### PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

MEAN ROOF HEIGHT: 18'-0	HEIGHT TO R	RIDGE:23'-10						
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A					
FENESTRATION U-FACTOR	0.35	0.35	0.35					
SKYLIGHT U-FACTOR	0.55	0.55	0.55					
GLAZED FENESTRATION SHGC	0.30	0.30	0.30					
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci					
WALL R-VALUE	15	15	19					
FLOOR R-VALUE	19	19	30					
* BASEMENT WALL R-VALUE	5/13	10/15	10/15					
** SLAB R-VALUE	Ö	10	10					
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19					

\* "10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION \*\* INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF

FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL DESIGNED FOR WIND SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B"

		•••••••			1						
COMPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS											
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'			
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8			
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2			
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2			
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9			
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4			
DESIGNED FOR WIN	DESIGNED FOR WIND SPEED OF 130 MPH, 3 SECOND GUST (101 FASTEST MILE) EXPOSURE "B"										
COMPONENT	& CLA	DDING	DESIG	NED FC	R THE	FOLLO	WING	LOADS			

COMPONENT	& CLA	DDING	DESIG	NED FC	<u>)r the</u>	FOLLO	WING	LOADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE 5	18.2	-24.0	19.1	-25.2	19.8	-26.2	20.4	-26.9

# **ROOF VENTILATION**

### SECTION R806

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Sinclair

Sinclair/200606B

Inc\200606B

**Builders**,

Home

Z:\Builder\Signature

**R806.1 Ventilation required.** Enclosed *attics* and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware doth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7.

R806.2 Minimum area. The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1/300 when a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling. Exceptions:

1. Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only. 2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,619 SQ.FT.

NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 17.46 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 8.73 SQ.FT.

# **GUARD RAIL NOTES**

### SECTION R312

**R312.1 Where required.** *Guards* shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or grade below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a guard.

R312.2 Height. Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads. Exceptions:

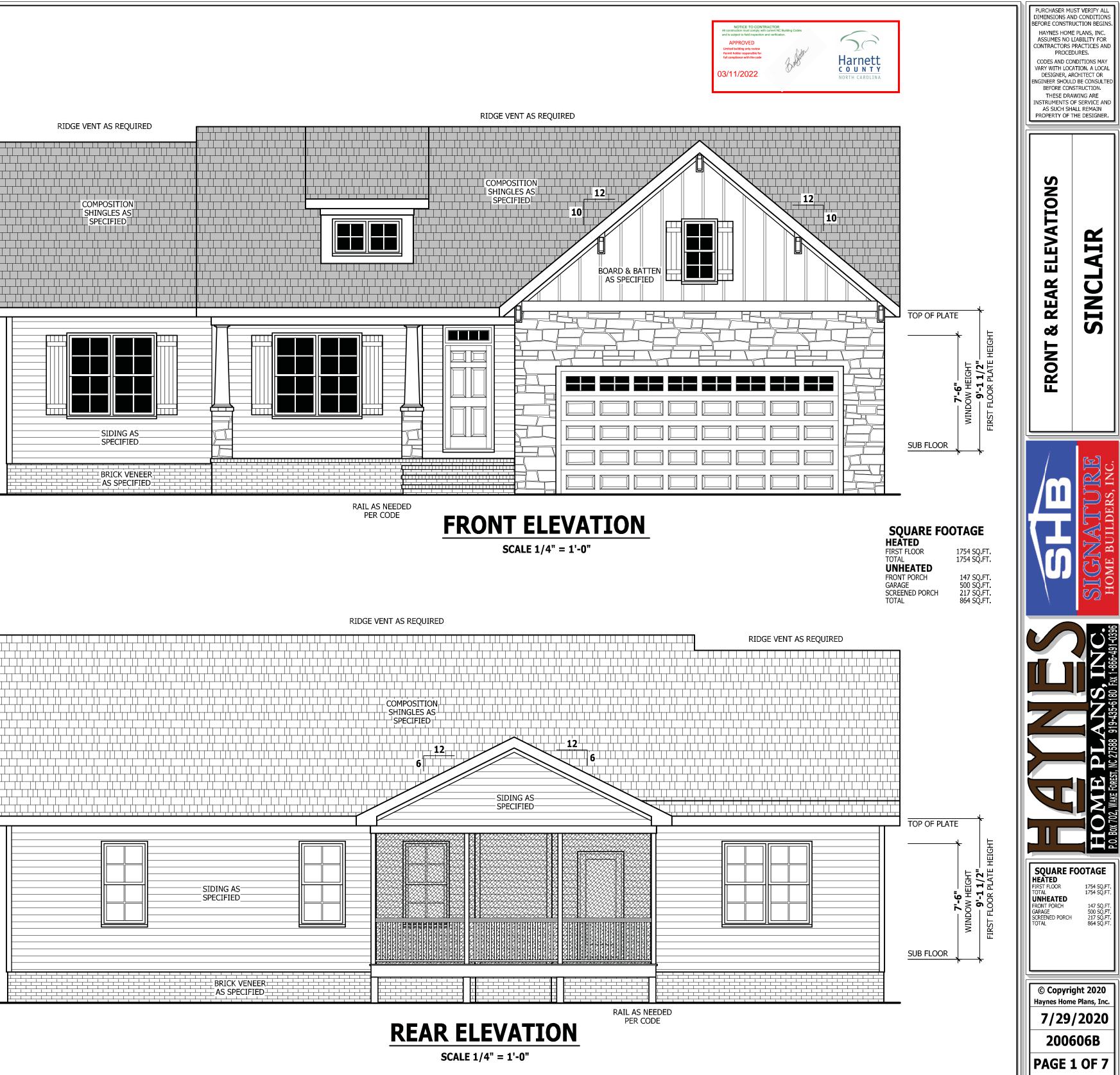
1. *Guards* on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the treads.

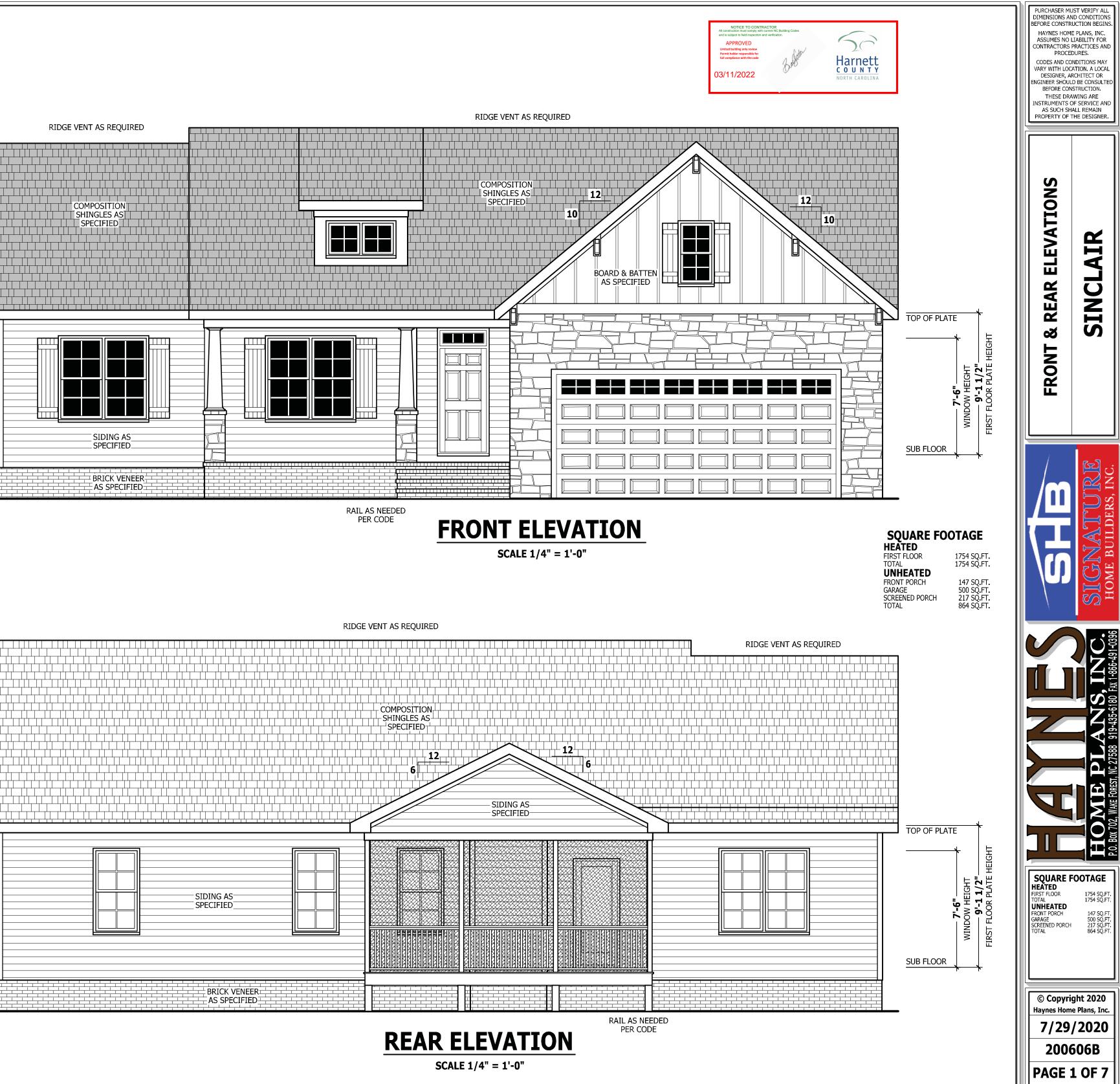
2. Where the top of the *guard* also serves as a handrail on the open sides of stairs, the top of the *guard* shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

