#### DESIGN LOADS LIVE LOAD DEAD LOAD TABLE R301.4 DWELLING UNITS (PSF) (PSF) SLEEPING ROOMS ATTICS WITH STORAGE ATTICS WITHOUT STORAGE ROOF SNOW DECKS EXTERIOR BALCONIES PASSENGER VEHICLE GARAGES FIRE ESCAPES GUARDRAILS AND HANDRAILS 200

#### MATERIALS

1. FRAMING LUMBER SHALL BE #2 SPRUCE PINE FIR (SPF) WITH THE FOLLOWING DESIGN PROPERTIES: Fb = 875 PSI Fv = 70 PSI E = 1.4E6 PSI

2. FRAMING LUMBER EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE #2 SOUTHERN YELLOW PINE (SYP) TREATED IN ACCORDANCE WITH AWPA C22 WITH THE FOLLOWING DESIGN PROPERTIES: Fb = 1050 PSI Fv = 95 PSI E = 1.6E6 PSI

3. ENGINEERED WOOD BEAMS SHALL BE LAMINATED VENEER LUMBER (LVL) OR PARALLEL STRAND LUMBER (PSL) WITH THE FOLLOWING MINIMUM DESIGN PROPERTIES: Fb = 2900 PSI Fv = 285 PSI E = 1.9E6 PSI

4. STRUCTURAL STEEL SHALL CONFORM TO ASTM A-36 MINIMUM GRADE.

5. BOLTS SHALL CONFORM TO A307 MINIMUM GRADE.

6. REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60.

7. POURED CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. MATERIALS USED TO PRODUCE CONCRETE SHALL COMPLY WITH THE APPLICABLE STANDARDS LISTED IN ACI 318 OR ASTM C 1157. 8. CONCRETE LOCATED PER <u>TABLE R402.2</u> SHALL BE AIR ENTRAINED WITH THE TOTAL AIR CONTENT NOT LESS THAN 5 PERCENT OR MORE THAN 7 PERCNET.

9. MASONRY UNITS SHALL CONFORM TO ACI 530/ASCE 5/TMS 402 AND MORTAR SHALL COMPLY WITH ASTM C 270. 10. ALLOWABLE SOIL BEARING PRESSURE 2000 PSF.

#### GENERAL

ENGINEER'S SEAL APPLIES TO STRUCTURAL COMPONENTS ONLY AND DOES NOT CERTIFY ARCHITECTURAL LAYOUT OR DIMENSIONAL ACCURACY. ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION METHODS OR ANY DEVIATION FROM THE PLANS.

ALL CONSTRUCTION, WORKMANSHIP, MATERIAL QUALITY AND SELECTION SHALL BE IN ACCORDANCE WITH THE <u>NORTH</u> <u>CAROLINA STATE BUILDING CODE - RESIDENTIAL CODE 2018 EDITION</u> FROM THE INTERNATIONAL RESIDENTIAL CODE 2018 (IRC), AND LOCAL CODES AND REGULATIONS. DIMENSIONS SHALL GOVERN OVER SCALE AND CODE SHALL GOVERN OVER DIMENSIONS.

#### ADDITIONAL LOADS

FIGURE R301.2(4) - BASIC DESIGN WIND SPEED 100 MPH

FIGURE R301.2(2) - SEISMIC DESIGN CATEGORY B

TABLE R301.2(4) - DESIGN POSITIVE AND NEGATIVE PRESSURE FOR DOORS AND WINDOW FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF

TABLE R301.2(2) - COMPONENT AND CLADDING LOADS FOR A MEAN ROOF HEIGHT OF 30 FEET OR LESS LOCATED IN EXPOSURE B ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE DESIGNED BASED ON ROOF PITCHES AS FOLLOWS: 45.4 PSF FOR 0:12 TO 2.25:12, 34.8 PSF FOR 2.25:12 TO 7:12 AND 21 PSF FOR 7:12 TO 12:12 WALL CLADDING IS DESIGNED FOR A 24.1 PSF POSITIVE AND NEGATIVE PRESSURE

#### ENERGY COMPLIANCE:

TABLE N1102.1 - REFER TO TABLE N1101.1 TO DETERMINE THE CLIMATE ZONE BY COUNTY AND REFER TO TABLE N1102.1 FOR R VALUE INSULATION REQUIREMENTS LISTED BY ZONE.

TABLE N1102.1 - ZONE 7 - MAX. GLAZING U FACTOR: 0.40. MIN. INSULATION R VALUES: CEILING R-30, WALLS R-13, FLOORS R-19, BASEMENT WALLS R-7, SLAB PERIMETER R-0, CRAWL SPACE WALLS R-7.

TABLE N1102.1 - ZONE 8 - MAX. GLAZING U FACTOR: 0.40. MIN. INSULATION R VALUES: CEILING R-30, WALLS R-13, FLOORS R-19, BASEMENT WALLS R-8, SLAB PERIMETER R-5 (2 FT DEEP), CRAWL SPACE WALLS R-10.

#### TRUCTIO

1. STEEL FLITCH BEAMS SHALL BE FASTENED TOGETHER WITH 1/2" DIAMETER BOLTS WITH WASHERS PLACED UNDER THE THREADE END OF THE BOLT. BOLTS SHALL BE SPACED AT MAXIMUM 2" O.C. STAGGERED TOP AND BOTTOM OF BEAM WITH A MINIMUM 2" EDGE DISTANCE. TWO BOLTS SHALL BE LOCATED AT 6" FROM EACH END OF FLITCH BEAM.

2. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" AND FULL FLANGE WIDTH. BEAMS MUST BE ANCHORED AT EACH END WITH A MINIMUM OF FOUR 16d NAILS OR TWO 1/2" x 4" LAG SCREWS. 3. ENGINEERED WOOD BEAMS SHALL BE INSTALLED WITH ALL CONNECTIONS PER MANUFACTURER'S INSTRUCTIONS.

4. ALL BEAMS SHALL BE CONTINUOUSLY SUPPORTED LATERALLY AND SHALL BEAR FULL WIDTH ON THE SUPPORTING WALLS OR COLUMNS INDICATED WITH A MINIMUM OF THREE STUDS.

5. SOLID BLOCKING SHALL BE PROVIDED AT ALL POINT LOADS TO TRANSFER LOADS THROUGH FLOOR LEVELS. COLUMNS SHALL BE CONTINUOUS TO THE FOUNDATION OR TO OTHER STRUCTURAL ELEMENTS.

6. ENGINEERED WOOD FLOOR SYSTEMS AND ROOF TRUSS SYSTEMS SHALL BE PROVIDED FOR REVIEW AND COORDINATED

WITH THE ENGINEER OF RECORD. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. 7. WALL BRACING REQUIREMENTS SHALL BE IN ACCORDANCE WITH SECTION R602.10 OF THE NORTH CAROLINA RESIDENTIAL CODE.

8. BRICK LINTELS SHALL BE 3 1/2 x 3 1/2 x 1/4 STEEL ANGLE FOR UP TO 6'0" MAXIMUM SPAN AND 6 x 4 x 5/16 FOR SPANS GREATER THAN 6'0".

9. BRICK LINTELS AT SLOPED AREAS SHALL BE 4 x 3 1/2 x 1/4 STEEL ANGLE WITH 16d NAILS IN 3/16" HOLES IN 4" ANGLE LEG AT 12" o.c. TO DOUBLE RAFTER. WHEN THE SLOPE EXCEEDS 4:12 A MINIMUM OF 3 x 3 x 1/4 PLATES SHALL BE WELDED AT 24" o.c. ALONG THE STEEL ANGLE.

# **Prince Place Lot 13**

ATTIC VENT SCHEDULE											
ELEVATION											
MAIN	MAIN HOUSE SQ FTG 2453 AT / NEAR RIDGE					AT / NEAR EAVE					
SQ. FT. VENT TYPE REQUIRED			SQ. FT.	PERCENT OF TOTAL	POT LARGE (SQ. FT. EACH)	POT SMALL (SQ. FT. EACH)	RIDGE VENT (SQ. FT. PER LF)	EAVE VENT (SQ. IN. EACH)	CONT. VENT (SQ. IN. PER LF)		
	RANGE				SUPPLIED SUPPLIED		0.4236	0.2778	0.125	0.1944	0.0625
					-	-					
RIDGE VENT	3.27	4.09	8.75	46.67	0	0	70.00				
SOFFIT VENTS	4.91	4.09	10.00	53.33		160.00					
FOTAL (MIN)	8.18	8.18	18.75	100.00	POT VENTS MAY BE REQUIRED IF THERE IS INSUFFICIENT RIDGE AVAILABLE						
* SCH	* SCHEDULE HAS BEEN CALCULATED ASSUMING EAVE VENTILATION AT 50-60% OF TOTAL AND RIDGE AT 40-50% OF TOTAL REQUIRED VENTILATION										

## **REVISION LOG**

DATE: 01/28/2022 1. ADD 3-CAR GARAGE TO PLAN.

## REVISION: 002

REVISION: 001

DATE: 02/08/2022

CHANGED THE 2 CAR GARAGE DOOR 16080 GARAGE DOOR TO 18080 EXTENDED 2 CAR GARAGE FRONT WALL 24" TO ACCOMMODATE NEW GARAGE DOOR SIZE

