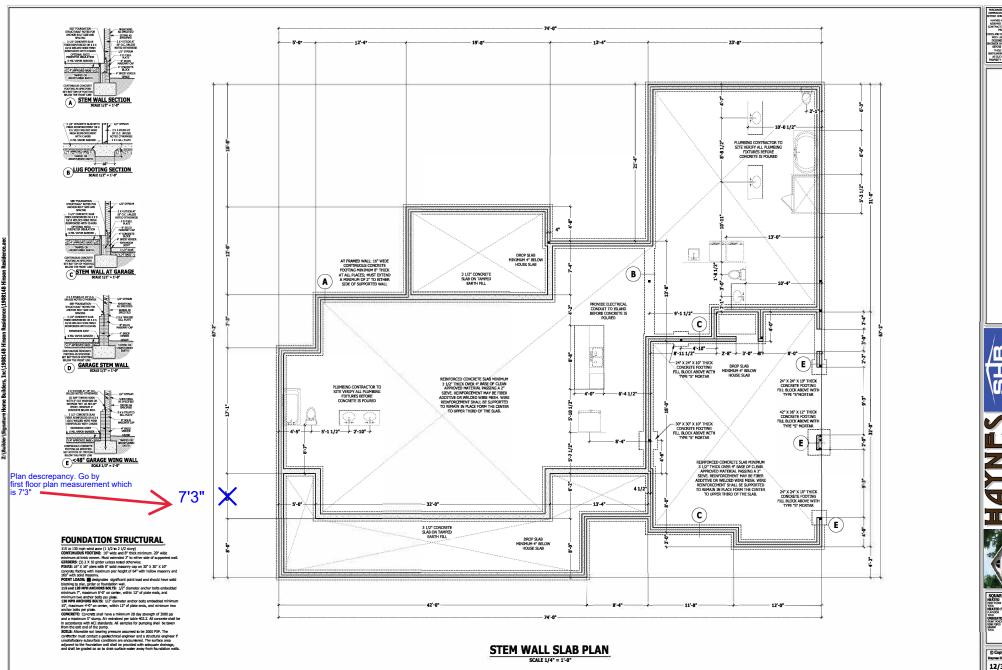








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PAGE 2 OF 8



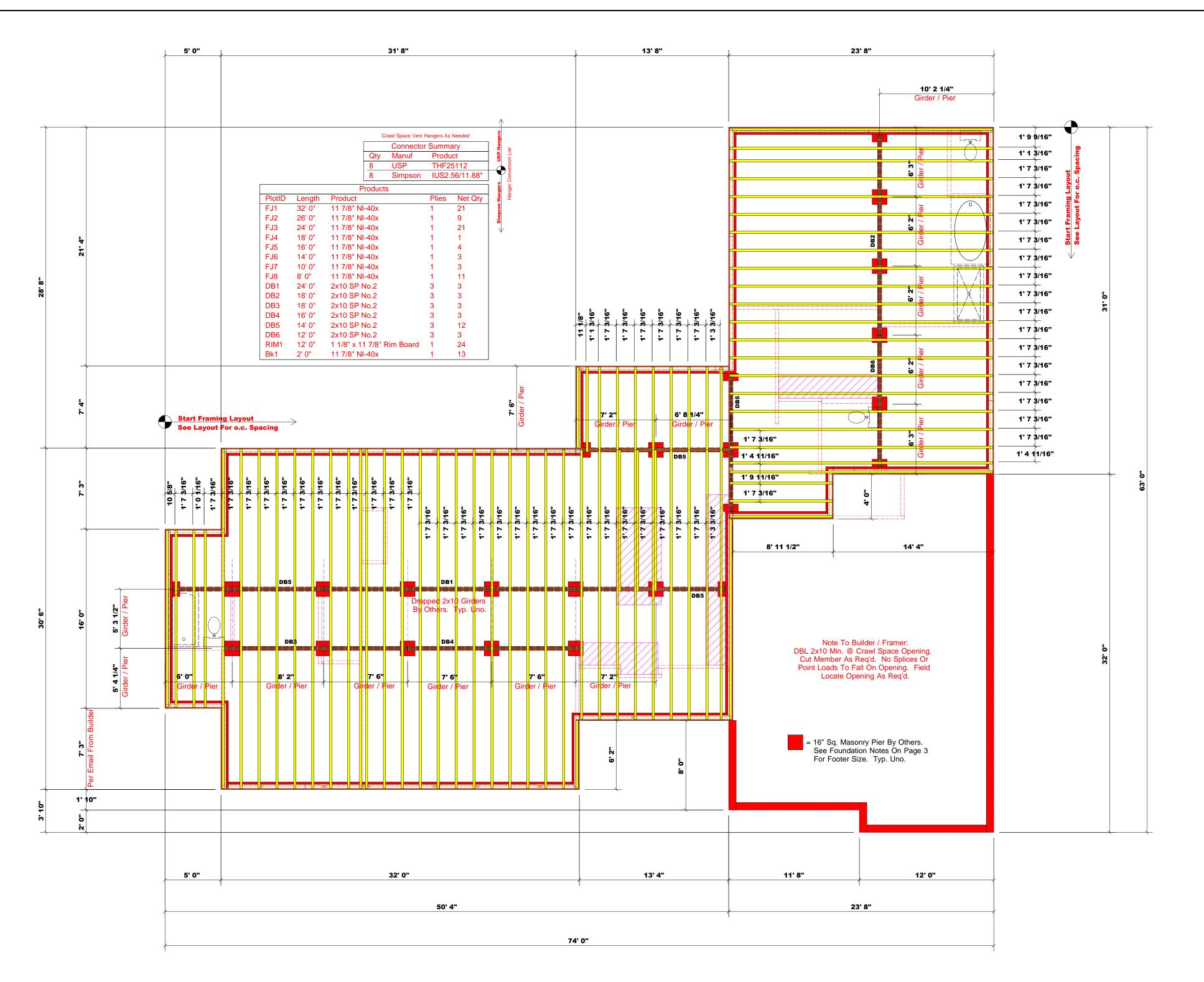
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Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the