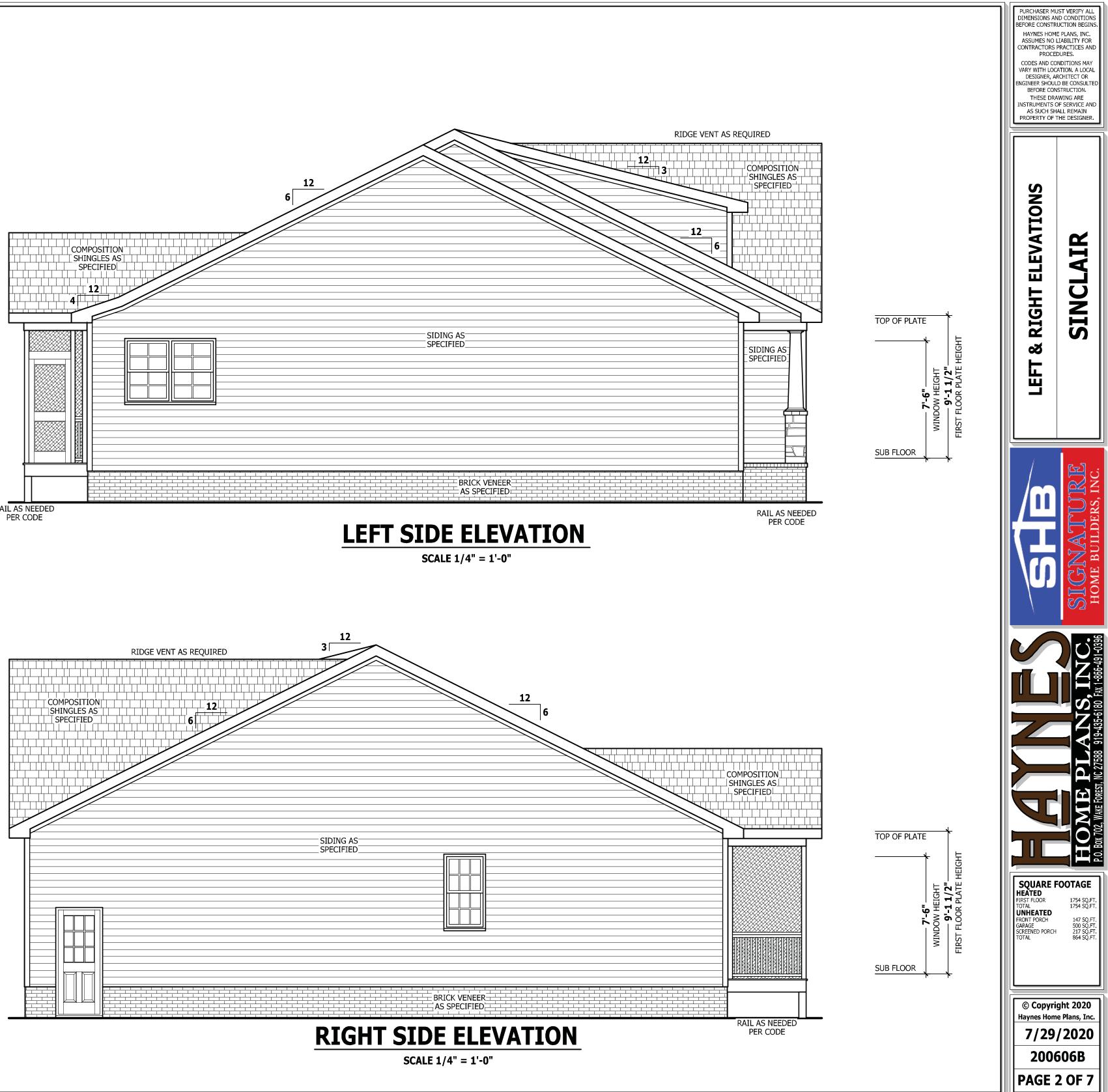
R312.3 Opening limitations. Required guards shall not have openings from the walking surface to the required *guard* height which allow passage of a sphere 4 inches (102 mm)in diameter Exceptions:

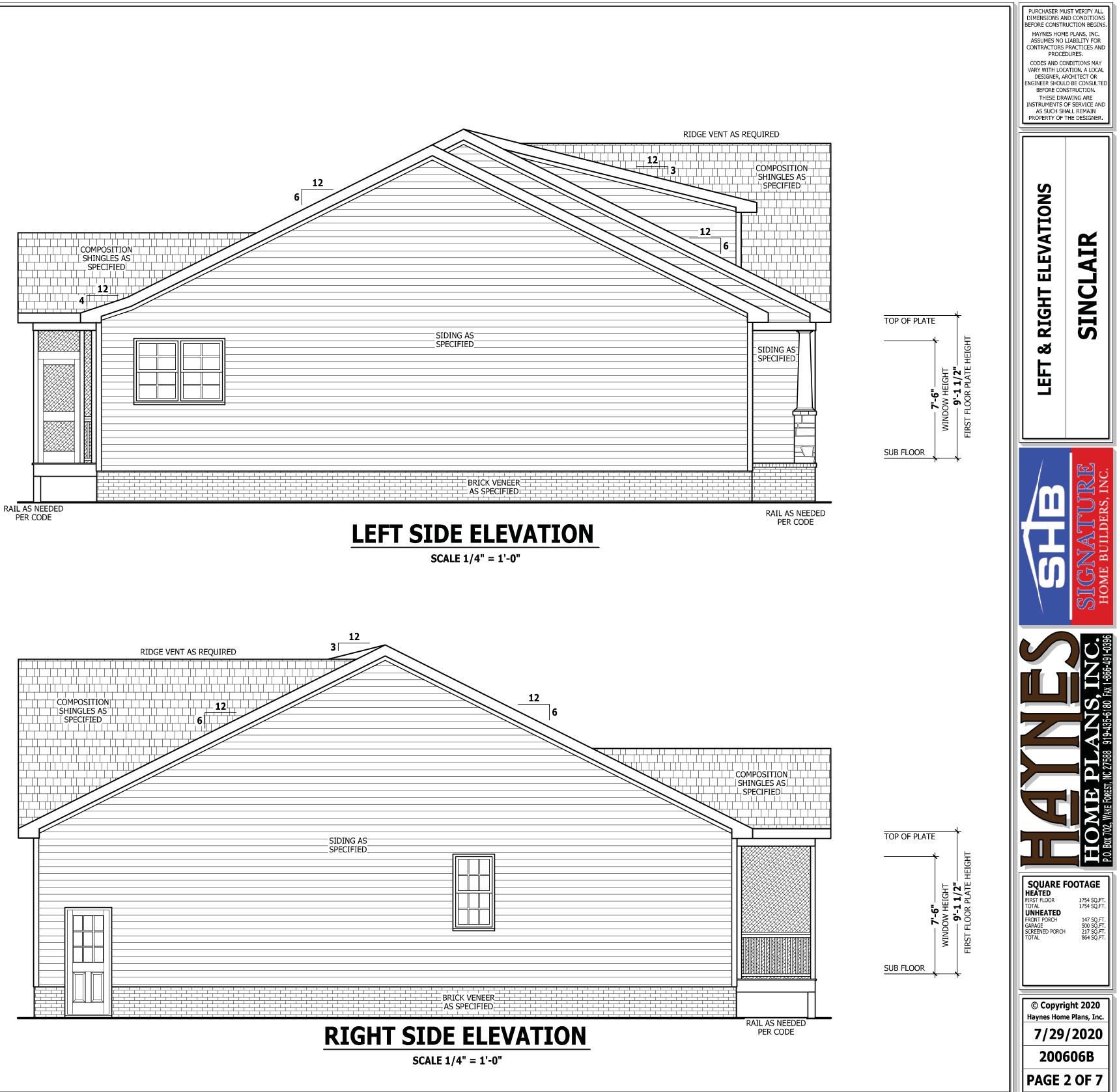
1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 mm) in diameter.