CHANGED THE 8080 GARAGE DOOR TO 9080

4. EXTENDED 1 CAR GARAGE SIDE WALL 12" TO ACCOMMODATE NEW GARAGE DOOR SIZE

## TABLE N1102.1 CLIMATE ZONES 3-5

CLIMATE	FENESTRATION U-FACTOR b	SKYLIGHT <sup>b</sup> U-FACTOR	GLAZED FENESTRATION	amuna k	WOOD	MASS		BASEMENT <sup>C</sup>	SLAB d	CRAWL SPACE C	1
201100		UTAUTON	SHGC b,e	ceiling <sup>k</sup> R-Value	FRAMED WALL R-VALUE	WALL R-VALUE I	floor R-Value	WALL R-VALUE	R-VALUE AND DEPTH	WALL R-VALUE	
3	0.35	0.65	0.30	30	13	5/10	19	10/13 <sup>f</sup>	0	5/13	
4	0.35	0.60	0.30	38 OR 30 CONT j	15 OR 13+2.5 <sup>h</sup>	5/10	19	10/13	10 <sup>d</sup>	10/13	
5	0.35	0.60	NR	38 OR 30 CONT j	19 OR 13+5 OR 15+3 <sup>e,h</sup>	13/17	30 <sup>g</sup>	10/13	10 <sup>d</sup>	10/13	
<ol> <li>"10/13" MEANS CRAWL SPACE</li> <li>FOR MONOLITI WHICHEVER IS REQUIRED SLA</li> <li>R-19 FIBERGL/ A 2x4 WALL S1</li> <li>R-19 FIBERGL/ A 2x4 WALL S1</li> <li>R-19 FIBERGL/ BASEMENT W/ 9. OR INSULATIO</li> <li>h. "13+5" MEANS PERCENT OR INSULATIO</li> <li>h. "13+5" MEANS</li> <li>h. "13+5" MEA</li></ol>	IIC SLABS, INSULATIO LESS, FOR FLOATING BEDGE R-VALUE FOR SOT DEEMED TO COMP SS BATTS COMPRESS SOT DEEMED TO COMP ILL INSULATION IS NOT ILL INSULATION IS NOT REFLOATING TO SATISFY EAVES. OTHERWISE R	D SHEATHING O N SHALL BE APF HEATED SLABS ED AND INSTAL PLY. THE FRAMING O ION PLUS R-5 IN FACUIRED IN 1 THE FRAMING O ION PLUS R-5 IN EATHING SHALL (ALUE APPLIES 1 THE CEILING IN 1-38 INSULATION	N THE INTERIOR OF PLED FROM THE INS ILED FROM THE INS LED IN A NOMINAL 2 WARM-HUMID LOCA' AVITY, R-19 MINIMU SULATED SHEATHIN SHEATHING IS NOT IS SUPPLEMENTEI WHEN MORE THAN H SULATION REQUIRED I IS REQUIRED WHEI	EXTERIOR OF PECTION GAP TO THE BOTT X6 CAVITY IS D TIONS AS DEFI M. (6, 15+3 MEAN REQUIRED WH O WITH INSULA WITH INSULA WITH INSULA WITH INSULA WITH INSULA	THE HOME OR R-1: DOWNWARD TO TH OM OF THE FOUND EEMED TO COMPLY NED BY FIGURE N11 S R-15 CAVITY INSU ERE STRUCTURAL & TED SHEATHING OF LATION IS ON THE I VER THE FULL HEIG CLEARANCE EXIST:	3 CAVITY INSU IE BOTTOM OF ATTON WALL O 7. FIBERGLASS 101.2 (1 AND 2) LATION PLUS SHEATHING IS AT LEAST R- INTERIOR OF 1 SOR INSULAT	LATION AT T THE FOOTIN R 24 INCHES BATTS RAT AND TABLE R-3 INSULATI USED. IF THI 2. 13+25 MEA HE MASS W/ ICOMPRESSI ION MUST ED	IG OR A MAXIMU WHICHEVER IS ED R-19 OR HIGH N1101.2. ED SHEATHING, STRUCTURAL S INS R-13 CAVITY LL. ED R-30 INSULAT (TEND TO EITHEI	M OF 18 INCHES LESS. R-5 SHALL IER COMPRESSE IF STRUCTURAL SHEATHING COVI INSULATION PLU 10N EXTENDS O' R THE INSULATIO	BELOW GRADE, BE ADDED TO THE ED AND INSTALLED IN SHEATHING COVERS : SINCRE THAN 25 PI SIS R2.5 SHEATHING, VER THE WALL TOP IN BAFFLE OR WITHIN	E

ſ	ABBREVIATIONS	<u>ì</u>
	CONC	CONCRETE
	CONT	CONTINUOUS
ш	DBL	DOUBLE
ш	DJ	DOUBLE JOIST
ш	DSP	DOUBLE STUD POCKET
ш	EA	EACH
ш	FL PT	FLAT PLATE
ш	FTG	FOOTING
ш	HGR	HANGER
ш	LVL	LAMINATED VENEER LUMBER
ш	NTS	NOT TO SCALE
ш	OC	ON CENTER
ш	PSL	PARALLEL STRAND LUMBER
ш	PT	PRESSURE TREATED
ш	SC	STUD COLUMN
ш	SP	STUD POCKET
ш	TJ	TRIPLE JOIST
	TYP	TYPICAL
	UNO	UNLESS NOTED OTHERWISE
2		

SQUARE FOOTAGE						
	HEATED S.F.	UNHEATED S.F.				
FIRST FLOOR	1534	0				
SECOND FLOOR	1256	0				
3 CAR GARAGE	0	827				
FRONT PORCH	0	171				
SCREEN PORCH	0	181				
TOTAL	2790	1179				
OPTIONS						
	HEATED S.F.	UNHEATED S.F.				

MEAN ROOF HEIGHT 1 STORY = 11'-0" CLADDING POSITIVE & NEGATIVE PRESSURE = 21 PSF

1 1/2 STORY = 19'-0"

CLADDING POSITIVE & NEGATIVE PRESSURE = 34.8 PSF

2 STORY = 19'-0" CLADDING POSITIVE & NEGATIVE PRESSURE = 34.8 PSF

ANCHOR BOLTS INSTALL ANCHOR BOLTS, NUTS, AND WASHERS PER CODE AT ALL EXTERIOR WALL TREATED PLATES AND AT INTERIOR BEARING WALL TREATED PLATES ON SLAB FOUNDATIONS. TO BE A MINIMUM OF 6' O.C. AND WITHIN 12" FROM THE ENDS OF EACH PLATE.

DESIGN PRESSURES MINIMUM RATING: 25 PSF

**MI WINDOWS 3500 SERIES** LOW E-GLASS WINDOWS



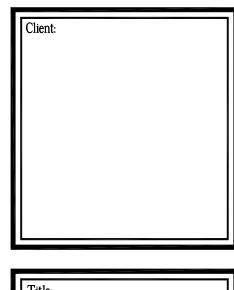
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Drawn By: RW	VB						
Checked By: RWB							
Date: 01-18-2021							
Revision No.	Revision Date						

### Designer Signature

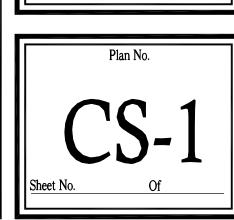
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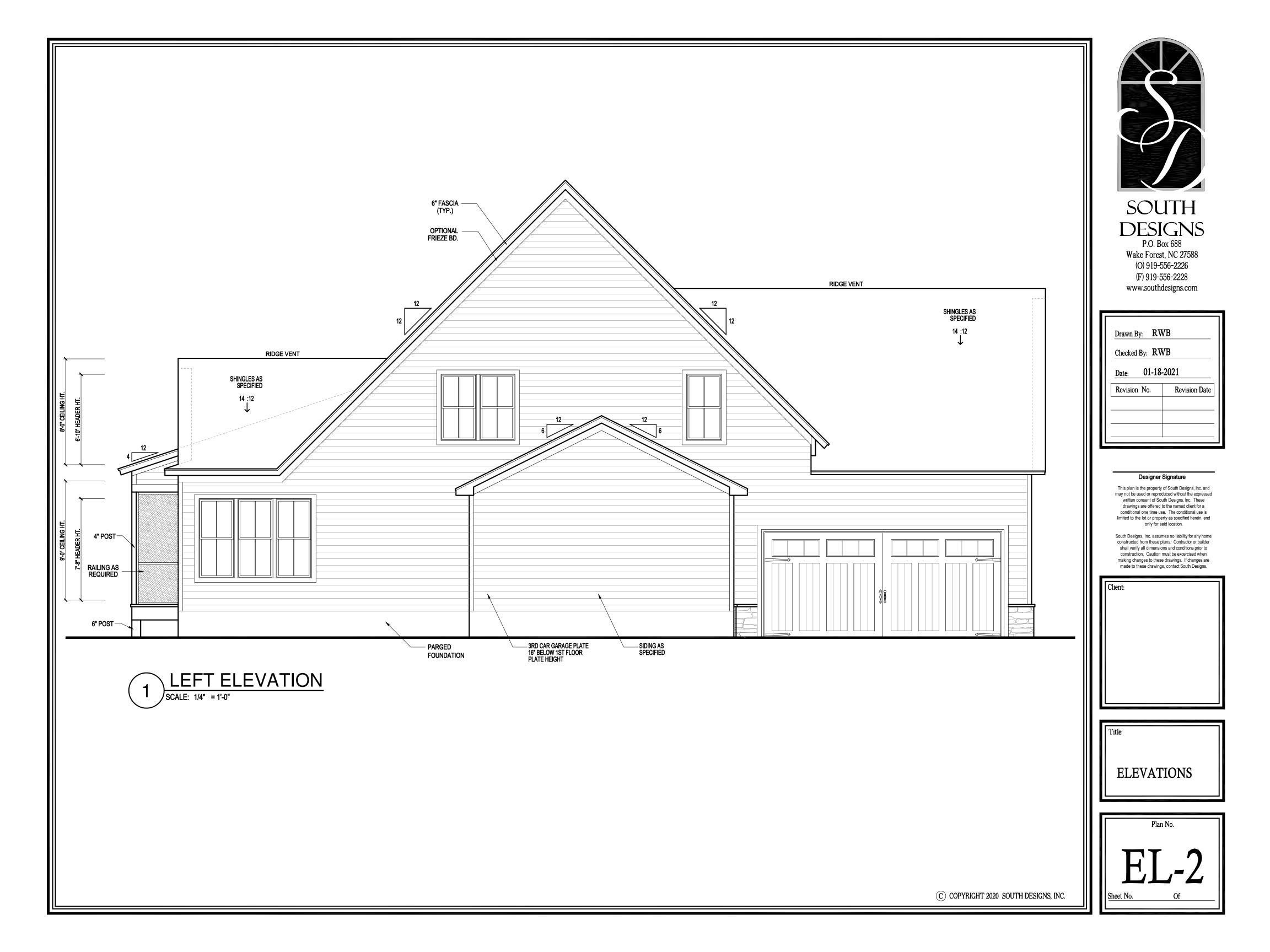


