

JDS JDS	Adda al to 300 iptive Coo prescrip the minii od studs ter than 3 istered di esign the hait exceed oles. A all be reta reactions Indry CK STU) 1 2 3 0 4 0 5	(910) is less th by with the contract (derived that 1500 and num port react than 1500 all be retar for any r in the att n profess ort syste Davi ART F(C CN TABL (K STUDS	Fax: g reactior d to comp ments. TI d Tables equireme tion size d to supp i greater t sional sha t system specified red desig the supp 15000#. Signature	deemei require attache Code r founda require but not profess suppor those s registe design exceed
Marshall Naylor	David Landry	03/18/22	Roof	708 Walker Road	Linden / Harnett
SALESMAN	DRAWN BY	DATE REV.	MODEL	ADDRESS	COUNTY
J0122-0300		N/A	The Fawnbrook	Lot 4 Walker Rd.	Ben Stout Real Estate
JOB #	QUOTE #	SEAL DATE	PLAN	JOB NAME	BUILDER

\sim			

support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



RE: J0122-0300 Lot 4 Walker Rd. Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Custom Lot/Blo

Customer: Benjamin Stout Real Estate	Project Name: J0122-0300
Lot/Block: 4	Model: Fawnbrook
Address: 708 Walker Road	Subdivision: Walker Rd.
City: Linden	State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 20 individual, dated Truss Design Drawings and 0 Additional Drawings.

	A		
No.	Seal#	Truss Name	Date
1	E16495456	A1	12/22/2021
2	E16495457	A1GE	12/22/2021
3	E16495458	A2	12/22/2021
4	E16495459	A3	12/22/2021
5	E16495460	A3A	12/22/2021
6	E16495461	A3GE	12/22/2021
7	E16495462	B1	12/22/2021
8	E16495463	B1-GR	12/22/2021
9	E16495464	B1GE	12/22/2021
10	E16495465	C1	12/22/2021
11	E16495466	C1-GR	12/22/2021
12	E16495467	C1GE	12/22/2021
13	E16495468	D1	12/22/2021
14	E16495469	D1GE	12/22/2021
15	E16495470	M1	12/22/2021
16	E16495471	M1GE	12/22/2021
17	E16495472	V1	12/22/2021
18	E16495473	V2	12/22/2021
19	E16495474	V3	12/22/2021
20	E16495475	V4	12/22/2021

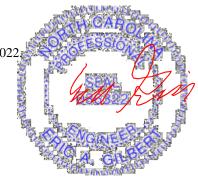
The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

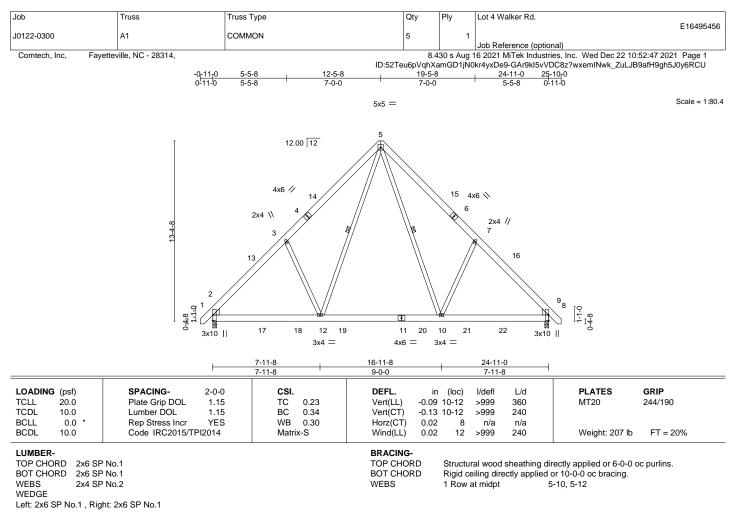
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



December 22, 2021



REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-317(LC 10) Max Uplift 2=-41(LC 12), 8=-41(LC 13) Max Grav 2=1185(LC 19), 8=1185(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1356/272, 3-5=-1240/456, 5-7=-1240/456, 7-8=-1356/272