Fax: (910) 864-4444

Joe Ciferni

LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER END REACTION (UP TO) REQ'D STUDS FOR (3) PLY HEADER END REACTION
(UP TO)
REQ'D STUDS FOF 3400 1 1700 1 2550 1 3400 2 6800 2 5100 2 5100 3 7650 3 10200 3 6800 4 10200 4 13600 4 8500 5 17000 5 12750 5 10200 6 15300 6 11900 7 13600 8 15300 9

Broadway, Anthony Williams Harnett County Joe Ciferni 3/6/24 6085 1st SALESMAN DRAWN BY DATE REV. ADDRESS COUNTY

Signature Home Builders Stewart J0324-1346 6085 Crawl Ž Z JOB NAME SEAL DATE QUOTE# BUILDER PLAN JOB THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

russ delivery package or online @ sbcindustry.co

SCALE 1/4" = 1'-0"

FIRSTFLOOR PLAN







12/30/2019 190814B PAGE 4 OF 8

FINDWESS NOT VISITY ALL
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8

FIRST FLOOR STRUCTURAL







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DESIGN LOADS	LIVE LOAD	DEAD L'OAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconles and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	-	-
Guardrall in-fill components	50	-	-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	-	L/360
Snow	20		-

Solve

ROOF TRUSS REQUIREMENTS

ROOF TRUSS REQUIREMENTS

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EXTERIOR HEADERS

-(2) 2 X S WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE - KING STUDS EACH END PER TABLE BELOW PEDDR SWIN 23 3 3 4 8 8-12 (12-16) KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTEODTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED

WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4° or as noted 2 X 6 are drawn as 6° to include 1/2° sheathing or gypsum. Subtract 1/2° for stuf face.

Interior walls are drawn as 3 1/2° or as noted 2 X 6 are drawn as 5 1/2°, and do not include gypsum.

ATTIC ACCESS

ATTIC ACCESS

SECTION 8807

1807.1 Aftic access. In sitic carcas gening shall beprovide

1807.1 Aftic access. In sitic carcas gening shall beprovide

1807.1 Aftic access. In sitic carcas gening shall be provided

access gening shall not be less throub incide by 30 inches (point

by 70 mm) and all to carcad in a history or other

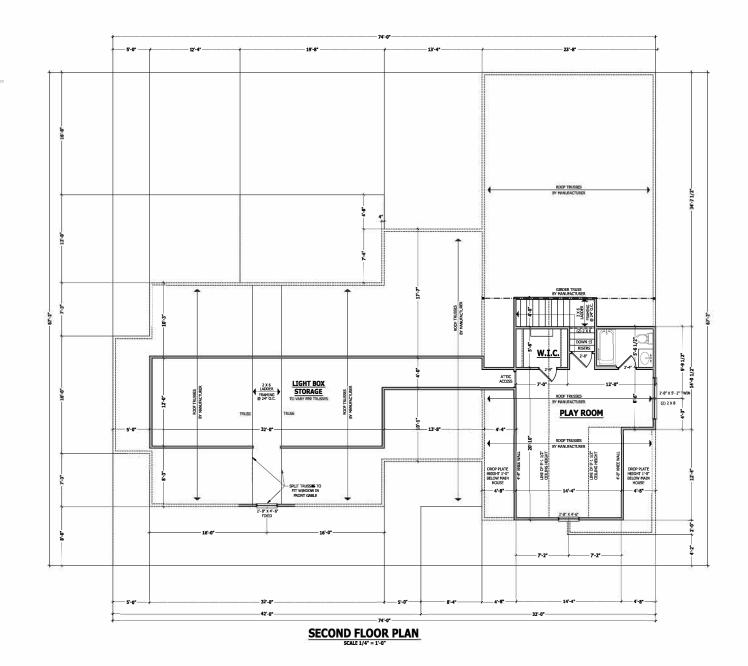
1807.1 Aftic access gening, site school incide by 30 inches (point

1807.1 Aftic access gening, site school in 1801.1 Aftic access shall be provided as

1807.1 Aftic access gening, site school in 1801.1 Aftic access shall be provided as

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1807.1 Aftic access gening site school in 1801.1 Aftic access gening site school in 1801.1



HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTIONS PRACTICES AN PROCEDURES.