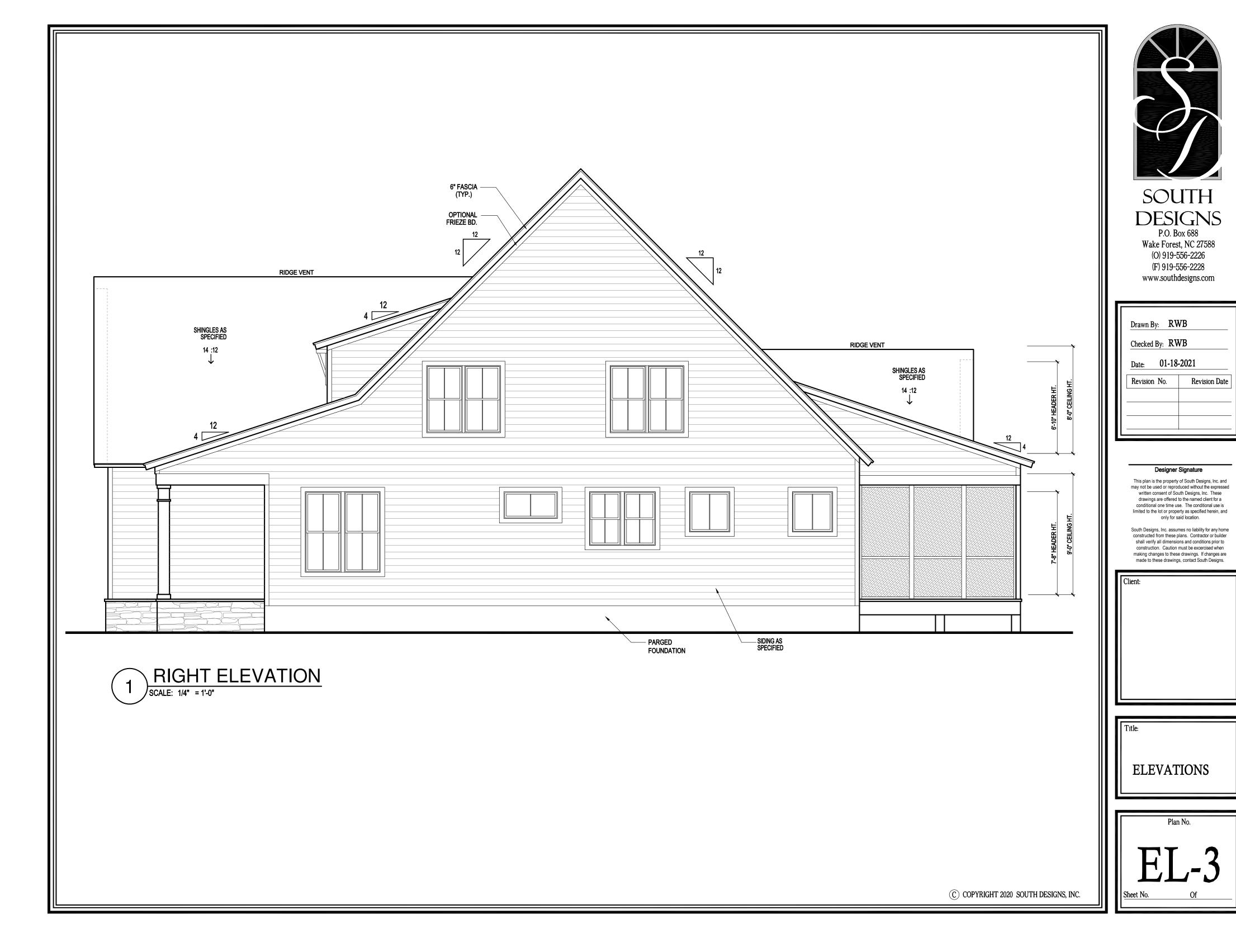


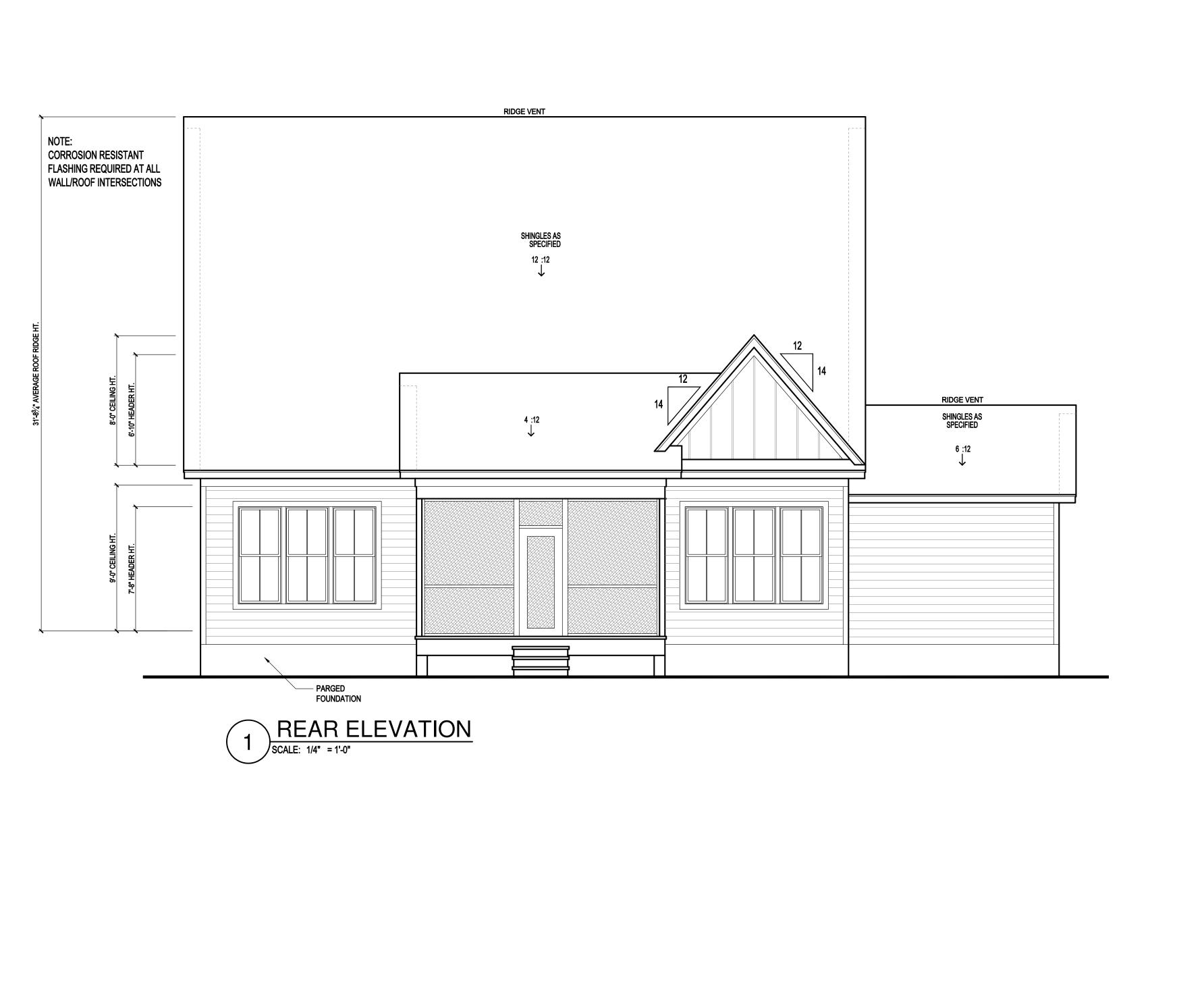
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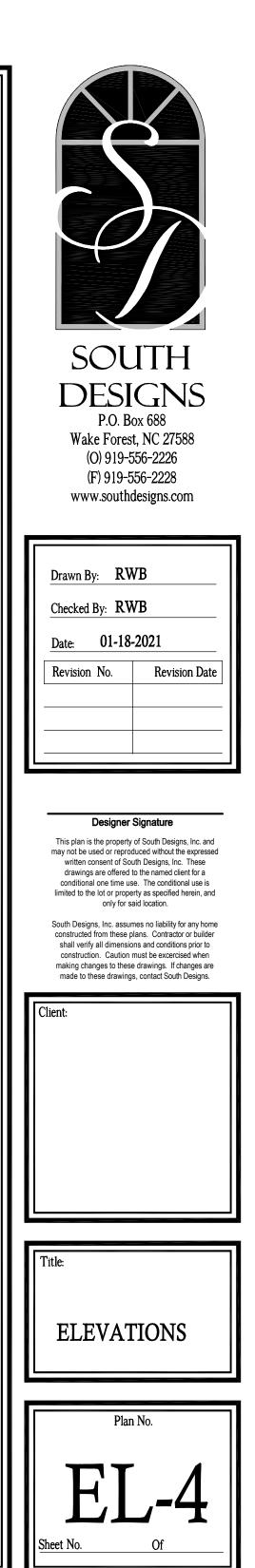


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Drawn By: RWB Checked By: RWB Date: 01-18-2021 Revision No. Revision Date
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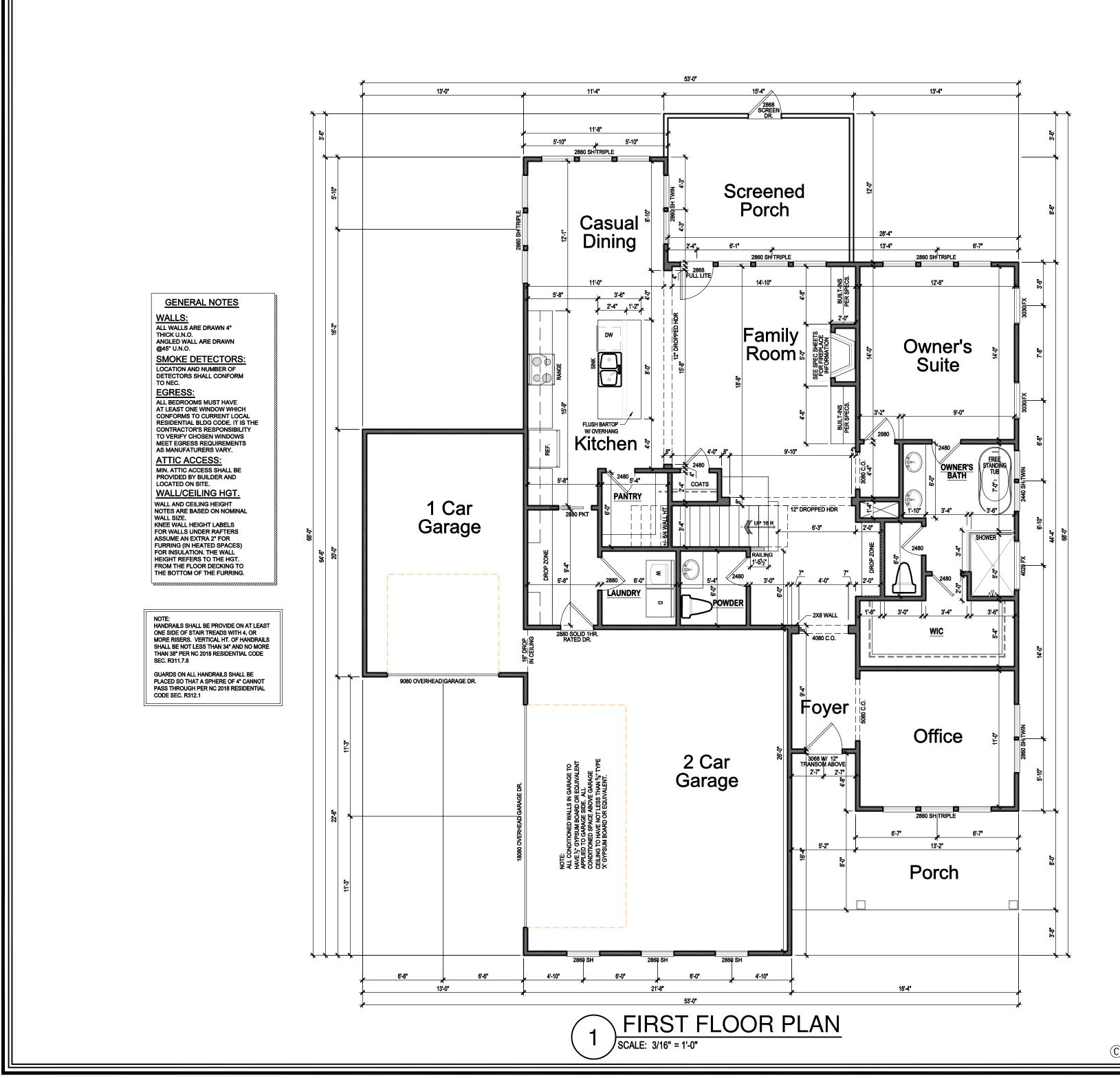


