

**PLANS DESIGNED TO THE
2018 NORTH CAROLINA STATE
RESIDENTIAL BUILDING CODE**

MEAN ROOF HEIGHT: 20'-4" HEIGHT TO RIDGE: 27'-10"

CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A
FENESTRATION U-FACTOR	0.35	0.35	0.35
SKYLIGHT U-FACTOR	0.55	0.55	0.55
GLAZED FENESTRATION SHGC	0.30	0.30	0.30
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci
WALL R-VALUE	15	15	19
FLOOR R-VALUE	19	19	30
* BASEMENT WALL R-VALUE	5/13	10/15	10/15
** SLAB R-VALUE	0	10	10
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19

* 10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION
** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL

DESIGNED FOR WIND SPEED OF 115 MPH, 3 SECOND GUST (89 FASTEST MILE) EXPOSURE "B"

COMPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS	MEAN ROOF UP TO 30'	30'-1" TO 35'	35'-1" TO 40'	40'-1" TO 45'
ZONE 1	13.1 -14.0	13.8 -14.7	14.3 -15.3	14.7 -15.7
ZONE 2	13.0 -13.0	13.7 -13.7	14.2 -14.2	14.6 -14.6
ZONE 3	13.1 -16.0	13.8 -16.8	14.3 -17.4	14.7 -17.9
ZONE 4	14.3 -15.0	15.0 -15.8	15.6 -16.4	16.0 -16.8
ZONE 5	14.3 -19.0	15.0 -20.0	15.6 -20.7	16.0 -21.3

ROOF VENTILATION

SECTION R806

R806.1 Ventilation required. Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware cloth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7.

R806.2 Minimum area. The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1/300 when a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.

Exceptions:

1. Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only.
2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,744 SQ.FT.

NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 18.29 SQ.FT.
WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 9.15 SQ.FT.

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or grade below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a guard.

R312.2 Height. Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads.

Exceptions:

1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the treads.
2. Where the top of the guard also serves as a handrail on the open sides of stairs, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations. Required guards shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm) in diameter.

Exceptions:

1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 mm) in diameter.
2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.

SQUARE FOOTAGE

HEATED

FIRST FLOOR	1941 SQ.FT.
SECOND FLOOR	685 SQ.FT.
TOTAL	2626 SQ.FT.

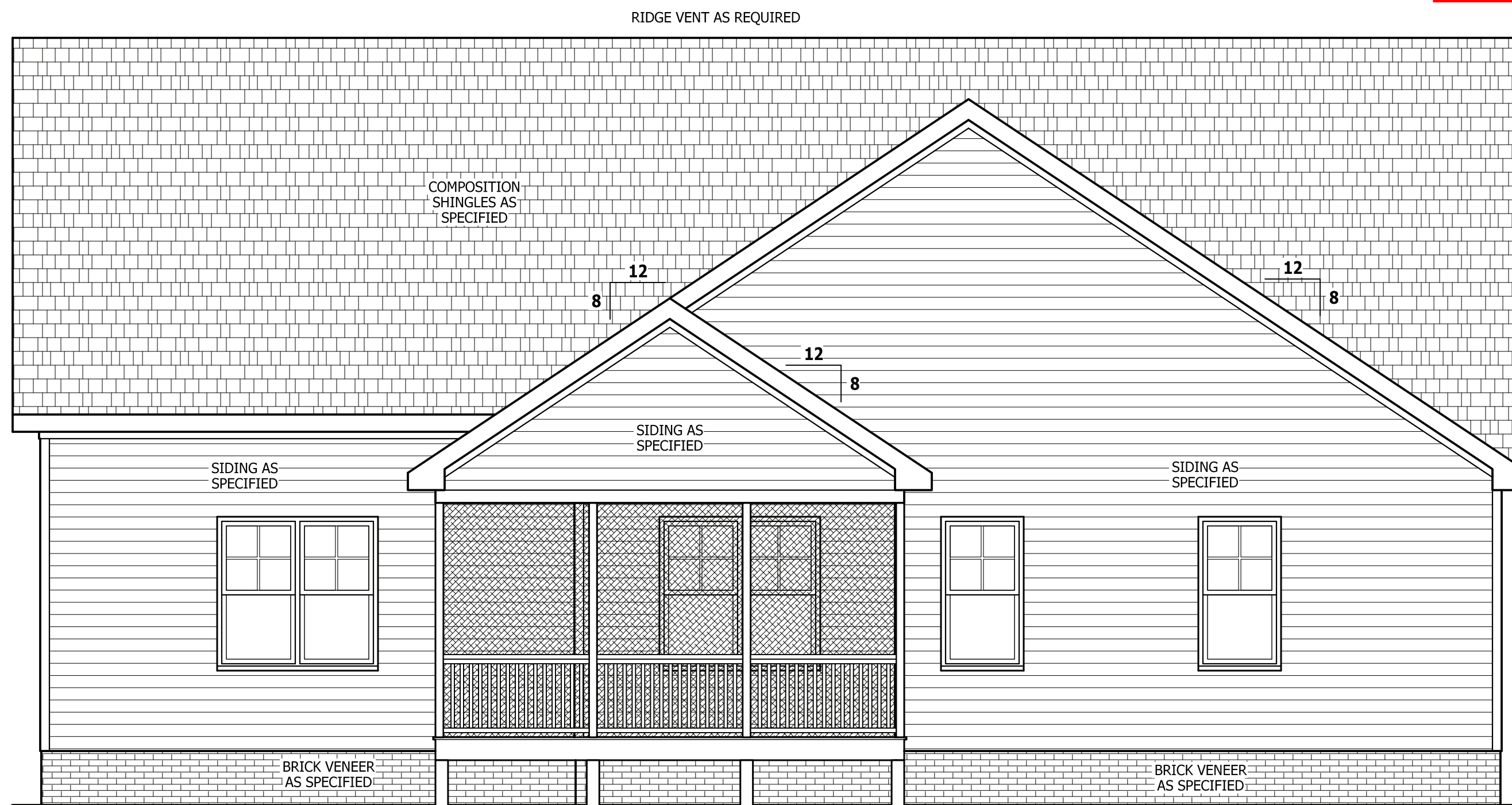
UNHEATED

GARAGE	495 SQ.FT.
FRONT PORCH	119 SQ.FT.
SCREENED PORCH	189 SQ.FT.
DECK	80 SQ.FT.
STORAGE	759 SQ.FT.
TOTAL	1642 SQ.FT.



FRONT ELEVATION

SCALE 1/4" = 1'-0"



REAR ELEVATION

SCALE 1/4" = 1'-0"

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FRONT & REAR ELEVATIONS
Oakridge



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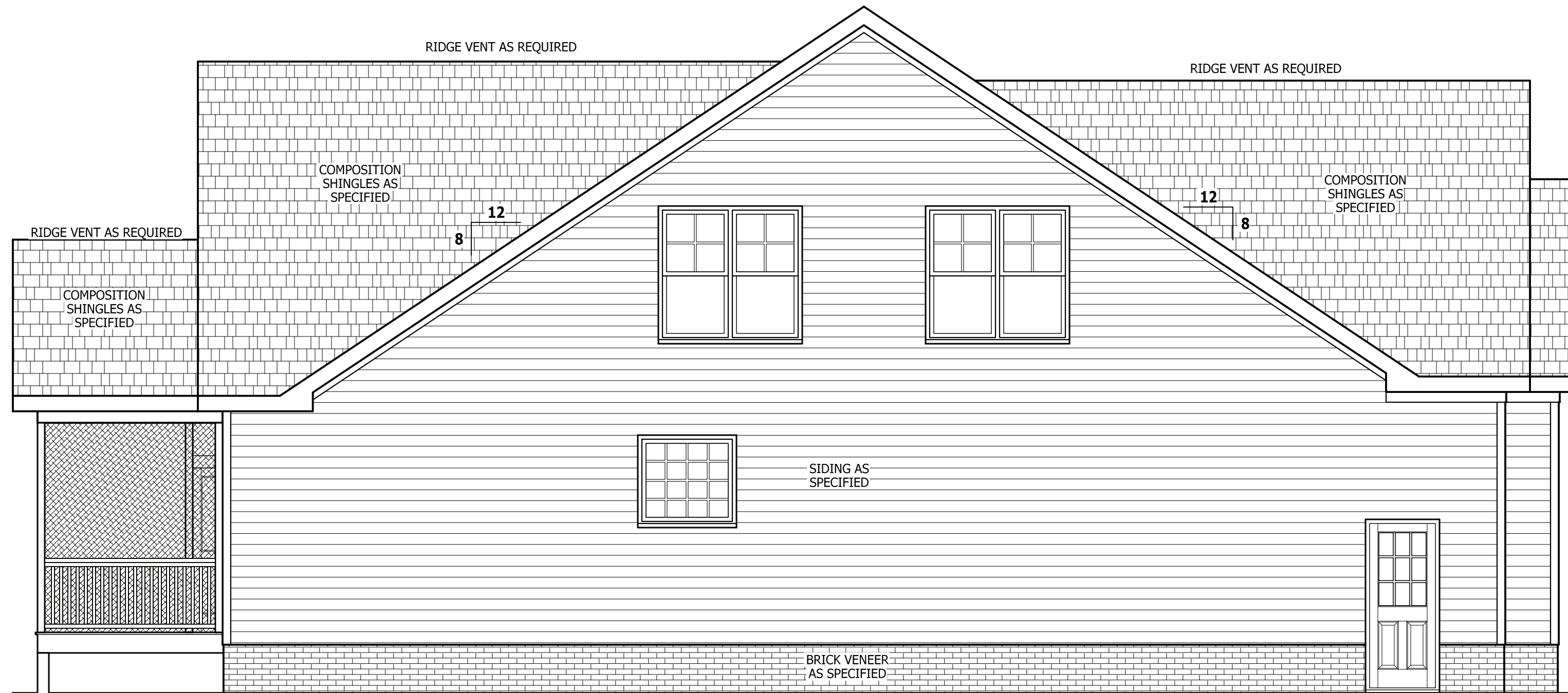
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AIR LEAKAGE

Section N1102.4

N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code:

1. Blocking and sealing floor/ceiling systems and under knee walls open to unconditioned or exterior space.
2. Capping and sealing shafts or chases, including flue shafts.
3. Capping and sealing soffit or dropped ceiling areas.



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LEFT & RIGHT ELEVATIONS
Oakridge



HAYNES HOME PLANS, INC.
P.O. Box 702, Wake Forest, NC 27788 919-435-6180 Fax: 1-866-491-0396

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TOTAL	2626 SQ. FT.
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FOUNDATION PLAN
Oakridge

Glover Design Build
Gloverdesignbuild@gmail.com

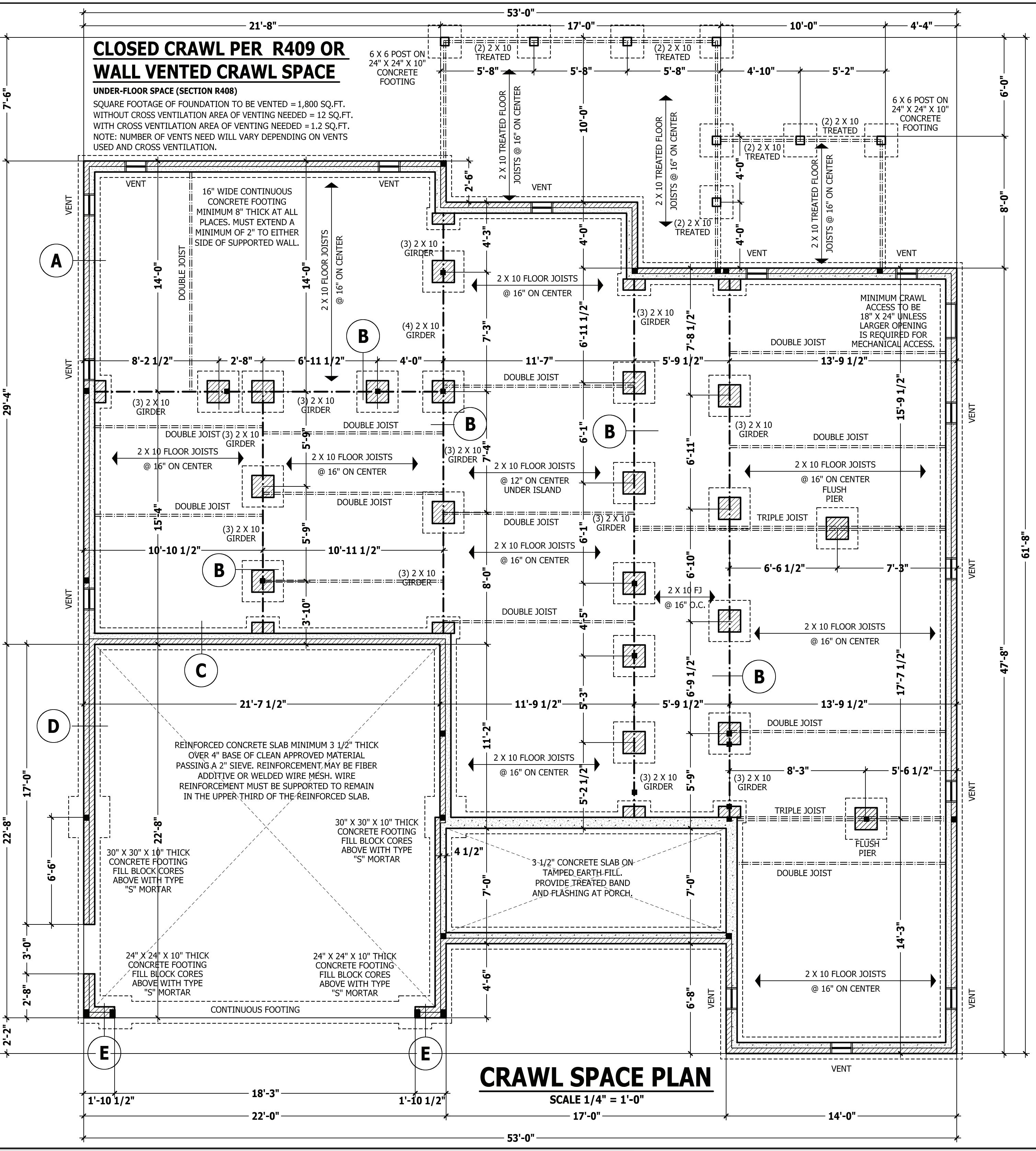
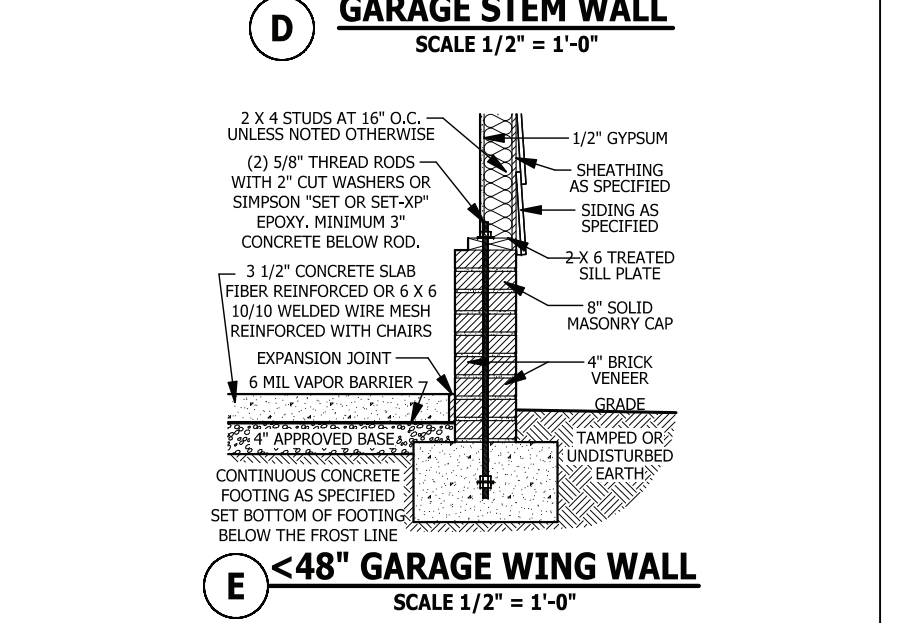
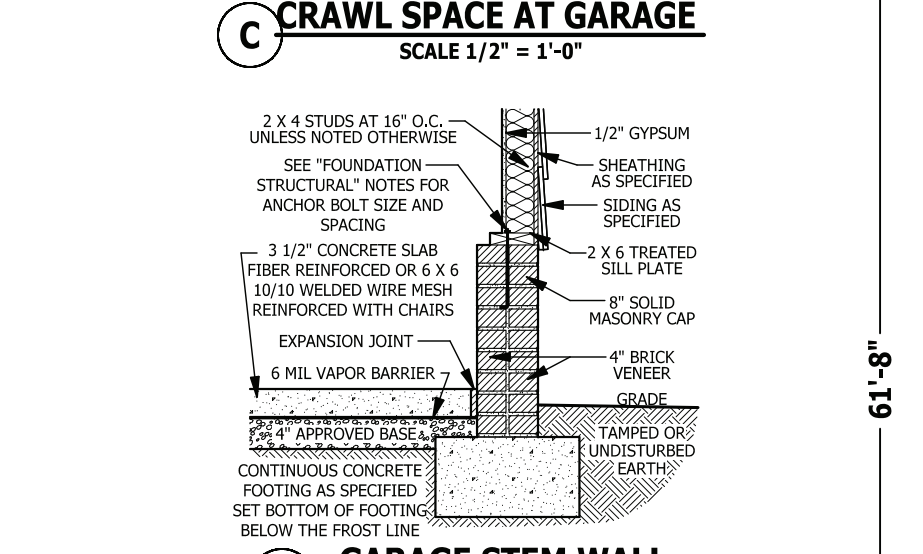
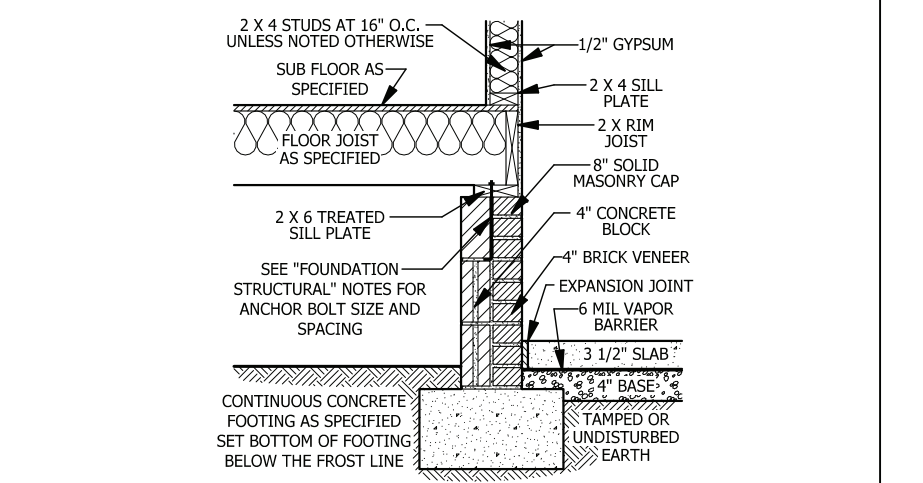
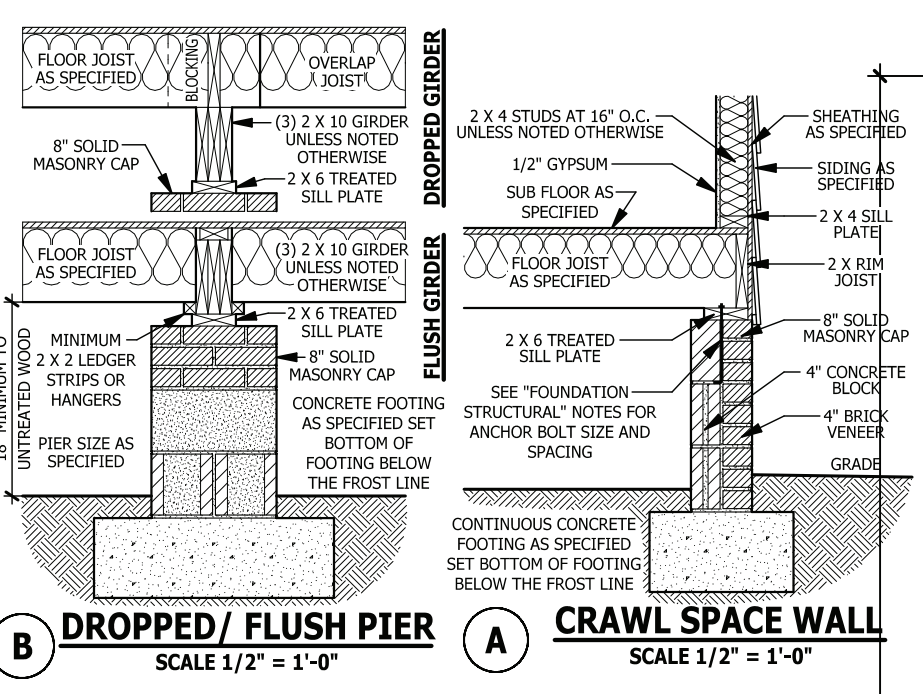
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CLOSED CRAWL PER R409 OR WALL VENTED CRAWL SPACE

UNDER-FLOOR SPACE (SECTION R408)
SQUARE FOOTAGE OF FOUNDATION TO BE VENTED = 1,800 SQ. FT. WITHOUT CROSS VENTILATION AREA OF VENTING NEEDED = 12 SQ. FT. WITH CROSS VENTILATION AREA OF VENTING NEEDED = 1.2 SQ. FT. NOTE: NUMBER OF VENTS NEED WILL VARY DEPENDING ON VENTS USED AND CROSS VENTILATION.



FOUNDATION STRUCTURAL

115 to 130 mph wind zone (1 1/2 to 2 1/2 story)
CONTINUOUS FOOTING: 16" wide and 8" thick minimum. 20" wide minimum at brick veneer. Must extend 2" to either side of supported wall.
GIRDERS: (3) 2 X 10 girder unless noted otherwise.
PIERS: 16" X 16" piers with 8" solid masonry cap on 30" X 30" X 10" concrete footing with maximum pier height of 64" with hollow masonry and 160" with solid masonry.
POINT LOADS: ■ designates significant point load and should have solid blocking to pier, girder or foundation wall.
115 and 120 MPH ANCHORS BOLTS: 1/2" diameter anchor bolts embedded minimum 7", maximum 6'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.
130 MPH ANCHORS BOLTS: 1/2" diameter anchor bolts embedded minimum 15", maximum 4'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.
CONCRETE: Concrete shall have a minimum 28 day strength of 3000 psi and a maximum 5" slump. Air entrained per table 402.2. All concrete shall be in accordance with ACI standards. All samples for pumping shall be taken from the exit end of the pump.
SOILS: Allowable soil bearing pressure assumed to be 2000 PSF. The contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage, and shall be graded so as to drain surface water away from foundation walls.

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FIRST FLOOR PLAN
Oakridge

Glover Design Build
 Gloverdesignbuild@gmail.com

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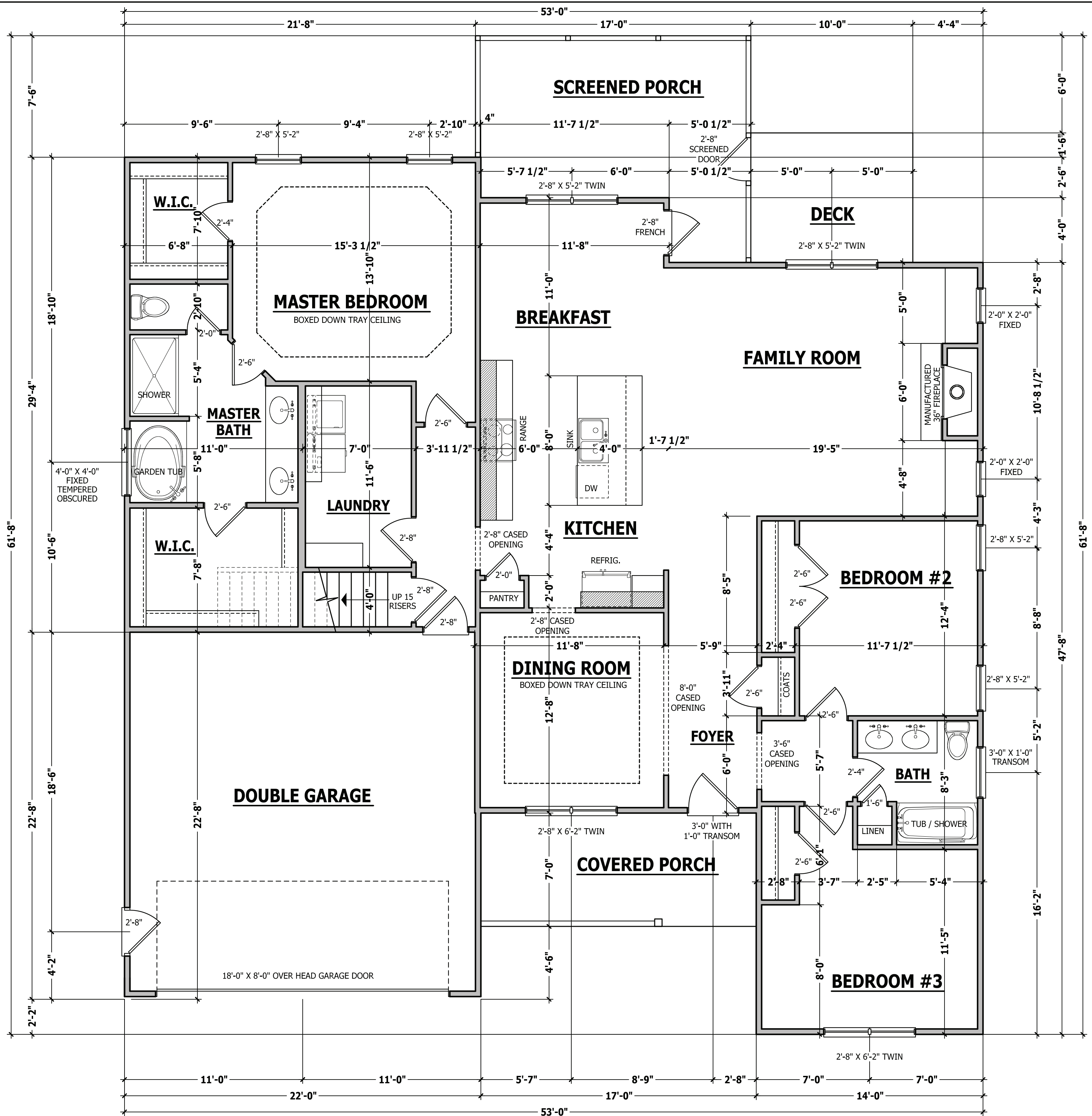
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WALL THICKNESSES

Exterior walls and walls adjacent to a garage are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for stud face.

Interior walls are drawn as 3 1/2" or as noted 2 X 6 are drawn as 5 1/2", and do not include gypsum.

ATTIC ACCESS

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m²) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.
2. Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.

DWELLING / GARAGE SEPARATION

REFER TO SECTIONS R302.5, R302.6, AND R302.7

WALLS. A minimum 1/2" gypsum board must be installed on all walls supporting floor/ceiling assemblies used for separation required by this section.

STAIRS. A minimum of 1/2" gypsum board must be installed on the underside and exposed sides of all stairways.

CEILINGS. A minimum of 1/2" gypsum must be installed on the garage ceiling if there are no habitable room above the garage. If there are habitable room above the garage a minimum of 5/8" type X gypsum board must be installed on the garage ceiling.

OPENING PENETRATIONS. Openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute fire-rated doors.