BOT CHORD 2-12=-123/1040, 10-12=-10/672, 8-10=-51/884

WEBS 5-10=-217/739, 7-10=-443/340, 5-12=-217/738, 3-12=-443/340

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 12-5-8, Exterior(2) 12-5-8 to 16-10-5, Interior(1) 16-10-5 to 25-8-6 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

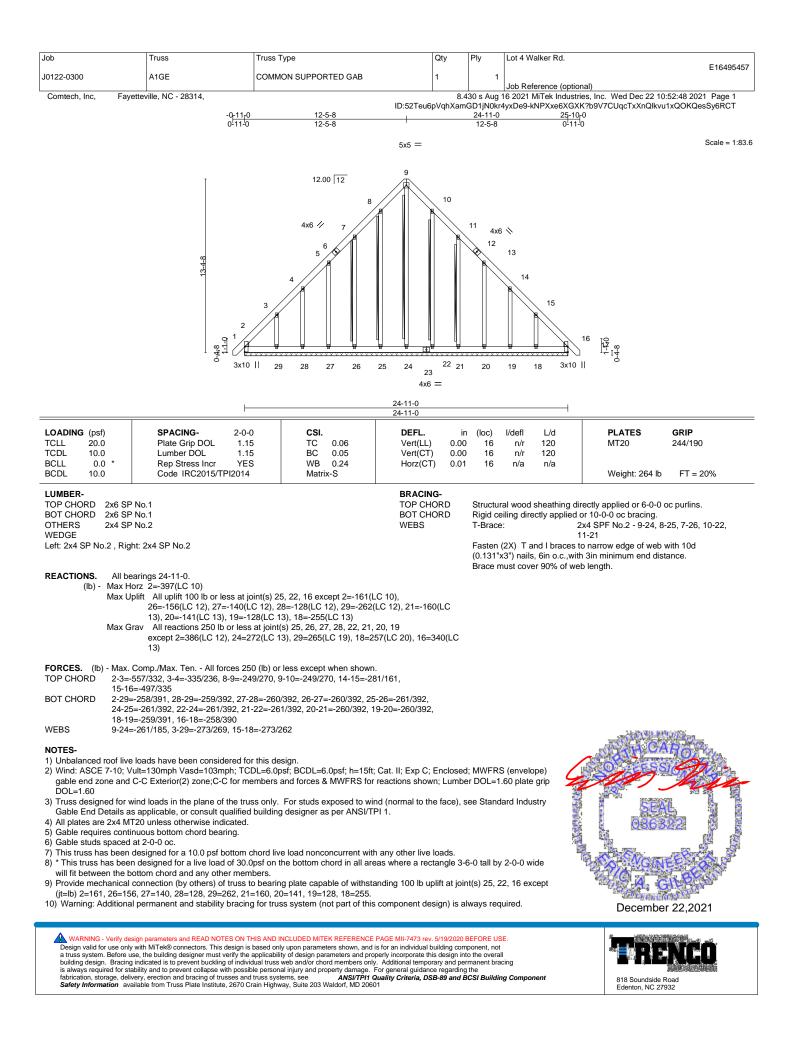
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