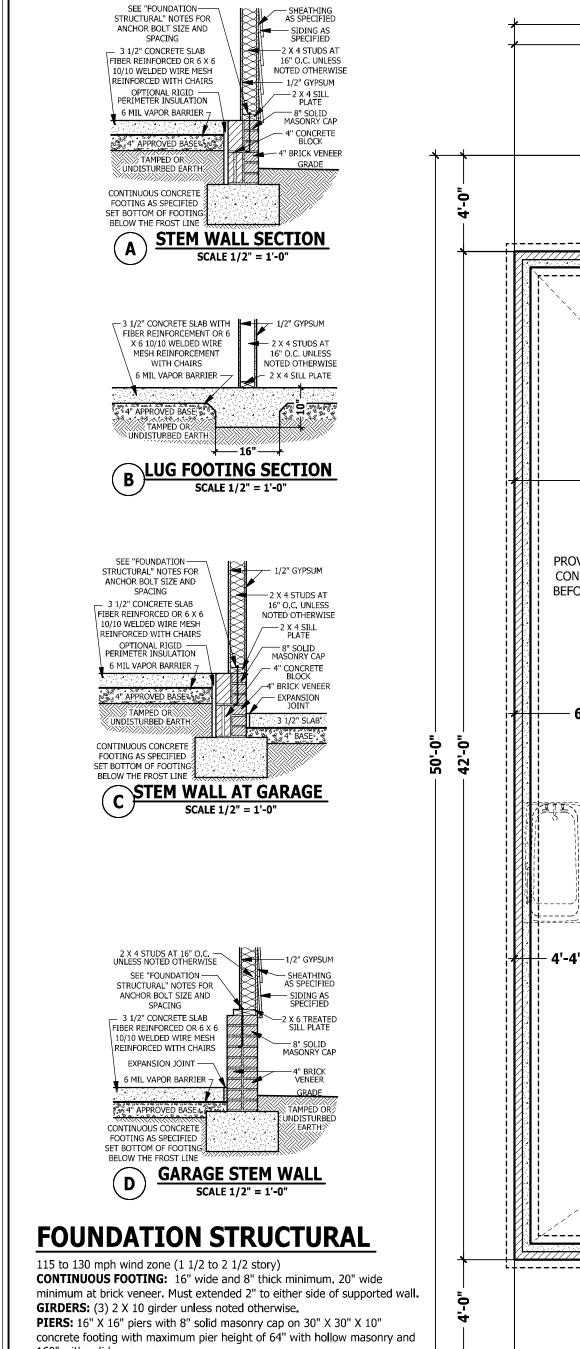
2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 43/8 inches (111 mm) in diameter.











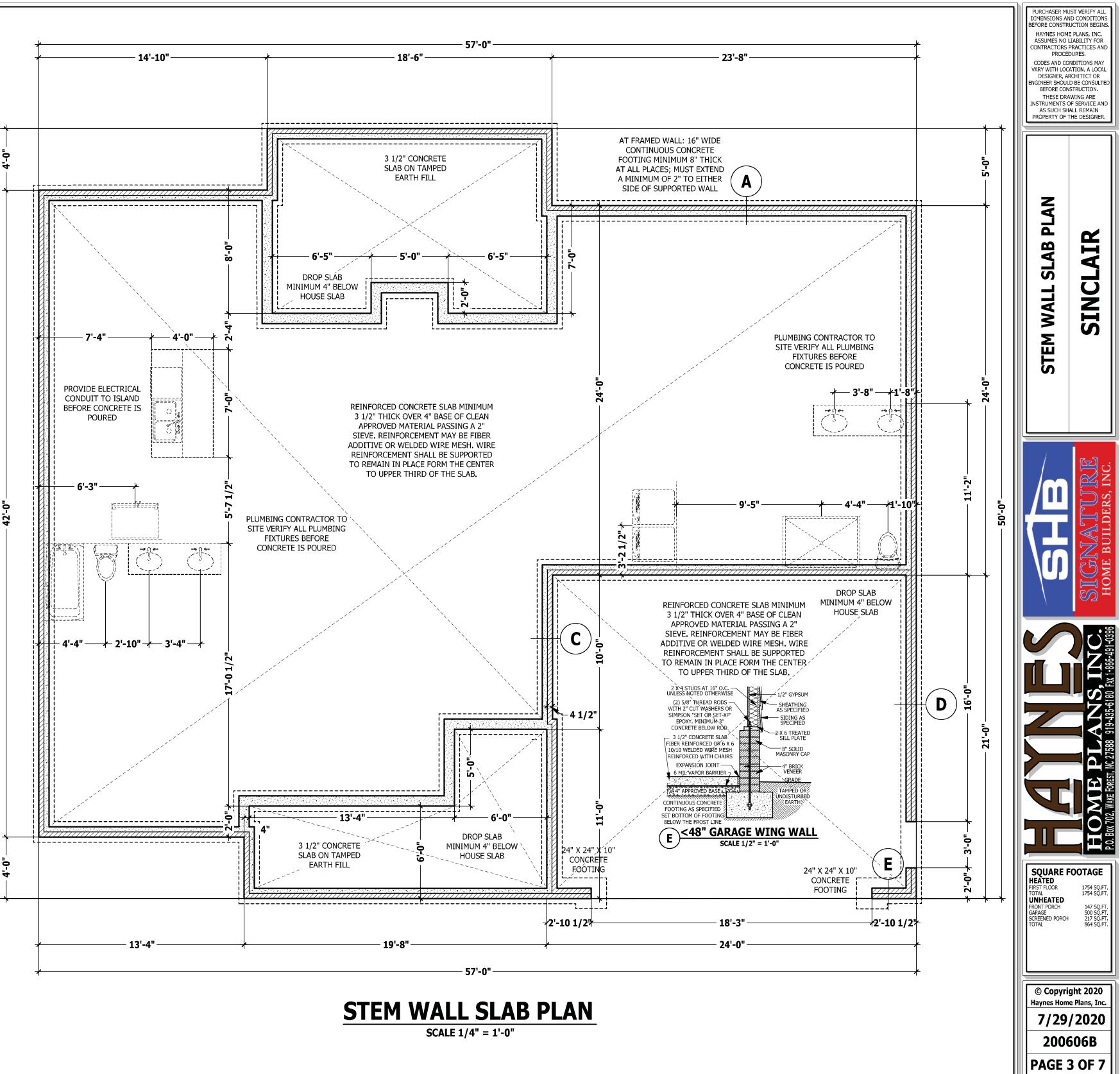
160" with solid masonry. **POINT LOADS:** designates significant point load and should have solid blocking to pier, girder or foundation wall.

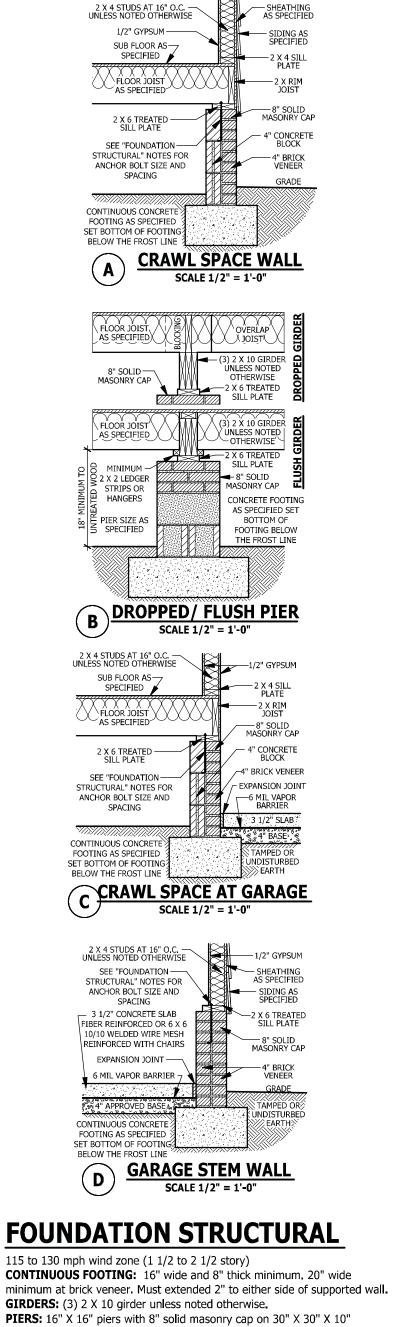
**115 and 120 MPH ANCHORS BOLTS:** 1/2" diameter anchor bolts embedded minimum 7", maximum 6'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

**130 MPH ANCHORS BOLTS:** 1/2" diameter anchor bolts embedded minimum 15", maximum 4'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

**CONCRETE:** Concrete shall have a minimum 28 day strength of 3000 psi and a maximum 5" slump. Air entrained per table 402.2. All concrete shall be in accordance with ACI standards. All samples for pumping shall be taken from the exit end of the pump. **SOILS:** Allowable soil bearing pressure assumed to be 2000 PSF. The

contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage, and shall be graded so as to drain surface water away from foundation walls.





PIERS: 16" X 16" piers with 8" solid masonry cap on 30" X 30" X 10" concrete footing with maximum pier height of 64" with hollow masonry and 160" with solid masonry. **POINT LOADS:** designates significant point load and should have solid