DUCT PENETRATIONS. Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage.

OTHER PENETRATIONS. Penetrations through the separation required in Section R302.6 shall be protected as required by Section R302.11, Item 4.

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FIRST FLOOR PLAN

SCALE 1/4" = 1'-0"

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION (LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	--	--
Guardrail in-fill components	50	--	--
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	--	L/360
Snow	20	--	--

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted otherwise.

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10⁶ PSI
Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x10⁶ PSI
Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x10⁶ PSI
Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacturer's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc.

LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters.

CONCRETE AND SOILS: See foundation notes.

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

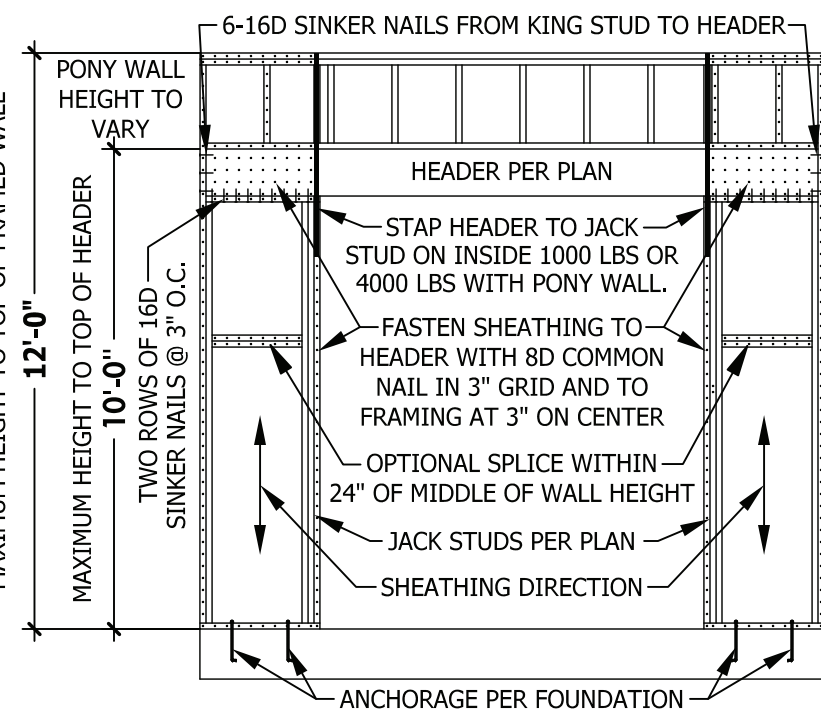
Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter).

CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing nails.

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws.

PF: Portal frame per figure R602.10.1



PF PORTAL FRAME AT OPENING

(METHOD PF PER FIGURE AND SECTION R602.10.1)
SCALE 1/4" = 1'-0"

EXTERIOR HEADERS

(2) 2 X 6 WITH 1 JACK STUD EACH END
UNLESS NOTED OTHERWISE

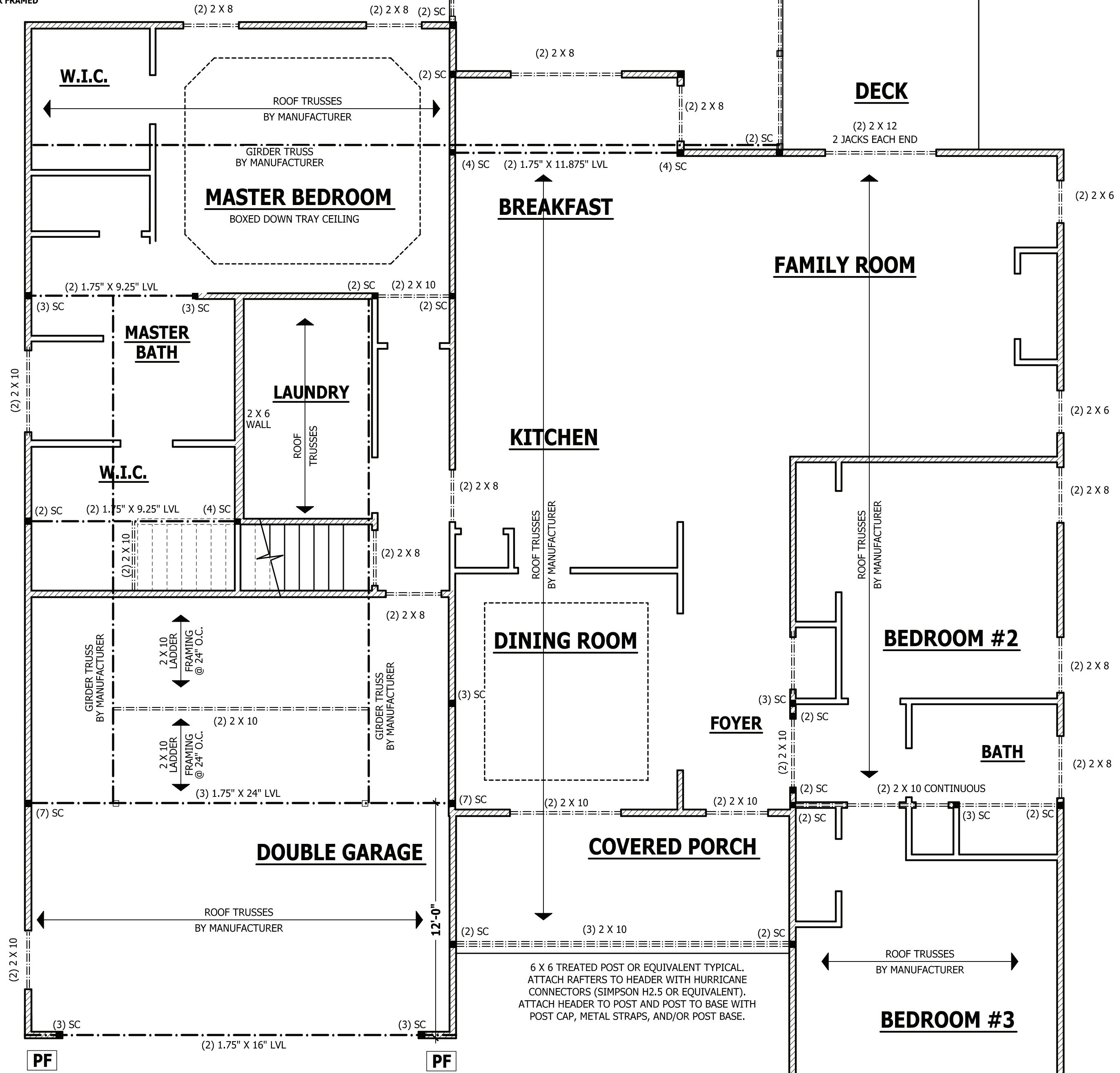
- KING STUDS EACH END PER TABLE BELOW

HEADER SPAN	< 3'	3'-4'	4'-8'	8'-12'	12'-16'
KING STUD(S)	1	2	3	5	6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END
UNLESS NOTED OTHERWISE

- NON LOAD BEARING HEADERS TO BE LADDER FRAMED



FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0"

4 X 4 TREATED POST OR EQUIVALENT TYPICAL. ATTACH RAFTERS TO HEADER WITH HURRICANE CONNECTORS (SIMPSON H2.5 OR EQUIVALENT). ATTACH HEADER TO POST AND POST TO BASE WITH POST CAP, METAL STRAPS, AND/OR POST BASE.

6 X 6 TREATED POST OR EQUIVALENT TYPICAL. ATTACH RAFTERS TO HEADER WITH HURRICANE CONNECTORS (SIMPSON H2.5 OR EQUIVALENT). ATTACH HEADER TO POST AND POST TO BASE WITH POST CAP, METAL STRAPS, AND/OR POST BASE.

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FIRST FLOOR STRUCTURAL

Oakridge

Glover Design Build
Gloverdesignbuild@gmail.com

HAYNES

HOME PLANS, INC.

P.O. Box 702, Wake Forest, NC 27788 919-435-6180 Fax: 1-866-491-0396

SQUARE FOOTAGE	
HEATED	
FIRST FLOOR	1941 SQ. FT.
SECOND FLOOR	685 SQ. FT.
TOTAL	2626 SQ. FT.
UNHEATED	
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DESIGN LOADS	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION (LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	--	--
Guardrail in-fill components	50	--	--
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	--	L/360
Snow	20	--	--

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted otherwise.

ENGINEERED WOOD BEAMS:
 Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10⁶ PSI
 Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x10⁶ PSI
 Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x10⁶ PSI
 Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacturer's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc.

LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters.

CONCRETE AND SOILS: See foundation notes.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE
- KING STUDS EACH END PER TABLE BELOW

HEADER SPAN	< 3'	3'-4'	4'-8'	8'-12'	12'-16'
KING STUD(S)	1	2	3	5	6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE
- NON LOAD BEARING HEADERS TO BE LADDER FRAMED

WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for stud face.

Interior walls are drawn as 3 1/2" or as noted 2 X 6 are drawn as 5 1/2", and do not include gypsum.

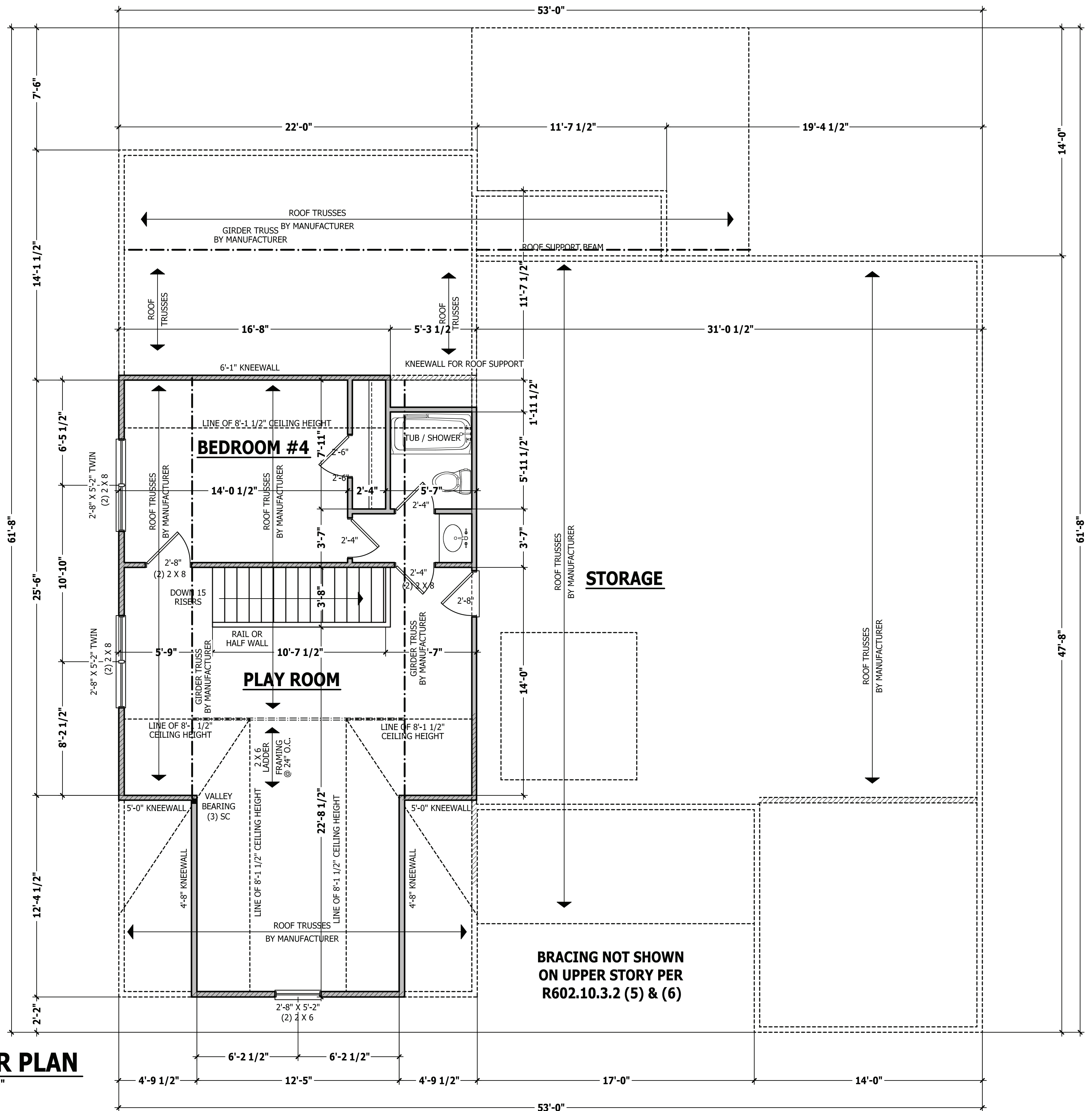
ATTIC ACCESS

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m²) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.
2. Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.



SECOND FLOOR PLAN

SCALE 1/4" = 1'-0"

PURCHASER MUST VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE CONSTRUCTION BEGINS. HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES. CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR ENGINEER SHOULD BE CONSULTED BEFORE CONSTRUCTION. THESE DRAWING ARE INSTRUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER.

SECOND FLOOR PLAN
Oakridge



SQUARE FOOTAGE HEATED	
FIRST FLOOR	1941 SQ. FT.
SECOND FLOOR	685 SQ. FT.
TOTAL	2626 SQ. FT.
UNHEATED	
GARAGE	495 SQ. FT.
FRONT PORCH	119 SQ. FT.
SCREENED PORCH	189 SQ. FT.
DECK	80 SQ. FT.
STORAGE	759 SQ. FT.
TOTAL	1642 SQ. FT.

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BRACING NOT SHOWN ON UPPER STORY PER R602.10.3.2 (5) & (6)

PURCHASER MUST VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE CONSTRUCTION BEGINS. HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES. CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR ENGINEER SHOULD BE CONSULTED BEFORE CONSTRUCTION. THESE DRAWING ARE INSTRUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER.

FIRST FLOOR PLAN
Oakridge


Glover Design Build
Gloverdesignbuild@gmail.com

HAYNES HOME PLANS, INC.
P.O. Box 702, Wake Forest, NC 27788 919-435-6180 Fax 1-866-491-0396

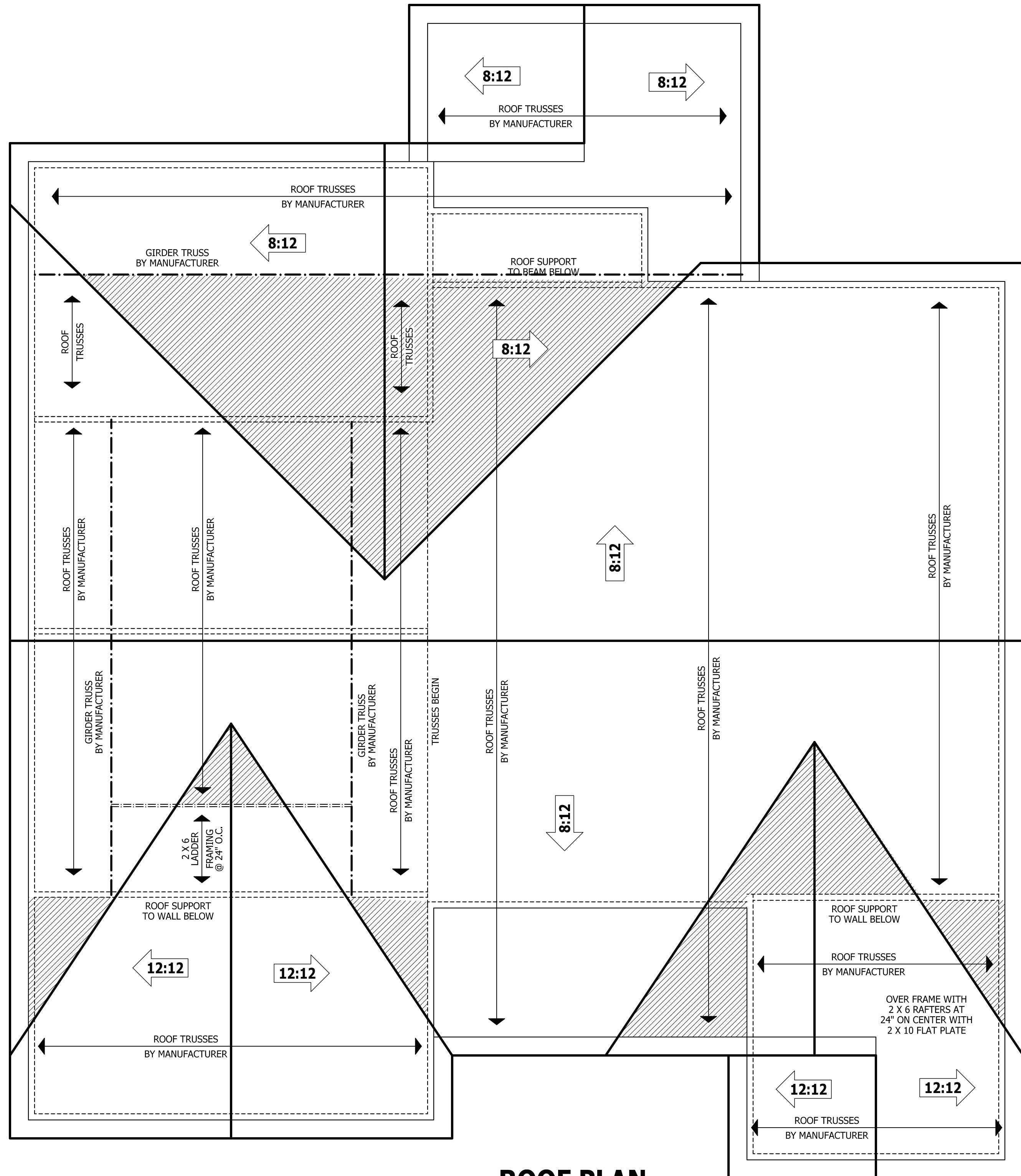
SQUARE FOOTAGE	
HEATED	
FIRST FLOOR	1941 SQ. FT.
SECOND FLOOR	685 SQ. FT.
TOTAL	2626 SQ. FT.
UNHEATED	
GARAGE	495 SQ. FT.
FRONT PORCH	119 SQ. FT.
SCREENED PORCH	189 SQ. FT.
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TOTAL	1642 SQ. FT.

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ROOF TRUSS REQUIREMENTS



TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the responsibility of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics.

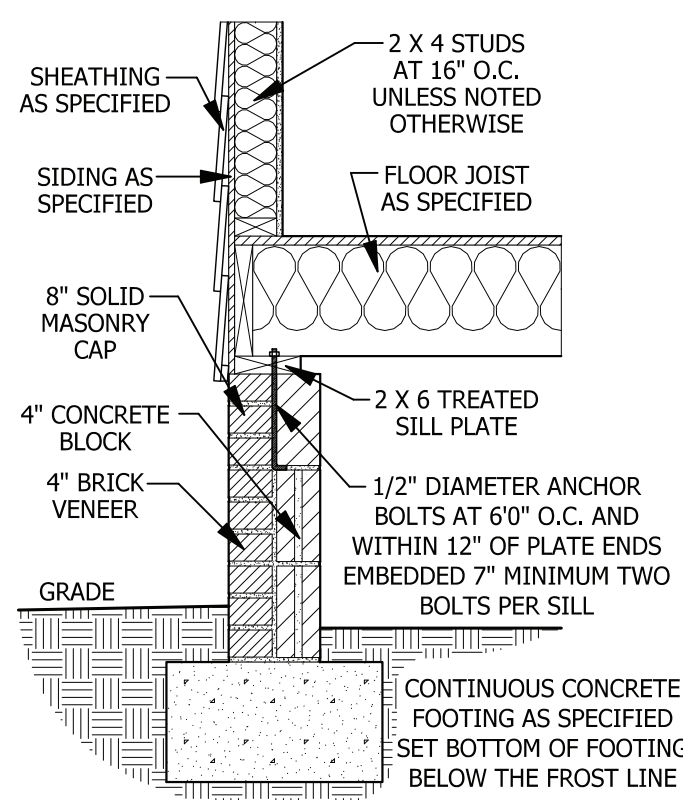
BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

-  HEEL HEIGHT ABOVE FIRST FLOOR PLATE
-  HEEL HEIGHT ABOVE SECOND FLOOR PLATE

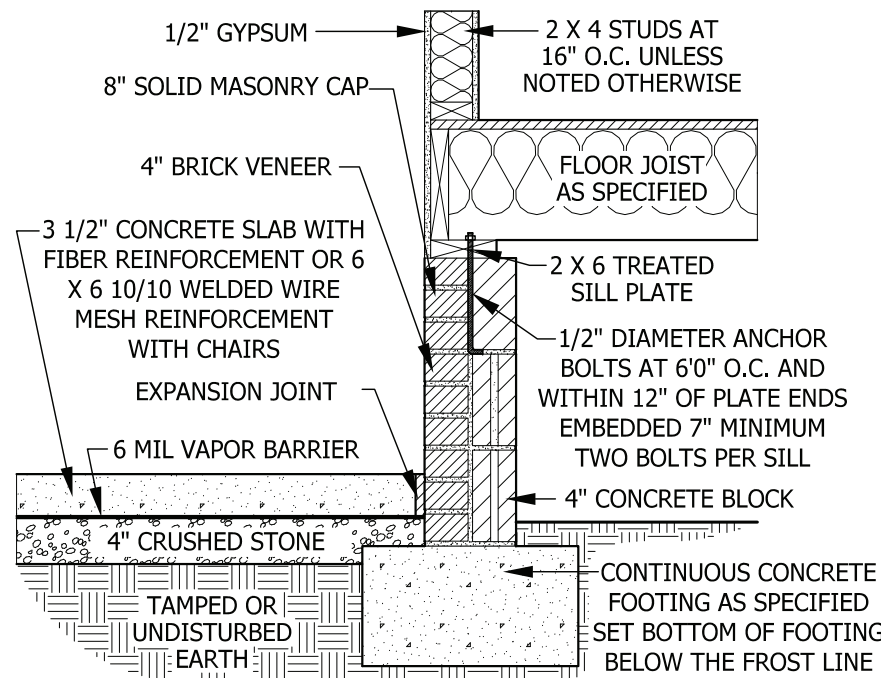
ROOF PLAN

SCALE 1/4" = 1'-0"



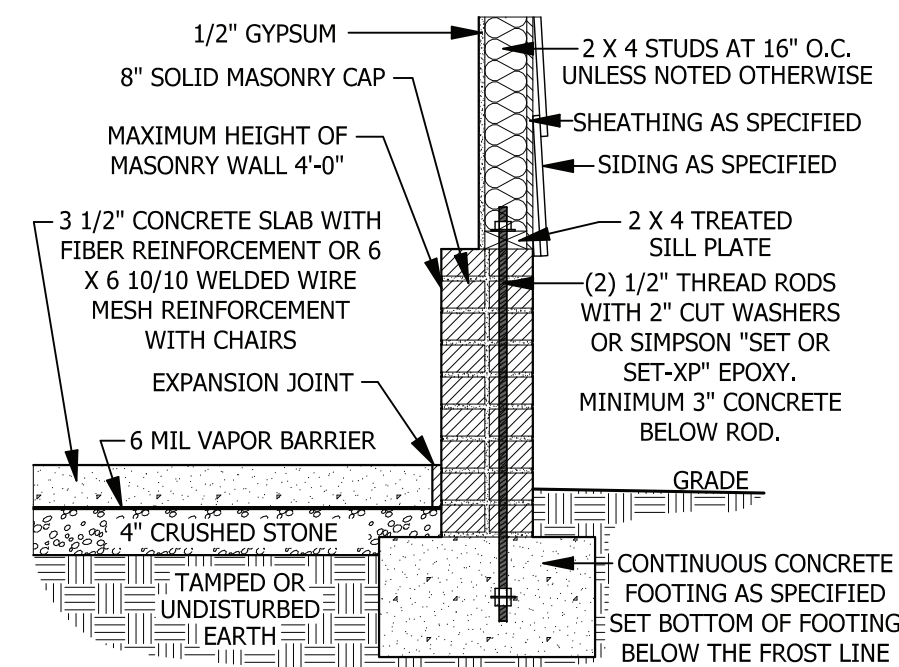
CRAWL SPACE FOUNDATION WALL

A SCALE 3/4" = 1'-0"



CRAWL SPACE FOUNDATION WALL AT GARAGE SLAB

C SCALE 3/4" = 1'-0"



48" OR LESS GARAGE WING WALL

E SCALE 3/4" = 1'-0"

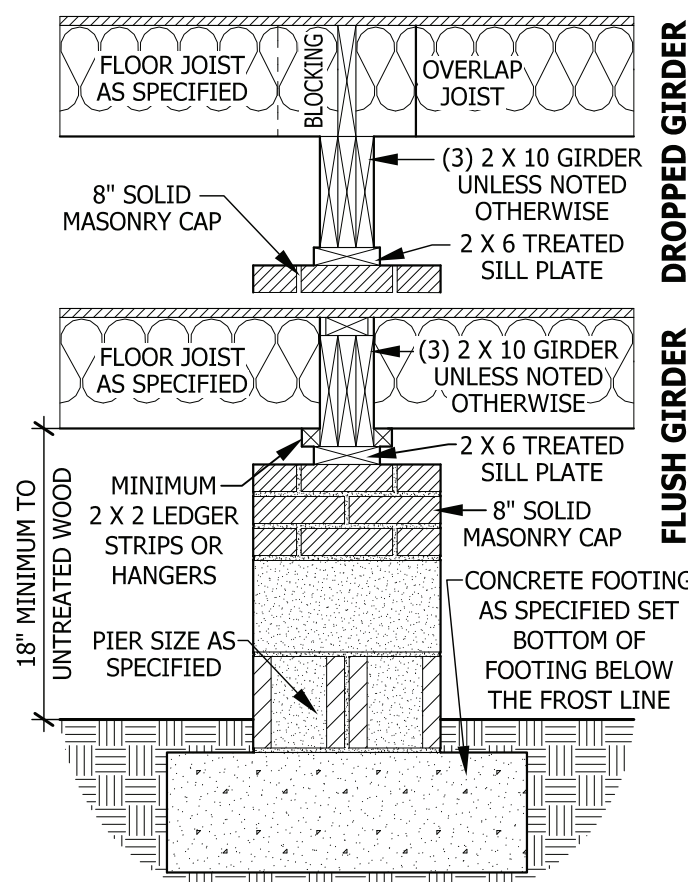
CARBON MONOXIDE ALARMS

SECTION R315

R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed by the alarm manufacturer.

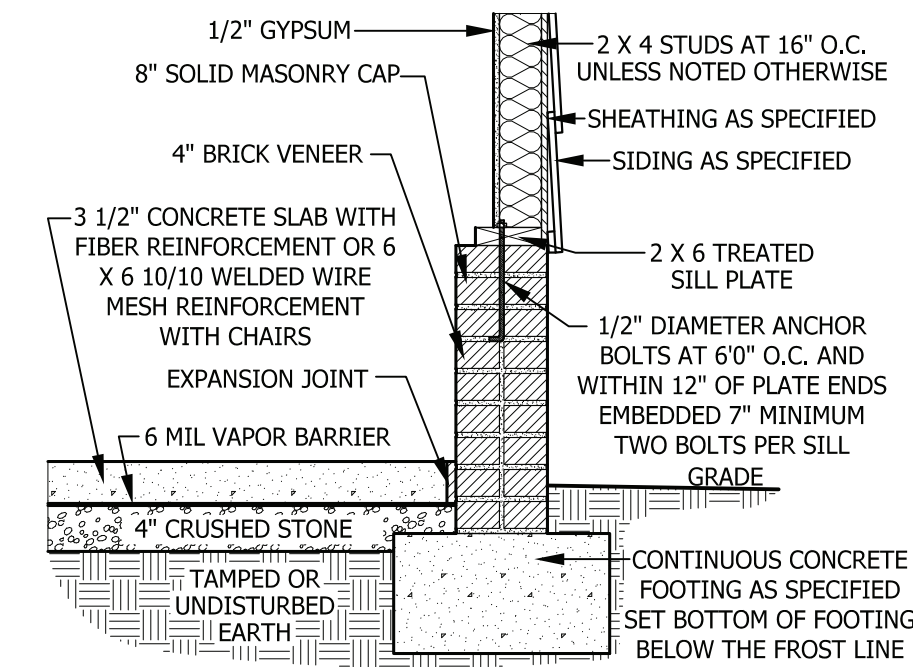
R315.2 Where required in existing dwellings. In existing dwellings, where interior alterations, repairs, fuel-fired appliance replacements, or additions requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section 315.1.