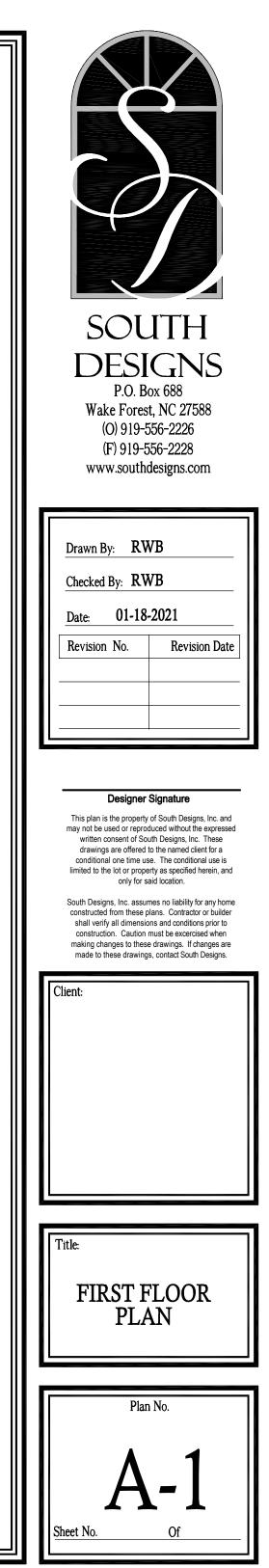




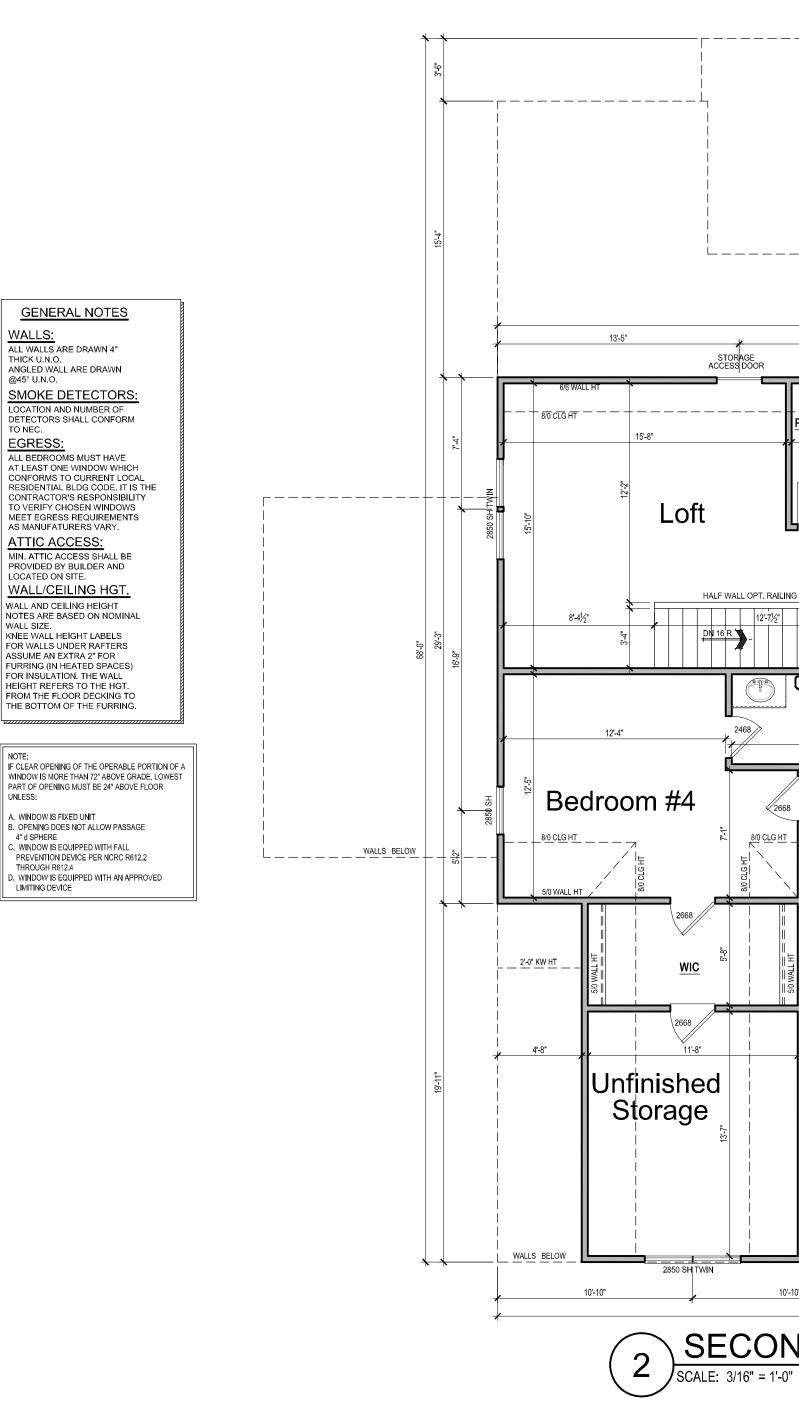


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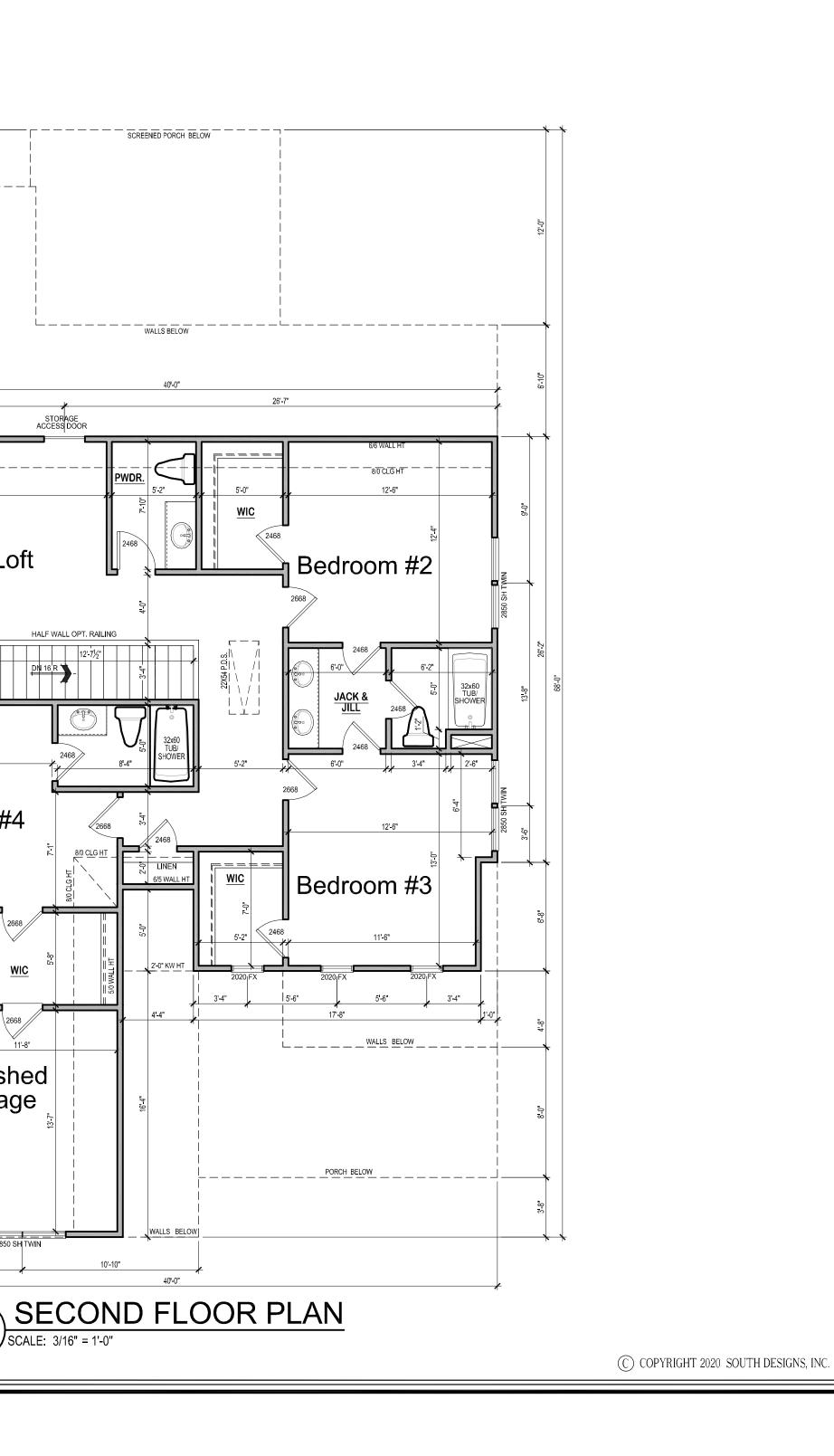


GENERAL NOTES WALLS: ALL WALLS ARE DRAWN 4" THICK U.N.O. ANGLED WALL ARE DRAWN @45° U.N.O. SMOKE DETECTORS: LOCATION AND NUMBER OF DETECTORS SHALL CONFORM TO NEC. ALL BEDROOMS MUST HAVE AT LEAST ONE WINDOW WHICH CONFORMS TO CURRENT LOCAL RESIDENTIAL BLDG CODE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIEV CHOSEN WINDOWS TO VERIFY CHOSEN WINDOWS MEET EGRESS REQUIREMENTS AS MANUFATURERS VARY. ATTIC ACCESS: MIN. ATTIC ACCESS SHALL BE PROVIDED BY BUILDER AND LOCATED ON SITE. WALL/CEILING HGT. WALL AND CEILING HEIGHT NOTES ARE BASED ON NOMINAL WALL SIZE. KNEE WALL HEIGHT LABELS FOR WALLS UNDER RAFTERS ASSUME AN EXTRA 2" FOR FURRING (IN HEATED SPACES) EOR INIT ATION THE WALL FOR INSULATION. THE WALL HEIGHT REFERS TO THE HGT.

## NOTE:

- UNLESS:

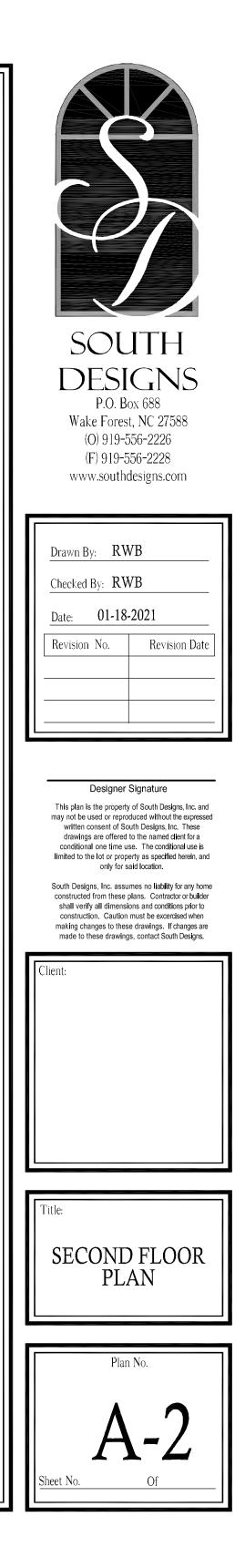
- LIMITING DEVICE

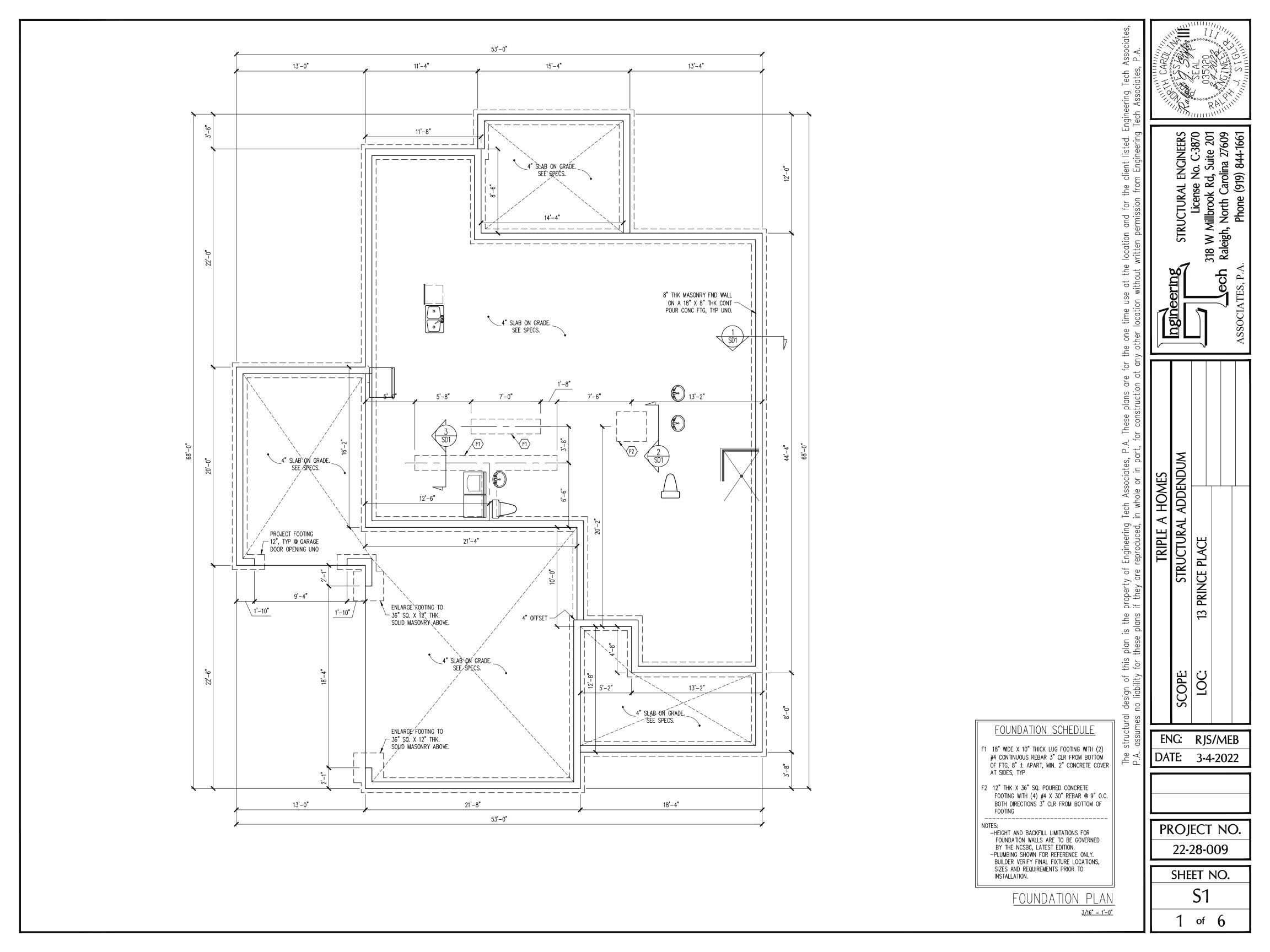


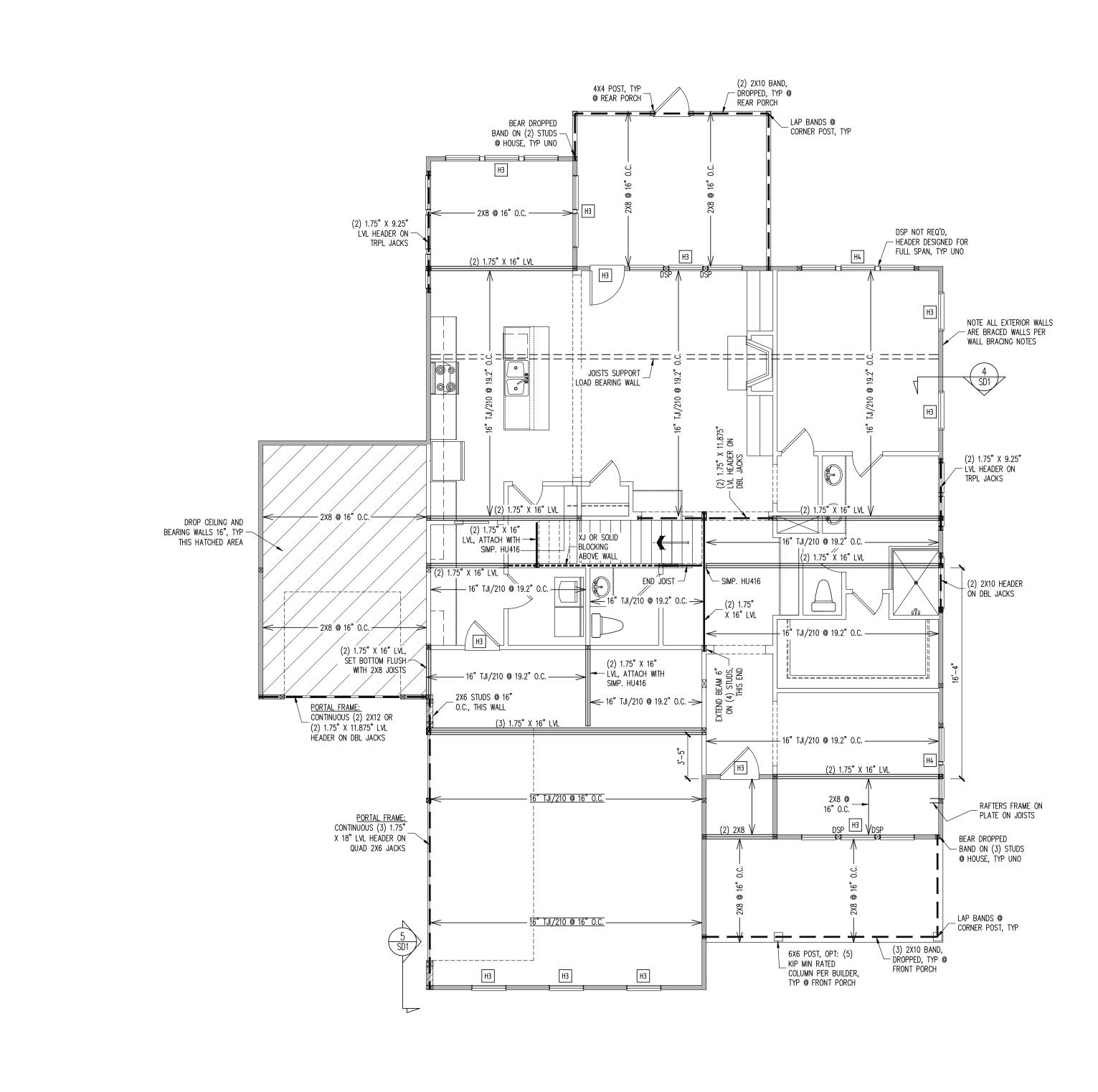
12'-7<sup>1</sup>/2"

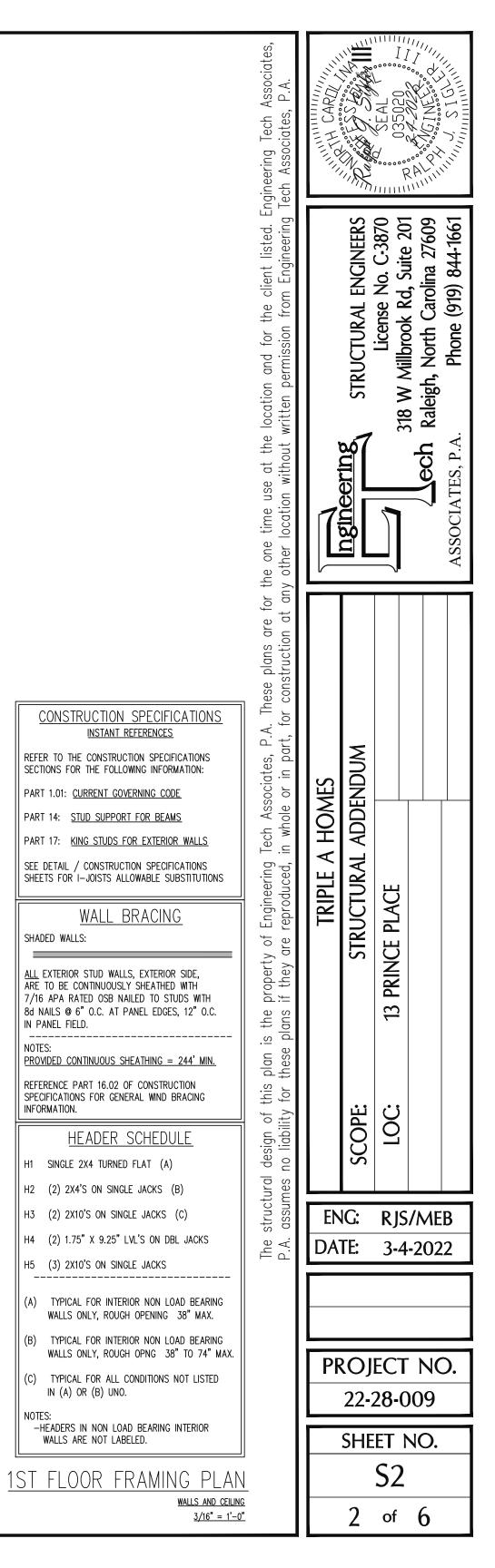
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8/0 CLG HT