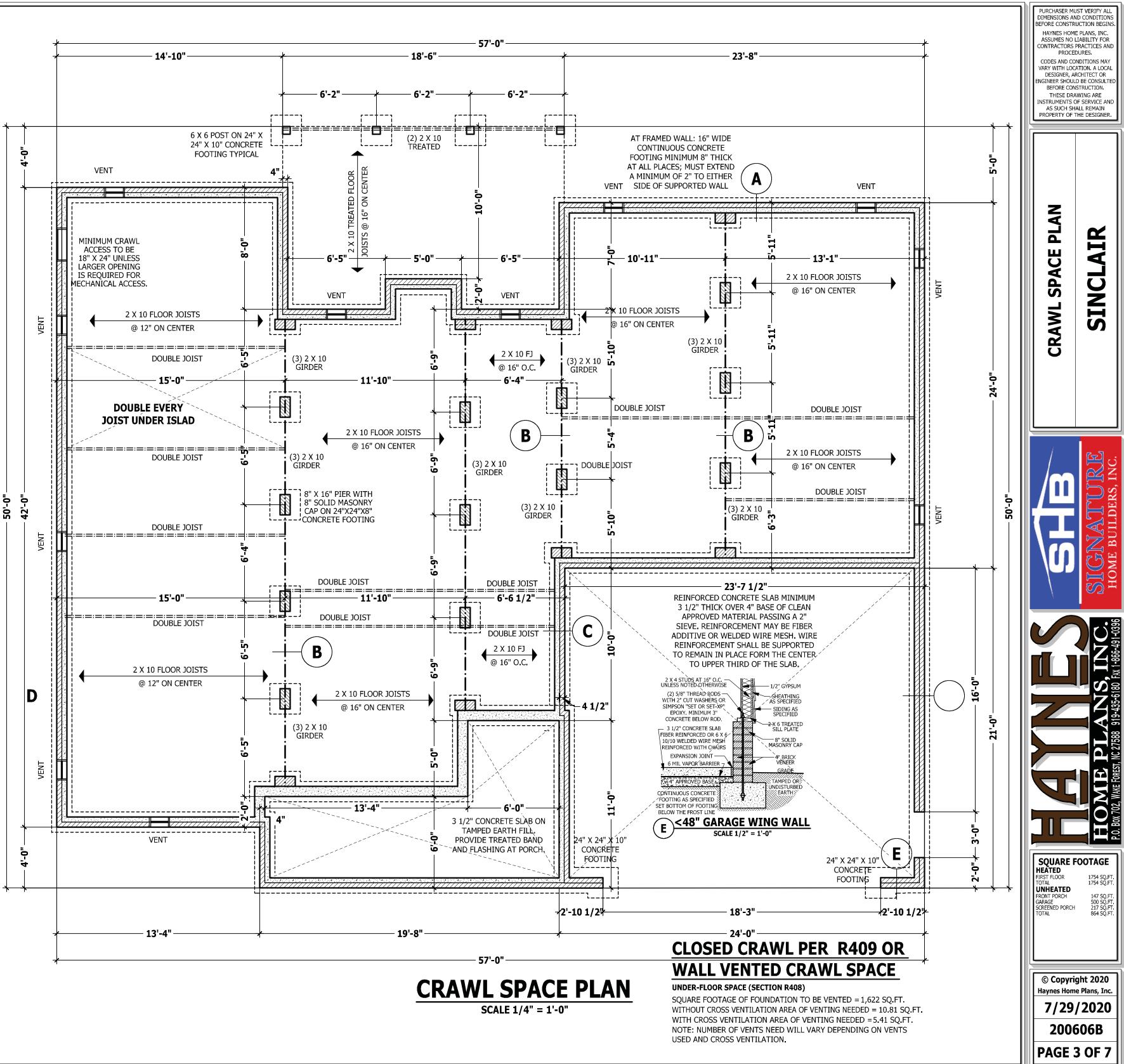
blocking to pier, girder or foundation wall. **115 and 120 MPH ANCHORS BOLTS:** 1/2" diameter anchor bolts embedded

minimum 7", maximum 6'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate. **130 MPH ANCHORS BOLTS:** 1/2" diameter anchor bolts embedded minimum

15", maximum 4'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

**CONCRETE:** Concrete shall have a minimum 28 day strength of 3000 psi and a maximum 5" slump. Air entrained per table 402.2. All concrete shall be in accordance with ACI standards. All samples for pumping shall be taken from the exit end of the pump. SOILS: Allowable soil bearing pressure assumed to be 2000 PSF. The

contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage, and shall be graded so as to drain surface water away from foundation walls.





# WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for stud face.

**Interior walls** are drawn as 3 1/2" or as noted 2 X 6 are drawn as 5 1/2", and do not include gypsum.

## **DWELLING / GARAGE SEPARATION**

REFER TO SECTIONS R302.5, R302.6, AND R302.7 **WALLS.** A minimum 1/2" gypsum board must be installed on all walls supporting floor/ceiling assemblies used for separation required by this section.

**STAIRS.** A minimum of 1/2" gypsum board must be installed on the underside and exposed sides of all stairways. **CEILINGS.** A minimum of 1/2" gypsum must be installed on the garage ceiling if there are no habitable room above the garage. If there are habitable room above the garage a minimum of 5/8" type X gypsum board must be installed on the garage ceiling.

**OPENING PENETRATIONS.** Openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute fire-rated doors.

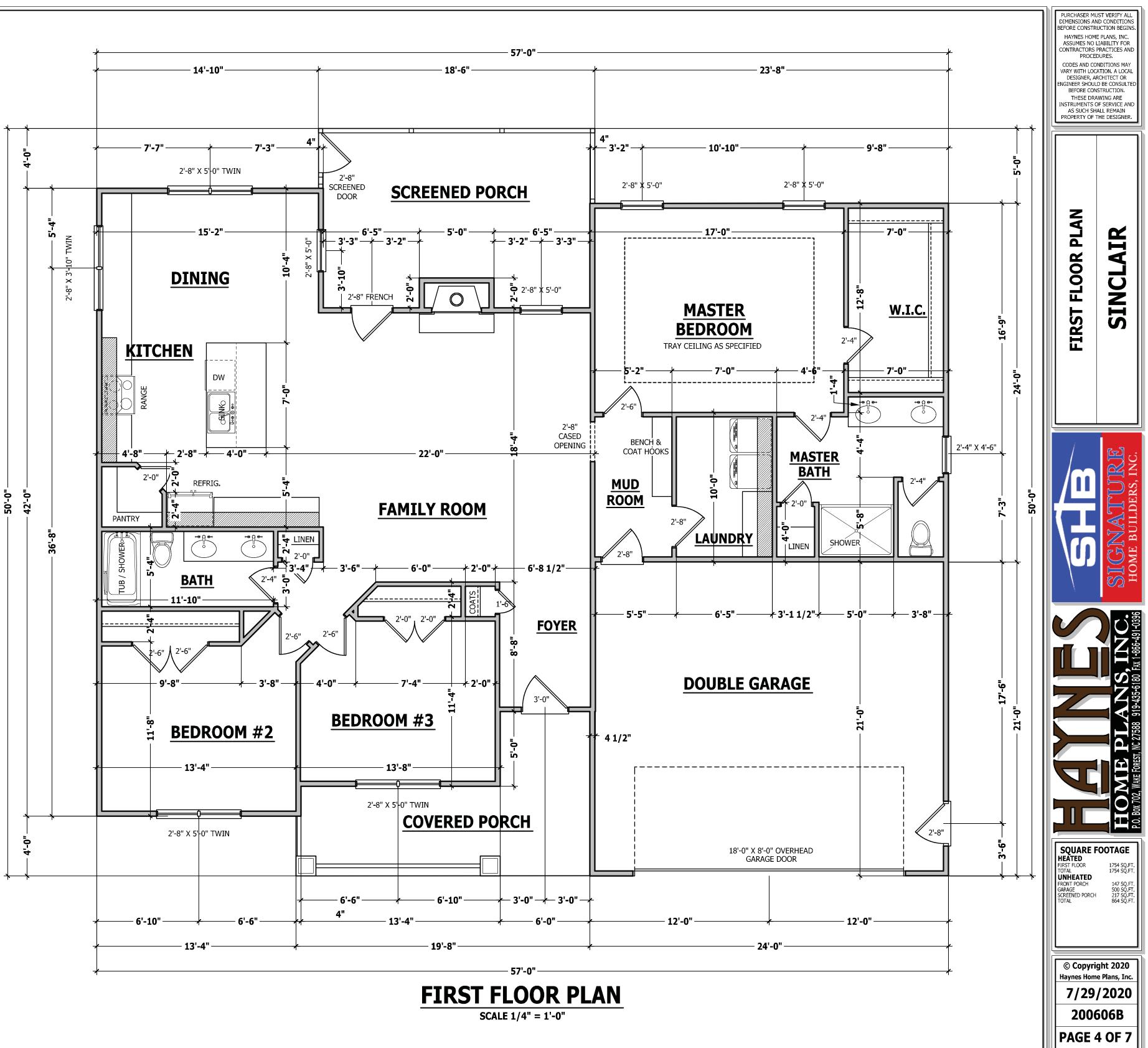
**DUCT PENETRATIONS.** Ducts in the garage and ducts penetrating the walls or ceilings separating the *dwelling* from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other *approved* material and shall have no openings into the garage.

OTHER PENETRATIONS. Penetrations through the separation required in Section R302.6 shall be protected as required by Section R302.11, Item 4.