R315.3 Alarm requirements. The required carbon monoxide alarms shall be audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with UL 2034 and shall be installed in accordance with this code and the manufacturer's installation instructions.



DROPPED/ FLUSH PIER

B SCALE 3/4" = 1'-0"



GARAGE FOUNDATION WALL

D SCALE 3/4" = 1'-0"

SMOKE ALARMS

SECTION R314

R314.1 Smoke detection and notification. All smoke alarms shall be listed in accordance with UL 217 and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

R314.2 Smoke detection systems. Household fire alarm systems installed in accordance with NFPA 72 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification device(s), it shall become a permanent fixture of the occupancy and owned by the homeowner. The system shall be monitored by an approved supervising station and be maintained in accordance with NFPA 72.

Exception: Where smoke alarms are provided meeting the requirements of Section R314.4.

R314.3 Location. Smoke alarms shall be installed in the following locations:

1. In each sleeping room.
2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.
3. On each additional story of the dwelling, including basements and habitable attics (finished) but not including crawl spaces, uninhabitable (unfinished) attics and uninhabitable (unfinished) attic-stories. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level.

When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit.

R314.4 Power source. Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commercial source, and when primary power is interrupted, shall receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected.

EXTERIOR WINDOWS AND DOORS

SECTION R612

R612.1 General. This section prescribes performance and construction requirements for exterior windows and doors installed in walls. Windows and doors shall be installed and flashed in accordance with the fenestration manufacturer's written installation instructions. Window and door openings shall be flashed in accordance with Section R703.8. Written installation instructions shall be provided by the fenestration manufacturer for each window or door.

R612.2 Window sills. In dwelling units, where the opening of an operable window is located more than 72 inches (1829 mm) above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches (610 mm) above the finished floor of the room in which the window is located. Operable sections of windows shall not permit openings that allow passage of a 4 inch (102 mm) diameter sphere where such openings are located within 24 inches (610 mm) of the finished floor.

Exceptions:

1. Windows whose openings will not allow a 4-inch diameter (102 mm) sphere to pass through the opening when the opening is in its largest opened position.
2. Openings that are provided with window fall prevention devices that comply with Section R612.3.
3. Openings that are provided with fall prevention devices that comply with ASTM F 2090.
4. Windows that are provided with opening limiting devices that comply with Section R612.4.

R612.3 Window fall prevention devices. Window fall prevention devices and window guards, where provided, shall comply with the requirements of ASTM F 2090.

DWELLING / GARAGE SEPARATION

REFER TO SECTIONS R302.5, R302.6, AND R302.7

WALLS. A minimum 1/2" gypsum board must be installed on all walls supporting floor/ceiling assemblies used for separation required by this section.

STAIRS. A minimum of 1/2" gypsum board must be installed on the underside and exposed sides of all stairways.

CEILING. A minimum of 1/2" gypsum must be installed on the garage ceiling if there are no habitable room above the garage. If there are habitable room above the garage a minimum of 5/8" type X gypsum board must be installed on the garage ceiling.

OPENING PENETRATIONS. Openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute fire-rated doors.

DUCT PENETRATIONS. Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage.

OTHER PENETRATIONS. Penetrations through the separation required in Section R302.6 shall be protected as required by Section R302.11, Item 4.

STAIRWAY NOTES

R311.7

R311.7.2 Headroom. The minimum headroom in all parts of the stairway shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stairway.

R311.7.4 Stair treads and risers. Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners.

R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of the adjacent treads.

R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a minimum tread depth of 4 inches (102 mm) at any point.

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid risers.

R311.7.7 Handrails. Handrails shall be provided on at least one side of each continuous run of treads or flight with four or more risers.

R311.7.7.1 Height. Handrail height, measured vertically from the sloped plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm) and not more than 38 inches (965 mm).

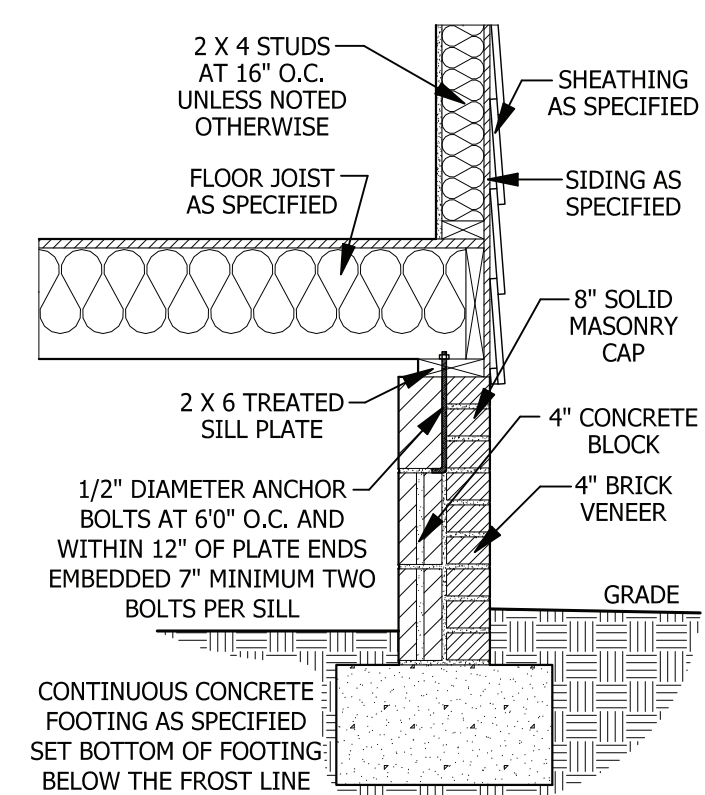
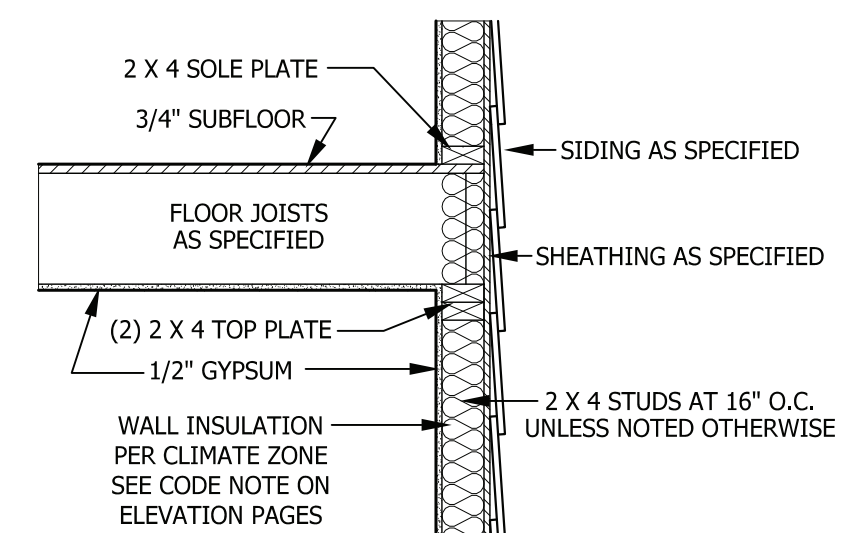
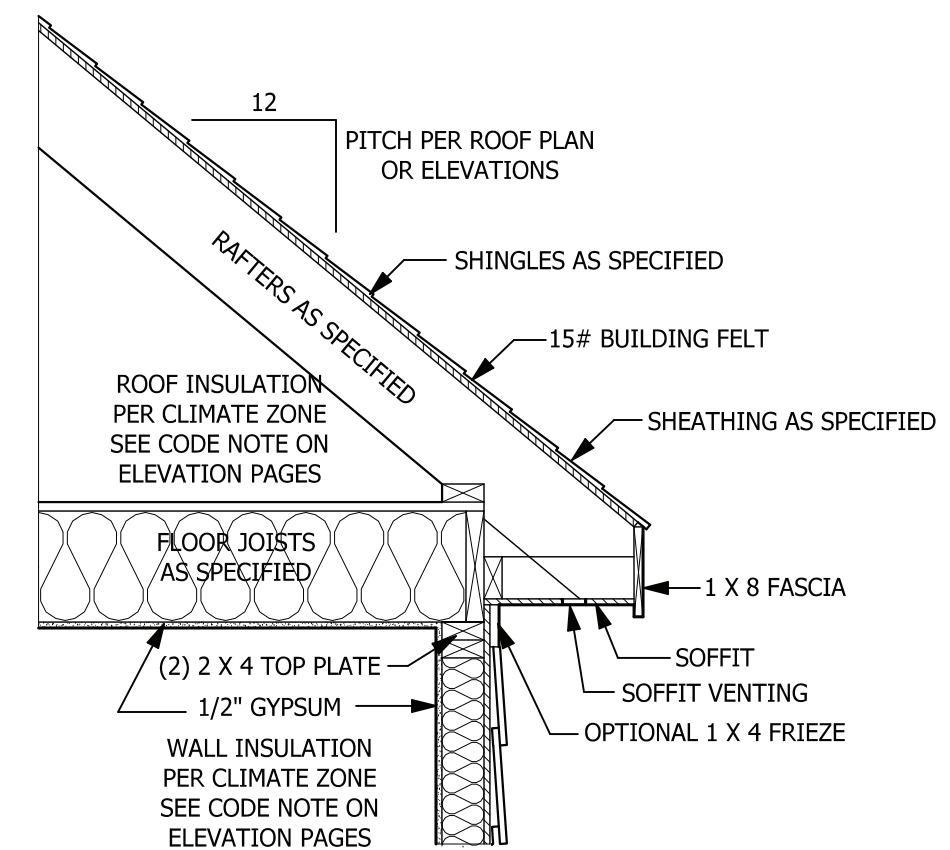
Exceptions:

1. The use of a volute, turnout or starting easing shall be allowed over the lowest tread.
2. When handrail fittings or bendings are used to provide continuous transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall be permitted to exceed the maximum height.

R311.7.7.2 Continuity. Handrails for stairways shall be continuous for the full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails adjacent to a wall shall have a space of not less than 1 1/2 inch (38 mm) between the wall and the handrails.

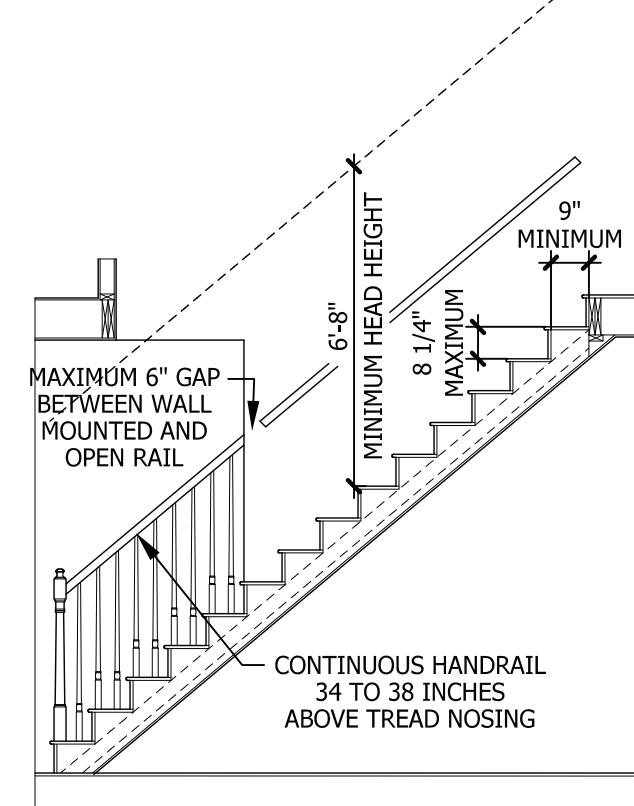
Exceptions:

1. Handrails shall be permitted to be interrupted by a newel post.
2. The use of a volute, turnout, starting easing or starting newel shall be allowed over the lowest tread.
3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.



TYPICAL WALL SECTION

SCALE 3/4" = 1'-0"



TYPICAL STAIR DETAIL

SCALE 1/4" = 1'-0"

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TYPICAL DETAILS
Oakridge

Glover Design Build
Gloverdesignbuild@gmail.com

HAYNES HOME PLANS, INC.
P.O. Box 702, Wake Forest, NC 27788 919-485-6180 Fax 1-866-491-0396

SQUARE FOOTAGE	
HEATED	
FIRST FLOOR	1941 SQ.FT.
SECOND FLOOR	685 SQ.FT.
TOTAL	2626 SQ.FT.
UNHEATED	
GARAGE	495 SQ.FT.
FRONT PORCH	119 SQ.FT.
SCREENED PORCH	189 SQ.FT.
DECK	80 SQ.FT.
STORAGE	759 SQ.FT.
TOTAL	1642 SQ.FT.

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ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature **David Landry**
David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES MODEL: 5/03)

REAR REACTION (UP/TON)	FRONT REACTION (UP/TON)	FRONT REACTION (UP/TON)
1700	2550	3400
2400	3100	4800
3100	3750	6200
3800	4400	7600
4500	5050	9000
5200	5700	10400
5900	6350	11800
6600	7000	13200
7300	7650	14600
8000	8300	16000
8700	8950	17400
9400	9600	18800
10100	10250	20200
10800	10900	21600
11500	11550	23000
12200	12200	24400
12900	12850	25800
13600	13500	27200
14300	14150	28600
15000	14800	30000

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.
○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Walls Shown Are Considered Load Bearing

Roof Area = 3757.45 sq.ft.
Ridge Line = 138.11 ft.
Hip Line = 0 ft.
Horiz. OH = 105.7 ft.
Raked OH = 253.78 ft.
Decking = 129 sheets

Dimension Notes
1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of frame wall unless noted otherwise
3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

Hatch Legend

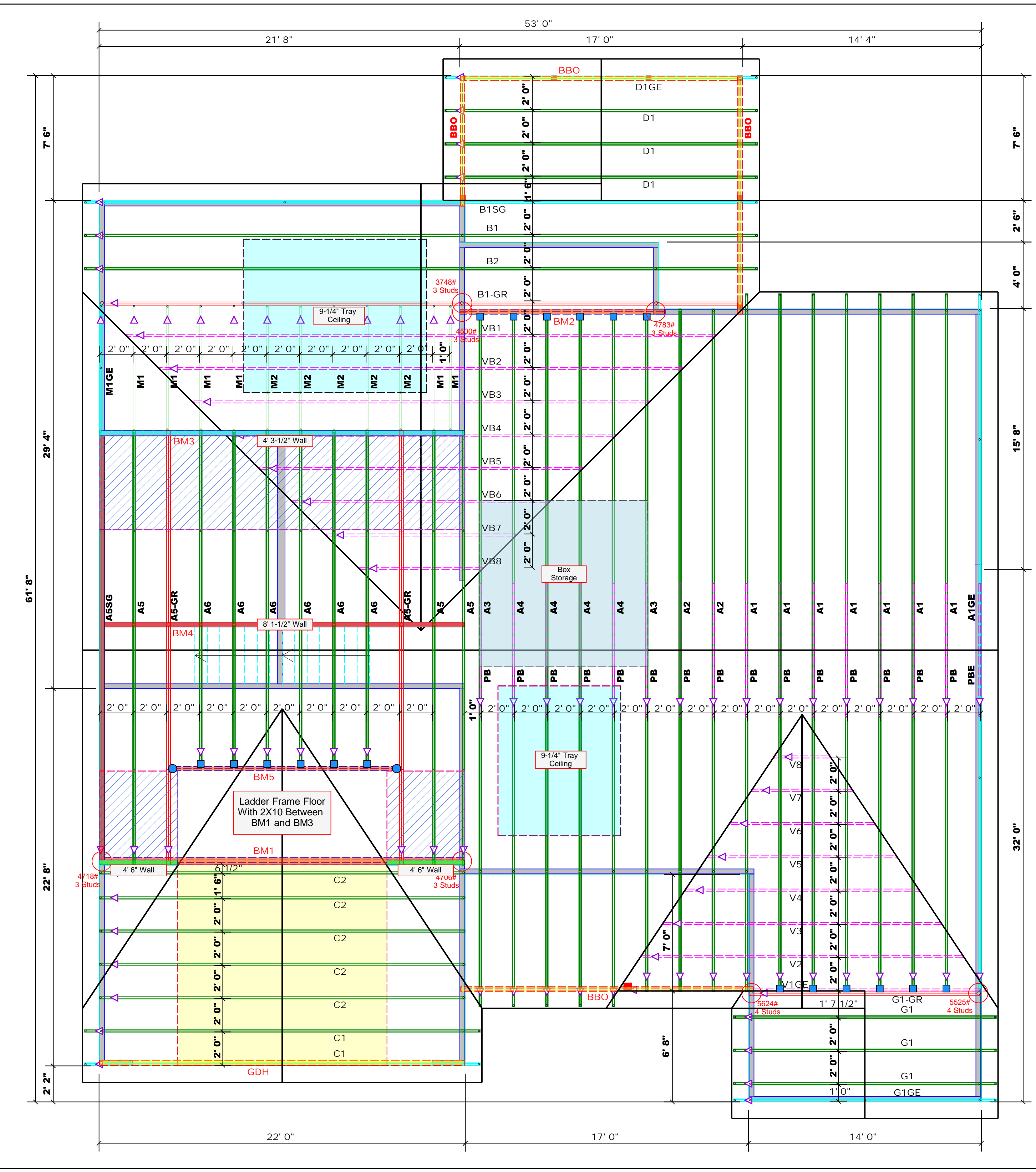
- 4' 3-1/2" Wall
- 4' 6" Walls
- Second Floor Walls
- Box Storage
- Vaulted Ceiling
- Tray Ceiling
- Drop Beam
- Flush Beam

Connector Information				Nail Information		
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
■	HUS26	USP	18	NA	16d/3-1/2"	16d/3-1/2"
●	HUS410	USP	2	NA	16d/3-1/2"	16d/3-1/2"

Products				
PlotID	Length	Product	Plies	Net Qty
BM1	22' 0"	1-3/4"x 23-7/8" LVL Kerto-S	3	3
BM2	13' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
BM3	10' 0"	2x10 SP No.1	2	2
BM4	12' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM5	14' 0"	2x10 SP No.1	2	2
GDH	22' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

1 Truss Placement Plan
Scale: 1/4"=1'

▲ = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards



BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #
Glover Design Build	Lot 8 Purfoy Place	Oakridge	N/A		J1121-6539
CITY / CO.	Fuquay Varina / Harnett	ADDRESS	176 Lambert Lane	MODEL	Roof
		DATE REV.	01/31/22	DRAWN BY	Jonathan Landry
				SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSH-B1 and BCSH-B3 provided with the truss delivery package or online @ sbindustry.com

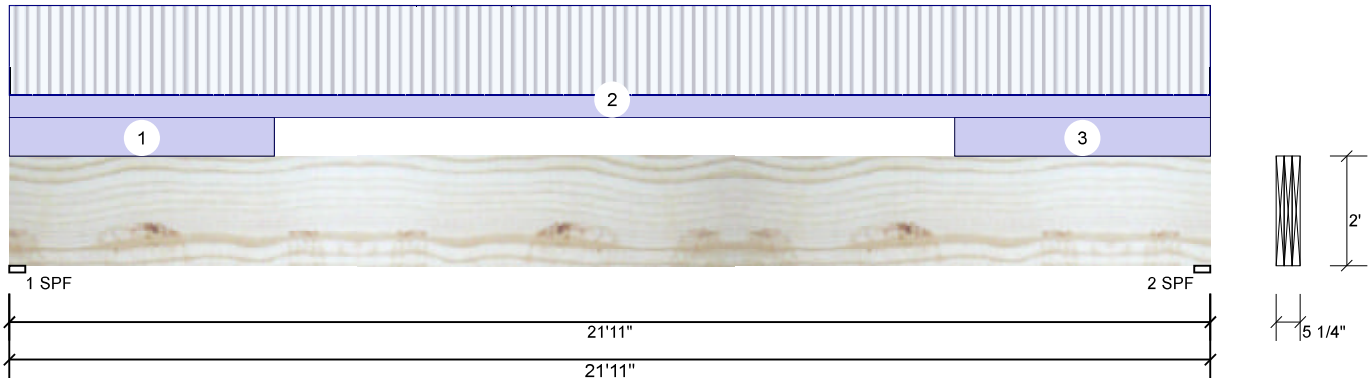


Client: Glover Design Build
 Project: Oakridge
 Address: 176 Lambert Lane
 Fuquay Varina, NC 27526

Date: 1/31/2022
 Input by: Jonathan Landry
 Job Name: Lot 8 Purfoy Place
 Project #: J1121-6539

BM1 Kerto-S LVL 1.750" X 24.000" 3-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	3
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	240
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	Yes
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	3068	1650	0	0	0
2	3068	1638	0	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total Ld.	Case	Comb.
1 - SPF	3.500"	60%	1650 / 3068	4718	L	D+L
2 - SPF	3.500"	60%	1638 / 3068	4706	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	23031 ft-lb	10'11 3/8"	114169 ft-lb	0.202 (20%)	D+L	L
Unbraced	23031 ft-lb	10'11 3/8"	23089 ft-lb	0.998 (100%)	D+L	L
Shear	3613 lb	2'2 5/8"	26880 lb	0.134 (13%)	D+L	L
LL Defl inch	0.126 (L/2052)	10'11 9/16"	0.537 (L/480)	0.230 (23%)	L	L
TL Defl inch	0.181 (L/1427)	10'11 7/16"	1.074 (L/240)	0.170 (17%)	D+L	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top must be laterally braced at a maximum of 11'9" o.c.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 4-10-0		Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
2	Tie-In	0-0-0 to 21-11-0	7-0-0	Top	10 PSF	40 PSF	0 PSF	0 PSF	0 PSF	Floor
3	Part. Uniform Self Weight	17-3-0 to 21-11-0		Top	120 PLF 28 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

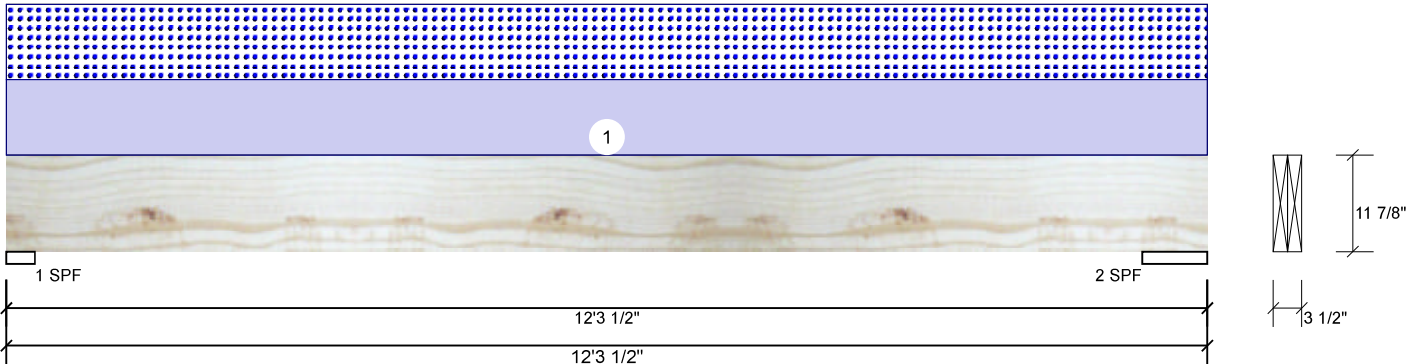
Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



Client: Glover Design Build
 Project: Oakridge
 Address: 176 Lambert Lane
 Fuquay Varina, NC 27526

Date: 1/31/2022
 Input by: Jonathan Landry
 Job Name: Lot 8 Purfoy Place
 Project #: J1121-6539

BM2 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED Level: Level



Member Information

Type: Girder
 Plies: 2
 Moisture Condition: Dry
 Deflection LL: 480
 Deflection TL: 240
 Importance: Normal
 Temperature: Temp <= 100°F

Application: Floor
 Design Method: ASD
 Building Code: IBC/IRC 2015
 Load Sharing: No
 Deck: Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	2277	2222	0	0
2	0	2421	2362	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	86%	2277 / 2222	4500	L	D+S
2 - SPF	8.000"	40%	2421 / 2362	4783	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	12395 ft-lb	5'11 1/2"	22897 ft-lb	0.541 (54%)	D+S	L
Unbraced	12395 ft-lb	5'11 1/2"	12441 ft-lb	0.996 (100%)	D+S	L
Shear	4318 lb	1'2 5/8"	10197 lb	0.423 (42%)	D+S	L
LL Defl inch	0.165 (L/833)	5'11 1/2"	0.286 (L/480)	0.580 (58%)	S	L
TL Defl inch	0.334 (L/411)	5'11 1/2"	0.573 (L/240)	0.580 (58%)	D+S	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top must be laterally braced at a maximum of 6'10 1/2" o.c.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Near Face	373 PLF	0 PLF	373 PLF	0 PLF	0 PLF	A4
	Self Weight				9 PLF					

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

- Handling & Installation**
- LVL beams must not be cut or drilled
 - Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 - Damaged Beams must not be used
 - Design assumes top edge is laterally restrained
 - Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
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 28314
 910-864-TRUS



This design is valid until 4/24/2023

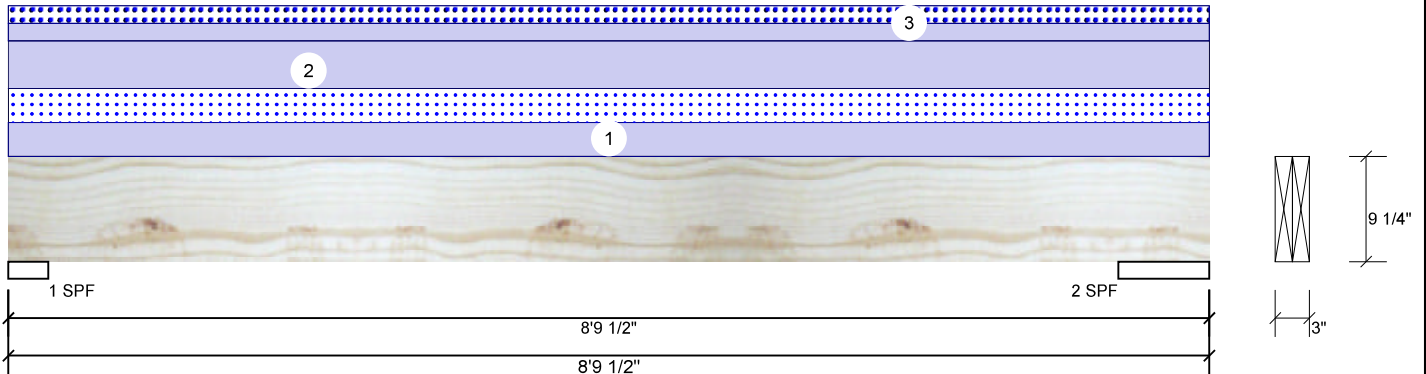


Client: Glover Design Build
 Project: Oakridge
 Address: 176 Lambert Lane
 Fuquay Varina, NC 27526