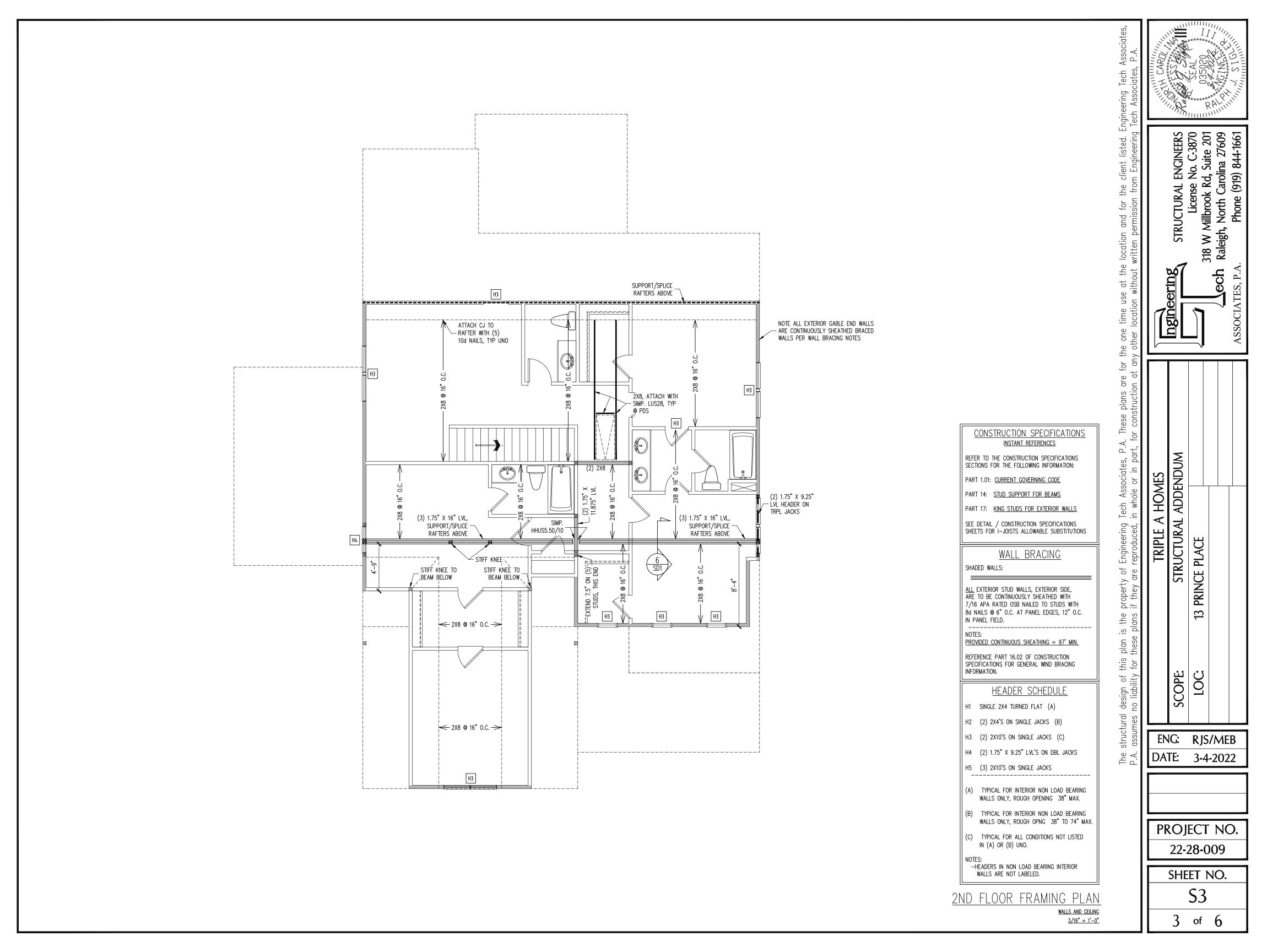
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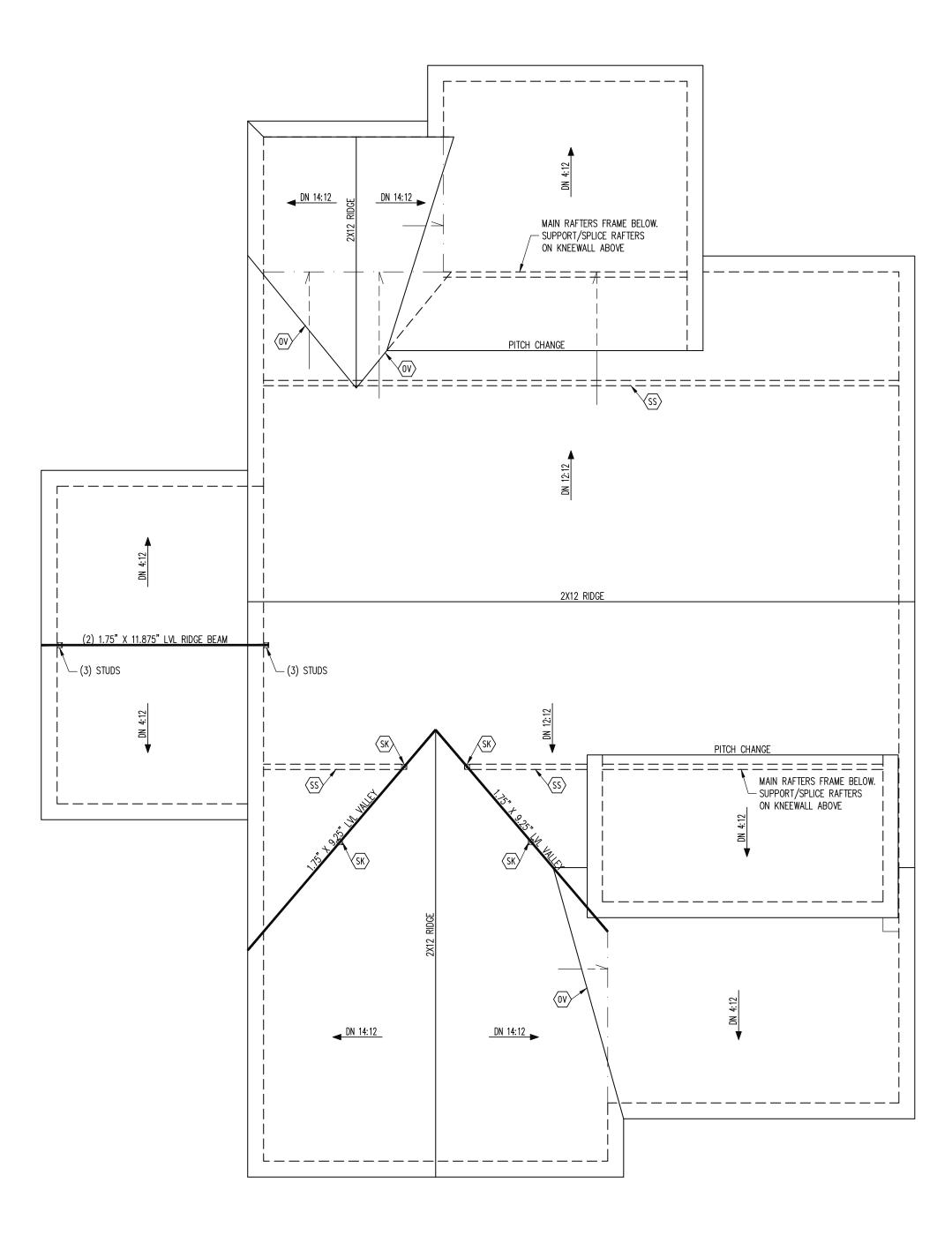
IN PANEL FIELD.

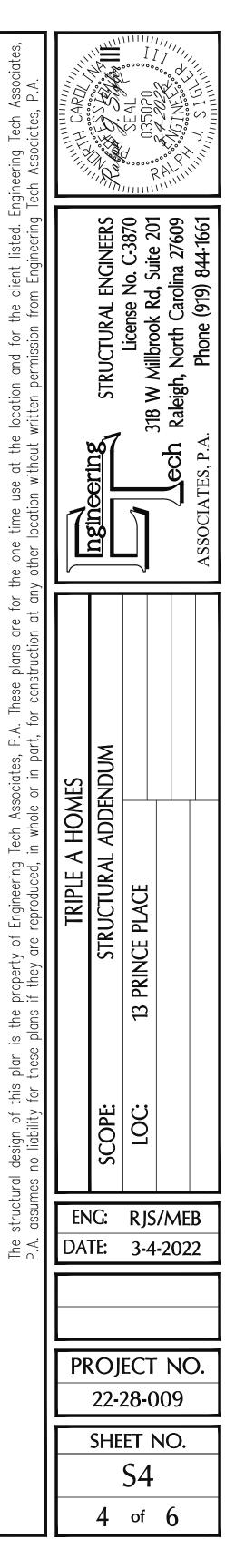
INFORMATION.

NOTES:

NOTES:







FRAMING NOTES

ROOF ONLY

-COMMON RAFTERS 2X8 @ 16" O.C. TYP U.N.O. -COLLAR TIES 2X4 EVERY 3RD SET OF RAFTERS TYP U.N.O. -ROOF PITCHES 12:12 TYP U.N.O.

-VERIFY ROOF PITCHES, OVERHANG LENGTHS, AND KNEEWALL FRAMING HGTS WITH ARCHITECTURAL DRAWINGS, TYPICAL.

FRAMING SCHEDULE

ROOF ONLY

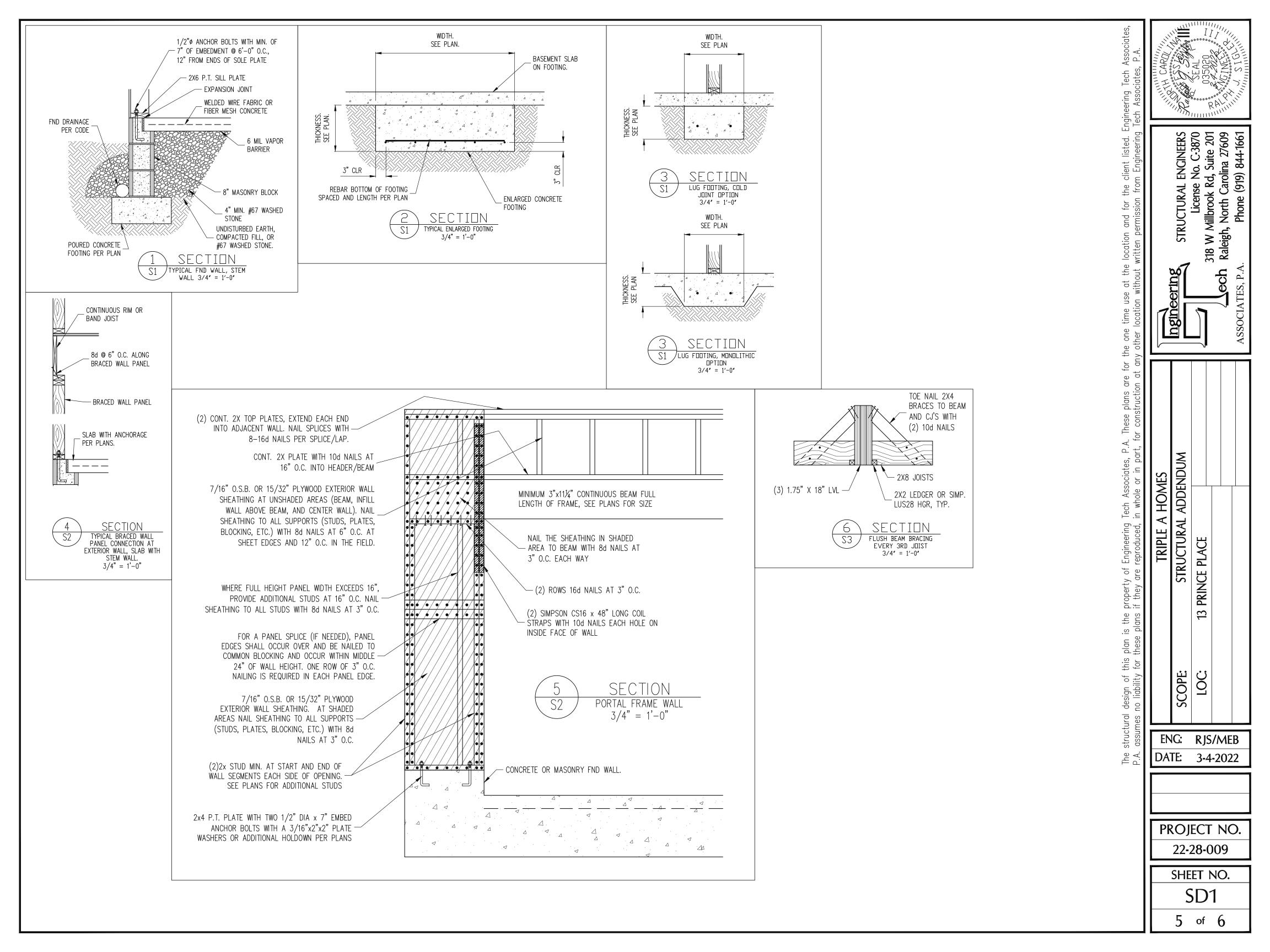
OV OVERFRAME VALLEY (2X10 SLEEPER)