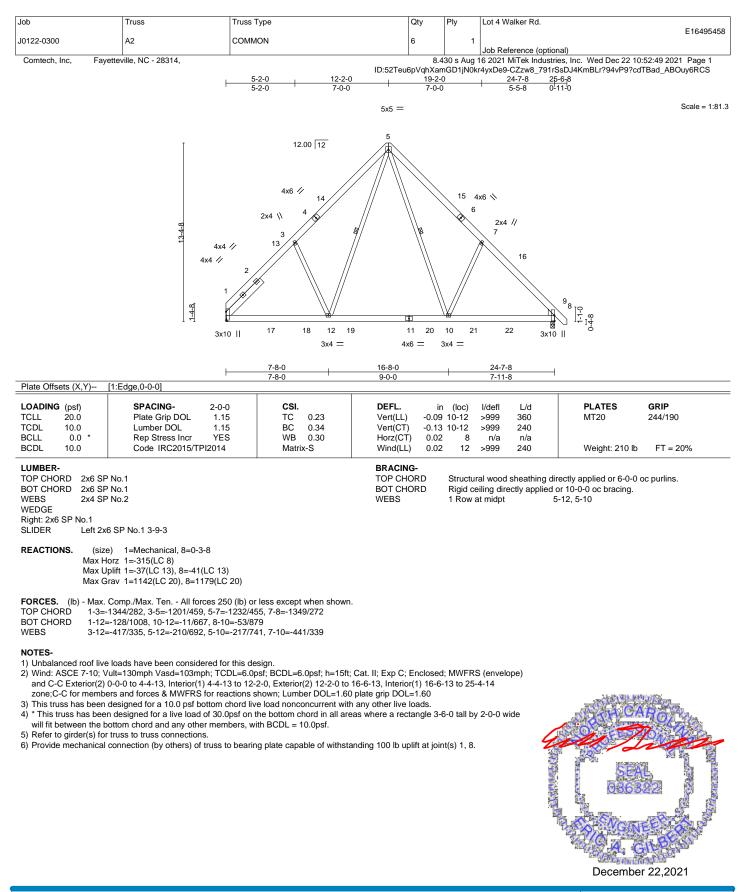


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

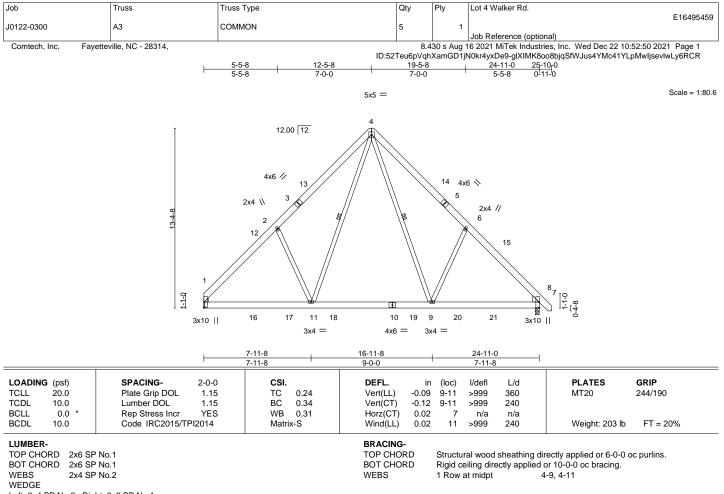


818 Soundside Road Edenton, NC 27932









Left: 2x4 SP No.2 , Right: 2x6 SP No.1

REACTIONS. (size) 1=Mechanical, 7=0-3-8 Max Horz 1=-315(LC 8) Max Uplift 1=-35(LC 13), 7=-41(LC 13) Max Grav 1=1146(LC 20), 7=1190(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-1367/280, 2-4=-1255/470, 4-6=-1246/458, 6-7=-1362/274 TOP CHORD

BOT CHORD WEBS

1-11=-122/1056, 9-11=-9/677, 7-9=-55/888 4-9=-217/737, 6-9=-443/340, 4-11=-223/756, 2-11=-454/346

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 12-5-8, Exterior(2) 12-5-8 to 16-10-5, Interior(1) 16-10-5 to 25-8-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

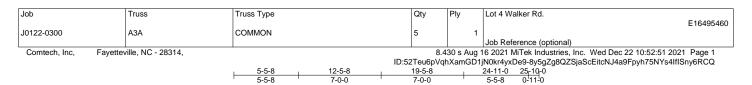
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

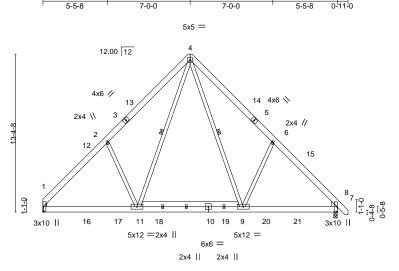


ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. 🛕 WARNING - Verify design pa Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLUDED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932





			7-11-8 7-11-8		16-11-8 9-0-0	-	-11-0 11-8			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI:	2-0-0 1.15 1.15 YES 2014	BC	0.24 0.34 0.31 S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.09 -0.12 0.02 0.02	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 223 lb	GRIP 244/190 FT = 20%