TOTAL UNHEATED FRONT PORCH GARAGE SCREENED PORCH TOTAL





## **STRUCTURAL NOTES**

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise. ENGINEERED WOOD BEAMS :

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10<sup>6</sup> PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise.  $3 \frac{1}{2} \times 3 \frac{1}{2} \times \frac{1}{4}$ " steel angle with  $\frac{1}{2}$ " bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. **ROOF SHEATHING:** OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters. **CONCRETE AND SOILS:** See foundation notes

## **BRACE WALL PANEL NOTES**

**EXTERIOR WALLS:** All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

**GYPSUM:** All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

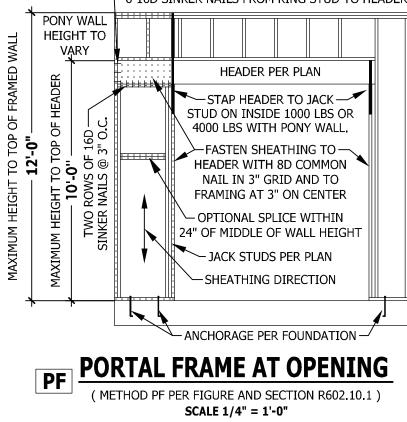
**REQUIRED LENGTH OF BRACING:** Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length. HD: 800 lbs hold down hold down device fastened to the edge

of the brace wall panel closets to the corner. Methods Per Table R602.10.1

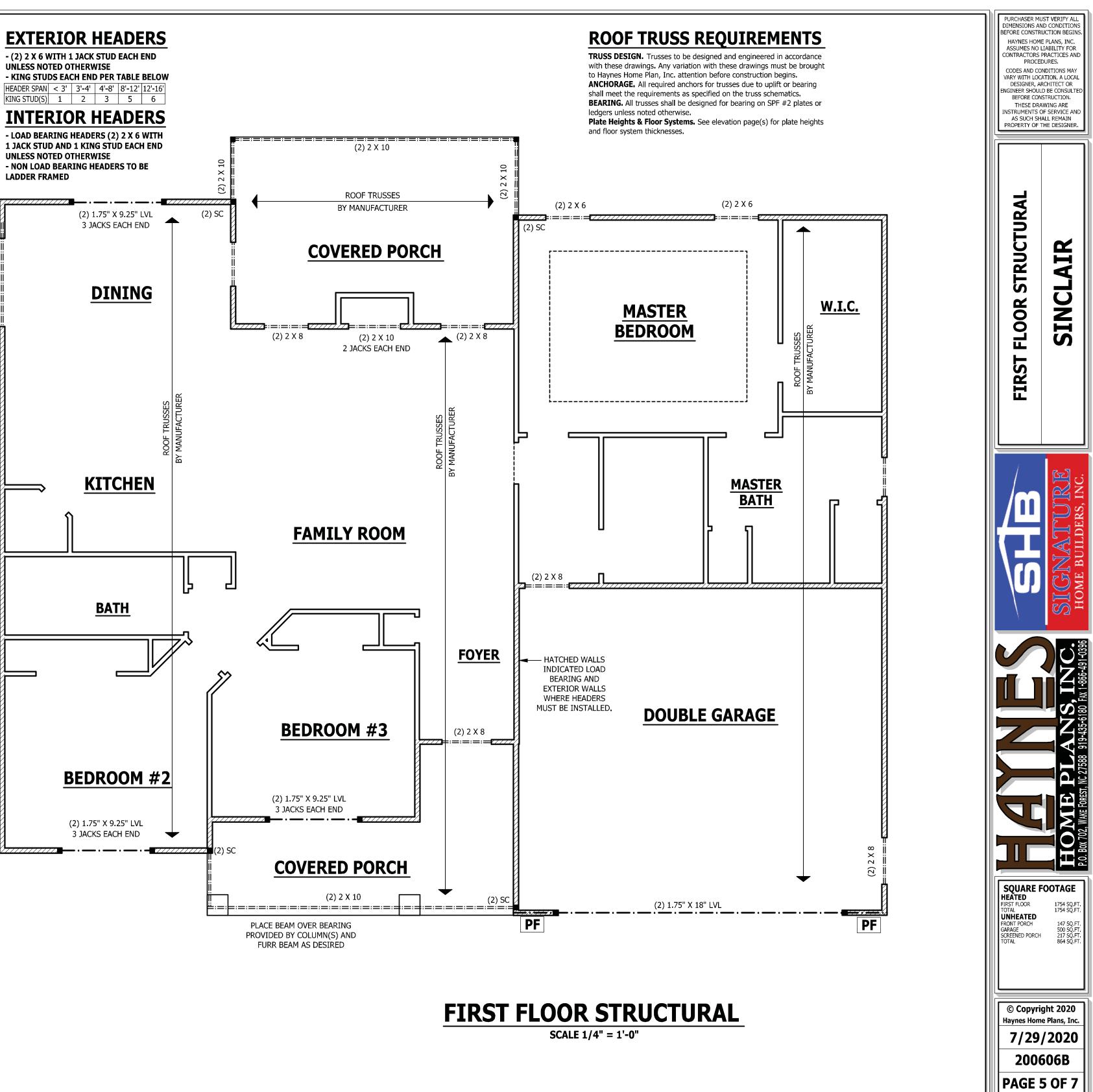
**CS-WSP:** Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or  $8d(2 1/2" \log x 0.113" diameter)$ . **CS-SFB:** Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing nails.

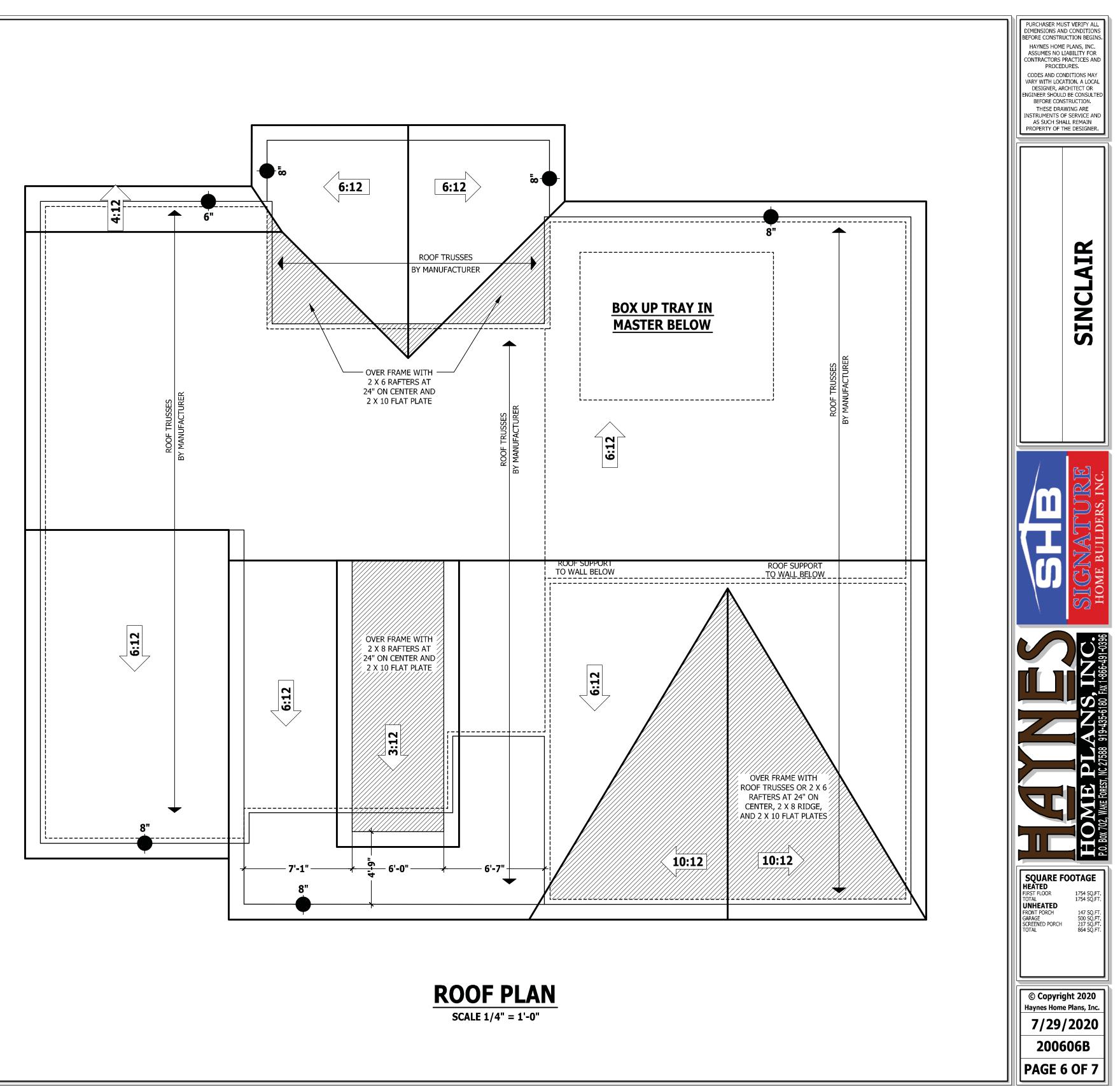
**GB:** Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. **PF:** Portal fame per figure R602.10.1

- 6-16D SINKER NAILS FROM KING STUD TO HEADER



- (2) 2 X 6 WITH 1 JACK STUD EACH END **UNLESS NOTED OTHERWISE** - KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16'