SECOND FLOOR PLAN

SQUARE FOOTAGE
HEATED
HIST TIOOR 299 SU.T.
TOTAL 299 SU.T.
TOTAL 299 SU.T.
TOTAL 499 SU.T.
TOT

© Copyright 2019

12/30/2019 190814B PAGE 6 OF 8 **ROOF TRUSS REQUIREMENTS**



ROOF TRUSSES BY MANUFACTURER

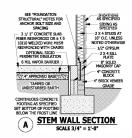


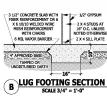




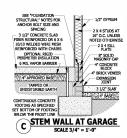


12/30/2019 190814B PAGE 7 OF 8



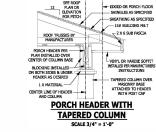


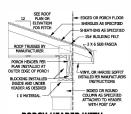
1/2" GYPSUM

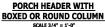


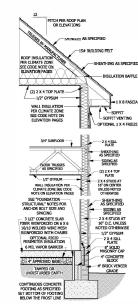
1/2* GYPSUM

SHEATHING AS SPECIFIED









HAYNES HOME FLAMS, INC. ACREMES NO LIABILITY FO CONTRACTORS PRACTICES A

PICAL DETAILS

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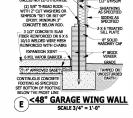
2250 SQ.FT. 2250 SQ.FT. HEATED FUT RESTROOM TOTAL HEATED FUT RESTROOM TOTAL UNHEATED RESET ASSOL RESERVICES SAACE 419 SQ.FT. 419 SQ.FT. 63 SD FT. 2015 ST FT. 600 SD FT. 100 SD FT.

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TYPICAL WALL DETAIL SCALE 3/4" = 1'-0"



GARAGE STEM WALL (**D**) SCALE 3/4" = 1'-0"



2 X 4 STUDS AT 16" O.C. UNLESS NOTED OTHERWISE

CARBON MONOXIDE ALARMS

SMOKE ALARMS

STAIRWAY NOTES

R311.7
R311.7.2 Readroom. The m inimumheadroom in all parts of the stainway shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the stand line adjoining the bread nosing or from the floor surface of the

fisces.

RS11.J.7 Mandralls. Handralls shall be provided on at least one side of each continuous run of tweets or flight with four ormore risers.

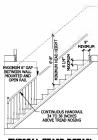
RS11.J.7.1 Height Handrall Height, measured vertically from the sloped plane adjoining the treed nosing, or finish surface of emp slope, shall be not less than 34 riches (864 mm) and not more than 38 inches (965 mm).

I. The use of a value, Lumak or starting saving shall be allowed now the towns from the control from the con

xceptions:
. Handrails shall be permitted to be interrupted by a newel post.
. The use of a volute, turnout, starting easing or starting newel shall billnowed over the lowest troad.

allowed over the lowest tread.

3. Two or more separate rais shall be considered continuous if the termination of the rail is occurs within 6 inches (152 mm) of each other. transitioning between a well-mounted handrail and a guardrail/hendrail, well-mounted rail must return into the wail.