Date: 1/31/2022
 Input by: Jonathan Landry
 Job Name: Lot 8 Purfoy Place
 Project #: J1121-6539

BM3 S-P-F #1 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
 Plies: 2
 Moisture Condition: Dry
 Deflection LL: 480
 Deflection TL: 240
 Importance: Normal
 Temperature: Temp <= 100°F

Application: Floor
 Design Method: ASD
 Building Code: IBC/IRC 2015
 Load Sharing: No
 Deck: Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	1048	543	0	0
2	0	1141	591	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	36%	1048 / 543	1591	L	D+S
2 - SPF	8.000"	17%	1141 / 591	1733	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2993 ft-lb	4'2 1/2"	3946 ft-lb	0.758 (76%)	D+S	L
Unbraced	2993 ft-lb	4'2 1/2"	3261 ft-lb	0.918 (92%)	D+S	L
Shear	1344 lb	1'	2872 lb	0.468 (47%)	D+S	L
LL Defl inch	0.042 (L/2272)	4'2 9/16"	0.199 (L/480)	0.210 (21%)	S	L
TL Defl inch	0.123 (L/775)	4'2 9/16"	0.398 (L/240)	0.310 (31%)	D+S	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	85 PLF	0 PLF	85 PLF	0 PLF	0 PLF	M1
2	Uniform			Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Uniform			Top	44 PLF	0 PLF	44 PLF	0 PLF	0 PLF	A6-GR

Manufacturer Info

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
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 910-864-TRUS

This design is valid until 4/24/2023

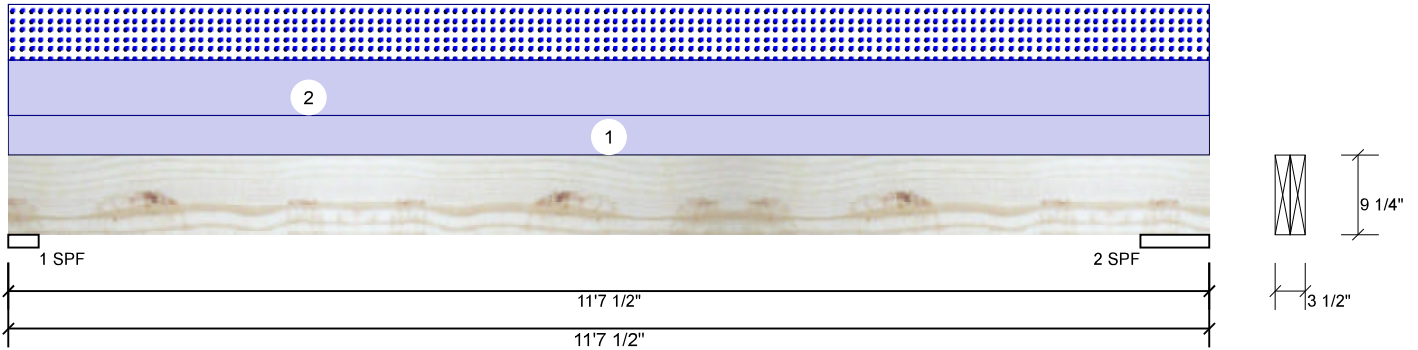


Client: Glover Design Build
 Project: Oakridge
 Address: 176 Lambert Lane
 Fuquay Varina, NC 27526

Date: 1/31/2022
 Input by: Jonathan Landry
 Job Name: Lot 8 Purfoy Place
 Project #: J1121-6539

BM4 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	240
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	1666	951	0	0
2	0	1777	1014	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	50%	1666 / 951	2617	L	D+S
2 - SPF	8.000"	23%	1777 / 1014	2791	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6772 ft-lb	5'7 1/2"	14423 ft-lb	0.470 (47%)	D+S	L
Unbraced	6772 ft-lb	5'7 1/2"	6979 ft-lb	0.970 (97%)	D+S	L
Shear	2152 lb	1'	7943 lb	0.271 (27%)	D+S	L
LL Defl inch	0.120 (L/1075)	5'7 1/2"	0.270 (L/480)	0.450 (45%)	S	L
TL Defl inch	0.332 (L/391)	5'7 1/2"	0.540 (L/240)	0.610 (61%)	D+S	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
2	Uniform			Top	169 PLF	0 PLF	169 PLF	0 PLF	0 PLF	A6-GR
	Self Weight				7 PLF					

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

- Handling & Installation**
- LVL beams must not be cut or drilled
 - Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 - Damaged Beams must not be used
 - Design assumes top edge is laterally restrained
 - Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

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 1001 S. Reilly Road, Suite #639
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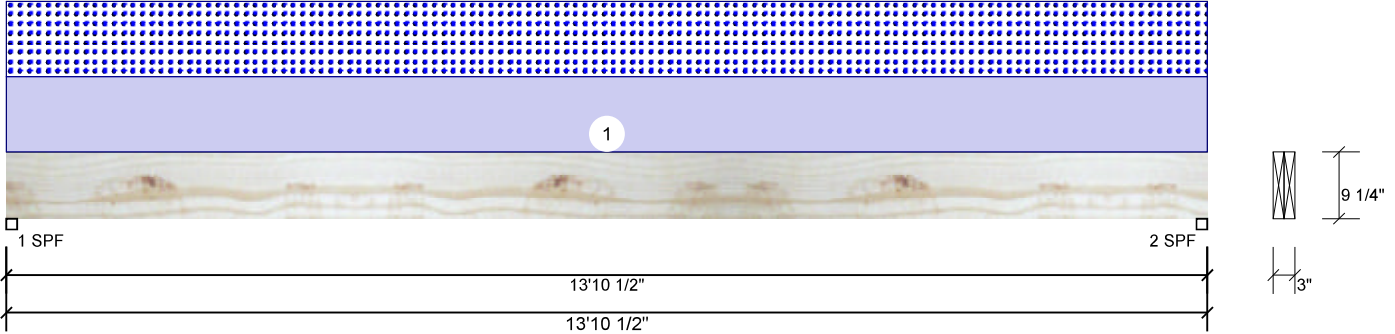


Client: Glover Design Build
 Project: Oakridge
 Address: 176 Lambert Lane
 Fuquay Varina, NC 27526

Date: 1/31/2022
 Input by: Jonathan Landry
 Job Name: Lot 8 Purfoy Place
 Project #: J1121-6539

BM5 S-P-F #1 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	240
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	312	312	0	0
2	0	312	312	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	1.500"	33%	312 / 312	624	L	D+S
2 - SPF	1.500"	33%	312 / 312	624	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2127 ft-lb	6'11 1/4"	3946 ft-lb	0.539 (54%)	D+S	L
Unbraced	2127 ft-lb	6'11 1/4"	2282 ft-lb	0.932 (93%)	D+S	L
Shear	549 lb	10"	2872 lb	0.191 (19%)	D+S	L
LL Defl inch	0.131 (L/1263)	6'11 5/16"	0.344 (L/480)	0.380 (38%)	S	L
TL Defl inch	0.261 (L/631)	6'11 5/16"	0.688 (L/240)	0.380 (38%)	D+S	L

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	45 PLF	0 PLF	45 PLF	0 PLF	0 PLF	A7

Manufacturer Info

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



This design is valid until 4/24/2023

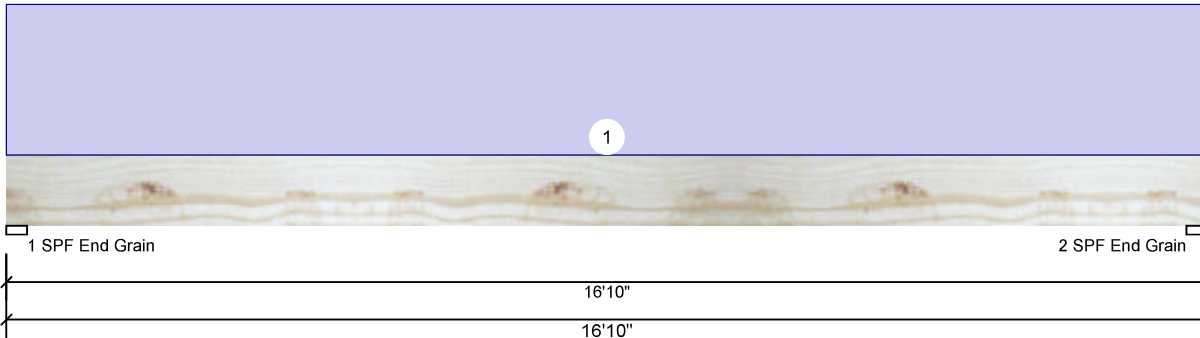


Client: Glover Design Build
 Project: Oakridge
 Address: 176 Lambert Lane
 Fuquay Varina, NC 27526

Date: 1/31/2022
 Input by: Jonathan Landry
 Job Name: Lot 8 Purfoy Place
 Project #: J1121-6539

GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	240
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	1719	0	0	0
2	0	1719	0	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	16%	1719 / 0	1719	Uniform	D
2 - SPF End Grain	3.500"	16%	1719 / 0	1719	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6845 ft-lb	8'5"	17919 ft-lb	0.382 (38%)	D	Uniform
Unbraced	6845 ft-lb	8'5"	6846 ft-lb	1.000 (100%)	D	Uniform
Shear	1470 lb	15'7 3/8"	7980 lb	0.184 (18%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.357 (L/550)	8'5 1/16"	0.819 (L/240)	0.440 (44%)	D	Uniform

Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top must be laterally braced at a maximum of 14'4 1/2" o.c.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	195 PLF	0 PLF	0 PLF	0 PLF	0 PLF	C1GE
	Self Weight				9 PLF					

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

- Handling & Installation**
- LVL beams must not be cut or drilled
 - Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 - Damaged Beams must not be used
 - Design assumes top edge is laterally restrained
 - Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Manufacturer Info

Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



RE: J1121-6539
Lot 8 Purfoy Place

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

Customer: Glover Design Build Project Name: J1121-6539
Lot/Block: 8 Model: Oakridge
Address: 176 Lambert Lane Subdivision: Purfoy Place
City: Fuquay Varina State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 41 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I49951722	A1	1/30/2022	21	I49951742	M1	1/30/2022
2	I49951723	A1GE	1/30/2022	22	I49951743	M1GE	1/30/2022
3	I49951724	A2	1/30/2022	23	I49951744	M2	1/30/2022
4	I49951725	A3	1/30/2022	24	I49951745	PB	1/30/2022
5	I49951726	A4	1/30/2022	25	I49951746	PBE	1/30/2022
6	I49951727	A5	1/30/2022	26	I49951747	V1GE	1/30/2022
7	I49951728	A5-GR	1/30/2022	27	I49951748	V2	1/30/2022
8	I49951729	A5SG	1/30/2022	28	I49951749	V3	1/30/2022
9	I49951730	A6	1/30/2022	29	I49951750	V4	1/30/2022
10	I49951731	B1	1/30/2022	30	I49951751	V5	1/30/2022
11	I49951732	B1-GR	1/30/2022	31	I49951752	V6	1/30/2022
12	I49951733	B1SG	1/30/2022	32	I49951753	V7	1/30/2022
13	I49951734	B2	1/30/2022	33	I49951754	V8	1/30/2022
14	I49951735	C1	1/30/2022	34	I49951755	VB1	1/30/2022
15	I49951736	C2	1/30/2022	35	I49951756	VB2	1/30/2022
16	I49951737	D1	1/30/2022	36	I49951757	VB3	1/30/2022
17	I49951738	D1GE	1/30/2022	37	I49951758	VB4	1/30/2022
18	I49951739	G1	1/30/2022	38	I49951759	VB5	1/30/2022
19	I49951740	G1-GR	1/30/2022	39	I49951760	VB6	1/30/2022
20	I49951741	G1GE	1/30/2022	40	I49951761	VB7	1/30/2022

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



January 30, 2022



RE: J1121-6539 - Lot 8 Purfoy Place

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

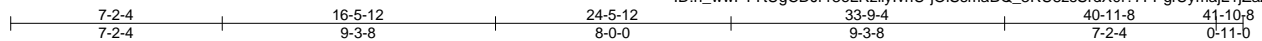
Project Customer: Glover Design Build Project Name: J1121-6539
Lot/Block: 8 Subdivision: Purfoy Place
Address: 176 Lambert Lane
City, County: Fuquay Varina State: NC

No.	Seal#	Truss Name	Date
41	I49951762	VB8	1/30/2022

Job J1121-6539	Truss A1	Truss Type PIGGYBACK BASE	Qty 7	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951722
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:40 2022 Page 1
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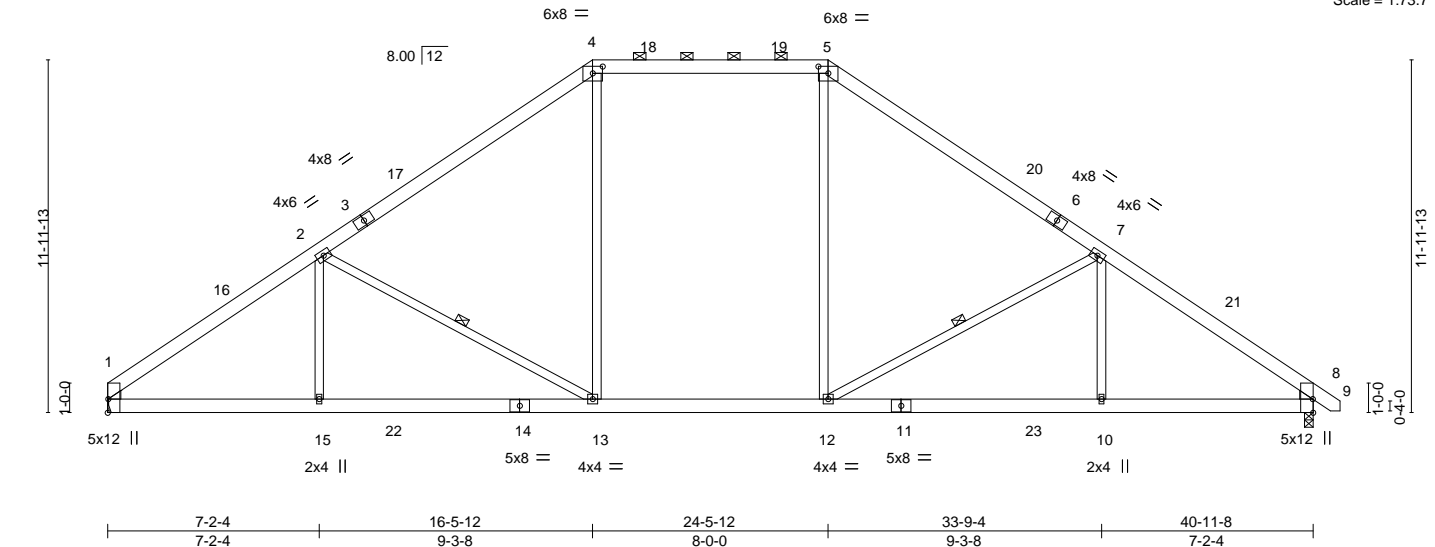


Plate Offsets (X,Y)-- [1:Edge,0-0-3], [4:0-4-0,0-2-13], [5:0-4-0,0-2-13], [8:Edge,0-0-3]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.52	Vert(LL)	-0.35	13-15	>999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.60	Vert(CT)	-0.44	13-15	>999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.39	Horz(CT)	0.07	8	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.30	13-15	>999	Weight: 290 lb	FT = 20%

LUMBER-
 TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.2
 WEDGE
 Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 4-8-9 oc purlins, except 2-0-0 oc purlins (5-10-12 max.): 4-5.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 2-13, 7-12

REACTIONS. (size) 1=Mechanical, 8=0-3-8
 Max Horz 1=276(LC 10)
 Max Uplift 1=68(LC 12), 8=80(LC 13)
 Max Grav 1=1742(LC 19), 8=1793(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-2651/532, 2-4=-2033/549, 4-5=-1583/537, 5-7=-2032/545, 7-8=-2656/530
 BOT CHORD 1-15=-341/2270, 13-15=-341/2270, 12-13=-92/1615, 10-12=-327/2040, 8-10=-327/2040
 WEBS 2-15=0/367, 2-13=-760/286, 4-13=-42/675, 5-12=-37/671, 7-12=-734/279, 7-10=0/361

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-5-12, Exterior(2) 16-5-12 to 22-8-7, Interior(1) 22-8-7 to 24-5-12, Exterior(2) 24-5-12 to 30-8-7, Interior(1) 30-8-7 to 41-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 68 lb uplift at joint 1 and 80 lb uplift at joint 8.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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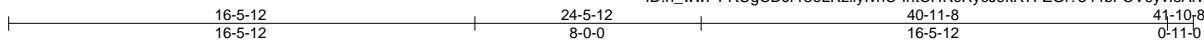


818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss A1GE	Truss Type PIGGYBACK BASE SUPPO	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951723
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:42 2022 Page 1



Scale = 1:76.7

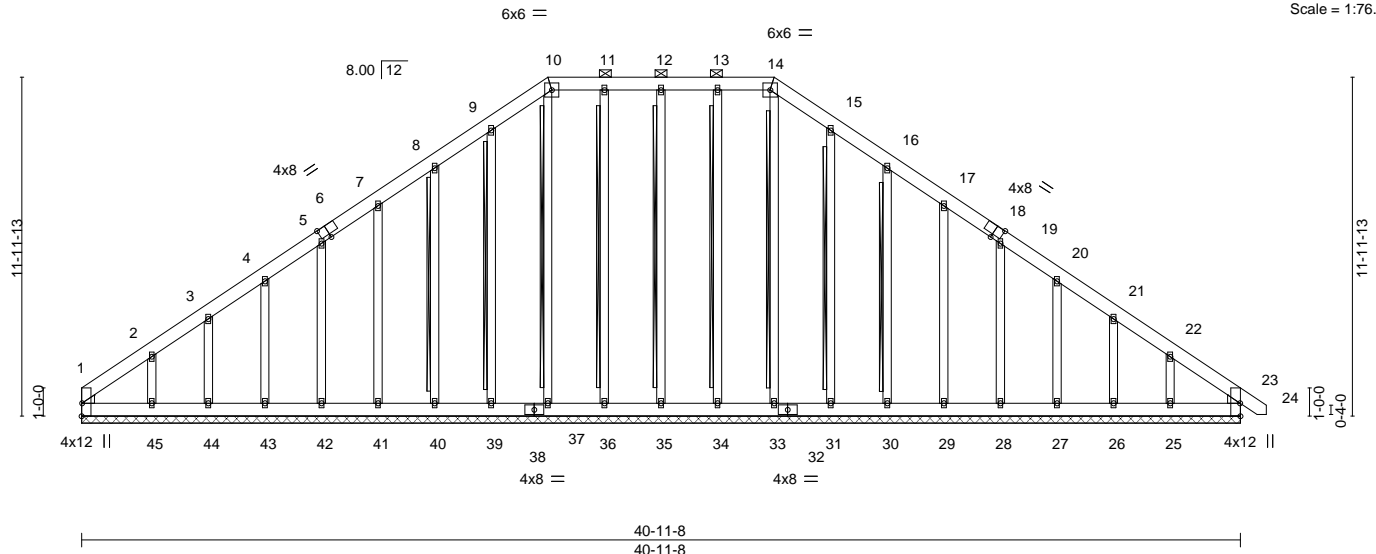


Plate Offsets (X,Y)-- [1:Edge,0-0-3], [6:0-3-11,Edge], [18:0-3-11,Edge], [23:Edge,0-0-3]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.04	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.02	Vert(LL) 0.00 23 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.14	Vert(CT) 0.00 23 n/r 120		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 23 n/a n/a		
	Code IRC2015/TPI2014			Weight: 414 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
OTHERS 2x4 SP No.2
WEDGE
Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 10-14.
Rigid ceiling directly applied or 10-0-0 oc bracing.
BOT CHORD
WEBS T-Brace: 2x4 SPF No.2 - 14-33, 13-34, 12-35, 11-36, 10-37, 9-39, 8-40, 15-31, 16-30
Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.
Brace must cover 90% of web length.

REACTIONS. All bearings 40-11-8.
(lb) - Max Horz 1=-276(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 34, 35, 36, 39, 40, 41, 42, 43, 44, 31, 30, 29, 28, 27, 26, 25, 23 except 45=-103(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 1, 33, 34, 35, 36, 37, 39, 40, 41, 42, 43, 44, 45, 31, 30, 29, 28, 27, 26, 25, 23

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-284/251, 8-9=-249/280, 9-10=-298/338, 10-11=-269/314, 11-12=-269/314, 12-13=-269/314, 13-14=-269/314, 14-15=-298/338, 15-16=-249/280

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) 0-0-0 to 4-5-12, Exterior(2) 4-5-12 to 16-6-9, Corner(3) 16-6-9 to 20-11-6, Exterior(2) 20-11-6 to 24-4-15, Corner(3) 24-4-15 to 28-9-11, Exterior(2) 28-9-11 to 41-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 34, 35, 36, 39, 40, 41, 42, 43, 44, 31, 30, 29, 28, 27, 26, 25, 23 except (jt=lb) 45=103.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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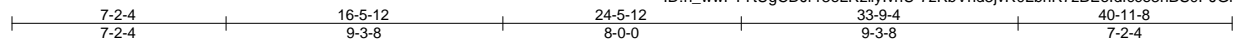


818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss A2	Truss Type PIGGYBACK BASE	Qty 2	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	149951724
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Comtech, Inc. Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:43 2022 Page 1
ID:h_wwFTTtUgCDJr13eLRzilylvhU-7zRbVnd3jvR0Lbhr7zBEldfccc3hBSOPJGN9vzqtFY



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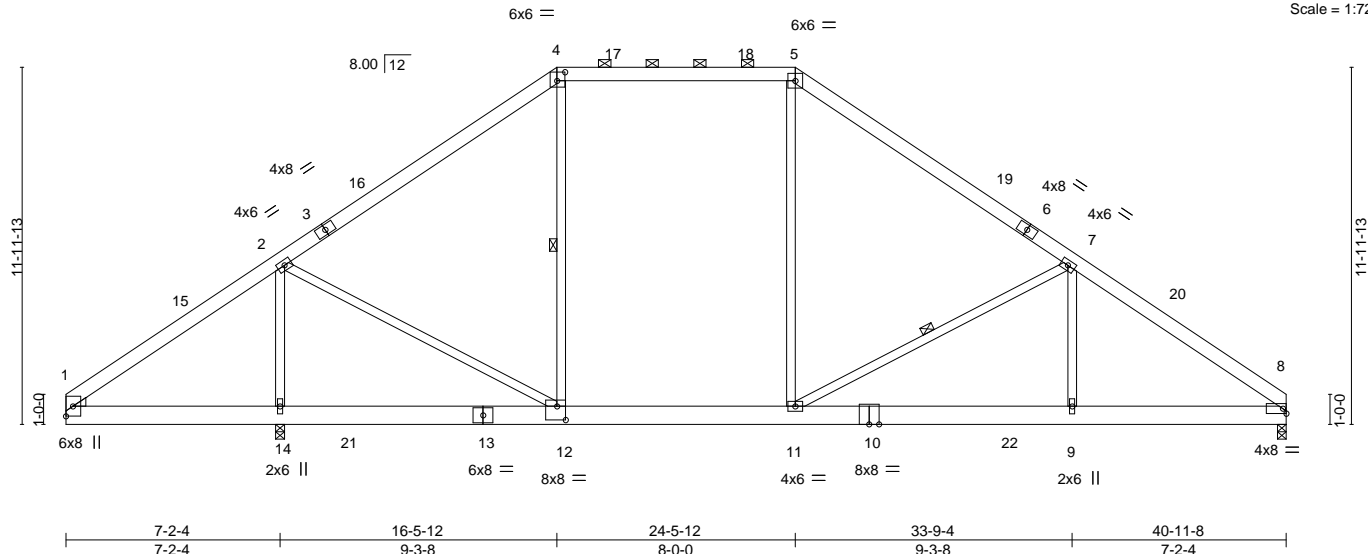


Plate Offsets (X,Y)-- [4:0-3-4,0-3-8], [12:0-3-8,0-5-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.39	Vert(LL)	-0.30 9-11	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.88	Vert(CT)	-0.57 9-11	>705	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.85	Horz(CT)	0.02 8	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.27 9-11	>999	240	Weight: 317 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x8 SP No.1
WEBS 2x4 SP No.2
WEDGE
Left: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-3-4 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 7-11

REACTIONS.

(size) 14=0-3-8, 8=0-3-8
Max Horz 14=-274(LC 8)
Max Uplift 14=-84(LC 12), 8=-70(LC 13)
Max Grav 14=2064(LC 2), 8=1391(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-344/563, 2-4=-1255/319, 4-5=-941/357, 5-7=-1282/340, 7-8=-2169/391
BOT CHORD 1-14=-347/355, 12-14=-479/363, 11-12=-5/982, 9-11=-214/1663, 8-9=-214/1663
WEBS 2-14=-1971/743, 2-12=-297/1456, 4-12=-129/257, 5-11=0/366, 7-11=-992/362, 7-9=0/513

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 16-5-12, Exterior(2) 16-5-12 to 22-8-7, Interior(1) 22-8-7 to 24-5-12, Exterior(2) 24-5-12 to 30-8-7, Interior(1) 30-8-7 to 40-9-12 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 8.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



January 30, 2022

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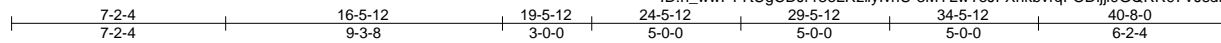
818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss A3	Truss Type PIGGYBACK BASE	Qty 2	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951725
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:45 2022 Page 1

ID: h_wwwFTTtUgCDJr13eLRzilylvhU-3MYLwTeJFXhkbvvrqFODijjiOQRR97VJsdIUENZqtFW



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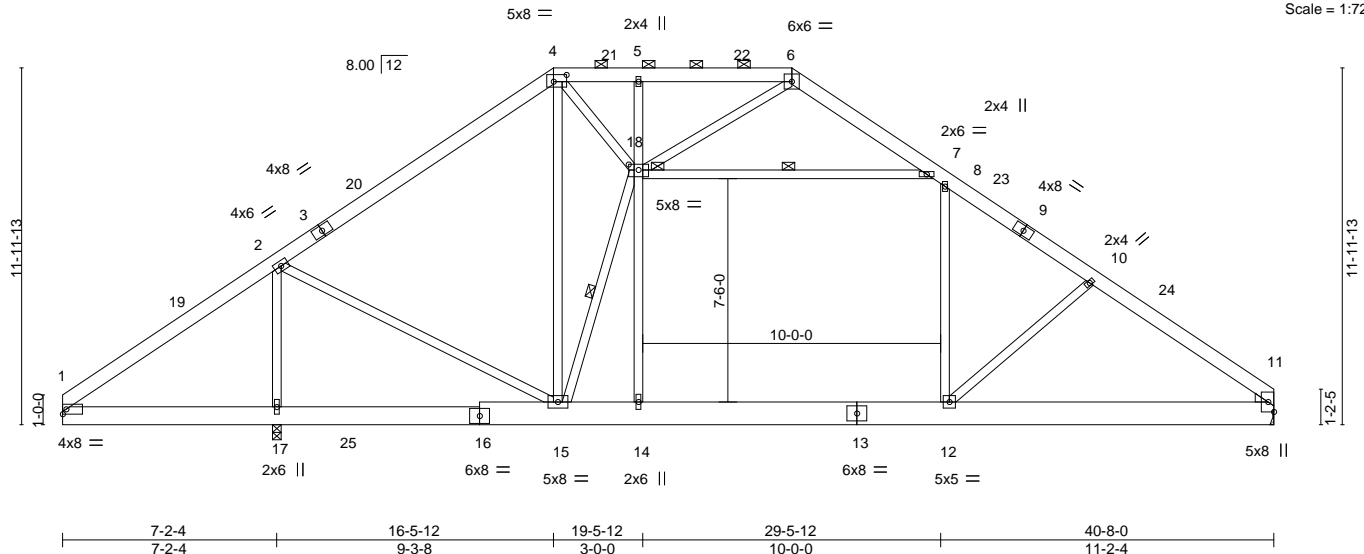


Plate Offsets (X,Y)-- [4:0-5-4,0-2-12], [18:0-4-0,0-2-4]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.38	Vert(LL)	-0.10 12-14	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.37	Vert(CT)	-0.18 12-14	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.75	Horz(CT)	0.02 11	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.07 11-12	>999	240	Weight: 376 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x10 SP No.1 *Except*
 1-16: 2x8 SP No.1
 WEBS 2x4 SP No.2
 WEDGE
 Right: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-6-10 oc purlins, except
 2-0-0 oc purlins (6-0-0 max.): 4-6.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 7-18, 15-18
 JOINTS 1 Brace at Jt(s): 18

REACTIONS.