### **ROOF TRUSS REQUIREMENTS**

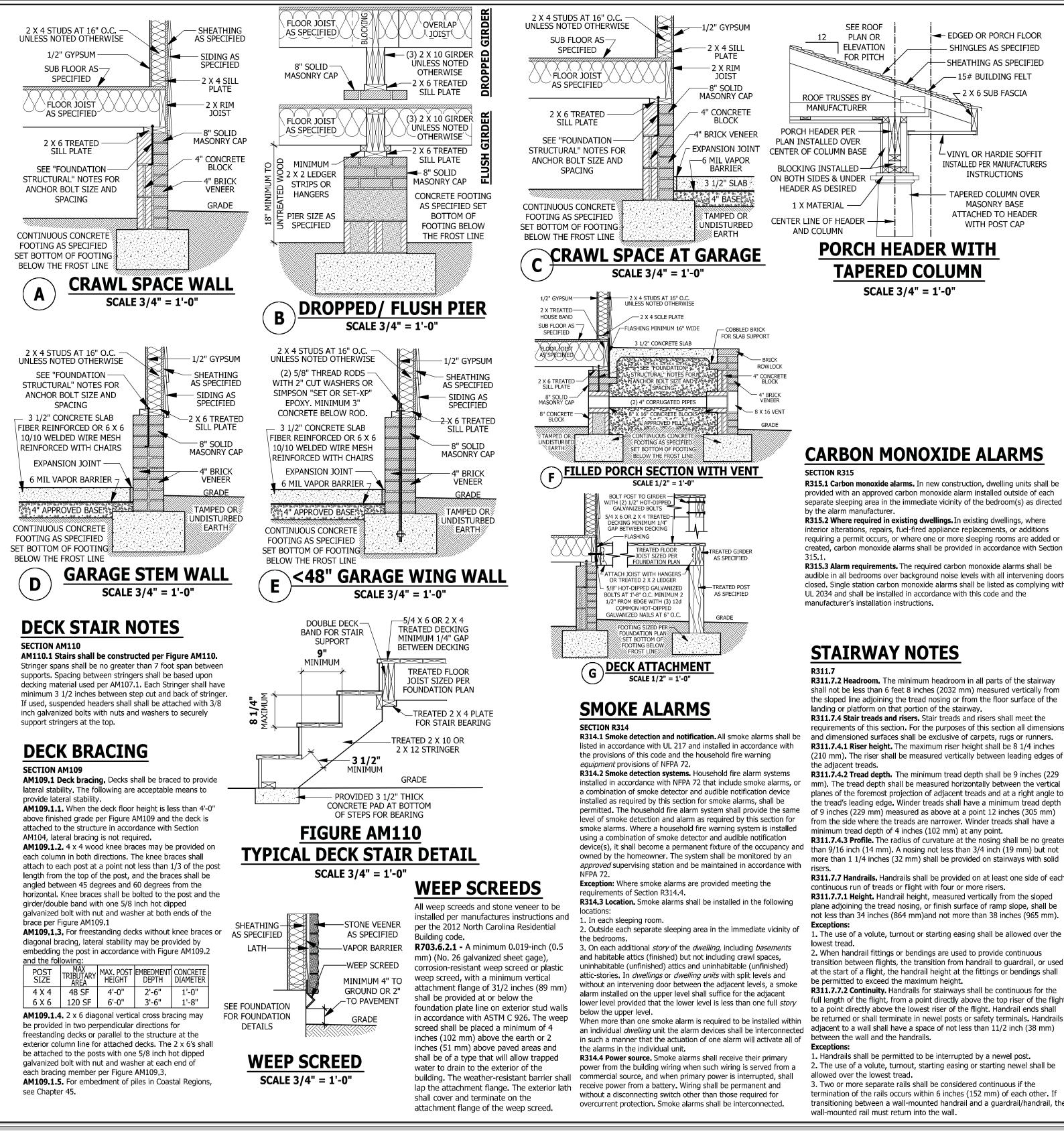
**TRUSS DESIGN.** Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

**ANCHORAGE.** All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

**Plate Heights & Floor Systems.** See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST FLOOR PLATE





R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed

requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with

shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the

requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches

mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

**R311.7.7 Handrails.** Handrails shall be provided on at least one side of each

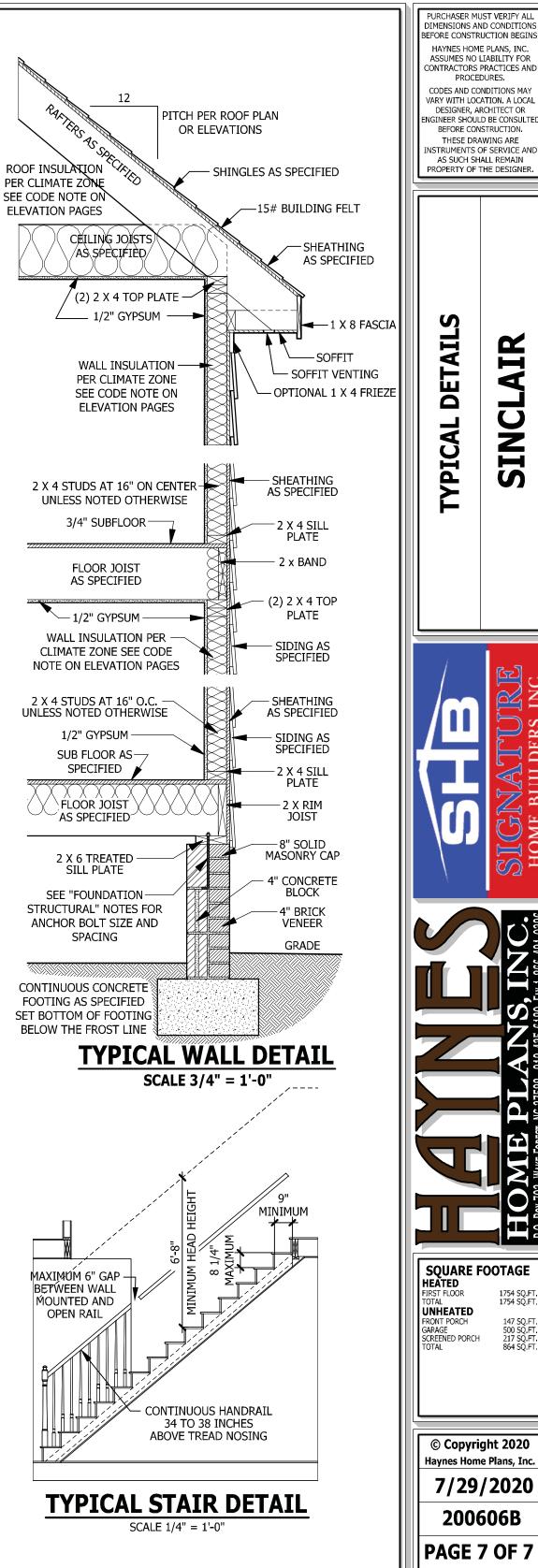
plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm).

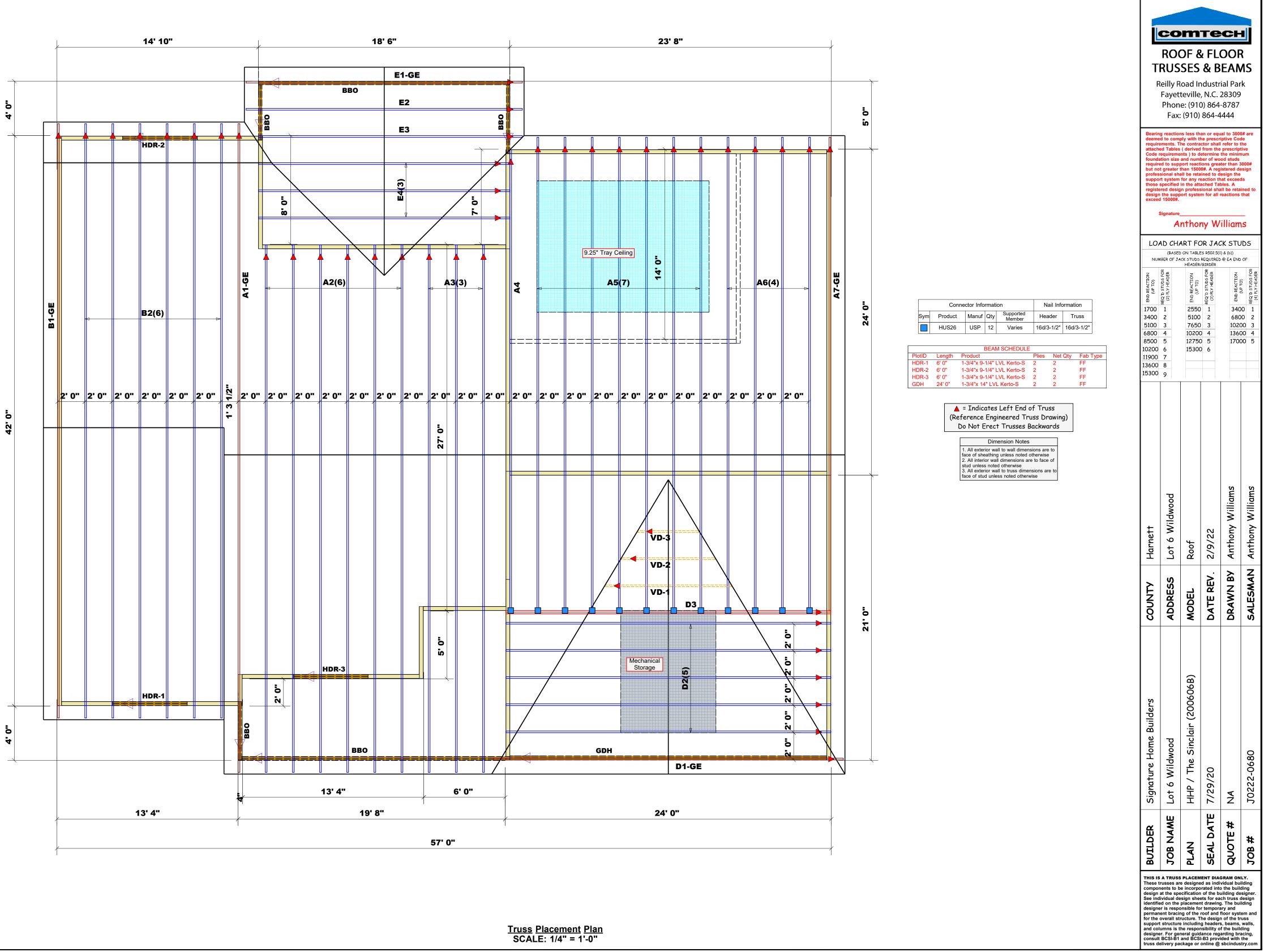
transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall

full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails

2. The use of a volute, turnout, starting easing or starting newel shall be

termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the





Project: Address: Lot 6 Wildwood HDR-1 & 2 Kerto-S LVL 1.750" X 9.250" 2-F	Input by: Anthony Williams Job Name: 6 Wildwood Project #: J0222-0680 Ply - PASSED
Participation         Participation           1DR-1 & 2         Kerto-S LVL         1.750" X 9.250"         2-F	Project #: J0222-0680
2	Ply - PASSED
• • • • •	
the second secon	9
1 SPF End Grain	2 SPF End Grain
5'6"	1 [] [] [] [] [] [] [] [] [] [] [] [] []
6'	
Iember Information           Type:         Girder         Application:         Floor	Reactions UNPATTERNED Ib (Uplift)           Brg Direction         Live         Dead         Snow         Wind         Const
Type:     Girder     Application:     Floor       Plies:     2     Design Method:     ASD	Brg         Direction         Live         Dead         Snow         Wind         Cons           1         Vertical         0         1447         1335         0
Moisture Condition: Dry Building Code: IBC/IRC 2015	2 Vertical 0 1447 1335 0
Deflection LL: 480 Load Sharing: No	
Deflection TL: 360 Deck: Not Checked	
Importance: Normal - II Temperature: Temp <= 100°F	
	Bearings
	Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb
	1 - SPF 3.000" Vert 30% 1447 / 1335 2782 L D+S
	End End
nalysis Results	Grain 2 - SPF 3.000" Vert 30% 1447 / 1335 2782 L D+S
Analysis Actual Location Allowed Capacity Comb. Ca Moment 3667 ft-lb 3' 14423 ft-lb 0.254 (25%) D+S L	
Moment         3667 ft-lb         3'         14423 ft-lb         0.254 (25%) D+S         L           Unbraced         3667 ft-lb         3'         10944 ft-lb         0.335 (34%) D+S         L	Grain
Shear 1841 lb 1' 1/4" 7943 lb 0.232 (23%) D+S L	
LL Defl inch 0.028 (L/2413) 3' 0.141 (L/480) 0.199 (20%) S L	
TL Defi inch 0.058 (L/1158) 3' 0.141 (L/460) 0.139 (20%) 3' L 3' 0.188 (L/360) 0.311 (31%) D+S L	
lesign Notes	
1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral su	oport
may also be required at the interior bearings by the building code. 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance	e not
to exceed 6".	
<ul><li>3 Refer to last page of calculations for fasteners required for specified loads.</li><li>4 Girders are designed to be supported on the bottom edge only.</li></ul>	
5 Top loads must be supported equally by all plies.	
6 Top must be laterally braced at end bearings.	
<ul><li>7 Bottom must be laterally braced at end bearings.</li><li>8 Lateral slenderness ratio based on single ply width.</li></ul>	
	d 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments
	5 PLF 0 PLF 445 PLF 0 PLF 0 PLF B2 TRUSS
	0 PLF 0 PLF 0 PLF 0 PLF WALL
	7 PLF
5	
Intes chemicals 6. For flat roofs	provide proper drainage to prevent Manufacturer Info Comtech, Inc.
alculated Structured Designs is responsible only of the Handling & Installation ponding	Metsä Wood Fayetteville, NC
tructural adequacy of this component based on the 1. LVL beams must not be cut or drilled esign criteria and loadings shown. It is the 2. Refer to manufacturer's product information sponsibility of the customer and/or the contractor to rearring in statisticing requirements multi-ny	Norwalk, CT 06851 28314 910-864-TRUS
sponsibility of the customer and/or the contractor to nsure the component suitability of the intended pplication, and to verify the dimensions and loads. approvals	(800) 622-5850 www.metsawood.com/us
umber 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained	ICC-ES: ESR-3633
5. Provide lateral support at bearing points to avoid	is valid until 3/30/2024

isDesig	Pr	ient: Signature Hor oject: ddress: Lot 6 Wildw	rood .	Date: 2/9/2022 nput by: Anthony Williams lob Name: 6 Wildwood Project #: J0222-0680	Page 2 of 6
HDR-1 & 2	Kerto-S LVL	1.750" X 9.		· · · · · · · · · · · · · · · · · · ·	
•	• •	•	• •	• <11/2"	9 1.
1 SPF End Grain	· · ·	5'6"	2 SPF End G		↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓
		6'			
Multi-Ply Analysis					
Fasten all plies usin Capacity Load	ng 2 rows of 10d Bo 0.0 % 0.0 PLF	x nails (.128x3") at	12" o.c Maximum end dist	ance not to exceed 6".	
Yield Limit per Foot	163.7 PLF				
Yield Limit per Fastener Yield Mode	81.9 lb. IV				
Edge Distance Min. End Distance	1 1/2" 3"				
Load Combination Duration Factor	1.00				
Notes	chemicals		6. For flat roofs provide proper drainage t	o prevent Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is re structural adequacy of this comp design criteria and loadings responsibility of the customer and	esponsible only of the onent based on the shown. It is the Vor the contractor to	& Installation s must not be cut or drilled manufacturer's product inform installation requirements, mu	ponding nation lti-ply	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	Fayetteville, NC
ensure the component suitabilit application, and to verify the dimensi <b>Lumber</b> 1. Dry service conditions, unless n 2. LVL not to be treated with fire r	ty of the intended fastening of approvals 3. Damaged f soted otherwise 5. Provide la	details, beam strength values, and Beams must not be used sumes top edge is laterally restrained teral support at bearing points to lacement and rotation	code	(800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	соттесн
Version 21 20 200 Deward k			This design is valid until 3/30/20	24	

CSD DESIGN

		Client: Signature Homes Project:		Date: Input by:	2/9/2022 Anthony William	s		Page 3 of
IS	Design	Address: Lot 6 Wildwood		Job Name Project #:	e: 6 Wildwood J0222-0680			
HDR-3	Kerto-S LVL	. 1.750" X 9.250"	2-Ply -	-	Level: Level			
	2							
		1						
•	•	•	•				Γ	$\overline{M}$ 1
		at the second	The line					)
•	•		a second second	• • • • • • • • • • • • • • • • • • •			L	
1 SPF E	nd Grain	5'6"	2	2 SPF End Grain			ļ	3 1/2"
/ /		6'		· · · · ·	1		I	10 112
Member Inf	<b>formation</b> Girder	Annlingtion Flags		Reactions UN		-	Snow Wi	nd Con
Type: Plies:	2	Application: Floor Design Method: ASD		Brg Direction 1 Vertical	Live 0	Dead 1594	Snow Wi 1482	nd Con 0
Moisture Cond	•	Building Code: IBC/IRC	C 2015	2 Vertical	0	1594	1482	0
Deflection LL: Deflection TL:	480 360	Load Sharing: No Deck: Not Che	ackad					
Importance:	Normal - II	Deck. Not Chi	ecked					
' Temperature:	Temp <= 100°F							
				Bearings				
				Bearing Length	n Dir. Cap.	React D/L lb	Total Ld. Ca	se Ld. Com
				1 - SPF 3.000"	Vert 34%	1594 / 1482	3076 L	D+S
Analysis Re	culte			_ End Grain				
Analysis	Actual Locat	ion Allowed Capacity Cor	nb. Case	2 - SPF 3.000"	Vert 34%	1594 / 1482	3076 L	D+S
Moment	4055 ft-lb	3' 14423 ft-lb 0.281 (28%) D+S		End				
Unbraced	4055 ft-lb	3' 10944 ft-lb 0.370 (37%) D+S		Grain				
Shear	2035 lb 4'11 3	3/4" 7943 lb 0.256 (26%) D+S						
LL Defl inch	0.031 (L/2174)	3' 0.141 (L/480) 0.221 (22%) S	L					
	0.064 (L/1047)	3' 0.188 (L/360) 0.344 (34%) D+S	S L					
Design Not	es			1				
1 Provide sup	port to prevent lateral mov	ement and rotation at the end bearing	s. Lateral support	1				
		arings by the building code. ox nails (.128x3") at 12" o.c. Maximum	end distance not					
to exceed 6	".							
	t page of calculations for fa designed to be supported	asteners required for specified loads.						
	nust be supported equally b							
•	e laterally braced at end be	0						
	st be laterally braced at end derness ratio based on sin	-						
ID	Load Type	Location Trib Width Side	Dead 0.9	Live 1 Sno	w 1.15 Wind	1.6 Const. 1.	.25 Comments	
1	Uniform	Тор	494 PLF		94 PLF 0 F		PLF A2 TRUSS	
2	Uniform	Тор	30 PLF	0 PLF	0 PLF 0 F		PLF WALL	
	Self Weight		7 PLF					
	5							
Notes		chemicals	6. For flat roofs provide r	proper drainage to prevent	Manufacturer Info		Comtech, Inc. 1001 S. Reilly Road, S	uite #639
Calculated Structured	Designs is responsible only of the Ha	andling & Installation	ponding		Metsä Wood 301 Merritt 7 Building	a 2nd Elocr	Fayetteville, NC USA	
design criteria and responsibility of the c	loadings shown. It is the 2. ustomer and/or the contractor to	LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply			Norwalk, CT 06851	, ∠πα <b>ΕΙΟ</b> ΟΓ	28314 910-864-TRUS	
ensure the compone	ent suitability of the intended fy the dimensions and loads.	fastening details, beam strength values, and code approvals			(800) 622-5850 www.metsawood.cor	n/us		
application, and to veri								
umber	4.	Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid			ICC-ES: ESR-3633			тесн