SS SUPPORT/SPLICE RAFTERS ON KNEEWALL BELOW

ROOF FRAMING PLAN

<u>3/16" = 1'-0"</u>

SK DBL 2X4 STIFF KNEE



CONSTRUCTION	SPECIFICATIONS		DECK SPEC
PART 1: GENERAL 1.01 CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.	f'M = 1,500 PSI MIN 7.02 CLAY MASONRY UNITS SHALL CONFORM TO ASTM C62-17 GRADE SW	2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 3" ONTO THE WALL AND BE SUPPORTED BY A DBL STUD GANGED COLUMN TYP UNO.	1. A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO A STRUCTURE OR BE FREE STANDING. ROOFED PORCHES, OPEN OR SCREENED IN, MAY BE CONSTRUCTED USING THESE PROVISIONS.
1.02 DIMENSIONS SHOWN SHALL GOVERN OVER SCALE ON THESE DRAWINGS.	7.03 MORTAR SHALL BE TYPE S. MORTAR AND GROUT SHALL CONFORM TO ASTM C476, MIN COMPRESSIVE STRENGTH OF 2000 PSI.	14.03 EXTRA JOISTS BEARING ON A STUD WALL PERPENDICULAR TO OR SKEWED RELATIVE TO THE BEAM SHALL BE SUPPORTED BY ONE ADDITIONAL STUD.	2. SUPPORT POSTS SHALL BE SUPPORTED BY A FOOTING.
<ul> <li>1.05 METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR, WHO SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION. PART 2: DESIGN LOADS</li> <li>2.01 DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW: USE LIVE LOAD (PSF) DEAD LOAD (PSF)</li> <li>BALCONIES, DECKS, ATTICS WITH FIXED STAIR ACCESS, DWELLING UNITS INCLUDING ATTICS WITH FIXED STAIR ACCESS, STARS, FIRE ESCAPES 40 10</li> <li>GARAGES (PASSENGER CARS ONLY) 50 ATTICS (NO STORAGE, LESS THAN 5' HEADROOM) 10 10 ATTICS (WITH STORAGE) 20 10 ROOF 20 10 (15 FOR VAULTS)</li> <li>NOTES: - INDIVIDUAL STAIR TREADS ARE TO BE DESIGNED FOR THE UNIFORMLY DISTRIBUTED LIVE LOAD OF 40 PSF OR A 300 LB. CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQ. WHICHEVER PRODUCES THE GREATER STRESS. - BUILDER TO VERIFY DEAD LOAD DOES NOT EXCEED 10 PSF WHEN HEAVY FLOOR OR ROOF FINSHES SUCH AS TILE OR SLATE ARE UTILIZED. NOTIFY ENGINEERING UNDER THESE CONDITIONS</li> <li>2.02 INTERIOR WALLS: 5 PSF LATERAL.</li> <li>2.03 BASIC WIND DESIGN VELOCITY OF 120 MPH.</li> </ul>	<ul> <li>7.04 MASONRY CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS OF ACI 530</li> <li>7.05 LADDER WIRE REINFORCEMENT SHALL CONFORM TO ASTM A951. 6" MIN LAPS FOR CONTINUOUS WALL APPLICATIONS</li> <li>PART 8: BOLTS AND LAG SCREWS</li> <li>8.01 BOLTS SHALL CONFORM TO ASTM A307 MINIMUM GRADE TYP UNO. INSTALL STANDARD STEEL WASHERS (ASTM F844-070) FOR THE NUT / BOLT HEAD WHEN BOLTING WOOD MEMBERS: HOLES FOR BOLTS SHALL BE AISC STANDARD HOLES UNO</li> <li>8.02 LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981. PILOT HOLES SHALL BE USED FOR LAG SCREW INSTALLATION AND SHALL BE BORED ACCORDING TO NDS SPECIFICATIONS. INSTALL STANDARD STEEL WASHERS (ASTM F844-070) FOR SCREW HEAD</li> <li>8.03 ANCHOR RODS AND BOLTS SHALL CONFORM TO ASTM F1554-15 GRADE 36 UNO. BENT ANCHOR BOLTS SHALL HAVE A 2" MIN HOOK UNO</li> <li>PART 9: DRIVEN FASTENERS</li> <li>9.01 NAILS, SPIKES AND STAPLES SHALL CONFORM TO ASTM F 1667- 05. NAILS ARE TO BE COMMON WIRE OR BOX</li> <li>PART 10: DIMENSIONAL LUMBER</li> <li>10.01 SOLID SAWN WOOD FRAMING DESIGN IS BASED ON NO. 2 SPRUCE PINE FIR OR SYP #2 FOR JOISTS, RAFTERS, GIRDERS, BEAMS, STUDS, ETC. MINIMUM ALLOWABLE DESIGN PROPERTIES ARE AS FOLLOWS: E = 1,400,000 PSI, Fc perp = 425 PSI, Fv = 285 PSI, SPECIFIC GRAVITY = 0.42 MIN Fb = 875 PSI FOR 2X4, 2X6, 2X8. Fb = 800 PSI FOR 2X10'S, 750 PSI FOR 2X12'S</li> </ul>	<ul> <li>14.04 STUDS THAT ARE GANGED TO FORM A COLUMN SHALL HAVE ADJACENT STUDS WITHIN THE COLUMN NAILED TOGETHER WITH ONE ROW OF 10d NAILS AT 8" O.C. (TWO ROWS OF 10d NAILS @ 8" O.C., 3" APART, FOR 2X8 OR 2X10 STUDS) ALL COLUMNS SHALL BE CONTINUOUS DOWN TO THE FOUNDATION OR OTHER PROPERLY DESIGNED STRUCTURAL ELEMENT SUCH AS A BEAM. COLUMNS TRANSFERRING LOADS THROUGH FLOOR LEVELS SHALL BE SOLIDLY BLOCKED <u>FOR THE FULL WIDTH</u> OF THE STUD COLUMN WITHIN THE CAVITY FORMED BY THE FLOOR JOISTS.</li> <li>PART 15: NAILING OF MULTI PLY WOOD BEAMS</li> <li>15.01 SOLID SAWN LUMBER JOISTS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM NAILED TOGETHER WITH THREE ROWS OF 10d NAILS @ 16" O.C. FOR 2X10 OR LARGER, TWO ROWS OF 10d NAILS @ 16" O.C. FOR 2X8, ONE ROW OF 10d NAILS @ 16" O.C. FOR 2X6 OR SMALLER. STAGGER ROWS 5" MIN.</li> <li>15.02 LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM COLUMN ADJACENT REMBERS IN THE BEAM STALE ON TOGETHER WITH THREE ROWS OF 10d NAILS @ 16" O.C. FOR 2X6 OR SMALLER. STAGGER ROWS 5" MIN.</li> <li>15.02 LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM FASTENED TOGETHER PER MANUFACTURERS RECOMMENDATIONS, TYP UNO</li> <li>PART 16: WALL FRAMING AND BRACING</li> <li>16.01 STUD WALLS SHALL CONSIST OF 2X4 STUDS SPACED AT 16" O.C. UNO. STUDS SHALL BE CONTINUOUS FROM SOLE PLATE AT FLOOR TO DOUBLE TOP PLATE AT THE CELING OR ROOF. NO INTERWEDIATE BANDS OR PLATES SHALL CAUSE DISCONTINUETIES IN A STUD WALL EXCEPT AS REQUIRED FOR DOOR OR WINDOW OPENINGS. THE KING STUDS FOR SUCH OPENINGS SHALL HEIGHTS FOR EXTERIOR STUD WALLS, INCLUSIVE OF SOLE PLATE AND 7/16" OSB EXTERIOR BRACING AND ROW OF 2X4 2X6 PURLINS AT 8" HEIGHT (AND AT 16" HEIGHT FOR TALL WALLS), TYP UNO: 2X4 @ 16" O.C.: 11'-11'2" 2X6 @ 16" O.C.: 17'-0" 2X4 @ 16" O.C.: 12'-11/2" 2X6 @ 12" O.C.: 18'-8"</li> </ul>	<ul> <li>WHEN ATTACHED TO A STRUCTURE, THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A TREATED WOOD BAND FOR THE LENGTH OF THE DECK, OR CORROSION RESISTANT FLASHING SHALL BE USED TO PREVENT MOISTURE FROM COMING IN CONTACT WITH THE UNTREATED FRAMING OF THE STRUCTURE. THE DECK BAND AND THE STRUCTURE BAND SHALL BE CONSTRUCTED IN CONTACT WITH EACH OTHER EXCEPT AT BRICK VENEER AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY FLASHED. SIDING SHALL NOT BE INSTALLED BETWEEN THE STRUCTURE AND THE DECK BAND. FOR THE BRICK STRUCTURE, NEITHER FLASHING NOR A TREATED DECK BAND. IF ATTACHED TO A BRICK STRUCTURE, NEITHER FLASHING NOR A TREATED DECK BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT WITH THE BRICK</li> <li>WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE, THE FOLLOWING ATTACHMENT SCHEDULES SHALL APPLY FOR ATTACHING THE DECK BAND TO THE STRUCTURE:         <ul> <li>A. ALL STRUCTURES EXCEPT BRICK STRUCTURES</li> <li>IOIST LENGTH</li> <li>UP TO 8' MAX.</li> <li>UP TO 16' MAX.</li> </ul> </li> <li>REQUIRED ONE - 5/8'' Ø BOLT @ 42'' O.C. AND ONE - 5/8'' Ø BOLT @ 20'' O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 32'' O.C. STAGGERED</li> <li>A. BRICK VENEER STRUCTURES</li> <li>JOIST LENGTH</li> <li>UP TO 8' MAX.</li> <li>UP TO 16' MAX.</li> </ul>
<ul> <li>2.04 SOIL BEARING CAPACITY 2000 PSF (PRESUMPTIVE).</li> <li><u>PART 3: STRUCTURAL STEEL</u></li> <li>3.01 WIDE FLANGE BEAMS AND TEE SECTIONS SHALL CONFORM TO ASTM A992 MINIMUM GRADE</li> <li>3.02 SQUARE AND RECTANGULAR TUBING SHALL CONFORM TO ASTM A500 GRADE B MINIMUM GRADE.</li> <li>3.03 STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B, TYPE S, MINIMUM GRADE</li> <li>3.04 ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 MINIMUM GRADE</li> <li>3.05 STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AISC</li> </ul>	PART 11: ENGINEERED LUMBER         11.01       LVL OR PSL MINIMUM ALLOWABLE DESIGN PROPERTIES ARE AS FOLLOWS: E = 1,900,000 PSI, $F_b = 2600$ PSI, $F_v = 285$ PSI, $F_c perp = 750$ PSI LSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E = 1.3 X 10E6 PSI, $F_b = 1700$ PSI, $F_v = 400$ PSI, $F_c perp = 680$ PSI         11.02       LVL OR PSL MEMBERS MAY BE RIPPED FROM DEEPER MEMBERS TO MATCH THE MEMBER DEPTH SPECIFIED IN THE PLANS         PART 12: PRESSURE TREATED LUMBER         12.01       LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED LUMBER SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2 OR BY ANY METHOD	DBL 2X4 @ 16" 0.C.: 13'-4" DBL 2X6 @ 16" 0.C.: 21'-0" 16.02 FOR WALL BRACING THE FOLLOWING SHALL APPLY: -BLOCKING AT UNSUPPORTED PANEL EDGES IS REQUIRED TYP UNO. -WALL BRACING IS BY ENGINEERED DESIGN AND NOT PRESCRIPTIVE PER SECTION 602.10 OF THE 2018 NCRC. CONTINUOUS SHEATHING HAS BEEN PROVIDED, ALONG WITH ALTERNATIVE METHODS TO INSURE THE MINIMUM INTENT OF SECTION 602.10 OF THE 2018 NCRC HAS BEEN MET AND EXCEEDED. -BRACED WALL PANELS SHALL BE FASTENED IN ACCORDANCE WITH TABLE 602.3(1) TO PROVIDE CONTINUOUS PANEL UPLIFT RESISTANCE AND COMPLIANCE WITH NCRBC R602.3.5 AND R802.11 UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS. -MAY SUBSTITUTE WSP FOR GB -SINGLE JOIST, CONTINUOUS RIM JOIST, OR BLOCKING OF EQUAL DEPTH IS REQUIRED ABOVE AND BELOW ALL BRACED WALLS. NAIL BLOCKING ABOVE WALL TO TOP PLATE	REQUIRED FASTENERS       ONE-5/8" Ø BOLT @ 28" O.C.       ONE-5/8" Ø BOLT @ 16" O.C.         5.       IF THE DECK BAND IS SUPPORTED BY A 1/2" MINIMUM MASONRY LEDGE ALONG THE FOUNDATION WALL, 5/8" Ø BOLTS SPACED @ 48" O.C. MAY BE USED FOR SUPPORT.         6.       OTHER MEANS OF SUPPORT, SUCH AS JOIST HANGERS, MAY BE USED TO CONNECT DECK JOISTS TO A TREATED STRUCTURE BAND         7.       GIRDERS SHALL BEAR DIRECTLY ON POSTS OR BE BE CONNECTED TO THE SIDES OF POSTS WITH 2-5/8" Ø BOLTS
3.03       STRUCTURAL STEEL CONSTRUCTION SHALL WELT THE REQUIREMENTS OF THE AISC SPECIFICATION FOR THE AISC FOR BUILDINGS.         PART 4: WELDING       PART 4: WELDING         4.01       WELDING ELECTRODES SHALL BE E70XX AND ALL WELDING SHALL BE PERFORMED BY AN	GIVING EQUAL PROTECTION. THE BUILDING CODE OFFICE MAY ALSO APPROVE A NATURAL DECAY RESISTANT WOOD PER SECTION 19-6(A) <u>PART 13: STEEL FLITCH PLATE BEAMS</u>	WITH 16d TOE NAILS @ 6" O.C. NAIL SOLE PLATE OF BRACED WALL TO BLOCKING BELOW WITH (3) 16d NAILS @ 16" O.C. BLOCKING AT HORIZONTAL JOINTS IN BRACED WALL LINES ONLY REQUIRED AT SHADED WALLS, UNO. PART 17: KING STUDS	8. FLOOR DECKING SHALL BE NO. 2 GRADE TREATED SOUTHERN PINE OR EQUIVALENT. THE MINIMUM FLOOR DECKING THICKNESS SHALL BE AS FOLLOWS:
<ul> <li>AWS CERTIFIED WELDER</li> <li>PART 5: CONCRETE AND SLABS ON GRADE</li> <li>5.01 CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 4–6% AIR ENTRAINMENT, FOR EXTERIOR CONCRETE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS TYP UNO.</li> <li>5.02 REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACE, TYP UNO.</li> <li>5.03 SLABS ON GRADE, IF ANY, SHALL BE CAST IN PLACE, CONTAIN SYNTHETIC POLYPROPYLENE FIBRILLATED MICRO FIBERS, FIBER LENGTH 1 1/2", DOSAGE RATE 1 1/2 LBS/CU YD. SLAB TO BE PLACED ON A 6 MIL VAPOR BARRIER ON 4" MIN GRANULAR FILL ON SOIL WITH 90% MIN STANDARD PROCTOR DENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS</li> </ul>	TWO PIECES OF CONTINUOUS LUMBER AS SIZED ON THE PLANS. BOLT PIECES TOGETHER USING 1/2" & BOLTS SPACED AT 16" O.C. STAGGERED TOP TO BOTTOM OF THE BEAM. MAINTAIN A 2" EDGE DISTANCE. PLACE TWO BOLTS, ONE ABOVE THE OTHER, 16" MAX FROM EACH END OF THE BEAM. TYP UNO PART 14: STUD SUPPORTS FOR BEAMS 14.01 STEEL, ENGINEERED LUMBER, AND FLITCH PLATE BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:	17.01 KING STUDS FOR OPENINGS IN EXTERIOR WALLS SHALL BE AS FOLLOWS: <ul> <li>NUMBER OF KING STUDS</li> <li>MAX OPENING WIDTH <math>5'-0"</math> <math>9'-0"</math> <math>13'-0"</math> <math>17'-0"</math> <math>21'-0"</math></li></ul>	
<ul> <li>PART 6: REBAR AND WIRE REINFORCEMENT</li> <li>6.01 REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60 TYP UNO</li> <li>6.02 LAP SPLICES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UNO</li> <li>6.03 WIRE REINFORCEMENT SHALL BE 9 GA AND SHALL CONFORM TO ASTM A1064.</li> <li>PART 7: MASONRY</li> <li>7.01 CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT,</li> </ul>	<ul> <li>2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 4 1/2" ONTO THE WALL AND BE SUPPORTED BY A TRPL STUD GANGED COLUMN TYP UNO.</li> <li>14.02 DIMENSIONAL LUMBER BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:</li> <li>1-WHEN THE BEAM IS PERPENDICULAR TO, OR SKEWED RELATIVE TO THE WALL. THE BEAM SHALL BEAR FULL WIDTH ON THE SUPPORTING WALL INDICATED (LESS 1 1/2" TO ALLOW FOR A CONTINUOUS RIM JOIST WHERE APPLICABLE) AND SHALL BE SUPPORTED BY A GANGED STUD COLUMN THE SAME WIDTH AS THE BEAM TYP UNO. (E.G. A TRIPLE 2X10 IS TO BE SUPPORTED BY (3) STUDS). FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM</li> </ul>	ASSOCIATES (ETA). THESE PLANS ARE FOR THE ONE TIME USE AT THE LOCATION INDICATED AND FOR THE CLIENT LISTED. ETA ASSUMES NO LIABILITY FOR THESE PLANS IF THEY ARE REPRODUCED, IN WHOLE OR IN PART, FOR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT WRITTEN PERMISSION FROM ETA	
NOTES	ABBREVIATIONS	ALLOWABLE I-JOIST SUBSTITUTION	
THE BUILDER IS RESPONSIBLE FOR REVIEWING PLANS PRIOR TO CONSTRUCTION. THE BUILDER SHALL IMMEDIATELY CONTACT THE ENGINEER OF RECORD (EOR) BEFORE PROCEEDING IF THE FOLLOWING CONDITIONS ARE NOTED BEFORE OR DURING CONSTRUCTION: 1) THE WORKING PLANS DO NOT BEAR THE SEAL OF THE EOR 2) THE PLANS CONTAIN DISCREPANT OR INCOMPLETE INFORMATION ANY ERRORS DUE TO A FAILURE TO FOLLOW THE ABOVE PROCEDURES SHALL NOT BE THE RESPONSIBILITY OF THE EOR. FURTHERMORE, IT IS THE RESPONSIBILITY OF THE BUILDER TO ENSURE THAN ANY REVISIONS ISSUED BY THE EOR ARE PROMPLY DISTRIBUTED TO THE SUBCONTRACTORS THE EOR DOES NOT PERFORM FENESTRATION OR VENTING CALCULATIONS OR ANY OTHER CALCULATIONS THAT ARE NOT DIRECTLY RELATED TO STRUCTURAL ENGINEERING. ROOF AND FLOOR TRUSSES TO BE DESIGNED BY AN ENGINEER REGISTERED BY THE STATE. FINAL TRUSS DRAWING SHOULD BE SUBMITTED TO THE EOR FOR REVIEW	ABV     ABOVE     FND     FOUNDATION     TJ     TRIPLE     JOIST       B.     BOTH     FTG     FOOTING     TYP     TYPICAL       B.E.     BOTH     ENDS     HDG     HOT DIPPED     TRIPLE     TRIPLE       BTWN     BETWEEN     GALVANIZED     TSP     TRIPLE     STOP       CIP     CAST IN PLACE     HGR     HANGER     UNO     UNLESS NOTED       CONC     CONCRETE     LVL     LAMINATED VENEER     OTHERWISE       DIA     DIAMETER     NTS     NOT TO SCALE     OTHERWISE       DJ     DOUBLE     O.C.     ON CENTER     LUMBER       DJ     DOUBLE     O.C.     ON CENTER     LUMBER       DJ     DOUBLE     OCC.     PSL     PARALLEL       DSP     DBL STUD POCKET     LUMBER     LUMBER       EQ     EQUAL     PT     PESSURE TREATED       EA     EACH     QJ     QUAD JOIST       FLG     FLANGE     SP     SPACE (OR SPACING)	NOTE:MAINTAINJOISTDEPTH,DIRECTION,ANDSPACINGSPECIFIEDONPLANS.SIMPSON FACESIMPSON FACESIMPSON TOPMANUFACTURERDEPTHSERIESMOUNTHGRFLANGEHGRBLUELINX16"BLI40IUS2.56/16ITS2.56/16BLUELINX16"BLI60IUS2.56/16ITS2.56/16BOISECASCADE16"BCI5000sIUS2.06/16BOISECASCADE16"BCI6000sIUS2.37/16INTERNATIONAL16"IB600IUS2.56/16ITS2.56/16BEAMSLPCORP16"LPI 20+IUS2.56/16ITS2.56/16NORDIC16"NI 40XIUS2.56/16ITS2.56/16ITS2.56/16ROSEBURG16"RFPI 60SIUS2.56/16ITS2.56/16WEYERHAEUSER16"TJI210IUS2.06/16ITS2.06/16	