BRACING-

WFBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2WEDGE

Left: 2x4 SP No.2 , Right: 2x6 SP No.1 **REACTIONS.** (size) 1=Mechanical, 7=0-3-8

Max Horz 1=-315(LC 8) Max Uplift 1=-35(LC 13), 7=-41(LC 13) Max Grav 1=1139(LC 20), 7=1183(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1357/280, 2-4=-1245/470, 4-6=-1236/458, 6-7=-1352/274

BOT CHORD 1-11=-122/1049, 9-11=-9/672, 7-9=-55/881

WEBS 4-9=-217/730, 6-9=-443/340, 4-11=-223/748, 2-11=-454/346

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 12-5-8, Exterior(2) 12-5-8 to 16-10-5, Interior(1) 16-10-5 to 25-8-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

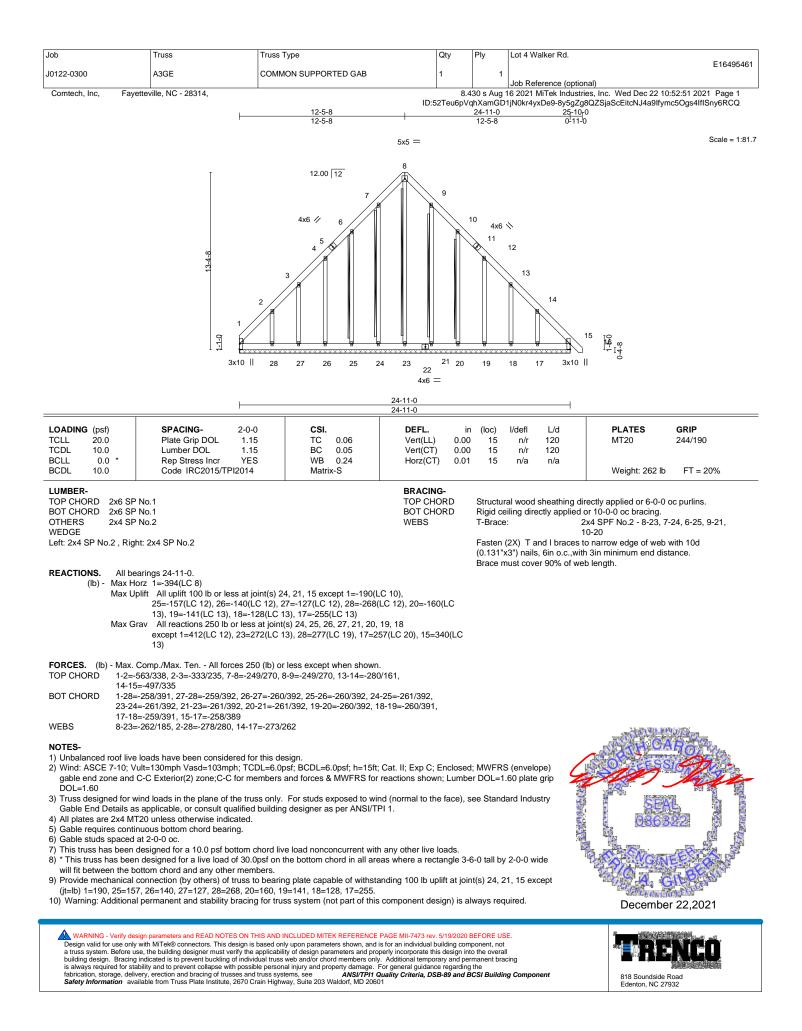
4-9, 4-11

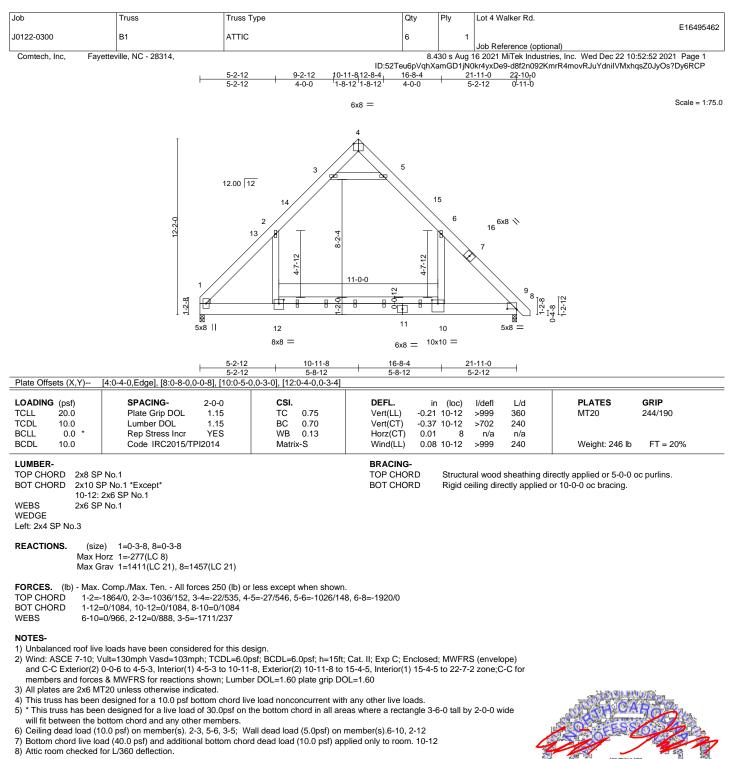
Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

Scale = 1:91.8





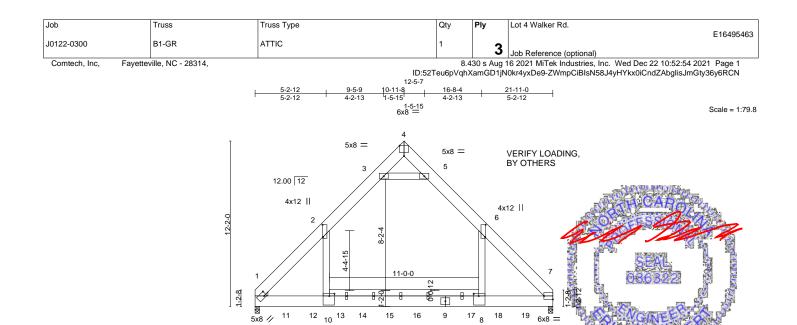




December 22,2021

🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLUDED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