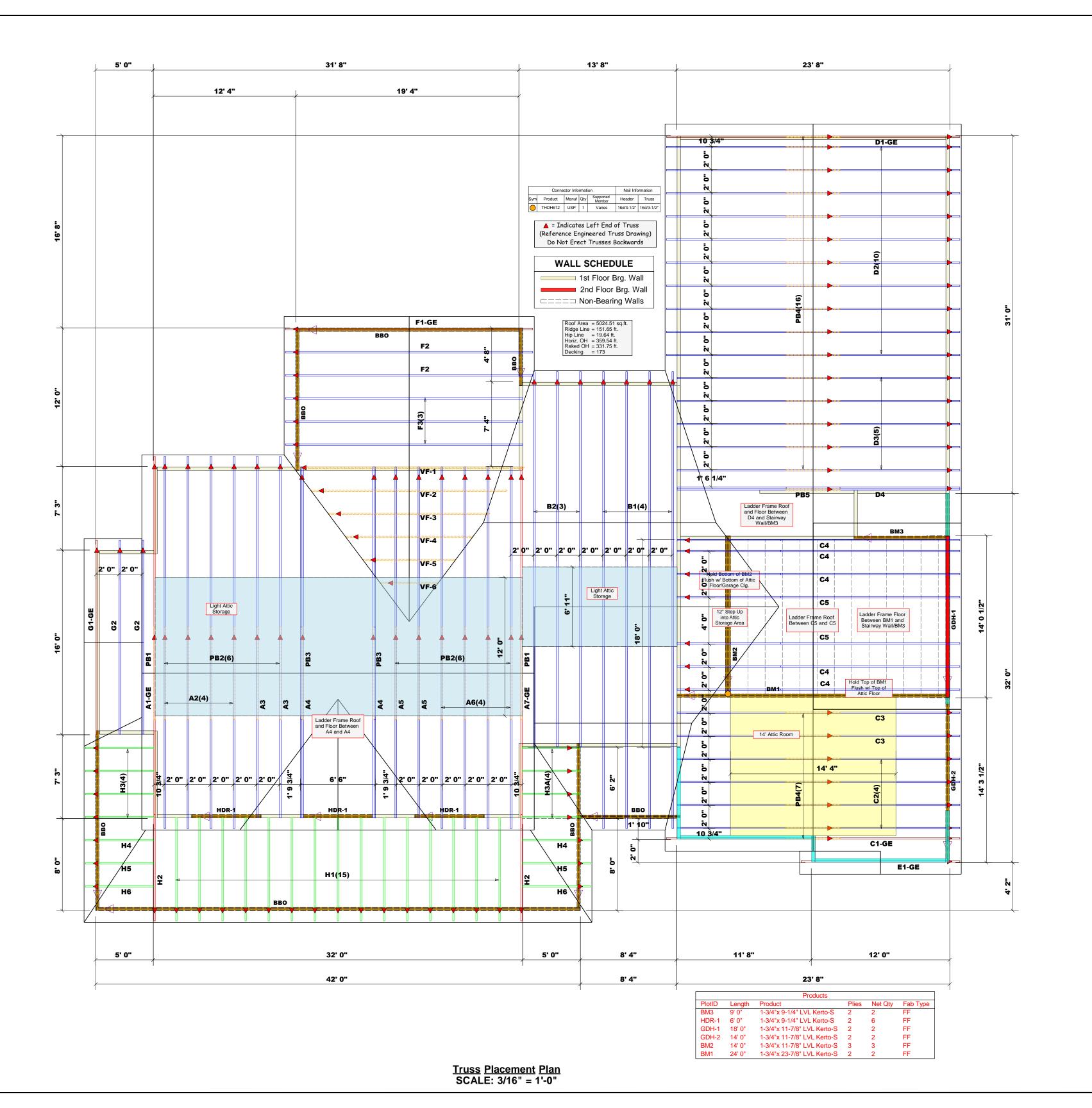


WEEP SCREEDS

All weep screeds and stone veneer to be installed per manufactures instructions and per the 2012 North Carolina Residential

per the 2012 North Carollia Residential Building code.

R703.6.2.1 - A minimum 0.019-inch (0.5 mm) No. 26 galvenized sheet gage), corrosion-resistant weep screed or plastic weep screed, with a minimum vertical mm), shall be provided at or belowthe for foundation plast line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be clased a minimum of 4





Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Anthony Williams

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

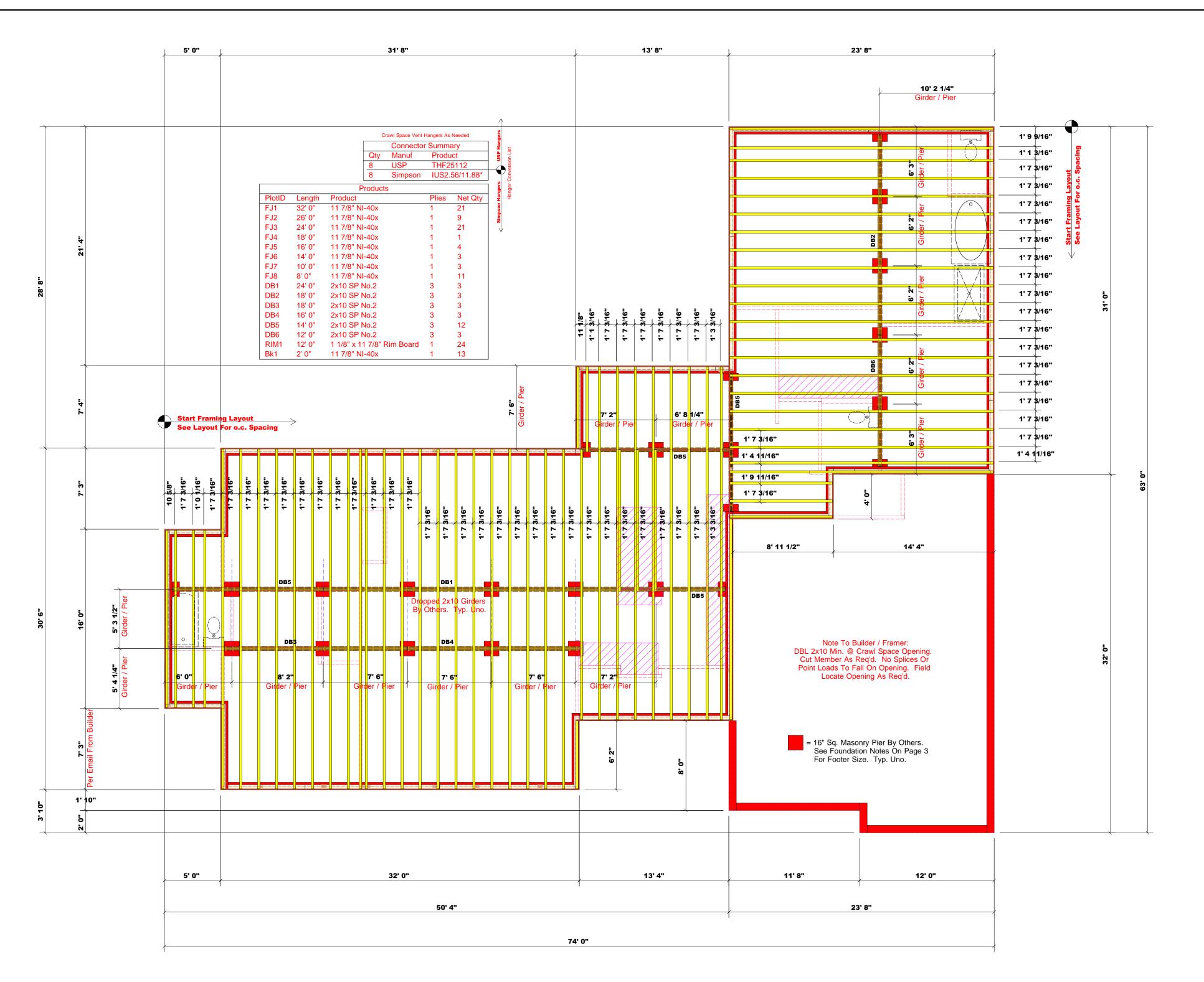
NUMBER OF JACK STUDS REQUIRED @ EA END
HEADER/GIRDER

		1	HEADER/	SIRDER	5		
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)	PEO'N STUDS FOR
1700	1		2550	1		3400	
3400	2		5100	2		6800	
5100	3		7650	3		10200	
6800	4		10200	4		13600	
8500	5		12750	5		17000	
10200	6		15300	6			
11900	7						
13600	8						
15300	9						

COUNTY	Harnett County
ADDRESS	6085 Cool Springs Rd / Broadway, NC
MODEL	Roof
DATE REV.	3/21/24
DRAWN BY	Anthony Williams
SALESMAN	SALESMAN Anthony Williams

BUILDER	Signature Home Builders
JOB NAME	6085 Cool Springs Rd
PLAN	Custom
SEAL DATE NA	¥Z
QUOTE #	Y Z
JOB #	J0324-1345

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com





TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the

Joe Ciferni

LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b)) NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER END REACTION (UP TO) REQ'D STUDS FOR (3) PLY HEADER END REACTION
(UP TO)
REQ'D STUDS FOF

3400 1 1700 1 2550 1 3400 2 6800 2 5100 2 5100 3 7650 3 10200 3 6800 4 10200 4 13600 4 8500 5 17000 5 12750 5 10200 6 15300 6 11900 7 13600 8 15300 9

æ Rd Anthony Williams Harnett County Joe Ciferni 3/6/24 6085 1st SALESMAN DRAWN BY DATE REV. ADDRESS COUNTY

Signature Home Builders Rd J0324-1346 6085 Crawl Ž Z JOB NAME SEAL DATE QUOTE# BUILDER PLAN JOB

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

russ delivery package or online @ sbcindustry.co



Client: Project: Address:

Signature Home Builders

Date: 3/27/2024

Input by: Anthony Williams Job Name: 6085 Cool Springs Rd Page 1 of 10

Const

Ld. Comb.

D+L

D+0.75(L+S)

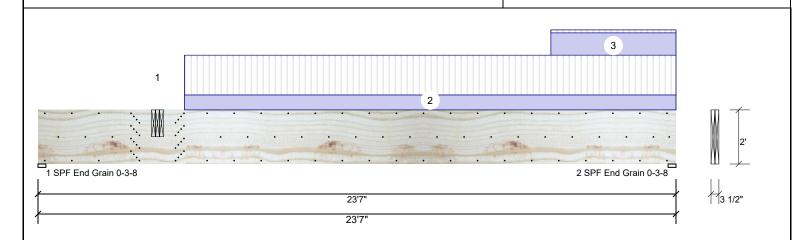
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Project #: J0324-1345

1.750" X 24.000" **Kerto-S LVL** 2-Ply - PASSED BM₁

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Wind Type: Floor Brg Direction Live Dead Snow Plies: Design Method: ASD 2094 3134 1991 0 Vertical Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 Vertical 3132 2452 441 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F **Bearings**

Bearing Length

1 - SPF 3.500"

2 - SPF 3.500"

End Grain

End Grain Dir.