		Client:	Signature Homes		Date:	2/9/2022	Page 4 of 6
		Project:			Input by:	Anthony Williams	
is	Design	Address:	Lot 6 Wildwood			e: 6 Wildwood	
					Project #:		
HDR-3	Kerto-S L\	VL 1.750	)" X 9.250"	2-Ply - PAS	SED	Level: Level	
					•		
							,
•	•	•	•	• •	٠	-	
						<1 1/2"	
						$\overline{\mathbf{V}}$	9 1/
	•	•	•	• •	• -		
	End Grain			2 SPF En	d Grain	$\Lambda$	
			5'6"		/		3 1/2"
					·	,	
1			6'		1		
Multi-Ply A	nalysis						
Fasten all pli	ies using 2 rows o	of 10d Box nails	(.128x3") at 12" of	o.c Maximum end d	istance no	ot to exceed 6".	
Capacity		.0 %	,				
Load		.0 PLF					
Yield Limit per F Yield Limit per F		63.7 PLF 1.9 lb.					
Yield Mode	IV						
Edge Distance		1/2"					
Min. End Distand Load Combination							
Duration Factor		.00					
Nete -		chemicals		<ol> <li>For flat roofs provide proper draina</li> </ol>	ide to provert	Manufacturer Info	Comtech, Inc.
Notes Calculated Structured	Designs is responsible only of the	Handling & Installa	tion	ponding	se to prevent	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC USA
structural adequacy of design criteria and	of this component based on the loadings shown. It is the	<ol> <li>LVL beams must not be</li> <li>Refer to manufactu</li> </ol>	cut or drilled rer's product information			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the compon	customer and/or the contractor to ent suitability of the intended ify the dimensions and loads.		requirements, multi-ply strength values, and code			(800) 622-5850 www.metsawood.com/us	
Lumber		<ol> <li>Damaged Beams must</li> <li>Design assumes top ed</li> </ol>	ge is laterally restrained			ICC-ES: ESR-3633	
<ol> <li>Dry service conditi</li> <li>LVL not to be treat</li> </ol>	ons, unless noted otherwise ted with fire retardant or corrosive	E Provide lateral cupper	at bearing points to avoid	This design is valid until 3/30	)/2024		соттесн
L					•		

			Client:	Signature Hom	es		Date		2/9/2022					Page 5 of 6
Tis	Design		Project: Address:	Lot 6 Wildwo	bod		•	it by: Name:	6 Wildw	<sup>,</sup> Williams ood				
				Lot o Wildin	Jou			ect #:	J0222-0					
GDH	Kerto-S L'	VL 1.	750" >	X 14.000	)" 2-	Ply - F	ASSED	L	evel: Leve					
						-								
		2						•••••						
	•••••		• • • • • • • • • • • •	•••••	1	• • • • • • • • • • •			•••••	•••••	•••••		m	- <i>f</i>
	and the second se				Marine .				Nin I				M	
		State of the second			•				13. J. M. M. M.		Stelling 1	• • •	M	1.2"
1 SPF End	d Grain										2 SPF E	ind Grain		
/					18'3"								3	1/2"
∤					18'10"								•	
												•		
Member In	formation						Reactions	UNP	ATTERM	NED Ih	(Uplift)			
Туре:	Girder		Applicat	ion: Flo	or		Brg Direc		Live		Dead	Snow	Wind	Const
Plies:	2		Design I				1 Vertica	al	C	)	2363	377	0	0
Moisture Cond Deflection LL:			Building Load Sh		C/IRC 2015		2 Vertica	al	C	)	2363	377	0	0
Deflection TL:			Deck:		t Checked									
Importance:	Normal - II													
Temperature:	Temp <= 10	0°F					Destination							
							Bearings		Dia	0	De a at D/l	U. T.4-1		L d. Oamah
							Bearing L	-	Dir. Vert	26%	React D/L 2363 / 37		Ld. Case	Ld. Comb. D+S
							End		VOIT	2070	2000701	2100	L	0.0
Analysis Re							Grain 2 - SPF 3	500"	Vert	26%	2363 / 37	77 2739		D+S
Analysis	Actual	Location A			Comb.	Case	End		ven	2070	2000701	2133	L	0.0
Moment Unbraced	10589 ft-lb 12277 ft-lb		24299 ft-lb 12288 ft-lb	0.436 (44%) 0.999	D D+S	Uniform L	Grain							
-				(100%)	_									
Shear	2009 lb 0.068 (L/3239)	17'4 1/2" 9		0.214 (21%) ) 0.148 (15%)		Uniform L								
	0.495 (L/445)			) 0.808 (81%)		L								
Design Not				, , ,			1							
1 Provide su	oport to prevent late				arings. Later	ral support	1							
	e required at the in plies using 3 rows o	0	,	0	num end dis	stance not								
to exceed 6	5".													
	t page of calculation designed to be su		•		as.									
	nust be supported													
	e laterally braced a st be laterally brace			D.C.										
8 Lateral sler	iderness ratio base	ed on single ply	y width.											
ID	Load Type	L	ocation		Side	Dead 0.9	Live 1		v 1.15		.6 Const		mments	
1	Uniform Uniform				Гор Гор	40 PLF 200 PLF	0 PLF 0 PLF		10 PLF 0 PLF	0 PL 0 PL		0 PLF R · 0 PLF WA		
2					Гор	200 FLF	UFLF		UFLF	UFL	.г	UFLF VVA		
	Self Weight													
Notes		chemica			6. For flat ponding	t roofs provide p	proper drainage to pr	eveni	Manufactur			Comtech 1001 S.	Reilly Road, Suite #	#639
structural adequacy	Designs is responsible only of this component based of loadings shown. It is	on the 1. LVL bea	g & Installation ms must not be cu	it or drilled		3		:	Metsä Wood 301 Merritt 7	7 Building,	2nd Floor	Fayettev USA 28314	lie, NC	
responsibility of the o ensure the comport	customer and/or the contract ent suitability of the int	ctor to regardin tended fastening	g installation	r's product informa requirements, mult strength values, and c	-ply				Norwalk, C1 (800) 622-5	06851 850		910-864-	TRUS	
application, and to ver Lumber	ify the dimensions and loads	<ol> <li>approva</li> <li>3. Damage</li> </ol>	Ís ed Beams must no	t be used				,	www.metsav ICC-ES: ES	wood.com	/us			
<ol> <li>Dry service condition</li> <li>LVL not to be treated</li> </ol>	ons, unless noted otherwise ted with fire retardant or con	5. Provide	assumes top edge lateral support a isplacement and r	is laterally restrained it bearing points to a ptation		lesign is valid	l until 3/30/2024						ют	есн
L					1113 (									

Version 21.20.299 Powered by iStruct<sup>™</sup> Dataset: embedded

	Client:	Signature Homes	Date:	2/9/2022	Page 6 of 6
	Project:	Signature nomes	Input by:		Fage 0 01 0
isDesign	Address:	Lot 6 Wildwood	Job Nam		
				J0222-0680 Level: Level	
GDH Kerto-S L	VL 1.750	K 14.000" 2-P	IY - PASSED		
		•••••	· · · ·		
		• • •			····
1 SPF End Grain				2 SPF End	Grain /
		18'3"			3 1/2"
<u>/</u>		18'10"			
Multi-Ply Analysis					
Fasten all plies using 3 rov	ws of 10d Box nails (	.128x3") at 12" o.c Ma	ximum end distance n	ot to exceed 6".	
Capacity	0.0 %				
₋oad ∕ield Limit per Foot	0.0 PLF 245.6 PLF				
/ield Limit per Fastener	81.9 lb.				
/ield Mode	IV 1.1/2"				
Edge Distance ⁄lin. End Distance	1 1/2" 3"				
Load Combination	0				
Duration Factor	1.00				
					r
Notes	chemicals		ofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is responsible on structural adequacy of this component based design criteria and loadings chown It	on the 1. LVL beams must not be c	It or drilled		Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA 28314
design criteria and loadings shown. It responsibility of the customer and/or the contr ensure the component suitability of the i	actor to regarding installation ntended fastening details beam	r's product information requirements, multi-ply strength values, and code		Norwalk, CT 06851 (800) 622-5850	28314 910-864-TRUS
application, and to verify the dimensions and load	<ul> <li>approvals</li> <li>3. Damaged Beams must no</li> </ul>	t be used		www.metsawood.com/us ICC-ES: ESR-3633	
<ol> <li>Dry service conditions, unless noted otherwis</li> <li>LVL not to be treated with fire retardant or c</li> </ol>	e 4. Design assumes top edge 5. Provide lateral support lateral displacement and	at bearing points to avoid			соттесн
l		I his des	ign is valid until 3/30/2024		