(size) 11=Mechanical, 17=0-3-8
 Max Horz 17=274(LC 9)
 Max Uplift 11=-69(LC 13), 17=-84(LC 12)
 Max Grav 11=1408(LC 20), 17=2093(LC 19)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-93/479, 2-4=-1102/356, 4-5=-389/314, 5-6=-382/315, 6-7=-431/245,
 7-8=-1303/429, 8-10=-1724/417, 10-11=-1946/430
 BOT CHORD 1-17=-318/150, 15-17=-383/185, 14-15=-75/1348, 12-14=-76/1347, 11-12=-231/1483
 WEBS 2-15=-85/1154, 4-15=-259/1121, 2-17=-1763/510, 7-18=-1202/297, 8-12=0/535,
 10-12=-338/218, 14-18=-10/747, 5-18=-261/120, 15-18=-1952/466, 4-18=-1149/351

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 16-5-12, Exterior(2) 16-5-12 to 22-8-7, Interior(1) 22-8-7 to 24-5-12, Exterior(2) 24-5-12 to 30-8-7, Interior(1) 30-8-7 to 40-7-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 17.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



January 30, 2022

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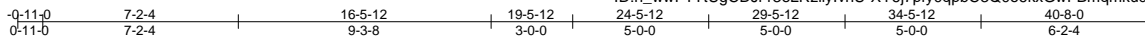
818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss A4	Truss Type PIGGYBACK BASE	Qty 4	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951726
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:46 2022 Page 1

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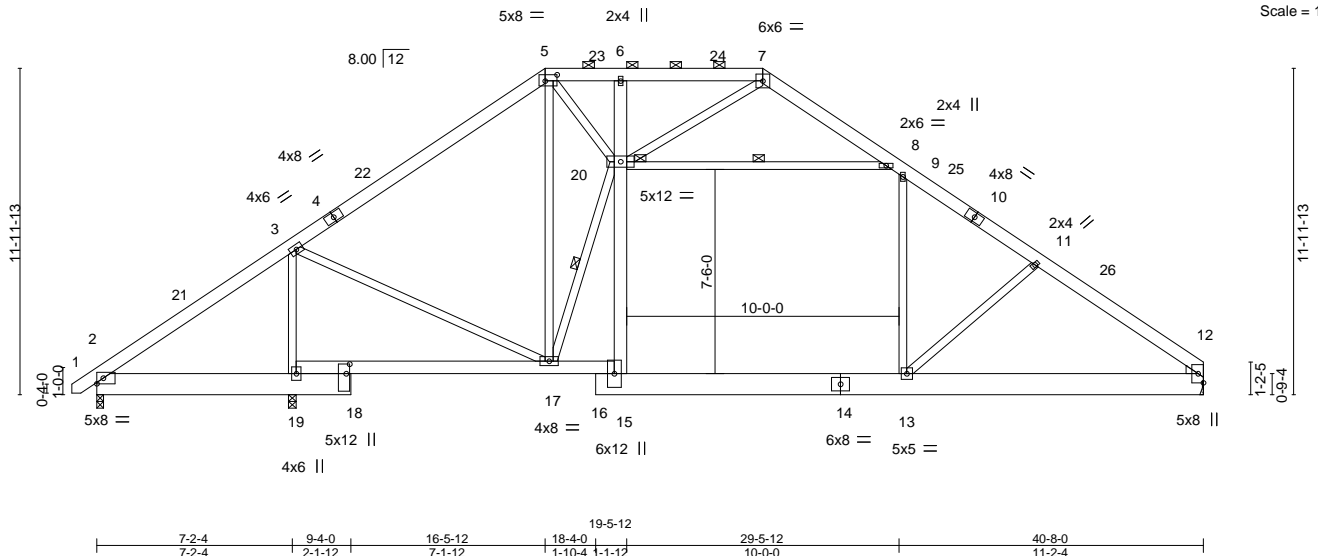


Plate Offsets (X,Y)--	[5:0-5-4,0-2-12], [18:0-4-4,0-1-8]
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LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	Vert(LL)	-0.09	13-15	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.43	Vert(CT)	-0.16	13-15	>999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.55	Horz(CT)	0.03	12	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Wind(LL)	0.07	12-13	>999		
	Code IRC2015/TPI2014						Weight: 389 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 5-4-5 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD 2x10 SP No.1 *Except* 15-19: 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.2 *Except* 6-15: 2x6 SP No.1	WEBS 1 Row at midpt 8-20, 17-20
WEDGE Right: 2x4 SP No.2	JOINTS 1 Brace at Jt(s): 20

REACTIONS. (size) 2=0-3-0, 19=0-3-8, 12=Mechanical
 Max Horz 2=275(LC 11)
 Max Uplift 2=86(LC 13), 19=83(LC 12), 12=87(LC 13)
 Max Grav 2=435(LC 20), 19=1652(LC 19), 12=1493(LC 20)


FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-548/201, 3-5=-1333/423, 5-6=-606/267, 6-7=-590/261, 7-8=-575/279,
 8-9=-1442/467, 9-11=-1868/448, 11-12=-2086/460
 BOT CHORD 2-19=-71/337, 17-19=-87/337, 15-17=-114/1439, 13-15=-117/1445, 12-13=-261/1592
 WEBS 3-19=-1386/396, 3-17=0/809, 5-17=-275/1196, 11-13=-326/215, 15-20=0/599,
 6-20=-259/122, 8-20=-1191/296, 9-13=0/516, 17-20=-1557/373, 5-20=-1058/320

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 16-5-12, Exterior(2) 16-5-12 to 22-8-7, Interior(1) 22-8-7 to 24-5-12, Exterior(2) 24-5-12 to 30-8-7, Interior(1) 30-8-7 to 40-7-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 12.
 - 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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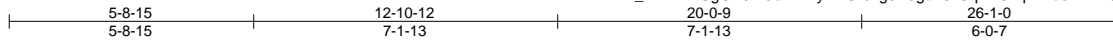


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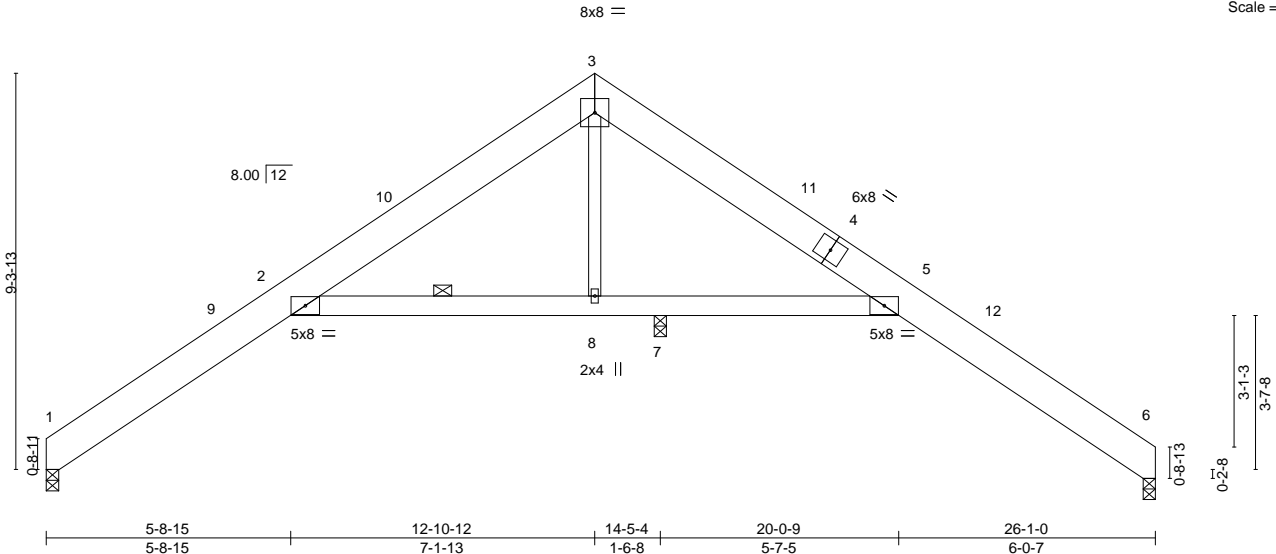
Job J1121-6539	Truss A5	Truss Type ROOF SPECIAL	Qty 3	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	149951727
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Comtech, Inc., Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:47 2022 Page 1
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Scale = 1:51.0



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL. in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.40	Vert(LL) -0.16	2	>999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.58	Vert(CT) -0.31	2-8	>560		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.44	Horz(CT) 0.18	6	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL) 0.14	2	>999		
							Weight: 166 lb	FT = 20%

LUMBER-

TOP CHORD 2x10 SP No.1
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
6-0-0 oc bracing: 2-8

REACTIONS.

(size) 1=0-3-8, 6=0-3-8, 7=0-3-8
Max Horz 1=208(LC 8)
Max Uplift 1=24(LC 12), 6=68(LC 13), 7=41(LC 12)
Max Grav 1=424(LC 23), 6=273(LC 24), 7=1523(LC 1)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=287/221, 2-3=-108/760, 3-5=-81/643
BOT CHORD 2-8=-639/304, 7-8=-639/304, 5-7=-639/304
WEBS 3-8=-1165/293

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 12-10-12, Exterior(2) 12-10-12 to 17-3-9, Interior(1) 17-3-9 to 25-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 7.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



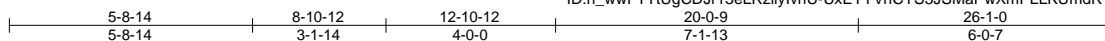
818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss A5-GR	Truss Type ROOF SPECIAL	Qty 2	Ply 2	Lot 8 Purfoy Place Job Reference (optional)	149951728
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:48 2022 Page 1

ID:h_wvFTTgUGCDJr13eLRzilylvhU-UxETYVhCYSJ3SMaPwXmPLLKUmdRTMbWIZbz8r6zqtFT



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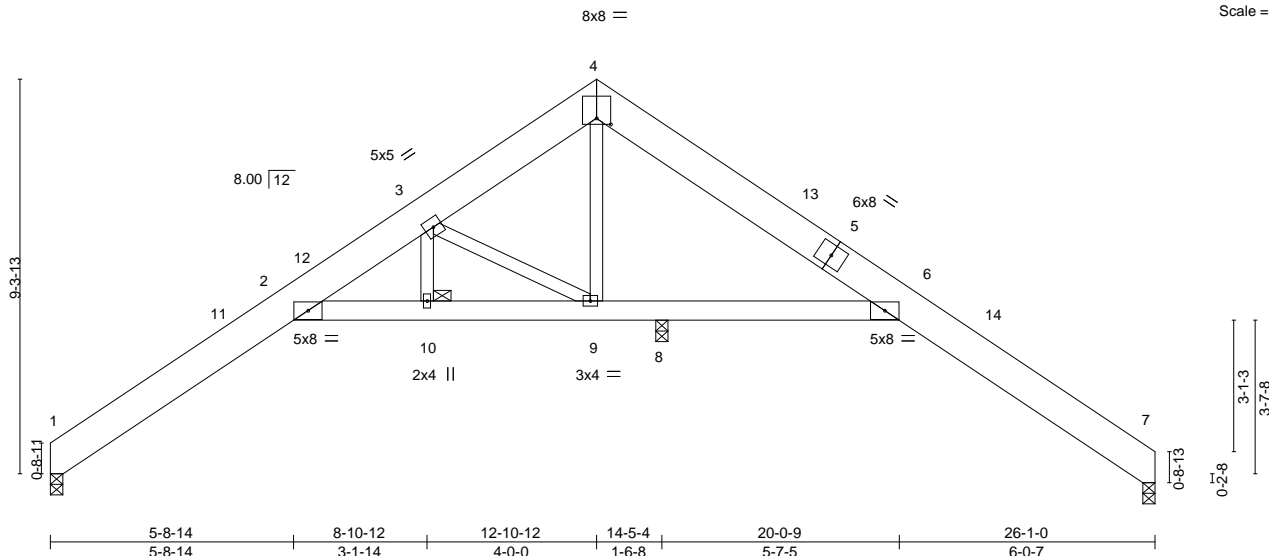


Plate Offsets (X,Y)-- [4:0-4-0,0-1-11]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.55	Vert(LL) -0.19	2	>914	360	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.48	Vert(CT) -0.36	2	>474	240		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.28	Horz(CT) 0.23	8	n/a	n/a		
BCDL 10.0	Rep Stress Incr NO	Matrix-S	Wind(LL) 0.18	2	>951	240		
	Code IRC2015/TPI2014						Weight: 350 lb	FT = 20%

LUMBER-

TOP CHORD 2x10 SP No.1 *Except*
1-4: 2x10 SP 2400F 2.0E
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 10

REACTIONS.

(size) 1=0-3-8, 7=0-3-8, 8=0-3-8
Max Horz 1=208(LC 8)
Max Uplift 1=131(LC 12), 7=215(LC 23), 8=159(LC 12)
Max Grav 1=1370(LC 23), 7=106(LC 9), 8=2755(LC 1)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-785/279, 2-3=-1074/244, 3-4=-194/1253, 4-6=-229/1276, 6-7=-48/307
BOT CHORD 2-10=-229/1815, 9-10=-235/1846, 8-9=-1265/464, 6-8=-1265/464
WEBS 4-9=-1260/292, 3-10=-180/1049, 3-9=-3276/802

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x10 - 2 rows staggered at 0-7-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 12-10-12, Exterior(2) 12-10-12 to 17-3-9, Interior(1) 17-3-9 to 25-11-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=131, 7=215, 8=159.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1923 lb down and 509 lb up at 5-8-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15



January 30, 2022

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 8 Purfoy Place	I49951728
J1121-6539	A5-GR	ROOF SPECIAL	2	2	Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:48 2022 Page 2
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LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=-84, 2-4=-60, 4-6=-60, 6-7=-84, 2-6=-20

Concentrated Loads (lb)

Vert: 2=-1900(F)

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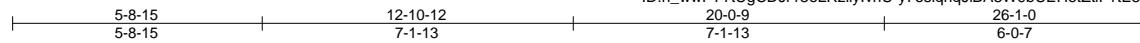


818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 8 Purfoy Place	I49951729
J1121-6539	A5SG	GABLE	1	1	Job Reference (optional)	

Comtech, Inc. Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:49 2022 Page 1
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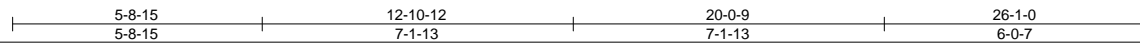
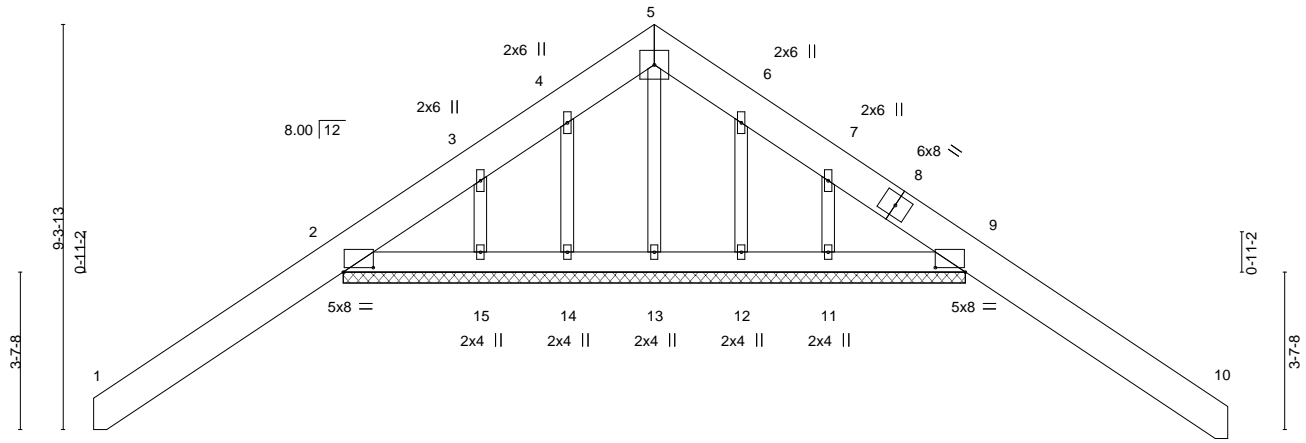


Plate Offsets (X,Y)-- [2:0-8-4,0-1-4], [9:0-8-4,0-1-4]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.38	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.12	Vert(LL) -0.09 10 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.07	Vert(CT) -0.15 10 n/r 120		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 9 n/a n/a		
	Code IRC2015/TPI2014			Weight: 181 lb	FT = 20%

LUMBER-

TOP CHORD 2x10 SP No.1
 BOT CHORD 2x6 SP No.1
 OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

All bearings 14-3-10.
 (lb) - Max Horz 2=-259(LC 10)
 Max Uplift All uplift 100 lb or less at joint(s) except 2=-406(LC 12), 9=-485(LC 13), 14=-128(LC 12),
 15=-237(LC 1), 12=-123(LC 13), 11=-292(LC 1)
 Max Grav All reactions 250 lb or less at joint(s) 13, 15 except 2=814(LC 23), 9=867(LC 1), 14=273(LC 19),
 12=269(LC 20), 11=283(LC 17)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-239/258, 7-9=-263/315
 BOT CHORD 2-15=-133/290, 14-15=-145/286, 13-14=-146/288, 12-13=-146/288, 11-12=-142/285,
 9-11=-149/299
 WEBS 3-15=-387/225, 7-11=-452/270

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 406 lb uplift at joint 2, 485 lb uplift at joint 9, 128 lb uplift at joint 14, 237 lb uplift at joint 15, 123 lb uplift at joint 12 and 292 lb uplift at joint 11.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.



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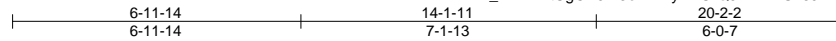
818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss A6	Truss Type Roof Special	Qty 6	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951730
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:50 2022 Page 1

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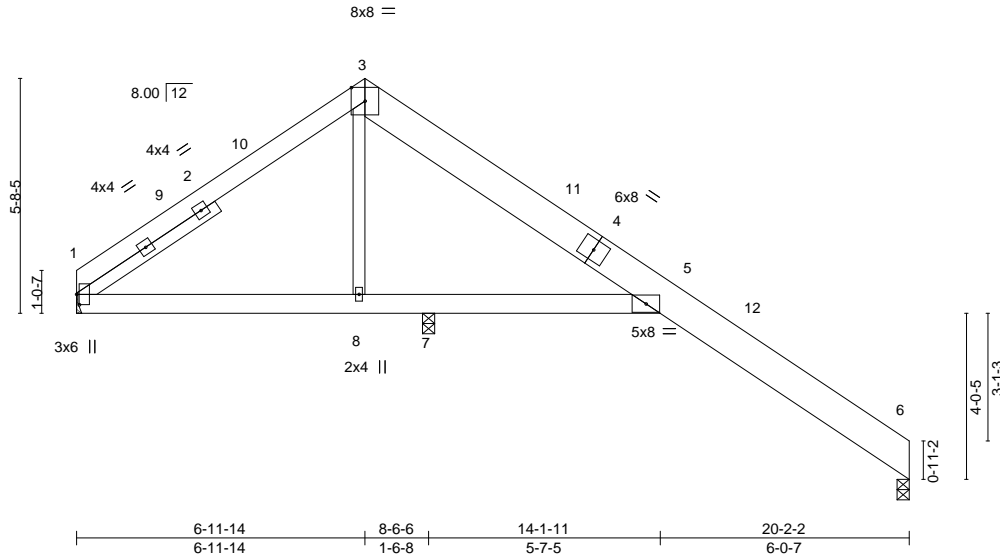


Plate Offsets (X,Y)-- [1:0-3-0,0-0-12]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.29	Vert(LL) -0.10	5	>999	360	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.74	Vert(CT) -0.19	5	>725	240		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.30	Horz(CT) 0.12	6	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Wind(LL) 0.07	5	>999	240		
	Code IRC2015/TPI2014						Weight: 131 lb	FT = 20%

LUMBER-

TOP CHORD 2x10 SP No.1 *Except*
1-3: 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2
SLIDER Left 2x4 SP No.2 4-0-8

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

(size) 1=Mechanical, 6=0-3-8, 7=0-3-8
Max Horz 1=207(LC 8)
Max Uplift 1=49(LC 12), 6=51(LC 13), 7=31(LC 13)
Max Grav 1=290(LC 23), 6=373(LC 1), 7=1047(LC 1)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-3=-117/327, 3-5=-77/397
BOT CHORD 1-8=-330/228, 7-8=-311/224, 5-7=-311/224
WEBS 3-8=-714/203

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 6-11-14, Exterior(2) 6-11-14 to 11-4-11, Interior(1) 11-4-11 to 20-0-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 51 lb uplift at joint 6 and 31 lb uplift at joint 7.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.



January 30, 2022

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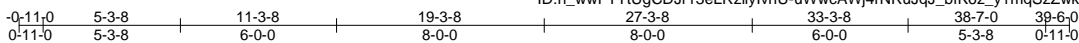


818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 8 Purfoy Place	I49951731
J1121-6539	B1	COMMON	1	1	Job Reference (optional)	

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:51 2022 Page 1



5x8 =

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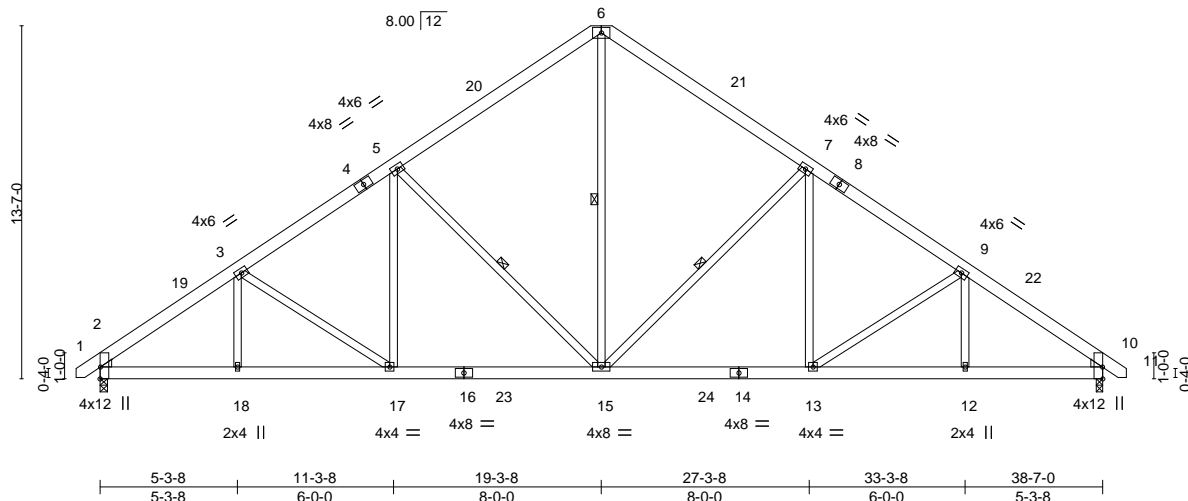


Plate Offsets (X,Y)-- [2:Edge,0-0-3], [10:Edge,0-0-3]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.44	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.49	Vert(LL) -0.09 13-15 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.44	Vert(CT) -0.16 13-15 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.06 10 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.05 15-17 >999 240	Weight: 310 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.2

WEDGE
 Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-0-7 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 6-15, 7-15, 5-15

REACTIONS.

(size) 2=0-3-8, 10=0-3-0
 Max Horz 2=320(LC 10)
 Max Uplift 2=92(LC 12), 10=92(LC 13)
 Max Grav 2=1658(LC 19), 10=1657(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2371/422, 3-5=-2090/451, 5-6=-1592/463, 6-7=-1592/463, 7-9=-2093/451, 9-10=-2377/424
 BOT CHORD 2-18=-232/2030, 17-18=-232/2030, 15-17=-132/1859, 13-15=-131/1691, 12-13=-234/1798, 10-12=-234/1798
 WEBS 6-15=-264/1212, 7-15=-783/262, 7-13=0/412, 9-13=-303/134, 5-15=-780/262, 5-17=0/408, 3-17=-298/132

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 19-3-8, Exterior(2) 19-3-8 to 23-8-5, Interior(1) 23-8-5 to 39-4-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 2 and 92 lb uplift at joint 10.



January 30, 2022

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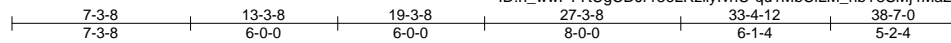


818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss B1-GR	Truss Type ROOF SPECIAL GIRDER	Qty 1	Ply 2	Lot 8 Purfoy Place	149951732
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Comtech, Inc. Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:53 2022 Page 1
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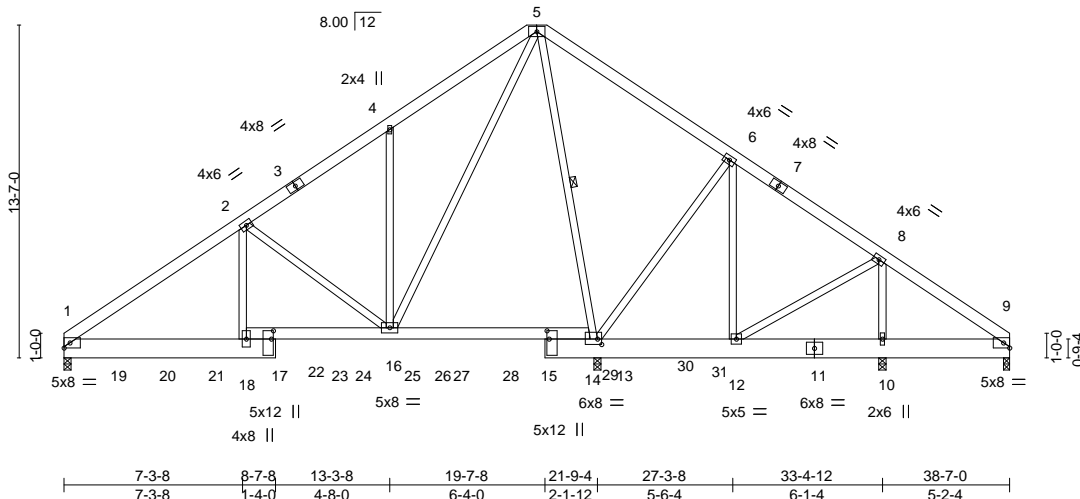


Plate Offsets (X,Y)-- [14:0-2-0,0-2-12], [15:0-4-0,0-1-0], [17:0-4-0,0-1-0]

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.23	Vert(LL) -0.05 14-16 >999 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.40	Vert(CT) -0.10 14-16 >999 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.64	Horz(CT) 0.03 13 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.01 16-18 >999 240	Weight: 715 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x10 SP No.1 *Except*
14-18: 2x6 SP No.1
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
10-0-0 oc bracing: 1-18,16-18.
WEBS 1 Row at midpt 5-14

REACTIONS.