Vert

Vert

Cap. React D/L lb

54%

3134 / 3064

2452 / 3132

Total Ld. Case

6198 L

5584 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	30390 ft-lb	11'2 1/16"	73185 ft-lb	0.415 (42%)	D+L	L
Unbraced	30390 ft-lb	11'2 1/16"	30490 ft-lb	0.997 (100%)	D+L	L
Shear	6192 lb	2'3 1/2"	20608 lb	0.300 (30%)	D+0.75(L+S)	L
LL Defl inch	0.224 (L/1240)	11'5 5/16"	0.579 (L/480)	0.387 (39%)	0.75(L+S)	L
TL Defl inch	0.415 (L/669)	11'3 13/16"	0.772 (L/360)	0.538 (54%)	D+0.75(L+S)	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be laterally braced at a maximum of 5'10" o.c.
- 8 Bottom must be laterally braced at end bearings.

9 Lateral slenderness ratio based on single ply width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Point	4-5-0		Far Face	2630 lb	273 lb	2433 lb	0 lb	0 lb	BM2 Brg 2	
2	Tie-In	5-5-0 to 23-7-0	6-8-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF		
3	Part. Uniform	18-11-8 to 23-7-0		Тор	150 PLF	20 PLF	0 PLF	0 PLF	0 PLF	ROOF & WALL	
	Self Weight				19 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
 - Damaged Beams must not be used

Handling & Installation

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer info	
Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	



Client: Project: Address: Signature Home Builders

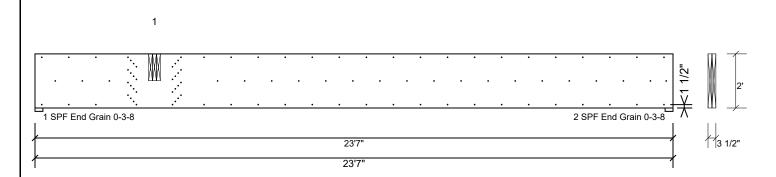
3/27/2024 Input by:

Anthony Williams Job Name: 6085 Cool Springs Rd Page 2 of 10

Project #: J0324-1345

1.750" X 24.000" 2-Ply - PASSED **Kerto-S LVL** BM₁

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6".

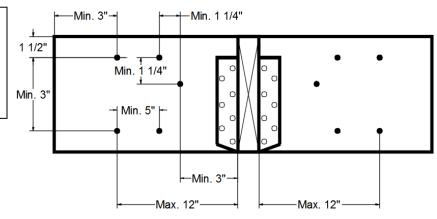
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 4-5-0 with a minimum of (32) - 10d Box nails (.128x3") in the pattern shown.

parte		
Capacity	84.1 %	
Load	2531.3lb.	
Total Yield Limit	3010.4 lb.	
Cg	0.9994	
Cg Cm	1	
Yield Limit per Fastener	94.1 lb.	
Yield Mode	IV	
Load Combination	D+S	
Duration Factor	1.15	

Min/Max fastener distances for Concentrated Side Loads



Notes NOtes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us



Client: Signature Home Builders

Project: Address: Date: 3/27/2024

Input by: Anthony Williams Job Name: 6085 Cool Springs Rd Page 3 of 10

Project #: J0324-1345 evel: Level

1.750" X 11.875" 3-Ply - PASSED Kerto-S LVL BM₂

Application:

Design Method:

Building Code:

Load Sharing:

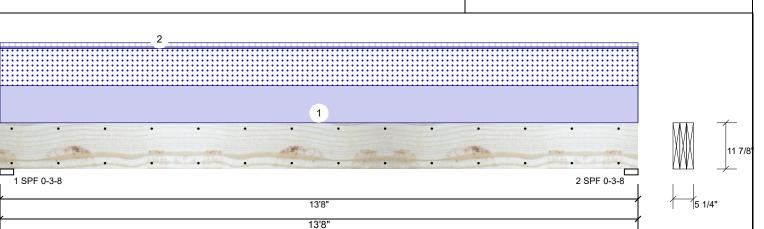
Deck:

ASD

Yes

IBC/IRC 2015

Not Checked



Member Information Type: Plies: 3

Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360

Importance: Normal - II Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	273	2630	2433	0	0
2	Vertical	273	2630	2433	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	65%	2630 / 2433	5063	L	D+S
2 - SPF	3.500"	Vert	65%	2630 / 2433	5063	L	D+S