JOISTS NOT LISTED IN THE ABOVE TABLE MAY BE USED PROVIDED THEY MEET OR EXCEED THE PROPERTIES OF THOSE LISTED. SUBSTITUTE USP BRAND HANGERS WITH EQUIVALENT VALUES AS DESIRED.

# CIFICATIONS

TERIOR WOOD FLOOR ST	TRUCTURE WHICH	MAY BE ATTACHED	TO
STANDING. ROOFED POR	RCHES, OPEN OR	SCREENED IN, MAY	BE

9. MA

	JOIST SPAN	DECKING
	12" O.C.	1" S4S
	16" O.C.	1" T&G
	24" O.C.	1 1/4″ S4S
	32" O.C.	2" S4S
XIMUM	HEIGHT OF DECK SUPPORT POSTS	S IS AS FOLLOWS:
	4X4	
	6X6	20'
	0/10	20
	ENGINEERED	20' +

10. DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING METHODS:

A. WHEN THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION 4, LATERAL BRACING IS NOT REQUIRED.

B. 4X4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45' AND 60' FROM THE HORIZONTAL. KNEE BRACES SHALL BE ATTACHED AT THE POPE TO THE OPPERT AND THE DORE WITH ONE SHOLL BE ATTACHED AT THE ENDS TO THE GIRDER AND THE POST WITH ONE - 5/8"Ø BOLT

C. FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN CONCRETE IN ACCORDANCE WITH THE FOLLOWING:

POST SIZE	TRIBUT. AREA	POST HEIGHT	emb. Depth	CONC. DIAM.
4X4 6X6	48 SQ. FT. 120 SQ. FT.	4'-0" 6'-0"	2'-6" 3'-6"	1'-0" 1'-8"
DIRECTIONS F	OR FREE STANDIN	ig decks or par ttached decks.	RALLEL TO THE S THE BRACES SH	I TWO PERPENDICUL TRUCTURE AT THE ALL BE ATTACHED RACE.

NOTES: 1) ALL NAILS AND BOLTS ARE TO BE HOT DIPPED GALVANIZED. 2) MINIMUM EDGE DISTANCE FOR BOLTS IS 2 1/2".

,						.,						
)	NAILS	MUST	PENETRATE	THE	SUPPORTING	STRUCTURE	BAND	A	MINIMUM	0F	1	1/2"

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the one time use at the location and for the client listed. Engineering Tech Associates, y other location without written permission from Engineering Tech Associates, P.A.		Ingineering Structural Engineers	318 W Millbrook Rd. Strite 201	<b>Cech</b> Raleigh, North Carolina 27609	ASSOCIATES, P.A. Phone (919) 844-1661
structural design of this plan is the property of Engineering Tech Associates, P.A. These plans are for the one time use at the location and for the client listed. Engineering Tech Asso assumes no liability for these plans if they are reproduced, in whole or in part, for construction at any other location without written permission from Engineering Tech Associates, P.A	TRIPLE A HOMES	SCOPE: STRUCTURAL ADDENDUM	LOC: 13 PRINCE PLACE		
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