All bearings 0-3-8 except (jt=length) 9=0-3-0.
(lb) - Max Horz 1=316(LC 25)
Max Uplift All uplift 100 lb or less at joint(s) 9 except 10=244(LC 28)
Max Grav All reactions 250 lb or less at joint(s) 9 except 1=1926(LC 1), 13=3748(LC 1), 10=535(LC 16)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=2655/0, 2-4=1407/0, 4-5=1388/45, 5-6=0/564, 6-8=89/521
BOT CHORD 1-18=0/2054, 16-18=0/2054, 14-16=-320/227, 13-14=-2291/0, 12-13=-399/20
WEBS 2-16=1230/0, 4-16=-379/212, 6-13=-376/327, 6-12=-278/0, 8-12=-312/0,
8-10=-313/350, 5-14=-2239/0, 2-18=0/1088, 5-16=0/2537

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 10=244.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 264 lb down at 2-0-12, 264 lb down at 4-0-12, 264 lb down at 6-0-12, 264 lb down at 8-0-12, 285 lb down at 10-0-12, 285 lb down at 12-0-12, 285 lb down at 14-0-12, 285 lb down at 16-0-12, 285 lb down at 18-0-12, and 264 lb down at 20-0-12, and 264 lb down at 21-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced); Lumber Increase=1.15, Plate Increase=1.15



January 30, 2022

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss B1-GR	Truss Type ROOF SPECIAL GIRDER	Qty 1	Ply 2	Lot 8 Purfoy Place I49951732 Job Reference (optional)
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:53 2022 Page 2
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LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-60, 1-9=-20

Concentrated Loads (lb)

Vert: 19=-264(F) 20=-264(F) 21=-264(F) 22=-264(F) 23=-285(F) 24=-285(F) 25=-285(F) 27=-285(F) 28=-285(F) 29=-264(F) 30=-264(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component**



818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss B1SG	Truss Type GABLE	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951733
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Comtech, Inc. Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:55 2022 Page 1

ID: h_wvFTTtUgCDJr13eLRzilylvhU-nH970umbubxJoRqlqVO27q7n7SwjVHnABA0aCzqtFM

0-11-0 5-3-8 11-3-8 19-3-8 27-3-8 33-3-8 38-7-0 39-6-0
0-11-0 5-3-8 6-0-0 8-0-0 8-0-0 6-0-0 5-3-8 0-11-0

5x8 =

Scale = 1:93.2

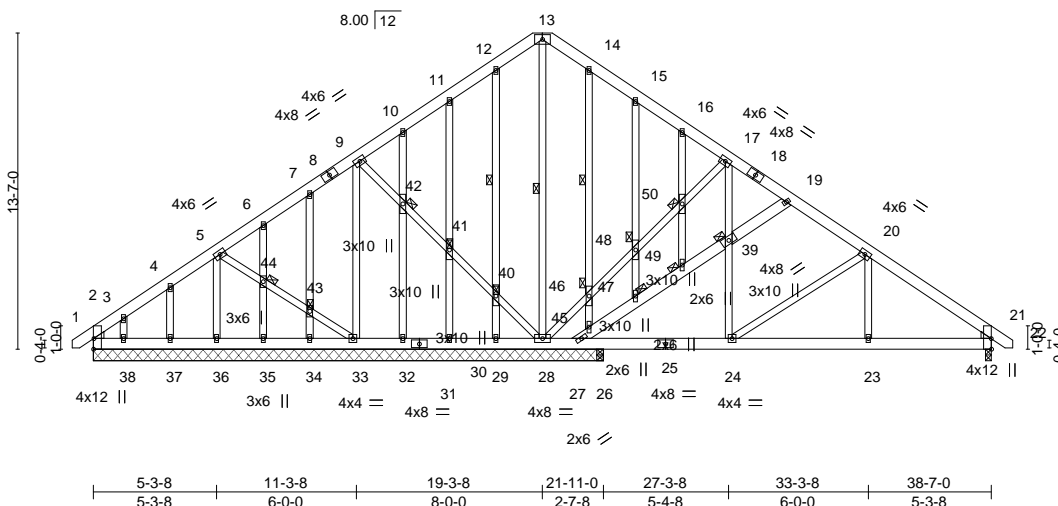


Plate Offsets (X,Y)-- [2:Edge,0-0-3], [21:Edge,0-0-3]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.16	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.12	Vert(LL) -0.01 23-24 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.34	Vert(CT) -0.03 23-24 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 21 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.01 23-24 >999 240	Weight: 439 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.2 *Except*
 27-39,19-39; 2x6 SP No.1
 OTHERS 2x4 SP No.2
 WEDGE
 Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 13-28, 12-40, 14-46
 JOINTS 1 Brace at Jt(s): 39, 40, 41, 42, 43, 44, 46, 47, 48, 49, 50

REACTIONS.

All bearings 21-11-0 except (jt=length) 21=0-3-0, 26=0-3-8.
 (lb) - Max Horz 2=400(LC 11)
 Max Uplift All uplift 100 lb or less at joint(s) 33, 29, 32, 34, 35, 37 except
 2=169(LC 10), 28=183(LC 13), 21=137(LC 13), 27=212(LC 13), 30=102(LC
 12), 38=170(LC 12)
 Max Grav All reactions 250 lb or less at joint(s) 2, 33, 36, 27, 29, 30, 32, 34, 35,
 37, 38 except 28=771(LC 1), 21=681(LC 1), 26=259(LC 3)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-343/315, 9-10=66/250, 14-15=0/263, 17-19=-292/120, 19-20=-442/132,
 20-21=-818/138
 BOT CHORD 2-38=-224/267, 37-38=-224/267, 36-37=-224/267, 35-36=-224/267, 34-35=-224/267,
 33-34=-224/267, 32-33=-307/366, 30-32=-307/366, 29-30=-307/366, 28-29=-307/366,
 27-28=-16/284, 26-27=0/323, 24-26=0/323, 23-24=-8/572, 21-23=-8/572
 WEBS 13-28=-286/0, 28-46=-608/298, 46-48=-622/311, 48-50=-595/290, 17-50=-601/295,
 24-39=-30/362, 17-39=-62/374, 20-24=-395/196, 27-45=-285/177, 14-46=-259/151,
 45-46=-277/170

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 33, 29, 32, 34, 35, 37 except (jt=lb) 2=169, 28=183, 21=137, 27=212, 30=102, 38=170.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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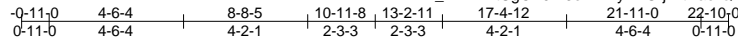


Job J1121-6539	Truss C1	Truss Type ATTIC	Qty 2	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951735
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:57 2022 Page 1

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6x8 =

Scale = 1:72.9

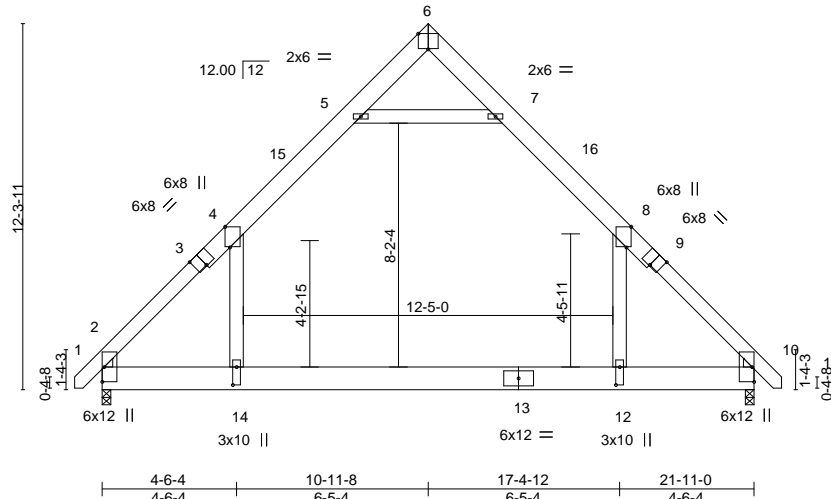


Plate Offsets (X,Y)-- [3:0-4-0,Edge], [4:0-8-6,Edge], [6:0-4-0,Edge], [8:0-8-6,Edge], [9:0-4-0,Edge], [12:0-7-4,0-1-8], [14:0-7-4,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.80	Vert(LL)	-0.28 12-14	>937	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.83	Vert(CT)	-0.46 12-14	>562	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.20	Horz(CT)	0.01 10	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.07 12-14	>999	240	Weight: 216 lb	FT = 20%

LUMBER-
TOP CHORD 2x8 SP No.1 *Except*
1-3,9-11: 2x6 SP No.1
BOT CHORD 2x10 SP No.1
WEBS 2x6 SP No.1
WEDGE
Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-11-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-13 oc bracing.

REACTIONS. (size) 2=0-3-8, 10=0-3-8
Max Horz 2=279(LC 10)
Max Grav 2=1518(LC 20), 10=1518(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-2055/0, 4-5=-1088/146, 5-6=0/413, 6-7=0/413, 7-8=-1088/146, 8-10=-2055/0
BOT CHORD 2-14=0/1111, 12-14=0/1111, 10-12=0/1111
WEBS 8-12=0/1119, 4-14=0/1119, 5-7=-1552/184

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-8-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Ceiling dead load (10.0 psf) on member(s). 4-5, 7-8, 5-7; Wall dead load (5.0psf) on member(s).8-12, 4-14
 - Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
 - Attic room checked for L/360 deflection.



January 30,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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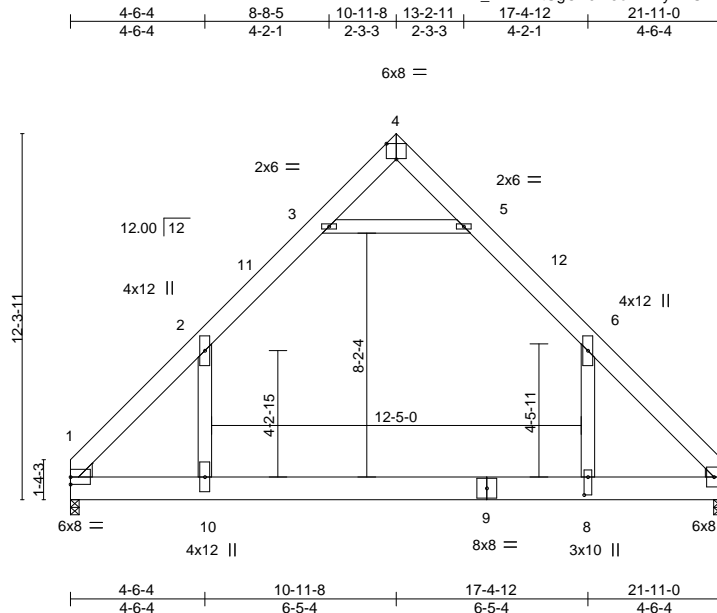


818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss C2	Truss Type ATTIC	Qty 5	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	149951736
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:58 2022 Page 1
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Scale = 1:72.9

Plate Offsets (X,Y)-- [4:0-4-0,Edge], [8:0-7-4,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.83	Vert(LL)	-0.29 8-10	>902	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.85	Vert(CT)	-0.49 8-10	>533	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.21	Horz(CT)	0.01 7	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.08 8-10	>999	240	Weight: 218 lb	FT = 20%

LUMBER-
TOP CHORD 2x8 SP No.1
BOT CHORD 2x10 SP No.1
WEBS 2x6 SP No.1
WEDGE
Left: 2x6 SP No.2 , Right: 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-7-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-9-4 oc bracing.

REACTIONS. (size) 1=0-3-8, 7=0-3-8
Max Horz 1=276(LC 8)
Max Grav 1=1488(LC 21), 7=1488(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2039/0, 2-3=-1107/148, 3-4=0/426, 4-5=0/426, 5-6=-1107/148, 6-7=-2039/0
BOT CHORD 1-10=0/1135, 8-10=0/1135, 7-8=0/1135
WEBS 6-8=0/1084, 2-10=0/1084, 3-5=-1596/185

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-6-4, Interior(1) 4-6-4 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 21-11-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-5; Wall dead load (5.0psf) on member(s).6-8, 2-10
 - Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-10
 - Attic room checked for L/360 deflection.



January 30,2022

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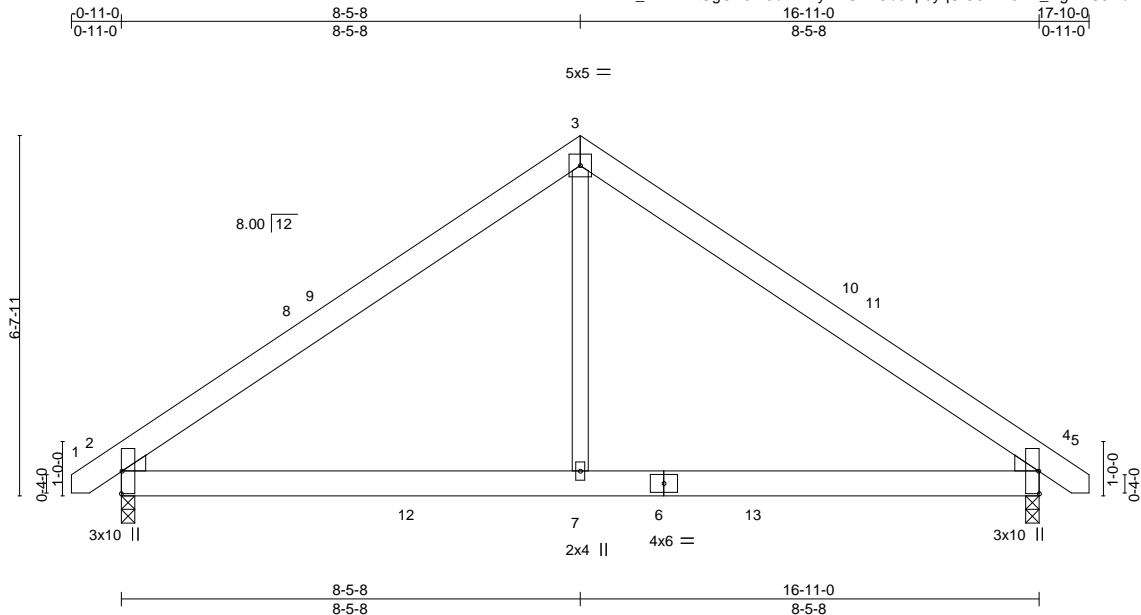


818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss D1	Truss Type COMMON	Qty 3	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951737
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Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:00:59 2022 Page 1
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Scale = 1:40.0

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.38	Vert(LL) -0.04 4-7 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.31	Vert(CT) -0.07 4-7 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 4 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.07 2-7 >999 240	Weight: 103 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2
WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 4=0-3-0, 2=0-3-0
Max Horz 2=147(LC 10)
Max Uplift 4=103(LC 8), 2=103(LC 9)
Max Grav 4=771(LC 2), 2=771(LC 2)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-914/694, 3-4=-914/694
BOT CHORD 2-7=-415/642, 4-7=-415/642
WEBS 3-7=-517/563

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 8-5-8, Exterior(2) 8-5-8 to 12-10-5, Interior(1) 12-10-5 to 17-8-1 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCCL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=103, 2=103.



January 30, 2022

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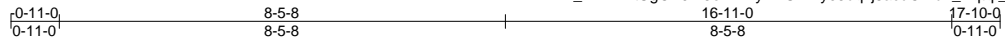


818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss D1GE	Truss Type GABLE	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951738
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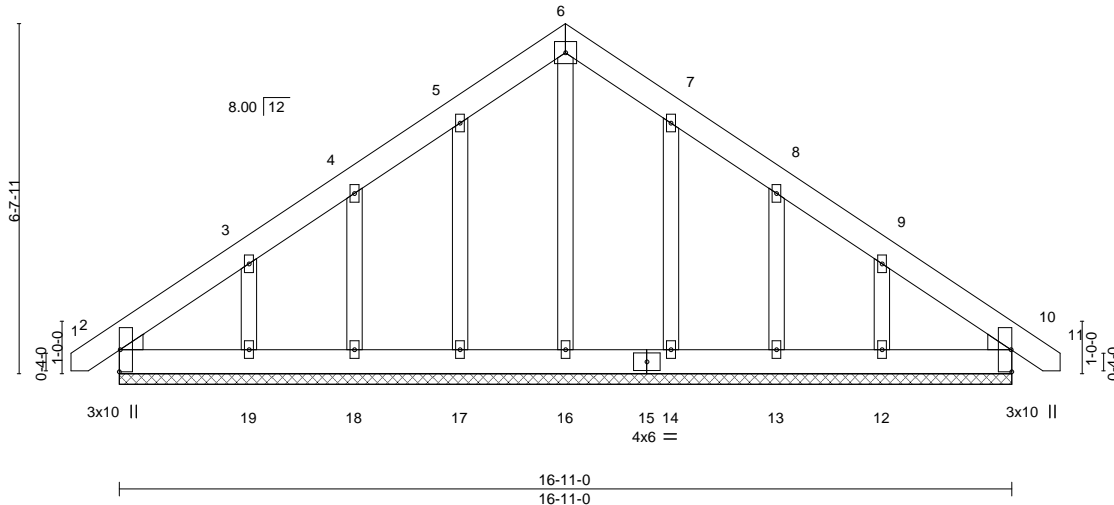
Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:00 2022 Page 1



5x5 =

Scale = 1:41.1



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.03	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.02	Vert(LL) 0.00 10 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.07	Vert(CT) 0.00 10 n/r 120		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 10 n/a n/a		
	Code IRC2015/TPI2014			Weight: 130 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 OTHERS 2x4 SP No.2
 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 16-11-0.
 (lb) - Max Horz 2=184(LC 10)
 Max Uplift All uplift 100 lb or less at joint(s) 10, 2, 17, 18, 14, 13 except 19=143(LC 12), 12=137(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 10, 2, 16, 17, 18, 19, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 2, 17, 18, 14, 13 except (jt=lb) 19=143, 12=137.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss G1	Truss Type COMMON	Qty 3	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951739
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Comtech, Inc. Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:01 2022 Page 1

ID:h_wvFTTtUgCDJr13eLRzilylvhU-bRWOHxrMURITWM4vBIVSN5MnYsyWvW_fY6dKoszqtFG
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Scale = 1:49.4

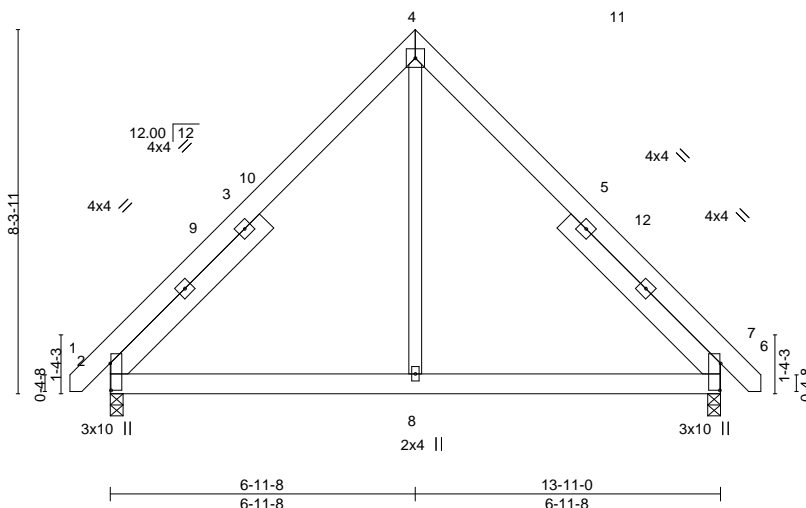


Plate Offsets (X,Y)-- [2:0-7-6,0-0-3], [6:0-7-6,0-0-3]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.23	Vert(LL)	-0.01	2-8	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.16	Vert(CT)	-0.02	2-8	>999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.07	Horz(CT)	0.00	6	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Wind(LL)	0.01	2-8	>999		
	Code IRC2015/TPI2014						Weight: 120 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.2
 SLIDER Left 2x6 SP No.1 4-11-14, Right 2x6 SP No.1 4-11-14

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 2=0-3-8, 6=0-3-8
 Max Horz 2=186(LC 8)
 Max Uplift 2=24(LC 13), 6=24(LC 12)
 Max Grav 2=603(LC 1), 6=603(LC 1)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-4=-575/174, 4-6=-575/174
 BOT CHORD 2-8=-8/317, 6-8=-8/317
 WEBS 4-8=0/321

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 6-11-8, Exterior(2) 6-11-8 to 11-4-5, Interior(1) 11-4-5 to 14-8-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

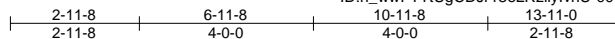


818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss G1-GR	Truss Type Common Girder	Qty 1	Ply 2	Lot 8 Purfoy Place Job Reference (optional)	I49951740
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Comtech, Inc. Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:04 2022 Page 1
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5x8 ||

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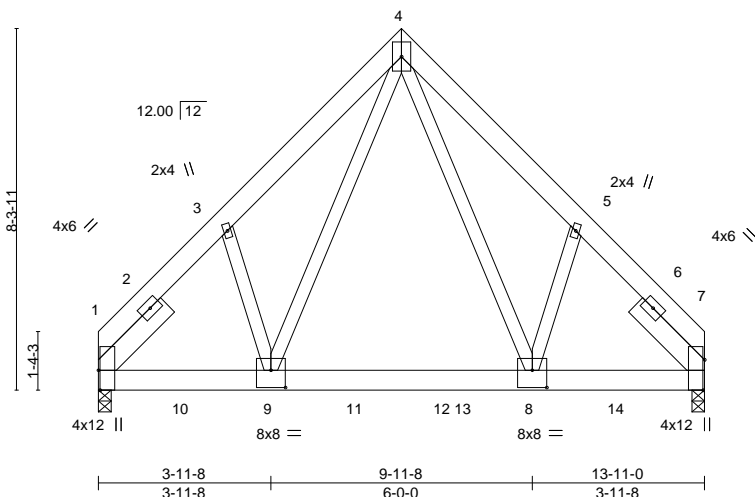


Plate Offsets (X,Y)-- [1:0-5-8,Edge], [7:0-8-6,Edge], [8:0-4-0,0-4-12], [9:0-4-0,0-4-12]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.60	Vert(LL) -0.11	8-9	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.98	Vert(CT) -0.21	8-9	>801	240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.50	Horz(CT) 0.02	7	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.07	8-9	>999	240	Weight: 248 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.2
 SLIDER Left 2x6 SP No.1 2-2-3, Right 2x6 SP No.1 2-2-3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-6-13 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=0-3-8, 7=0-3-8
 Max Horz 1=186(LC 25)
 Max Uplift 1=267(LC 9), 7=262(LC 8)
 Max Grav 1=5624(LC 2), 7=5525(LC 2)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-3=6301/328, 3-4=5710/402, 4-5=5694/402, 5-7=6287/327
 BOT CHORD 1-9=246/3832, 8-9=128/2642, 7-8=158/3820
 WEBS 4-8=294/4033, 5-8=174/1041, 4-9=295/4064, 3-9=173/1039

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
 Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BC DL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=267, 7=262.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1688 lb down and 88 lb up at 1-10-4, 1688 lb down and 88 lb up at 3-10-4, 1659 lb down and 88 lb up at 5-10-4, 1654 lb down and 88 lb up at 7-10-4, and 1688 lb down and 88 lb up at 9-10-4, and 1688 lb down and 88 lb up at 11-10-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-4=60, 4-7=60, 1-7=20



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Continued on page 2

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818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 8 Purfoy Place	I49951740
J1121-6539	G1-GR	Common Girder	1	2	Job Reference (optional)	

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LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 8=-1609(B) 9=-1609(B) 10=-1609(B) 11=-1609(B) 12=-1609(B) 14=-1609(B)

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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component**

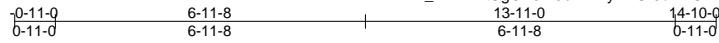


818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 8 Purfoy Place	I49951741
J1121-6539	G1GE	GABLE	1	1	Job Reference (optional)	

Comtech, Inc. Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:02 2022 Page 1



5x5 =

Scale = 1:48.7

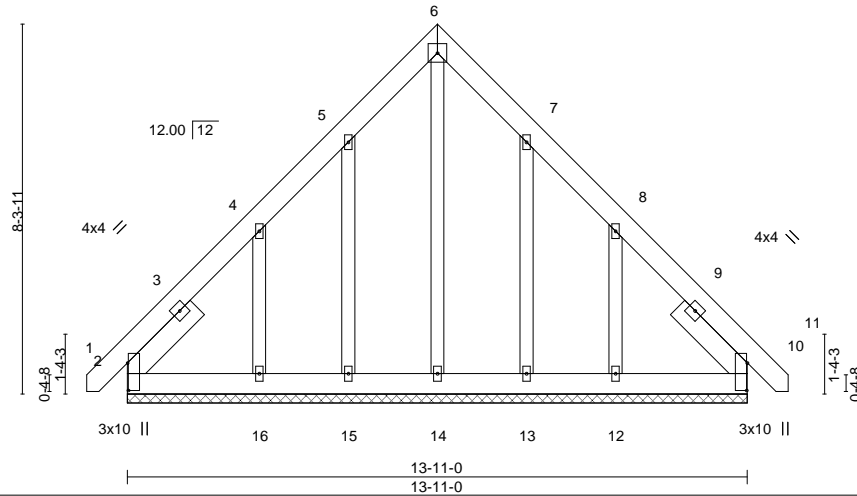


Plate Offsets (X,Y)-- [2:0-7-6,0-0-3], [10:0-7-6,0-0-3]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.06	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.04	Vert(LL) 0.00 10 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.20	Vert(CT) 0.00 10 n/r 120		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 10 n/a n/a		
	Code IRC2015/TPI2014			Weight: 132 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 OTHERS 2x4 SP No.2
 SLIDER Left 2x6 SP No.1 2-1-15, Right 2x6 SP No.1 2-1-15

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 13-11-0.
 (lb) - Max Horz 2=232(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 13 except 16=286(LC 12), 12=280(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 10, 14, 15, 13 except 2=260(LC 20), 16=296(LC 19), 12=289(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 4-16=300/289, 8-12=300/284

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 13 except (jt=lb) 16=286, 12=280.



January 30, 2022

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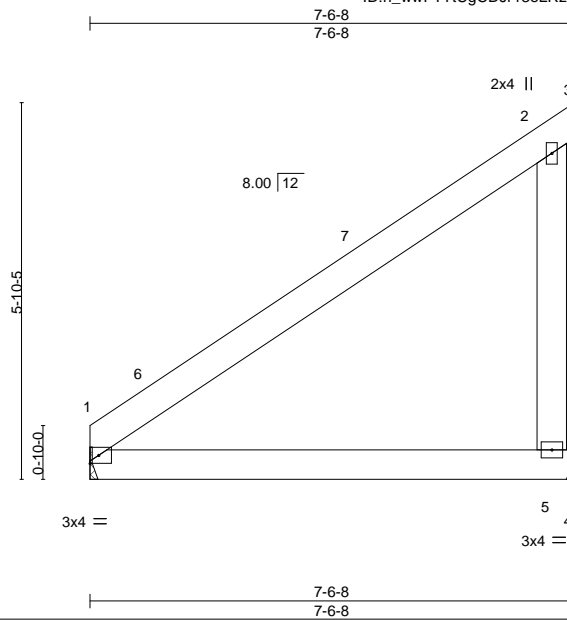


818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss M1	Truss Type JACK-PARTIAL	Qty 6	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951742
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8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:04 2022 Page 1
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Scale = 1:33.8

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	Vert(LL)	-0.03	1-5	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.20	Vert(CT)	-0.07	1-5	>999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.07	Horz(CT)	0.00	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Wind(LL)	0.00	1	****	Weight: 51 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x6 SP No.1

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=Mechanical, 5=Mechanical
Max Horz 1=173(LC 12)
Max Uplift 5=96(LC 12)
Max Grav 1=284(LC 1), 5=337(LC 19)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-5=309/243

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 7-6-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.