Analysis Results

, ,						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16156 ft-lb	6'10"	35719 ft-lb	0.452 (45%)	D+S	L
Unbraced	16156 ft-lb	6'10"	16165 ft-lb	0.999 (100%)	D+S	L
Shear	4127 lb	12'4 5/8"	15295 lb	0.270 (27%)	D+S	L
LL Defl inch	0.181 (L/877)	6'10"	0.330 (L/480)	0.547 (55%)	S	L
TL Defl inch	0.376 (L/421)	6'10"	0.440 (L/360)	0.854 (85%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". Nail from both sides.
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'4 5/8" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

			F-)									
ID)	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1		Uniform			Тор	356 PLF	0 PLF	356 PLF	0 PLF	0 PLF	C4	
2		Uniform			Тор	15 PLF	40 PLF	0 PLF	0 PLF	0 PLF	FLOOR	
		Self Weight				14 PI F						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

- - Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- This design is valid until 6/28/2026



Client: Signature Home Builders

Project: Address:

3/27/2024 Input by:

Anthony Williams Job Name: 6085 Cool Springs Rd Page 4 of 10

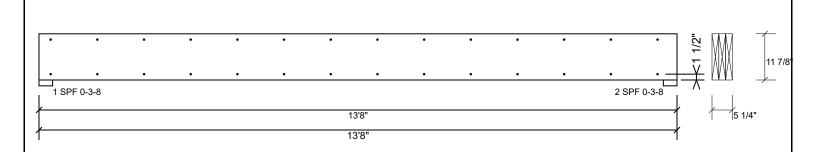
Project #: J0324-1345

Kerto-S LVL BM₂

1.750" X 11.875"

3-Ply - PASSED

evel: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1 00

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Infoculing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Manufacturer Info



Client:

Project: Address: Signature Home Builders

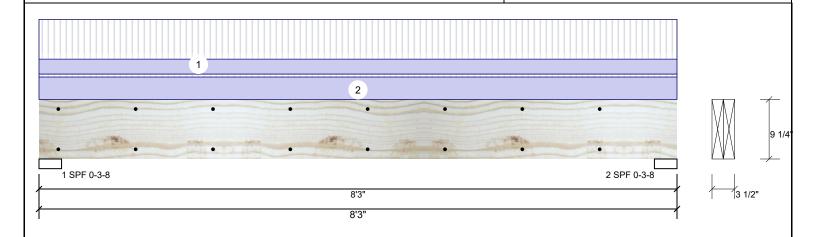
Date: 3/27/2024

Input by: Anthony Williams Job Name: 6085 Cool Springs Rd Page 5 of 10

Project #: J0324-1345

1.750" X 9.250" Kerto-S LVL 2-Ply - PASSED BM₃

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Snow Wind Type: Floor Brg Direction Live Dead Const Plies: Design Method: ASD 1183 1061 0 Vertical n 0 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 Vertical 1183 1061 0 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+L 1 - SPF 3.500" Vert 43% 1061 / 1183 2243 L

2 - SPF 3.500"

Vert

43%

1061 / 1183

Analysis Results

ĺ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	4127 ft-lb	4'1 1/2"	12542 ft-lb	0.329 (33%)	D+L	L
	Unbraced	4127 ft-lb	4'1 1/2"	8701 ft-lb	0.474 (47%)	D+L	L
	Shear	1671 lb	7'2 1/4"	6907 lb	0.242 (24%)	D+L	L
	LL Defl inch	0.059 (L/1579)	4'1 9/16"	0.195 (L/480)	0.304 (30%)	L	L
	TL Defl inch	0.112 (L/832)	4'1 9/16"	0.260 (L/360)	0.433 (43%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Tie-In	0-0-0 to 8-3-0	6-8-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	FLOOR	
2	Uniform			Тор	150 PLF	20 PLF	0 PLF	0 PLF	0 PLF	ROOF & WALL	
	Self Weight				7 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Client: Signature Home Builders

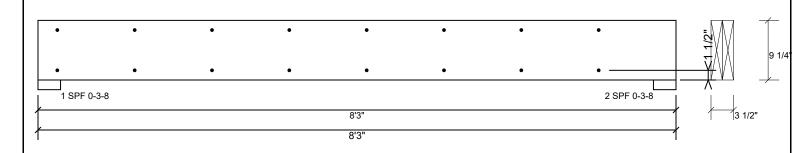
Project: Address: Date: 3/27/2024

Input by: Anthony Williams Job Name: 6085 Cool Springs Rd Page 6 of 10

Project #: J0324-1345

Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED **BM3**

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

·	-	•
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1 00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Manufacturer Info

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Client:

Project: Address: Signature Home Builders

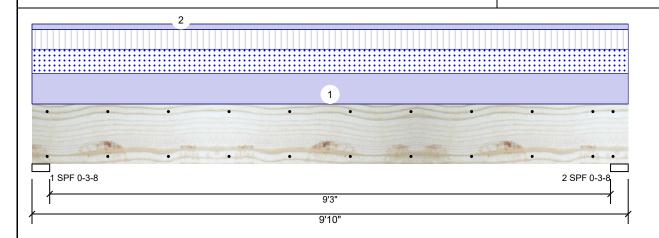
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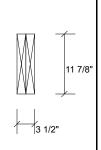
Input by: Anthony Williams Job Name: 6085 Cool Springs Rd

Project #: J0324-1345

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL GDH**

Level: Level





Page 7 of 10

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Live Wind Brg Direction Dead Snow Const 1092 2027 1323 0 Vertical 0 2 Vertical 1092 2027 1323 0 0

Bearings

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+0.75(L+S) 1 - SPF 3.500" Vert 2027 / 1811 3837 L 3.500" D+0.75(L+S) 2 - SPF Vert 74% 2027 / 1811 3837 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8575 ft-lb	4'11"	22897 ft-lb	0.374 (37%)	D+0.75(L+S)	L
Unbraced	8575 ft-lb	4'11"	9857 ft-lb	0.870 (87%)	D+0.75(L+S)	L
Shear	2847 lb	1'3 3/8"	10197 lb	0.279 (28%)	D+0.75(L+S)	L
LL Defl inch	0.077 (L/1466)	4'11"	0.234 (L/480)	0.327 (33%)	0.75(L+S)	L
TL Defl inch	0.163 (L/692)	4'11"	0.312 (L/360)	0.520 (52%)	D+0.75(L+S)	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

•		F-7									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	343 PLF	222 PLF	269 PLF	0 PLF	0 PLF	C3 R+F	
2	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
	Self Weight				9 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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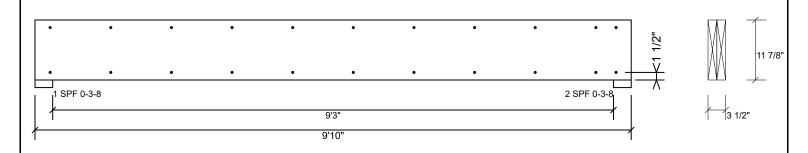
Project: Address: Date: 3/27/2024 Input by:

Anthony Williams Job Name: 6085 Cool Springs Rd Page 8 of 10

Project #: J0324-1345

1.750" X 11.875" 2-Ply - PASSED **GDH Kerto-S LVL**

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Project: Address: Signature Home Builders

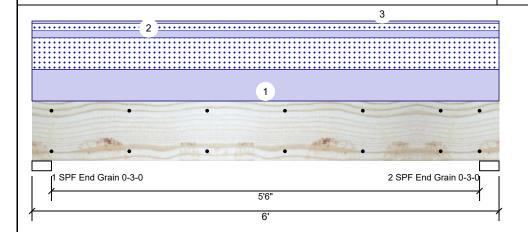
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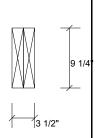
Input by: Anthony Williams Job Name: 6085 Cool Springs Rd

Project #: J0324-1345

1.750" X 9.250" **Kerto-S LVL** 2-Ply - PASSED HDR-1

Level: Level





Page 9 of 10

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	: Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1381	1269	0	0
2	Vertical	0	1381	1269	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3493 ft-lb	3'	14423 ft-lb	0.242 (24%)	D+S	L
Unbraced	3493 ft-lb	3'	10944 ft-lb	0.319 (32%)	D+S	L
Shear	1754 lb	1' 1/4"	7943 lb	0.221 (22%)	D+S	L
LL Defl inch	0.027 (L/2538)	3'	0.141 (L/480)	0.189 (19%)	S	L
TL Defl inch	0.056 (L/1216)	3'	0.188 (L/360)	0.296 (30%)	D+S	L

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	Vert	30%	1381 / 1269	2650	L	D+S
2 - SPF End Grain	3.000"	Vert	30%	1381 / 1269	2650	L	D+S

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

o Lateral cicinatinos ratio bassa on single pry matin											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	346 PLF	0 PLF	346 PLF	0 PLF	0 PLF	"A" TRUSSES	
2	Uniform			Тор	77 PLF	0 PLF	77 PLF	0 PLF	0 PLF	"H" TRUSSES	
3	Uniform			Тор	30 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
	Self Weight				7 PLF						

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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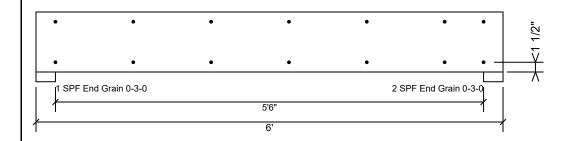
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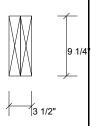
Input by: Anthony Williams Job Name: 6085 Cool Springs Rd

Project #: J0324-1345

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** HDR-1

Level: Level





Page 10 of 10

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1 00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Metsä Wood	

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