January 30, 2022

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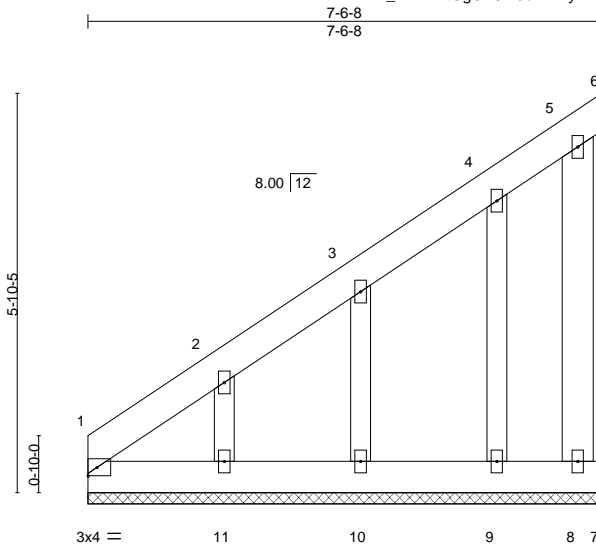
818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss M1GE	Truss Type GABLE	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951743
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8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:05 2022 Page 1

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Scale: 3/8"=1'

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.04	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.01	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.04	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) -0.00 6 n/a n/a		
	Code IRC2015/TPI2014			Weight: 62 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x6 SP No.1
 OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 7-6-8.
 (lb) - Max Horz 1=251(LC 12)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 6, 10, 9, 8 except 11=138(LC 12)
 Max Grav All reactions 250 lb or less at joint(s) 1, 6, 7, 11, 10, 9, 8

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=306/255

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 10, 9, 8 except (jt=lb) 11=138.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

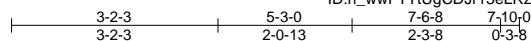


818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss M2	Truss Type JACK-PARTIAL	Qty 5	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951744
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:06 2022 Page 1
ID:h_wwFTTtUgCDJr13eLRzilylvhU-yOKHKfvVJ_Klc7yszl5e484dgtelanrPhOK5T3zqtFB



Scale = 1:33.4

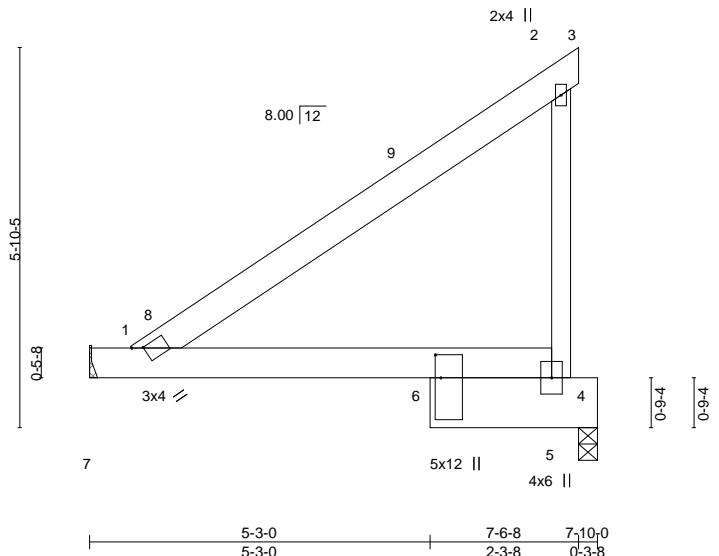


Plate Offsets (X,Y)-- [1:0-1-14,0-1-1], [6:0-4-4,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.27	Vert(LL)	-0.03	1-5	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.25	Vert(CT)	-0.08	1-5	>999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.10	Horz(CT)	0.01	4	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Wind(LL)	0.03	1-5	>999	Weight: 53 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1 *Except*
 4-6: 2x10 SP No.1
 WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

(size) 7=Mechanical, 4=0-3-8
 Max Horz 7=148(LC 12)
 Max Uplift 4=72(LC 12)
 Max Grav 7=305(LC 1), 4=305(LC 19)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 WEBS 2-5=-261/189

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-7-15 to 5-0-12, Interior(1) 5-0-12 to 7-6-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.



January 30,2022

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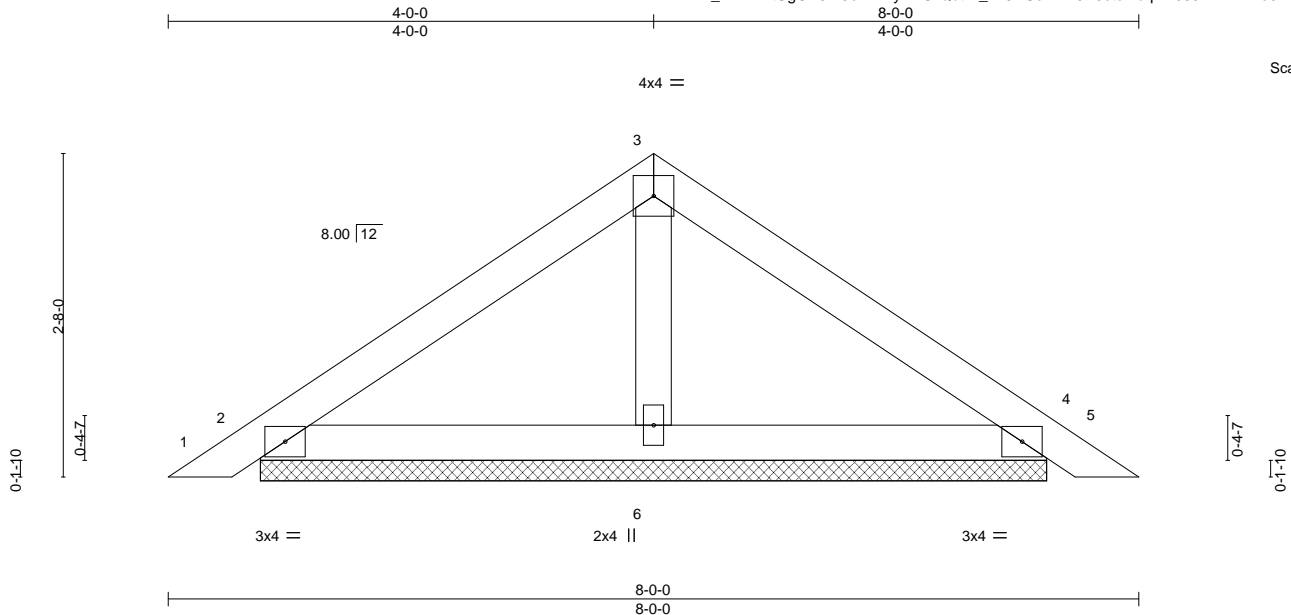
818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss PB	Truss Type PIGGYBACK	Qty 15	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951745
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:07 2022 Page 1

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Scale = 1:17.9

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.14	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.08	Vert(LL) 0.00 5 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.02	Vert(CT) 0.01 5 n/r 120		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 4 n/a n/a		
	Code IRC2015/TPI2014			Weight: 27 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 2=6-5-12, 4=6-5-12, 6=6-5-12
Max Horz 2=60(LC 10)
Max Uplift 2=31(LC 12), 4=36(LC 13)
Max Grav 2=173(LC 1), 4=173(LC 1), 6=232(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



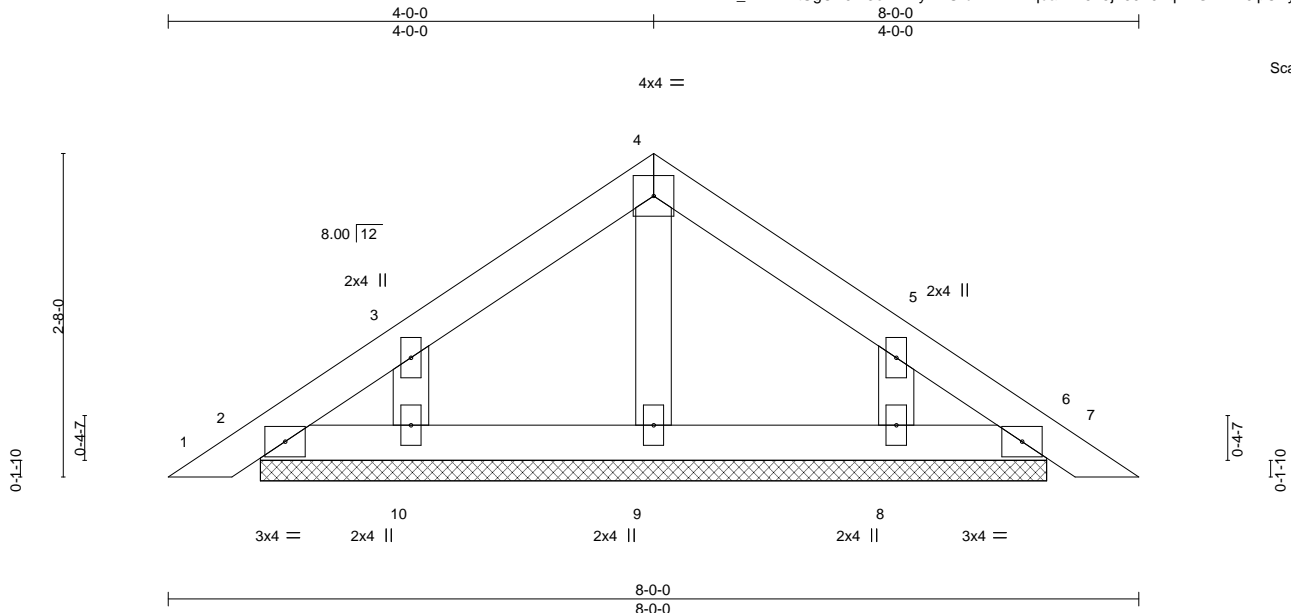
818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss PBE	Truss Type GABLE	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951746
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Comtech, Inc., Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:08 2022 Page 1

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Scale = 1:17.9

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.03	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.02	Vert(LL) -0.00 6 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.02	Vert(CT) -0.00 6 n/r 120		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 6 n/a n/a		
	Code IRC2015/TPI2014			Weight: 29 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS.

All bearings 6-5-12.
 (lb) - Max Horz 2=75(LC 11)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 10, 8
 Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2'-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6, 10, 8.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



January 30, 2022

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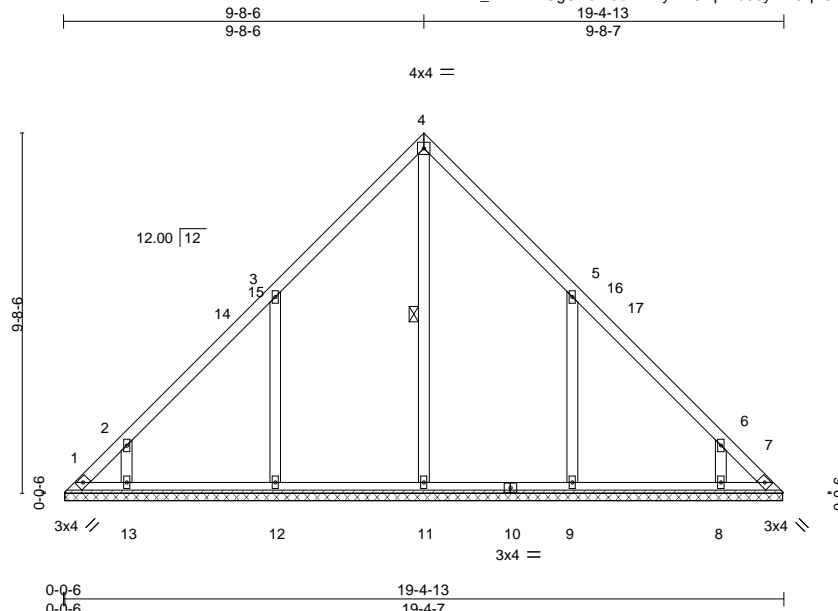


818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss V2	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951748
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:10 2022 Page 1
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Scale = 1:58.4

Plate Offsets (X,Y)-- [5:0-0-0,0-0-0], [6:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.16	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.19	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.20	Horz(CT)	0.00	7	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S						
	Code IRC2015/TPI2014						Weight: 100 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-11

REACTIONS.

All bearings 19-4-1.
(lb) - Max Horz 1=224(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 7 except 1=131(LC 10), 12=185(LC 12), 13=132(LC 12), 9=184(LC 13), 8=132(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=434(LC 22), 12=490(LC 19), 13=280(LC 19), 9=490(LC 20), 8=280(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-268/228, 6-7=-262/228
WEBS 3-12=-406/309, 2-13=-308/261, 5-9=-406/309, 6-8=-308/261

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 9-8-6, Exterior(2) 9-8-6 to 14-1-3, Interior(1) 14-1-3 to 19-0-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCCL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 1=131, 12=185, 13=132, 9=184, 8=132.
- Non Standard bearing condition. Review required.



January 30, 2022

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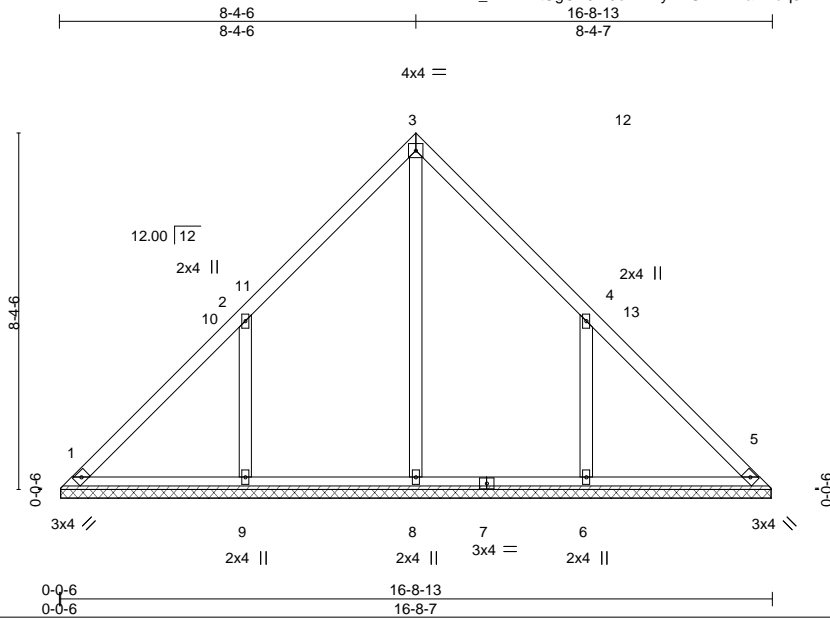


818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 8 Purfoy Place	I49951749
J1121-6539	V3	VALLEY	1	1	Job Reference (optional)	

Comtech, Inc. Fayetteville, NC - 28314.

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:12 2022 Page 1
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Scale = 1:50.9

Plate Offsets (X,Y)-- [4:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.19	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.18	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.15	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S					Weight: 81 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-
 TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 OTHERS 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 16-8-1.
 (lb) - Max Horz 1=192(LC 9)
 Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=201(LC 12), 6=201(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=416(LC 22), 9=523(LC 19), 6=523(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 WEBS 2-9=433/325, 4-6=433/325

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 8-4-6, Exterior(2) 8-4-6 to 12-9-3, Interior(1) 12-9-3 to 16-4-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=201, 6=201.
 - Non Standard bearing condition. Review required.



January 30, 2022

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Job	Truss	Truss Type	Qty	Ply	Lot 8 Purfoy Place	I49951750
J1121-6539	V4	VALLEY	1	1	Job Reference (optional)	

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8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:13 2022 Page 1

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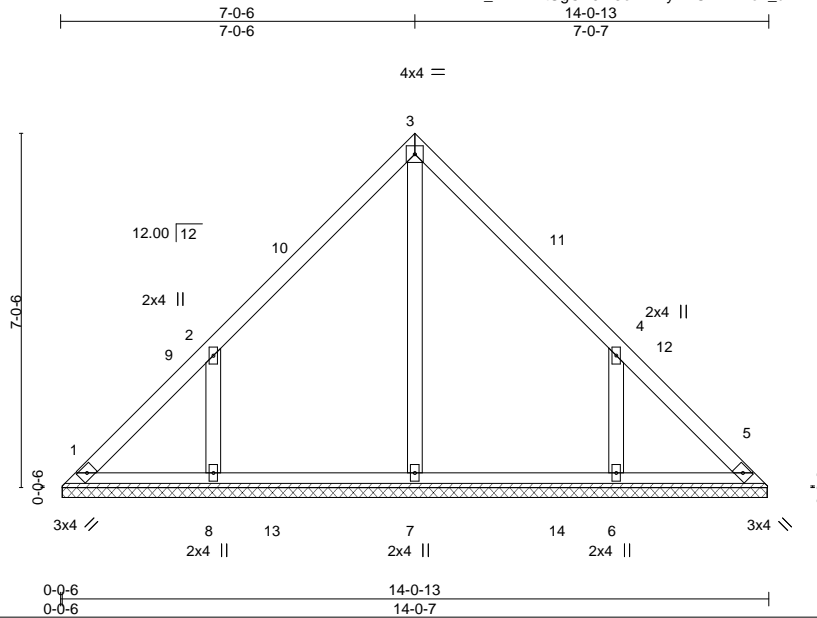


Plate Offsets (X,Y)-- [4:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.14	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.17	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.11	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 5 n/a n/a		
	Code IRC2015/TPI2014			Weight: 66 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-0-1.
(lb) - Max Horz 1=160(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=170(LC 12), 6=170(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=401(LC 19), 8=407(LC 19), 6=407(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=-371/294, 4-6=-371/294

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-0-6, Exterior(2) 7-0-6 to 11-5-3, Interior(1) 11-5-3 to 13-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=170, 6=170.
 - Non Standard bearing condition. Review required.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



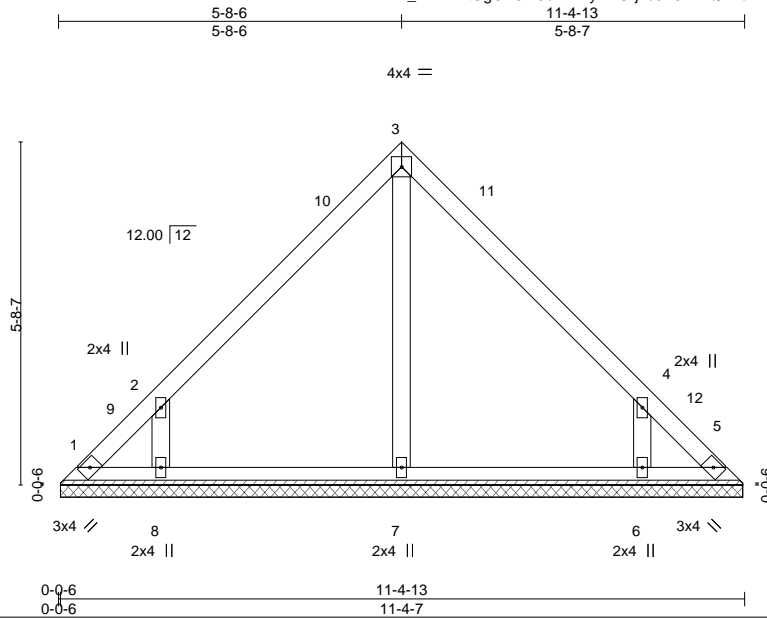
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 8 Purfoy Place	I49951751
J1121-6539	V5	VALLEY	1	1	Job Reference (optional)	

Comtech, Inc. Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:14 2022 Page 1

ID:h_wwFTTtUgCDJr13eLRzilylvhU-jxoJ?O?WQRLdZMZPR_EWOqP1c6PdSPHaXeGWlczqtF3



Scale = 1:36.0

Plate Offsets (X,Y)-- [4:0-0-0,0-0-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.14	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.09	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.07	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S					Weight: 50 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS. All bearings 11-4-1.
(lb) - Max Horz 1=128(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=163(LC 12), 6=163(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=341(LC 19), 6=341(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=365/308, 4-6=365/308

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-8-6, Exterior(2) 5-8-6 to 10-1-3, Interior(1) 10-1-3 to 11-0-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=163, 6=163.
 - Non Standard bearing condition. Review required.



January 30, 2022

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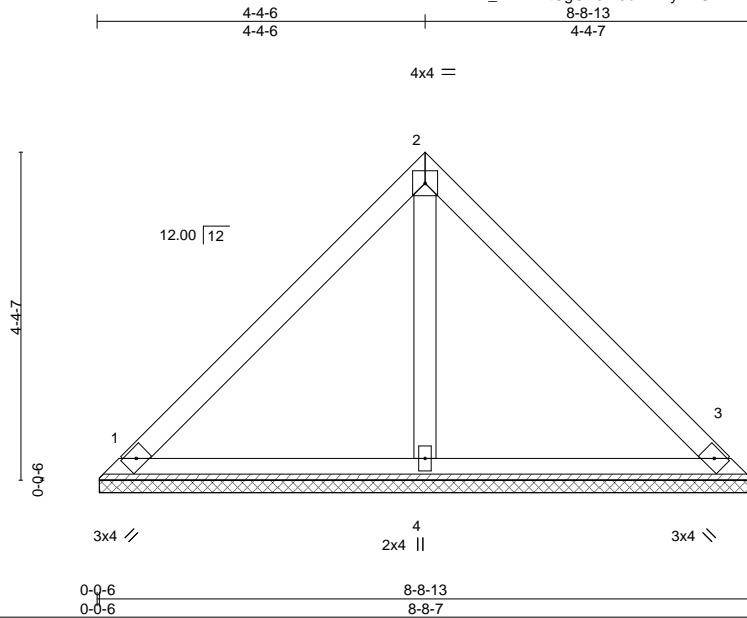


818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss V6	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951752
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Comtech, Inc., Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:15 2022 Page 1
ID:h_wvFTTtUgCDJr13eLRzilylvhU-B7MhDj08BITUBW8b?hlx1y9HWIOBtwjml03I2zqtF2



Scale = 1:28.9

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.28	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.12	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.04	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
	Code IRC2015/TPI2014			Weight: 35 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=8-8-1, 3=8-8-1, 4=8-8-1
Max Horz 1=96(LC 11)
Max Uplift 1=35(LC 13), 3=35(LC 13)
Max Grav 1=196(LC 1), 3=196(LC 1), 4=251(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- Non Standard bearing condition. Review required.



January 30, 2022

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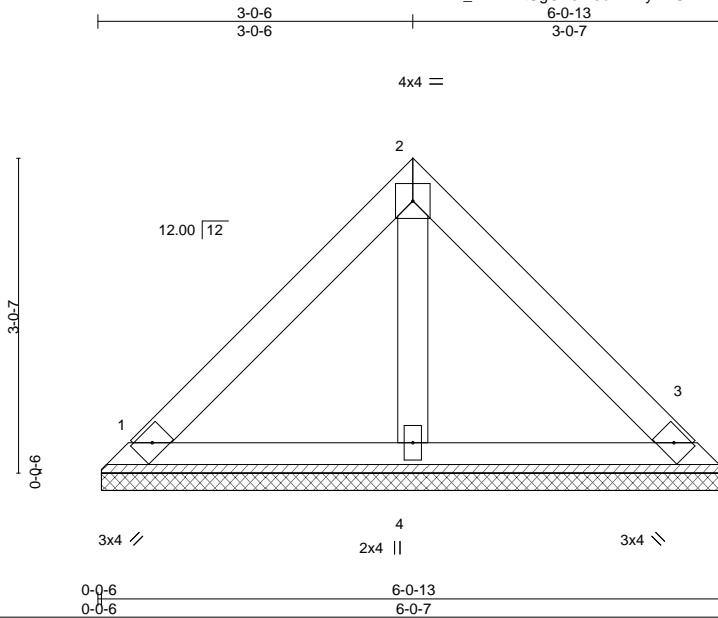


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Job J1121-6539	Truss V7	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951753
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Comtech, Inc., Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:15 2022 Page 1
ID:h_wwFTTtUgCDJr13eLRzilylvhU-B7MhDj08BITUBW8b?hlx1yCmWmQBtJml03l2zqtF2



Scale = 1:20.9

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.12	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.05	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.02	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
	Code IRC2015/TPI2014			Weight: 24 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=6-0-1, 3=6-0-1, 4=6-0-1
Max Horz 1=64(LC 8)
Max Uplift 1=23(LC 13), 3=23(LC 13)
Max Grav 1=131(LC 1), 3=131(LC 1), 4=168(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- Non Standard bearing condition. Review required.



January 30, 2022

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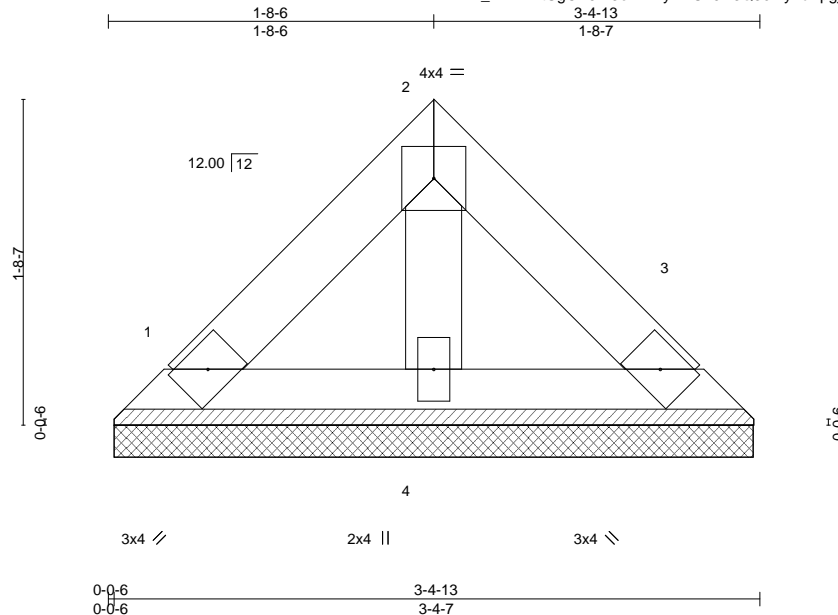


818 Soundside Road
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Job J1121-6539	Truss V8	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951754
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8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:16 2022 Page 1
ID:h_wwFTTtUgCDJr13eLRzilylvhU-fJw3Q30my2bKpgjnZPG_TFUOxv6HwKkt?yldqUzqtF1



Scale = 1:11.3

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.03	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.01	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.01	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
	Code IRC2015/TPI2014			Weight: 12 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-4-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=3-4-1, 3=3-4-1, 4=3-4-1
Max Horz 1=32(LC 9)
Max Uplift 1=12(LC 13), 3=12(LC 13)
Max Grav 1=66(LC 1), 3=66(LC 1), 4=84(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- Non Standard bearing condition. Review required.



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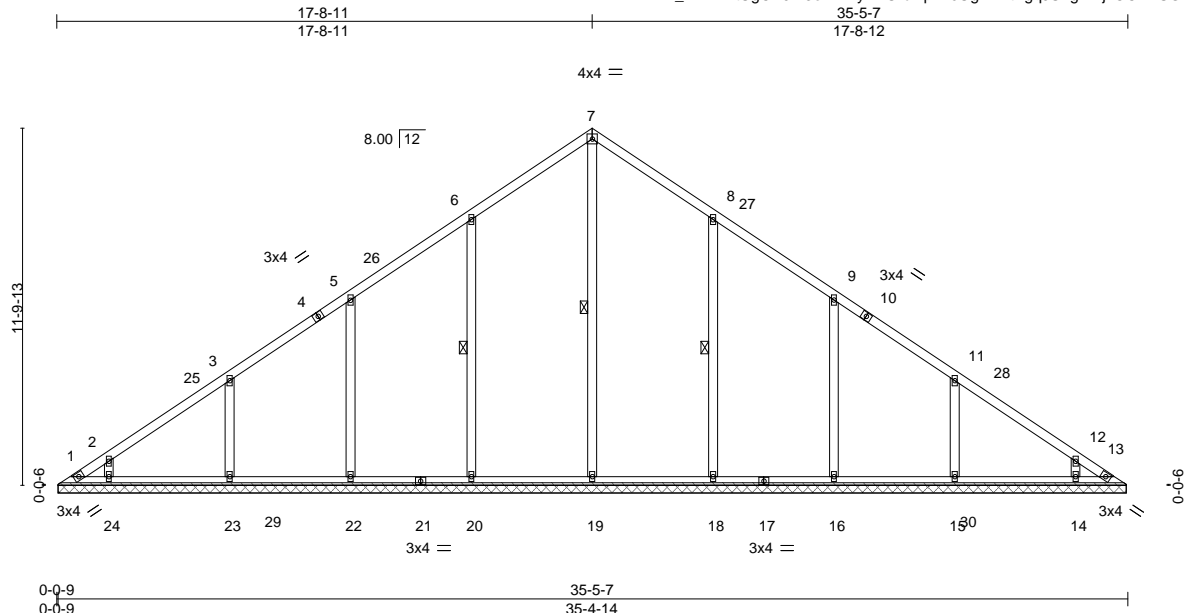


818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss VB1	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951755
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:18 2022 Page 1
ID:h_wwFTTtUgCDJr13eLRzilylvhU-bi2pri20Ugr22ztAgqJSZgZiXjllOCPASGEjuNzqtF?



Scale = 1:71.8

Plate Offsets (X,Y)--	[8:0-0-0,0-0-0], [9:0-0-0,0-0-0], [10:0-0-0,0-0-0], [11:0-0-0,0-0-0], [12:0-0-0,0-0-0]
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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.17	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.19	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 13 n/a n/a		
	Code IRC2015/TPI2014			Weight: 186 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x4 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.2	WEBS 1 Row at midpt 7-19, 6-20, 8-18

REACTIONS. All bearings 35-4-5.
 (lb) - Max Horz 1=276(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) 22, 23, 24, 16, 15, 14, 13 except 1=134(LC 10), 20=101(LC 12), 18=101(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 1, 13 except 19=466(LC 22), 20=540(LC 19), 22=526(LC 19), 23=425(LC 19), 24=270(LC 19), 18=540(LC 20), 16=527(LC 20), 15=425(LC 20), 14=269(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=284/235, 6-7=297/281, 7-8=297/281
 WEBS 6-20=307/201, 5-22=287/160, 3-23=301/183, 2-24=250/184, 8-18=307/201, 9-16=287/160, 11-15=301/183, 12-14=250/184

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 17-8-11, Exterior(2) 17-8-11 to 22-1-8, Interior(1) 22-1-8 to 34-11-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - All plates are 2x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 22, 23, 24, 16, 15, 14, 13 except (jt=lb) 1=134, 20=101, 18=101.
 - Non Standard bearing condition. Review required.



January 30, 2022

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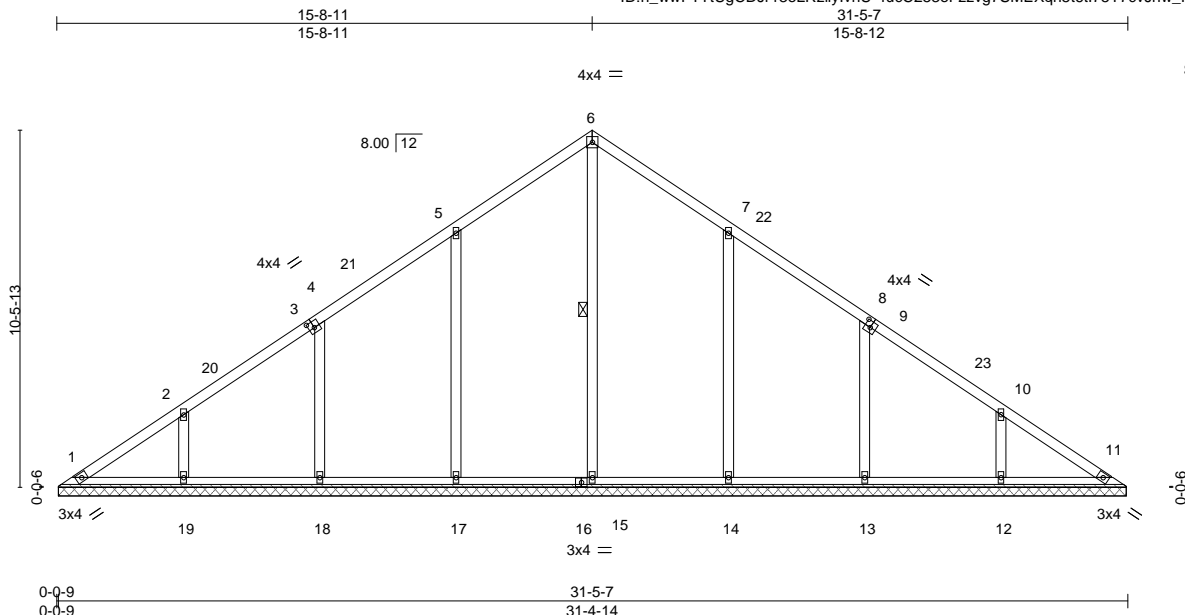
818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss VB2	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951756
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:19 2022 Page 1

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Scale: 3/16"=1'

Plate Offsets (X,Y)-- [3:0-2-0,0-2-4], [7:0-0-0,0-0-0], [9:0-2-0,0-2-4], [10:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.17	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.30	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 11 n/a n/a		
	Code IRC2015/TPI2014			Weight: 157 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 OTHERS 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 6-15

REACTIONS. All bearings 31-4-5.
 (lb) - Max Horz 1=244(LC 11)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 18, 19, 13, 12 except 17=-102(LC 12), 14=-101(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 1, 11 except 15=447(LC 22), 17=556(LC 19), 18=429(LC 19), 19=334(LC 19), 14=555(LC 20), 13=429(LC 20), 12=334(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 5-6=-264/248, 6-7=-264/248
 WEBS 5-17=-307/202, 4-18=-290/164, 2-19=-289/192, 7-14=-307/201, 8-13=-290/164, 10-12=-289/192

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 15-8-11, Exterior(2) 15-8-11 to 20-1-8, Interior(1) 20-1-8 to 30-11-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - All plates are 2x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCCL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 18, 19, 13, 12 except (jt=lb) 17=102, 14=101.
 - Non Standard bearing condition. Review required.



January 30, 2022

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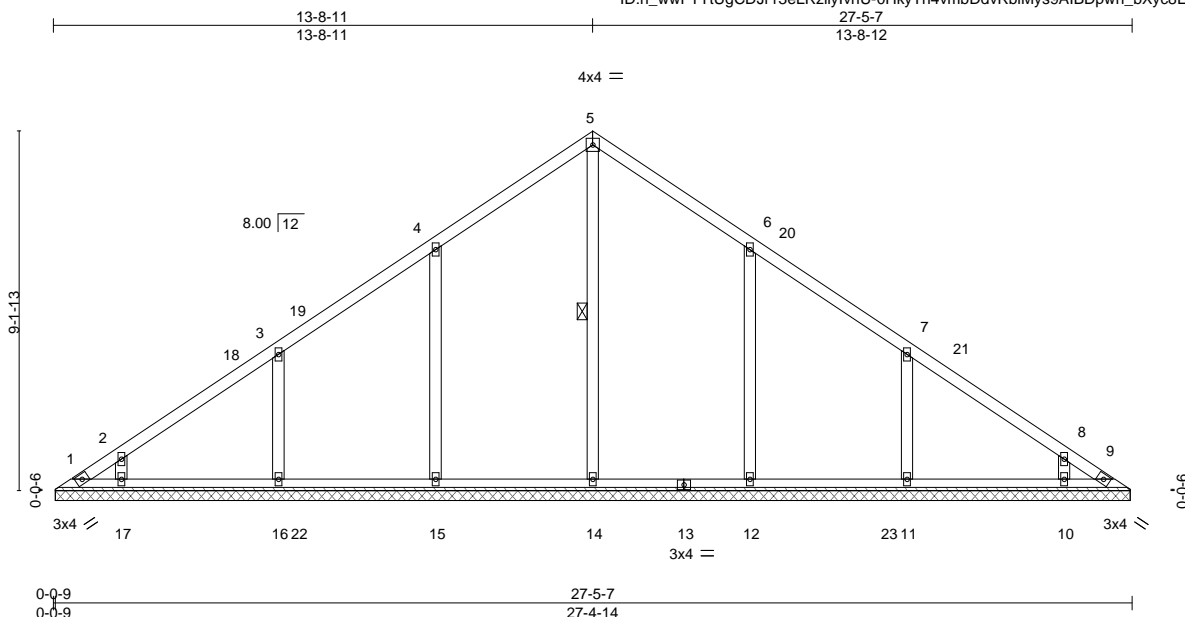


818 Soundside Road
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Job J1121-6539	Truss VB3	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951757
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:21 2022 Page 1
ID:h_wvFTTtUgCDJr13eLRzilylvhU-0HkyTn4vmbDdvRblMys9AIBDpwn_bXyc8ETOuizqtEy



Scale = 1:55.2

Plate Offsets (X,Y)-- [6:0-0-0,0-0-0], [7:0-0-0,0-0-0], [8:0-0-0,0-0-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.15	TC 0.15	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.17	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.20	Horz(CT)	0.00	9	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S							
								Weight: 130 lb	FT = 20%	

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-14

REACTIONS.

All bearings 27-4-5.
(lb) - Max Horz 1=212(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 9, 16, 17, 11, 10 except 1=105(LC 10), 15=101(LC 12), 12=101(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 9 except 14=436(LC 22), 15=551(LC 19), 16=418(LC 19), 17=272(LC 19), 12=551(LC 20), 11=418(LC 20), 10=272(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 4-15=-306/201, 3-16=-297/181, 2-17=-252/184, 6-12=-306/201, 7-11=-297/181, 8-10=-252/184

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 13-8-11, Exterior(2) 13-8-11 to 18-1-8, Interior(1) 18-1-8 to 26-11-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 16, 17, 11, 10 except (jt=lb) 1=105, 15=101, 12=101.
- Non Standard bearing condition. Review required.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

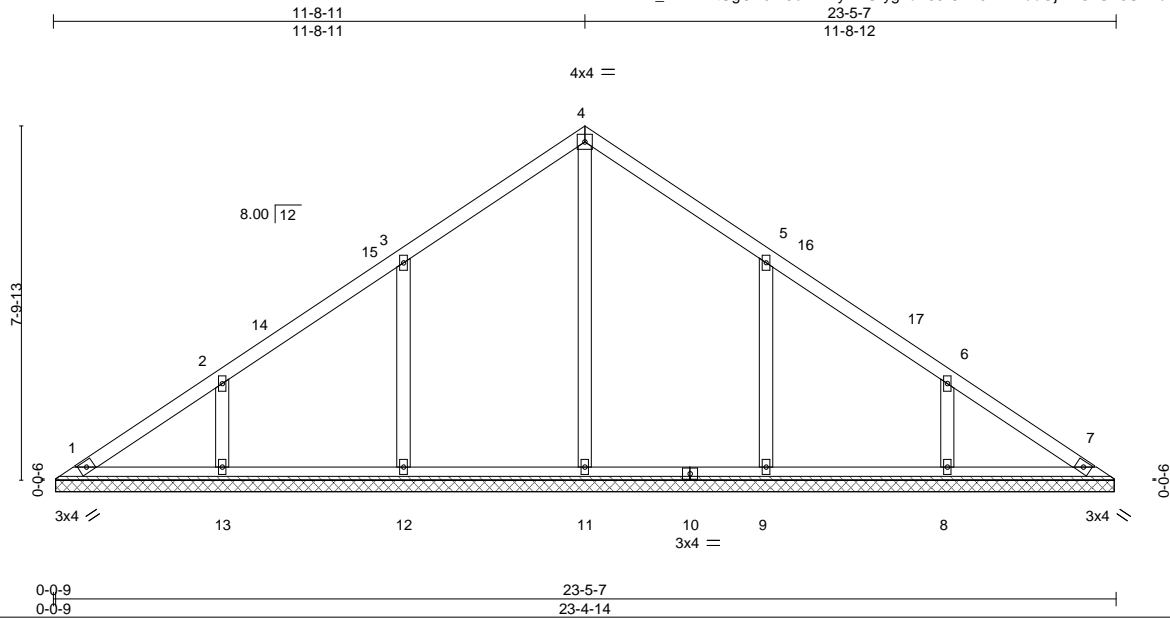


818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss VB4	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951758
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:23 2022 Page 1
 ID:h_wwFTTtUgCDJr13eLRzilylvhU-ygriuT69ICTL9II7TNudGjHZGkSB3SwcXyUZazqtEw
 23-5-7
 11-8-12



Scale: 1/4"=1'

Plate Offsets (X,Y)-- [5:0-0-0,0-0-0], [6:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.19	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.17	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 7 n/a n/a		
	Code IRC2015/TPI2014			Weight: 105 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 OTHERS 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 23-4-5.
 (lb) - Max Horz 1=180(LC 10)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 8 except 12=102(LC 12), 9=102(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=454(LC 22), 12=450(LC 19), 13=329(LC 19),
 9=450(LC 20), 8=329(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 WEBS 3-12=310/204, 2-13=284/191, 5-9=310/204, 6-8=284/191

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 11-8-11, Exterior(2) 11-8-11 to 16-1-8, Interior(1) 16-1-8 to 22-11-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - All plates are 2x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 8 except (jt=lb) 12=102, 9=102.
 - Non Standard bearing condition. Review required.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIP/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



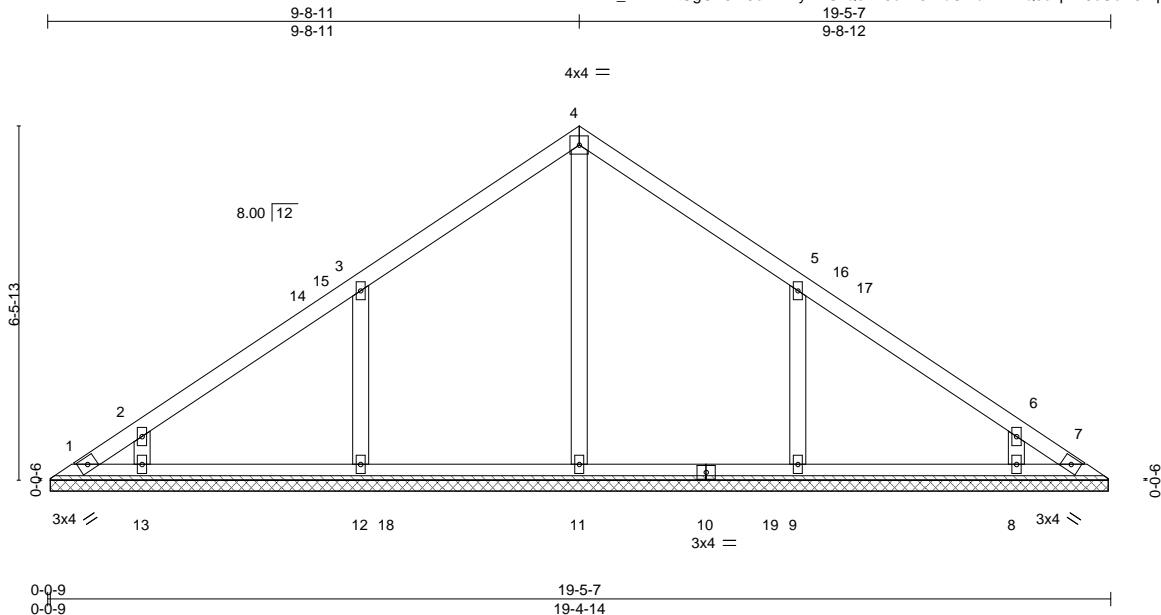
818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss VB5	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951759
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Comtech, Inc., Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:24 2022 Page 1

ID:h_wwFTTtUgCDJr13eLRzilylvhU-QsP45o7n3WbCmuKK14Qsoxpkv8oUow32qBh251zqtEv



Scale = 1:39.7

Plate Offsets (X,Y)-- [5:0-0-0,0-0-0], [6:0-0-0,0-0-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.15	TC 0.16	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.18	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.11	Horz(CT)	0.00	7	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S						Weight: 82 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS. All bearings 19-4-5.
(lb) - Max Horz 1=148(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 13, 8 except 12=106(LC 12), 9=105(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=442(LC 19), 12=441(LC 19), 13=263(LC 19),
9=441(LC 20), 8=263(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-12=318/209, 5-9=318/209

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 9-8-11, Exterior(2) 9-8-11 to 14-1-8, Interior(1) 14-1-8 to 18-11-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 13, 8 except (jt=lb) 12=106, 9=105.
- Non Standard bearing condition. Review required.



January 30,2022

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818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss VB6	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951760
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:25 2022 Page 1
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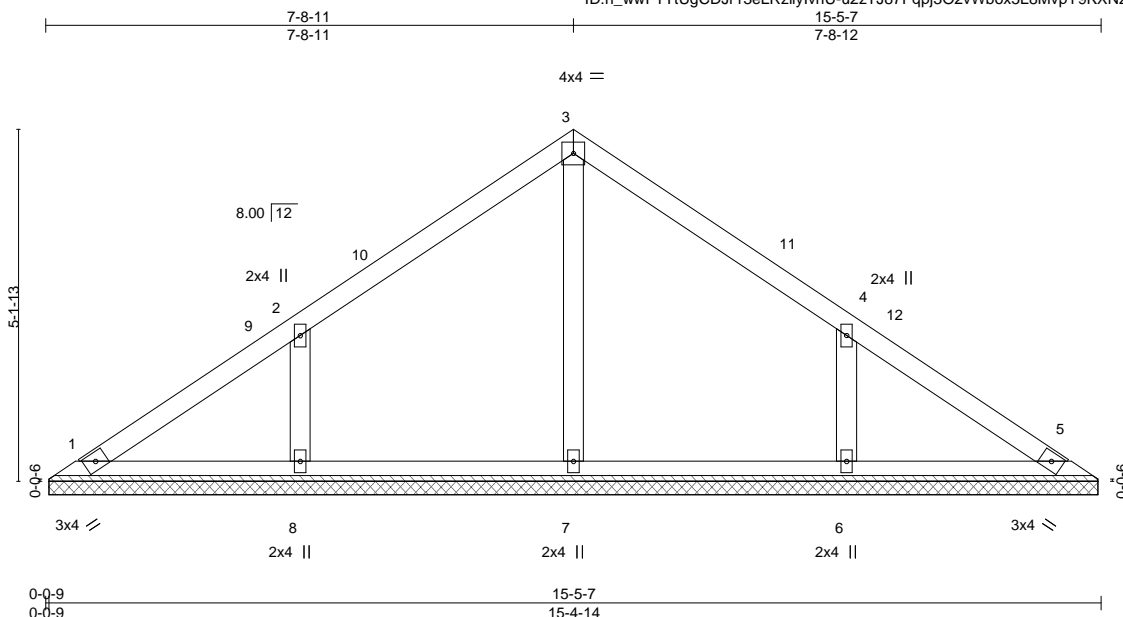


Plate Offsets (X,Y)-- [4:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.14	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.08	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.07	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 5 n/a n/a		
	Code IRC2015/TPI2014			Weight: 61 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-4-5.
(lb) - Max Horz 1=116(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=104(LC 12), 6=104(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=252(LC 1), 8=365(LC 19), 6=365(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=309/206, 4-6=309/206

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 7-8-11, Exterior(2) 7-8-11 to 12-1-8, Interior(1) 12-1-8 to 14-11-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=104, 6=104.
 - Non Standard bearing condition. Review required.



January 30, 2022

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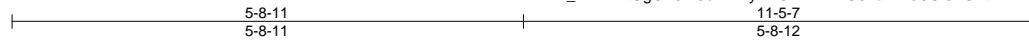
818 Soundside Road
Edenton, NC 27932

Job J1121-6539	Truss VB7	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951761
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:26 2022 Page 1

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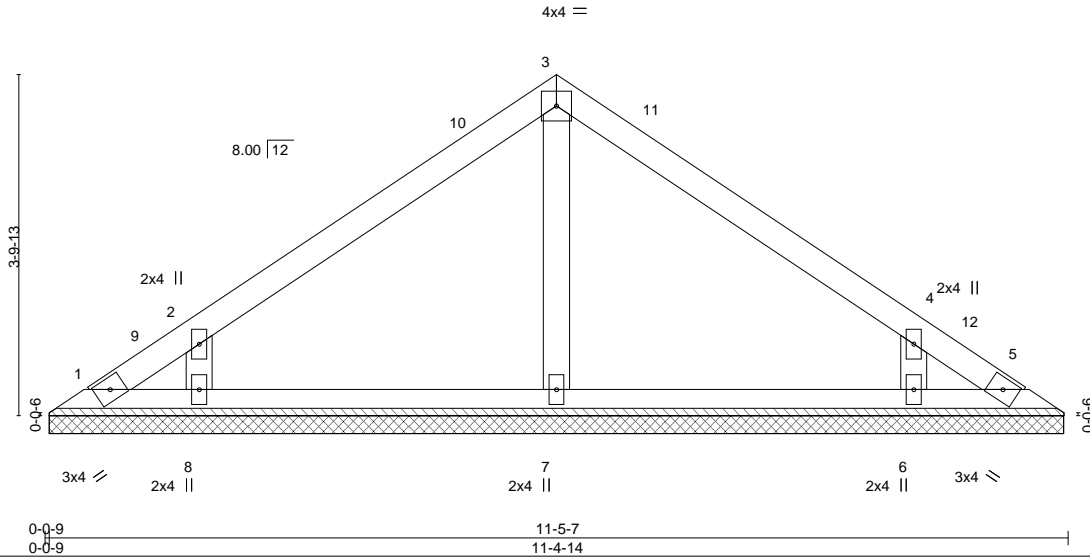


Plate Offsets (X,Y)-- [4:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.13	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.09	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.04	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 5 n/a n/a		
	Code IRC2015/TPI2014			Weight: 42 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 OTHERS 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS. All bearings 11-4-5.
 (lb) - Max Horz 1=84(LC 11)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6
 Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=262(LC 1), 8=321(LC 19), 6=320(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 WEBS 2-8=-290/215, 4-6=-290/215

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 5-8-11, Exterior(2) 5-8-11 to 10-1-8, Interior(1) 10-1-8 to 10-11-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.
 - Non Standard bearing condition. Review required.



January 30, 2022

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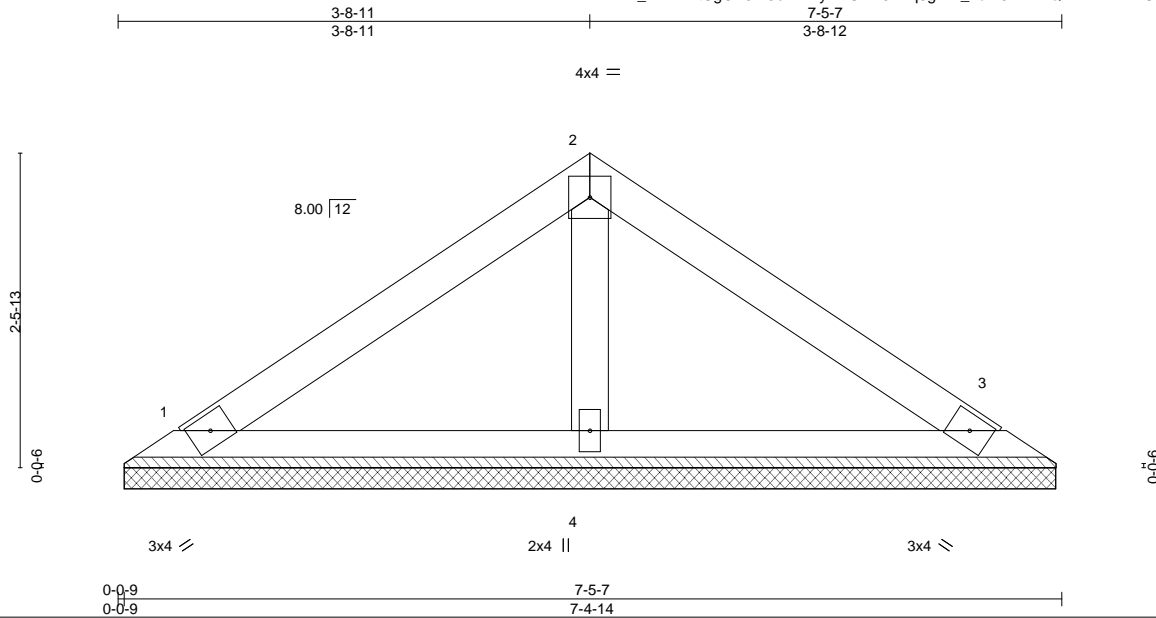


818 Soundside Road
 Edenton, NC 27932

Job J1121-6539	Truss VB8	Truss Type VALLEY	Qty 1	Ply 1	Lot 8 Purfoy Place Job Reference (optional)	I49951762
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Comtech, Inc, Fayetteville, NC - 28314,

8,430 s Aug 16 2021 MiTek Industries, Inc. Fri Jan 28 09:01:27 2022 Page 1
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Scale = 1:17.1

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.14	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.08	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.02	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
	Code IRC2015/TPI2014			Weight: 25 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=7-4-5, 3=7-4-5, 4=7-4-5
Max Horz 1=52(LC 8)
Max Uplift 1=22(LC 12), 3=27(LC 13)
Max Grav 1=141(LC 1), 3=141(LC 1), 4=236(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- Non Standard bearing condition. Review required.



January 30, 2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

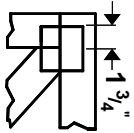
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



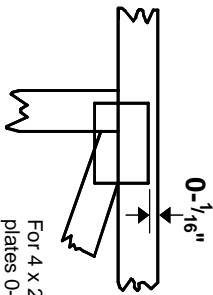
818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20** software or upon request.

PLATE SIZE

4 X 4

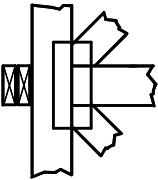
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



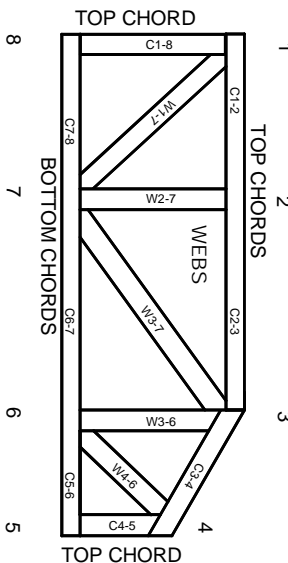
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

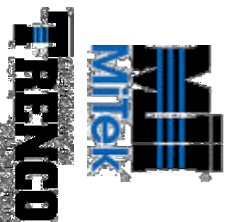
ICC-ES Reports:

ESR-1311, ESR-1352, ESR 1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: Mill-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.