10

+

5-2-12 5-2-12

10x10 =

2x6 || 2x6 || 2x6 ||

10-11-8 5-8-12

6x8 = 10x10 =

21-11-0 5-2-12

-1

2x6 || 2x6 ||

16-8-4 5-8-12

Plate Offsets (X,Y) [[1:0-2-3,0-3-8], [2:0-10-12,0-0-8], [4:0-4	-0,Edge], [6:0-10-12,0-0-8]], [7:Edge,0-3-0], [8:0-4	·8,0-2-0],	[10:0-4-8	,0-2-0]		
.OADING (psf) TCLL 20.0 TCDL 10.0 SCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNO	CSI. TC 0.78 BC 0.79 WB 0.40	Vert(LL) -0.31	8-10	l/defl >835 >614 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
CDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.01		>999	240	Weight: 801 lb	FT = 20%
OT CHORD 2x10 SF	^P 2400F 2.0E ^P No.1 *Except* 66 SP No.1 No.1		BRACING- TOP CHORD BOT CHORD				ectly applied or 6-0-0 c r 10-0-0 oc bracing.	oc purlins.
Max Ho) 1=0-4-0, 7=0-4-0 prz 1=-269(LC 4) rav 1=10108(LC 14), 7=10091(LC 14)							
OP CHORD 1-2=-1 OT CHORD 1-10=	Comp./Max. Ten All forces 250 (lb) or 10697/0, 2-3=-4448/35, 3-4=0/4719, 4-5 0/5996, 8-10=0/6065, 7-8=0/5996 /8663, 2-10=0/8686, 3-5=-12734/0		-7=-10688/0					
Top chords connecte Bottom chords conne Webs connected as f All loads are conside ply connections have Unbalanced roof live	hected together with 10d (0.131"x3") na d as follows: 2x10 - 2 rows staggered a cted as follows: 2x10 - 5 rows staggered follows: 2x6 - 2 rows staggered at 0-9-0 red equally applied to all plies, except i been provided to distribute only loads loads have been considered for this de	at 0-9-0 oc. ed at 0-4-0 oc. oc. f noted as front (F) or back noted as (F) or (B), unless isign.	otherwise indicated.	. ,				
Lumber DOL=1.60 pl Concentrated loads f MWFRS Wind (Pos. Left; #7 Dead + 0.6 N MWFRS Wind (Pos. (Pos. Internal) 4th Pe 2nd Parallel; #20 Dee Live (bal.) + 0.75 Atti	rom layout are not present in Load Cas Internal) Left; #5 Dead + 0.6 MWFRS V /WFRS Wind (Neg. Internal) Right; #8 Internal) 2nd Parallel; #10 Dead + 0.6 I rarllel; #12 Dead + 0.6 MWFRS Wind (I ad + 0.75 Roof Live (bal.) + 0.75 Attic F c Floor + 0.75(0.6 MWFRS Wind (Neg.	e(s): #3 Dead + Uninhabiti Vind (Pos. Internal) Right; Dead + 0.6 MWFRS Wind /WFRS Wind (Pos. Interna Veg. Internal) 1st Parallet; loor + 0.75(0.6 MWFRS W Int) Right); #22 Dead + 0.7	able Attic Without Stora #6 Dead + 0.6 MWFRS (Pos. Internal) 1st Para al) 3rd Parallel; #11 Dea #13 Dead + 0.6 MWFR (ind (Neg. Int) Left); #21 75 Roof Live (bal.) + 0.7	ge; #4 De Wind (Ne Ilel; #9 D ad + 0.6 N S Wind (N Dead + '5 Attic F	ead + 0.6 eg. Intern ead + 0.6 //WFRS V Neg. Inter 0.75 Root loor +	al) Vind nal)	Decembe	r 22 2021
0.75(0.6 MWFRS Wind of the local state of the local	nd (Neg. Int) 1st Parallel); #23 Dead +	0.75 Roof Live (bal.) + 0.75	5 Attic Floor + 0.75(0.6	MWFRS	Wind (Ne	g.	December	122,2021
WARNING - Verify des Design valid for use only a truss system. Before us building design. Bracing is always required for sta fabrication, storage, deliv	ign parameters and READ NOTES ON THIS AND I with MiTek® connectors. This design is based on se, the building designer must verify the applicabil indicated is to prevent buckling of individual truss bility and to prevent collapse with possible person ery, erection and bracing of trusses and truss sys ilable from Truss Plate Institute, 2670 Crain Highw	ly upon parameters shown, and is ty of design parameters and prop- web and/or chord members only. al injury and property damage. Fr tems, see ANS//TPI Q	for an individual building com erly incorporate this design in Additional temporary and pe or general guidance regarding uality Criteria, DSB-89 and E	ponent, not to the overa manent bra the	ll Icing	ent	818 Soundside R Edenton, NC 275	NCCO Road 32



Job	Truss	Truss Type	Qty	Ply	Lot 4 Walker Rd.	
J0122-0300	B1-GR	ATTIC	1	2	E16495463	
					Job Reference (optional)	
Comtech, Inc,	Fayetteville, NC - 28314,		8.4	30 s Aug	16 2021 MiTek Industries, Inc. Wed Dec 22 10:52:54 2021 Page 2	
		ID:52Teu6pVqhXamGD1jN0kr4yxDe9-ZWmpCiBIsN58J4yHYkx0iCndZAbqlisJmGty36y6RCN				

NOTES-

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-5; Wall dead load (5.0psf) on member(s).6-8, 2-10
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-10
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1870 lb down at 2-1-12, 1870 lb down at 4-1-12, 4072 lb down at 5-3-12, 371 lb down and 36 lb up at 7-9-4, 371 lb down and 36 lb up at 9-9-4, 371 lb down and 36 lb up at 11-9-4, 371 lb down and 36 lb up at 13-9-4, 371 lb down and 36 lb up at 13-9-4, 371 lb down and 36 lb up at 13-9-4, 371 lb down and 36 lb up at 15-9-4, 4072 lb down at 16-7-4, and 1870 lb down at 17-9-4, and 1870 lb down at 19-9-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 11) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

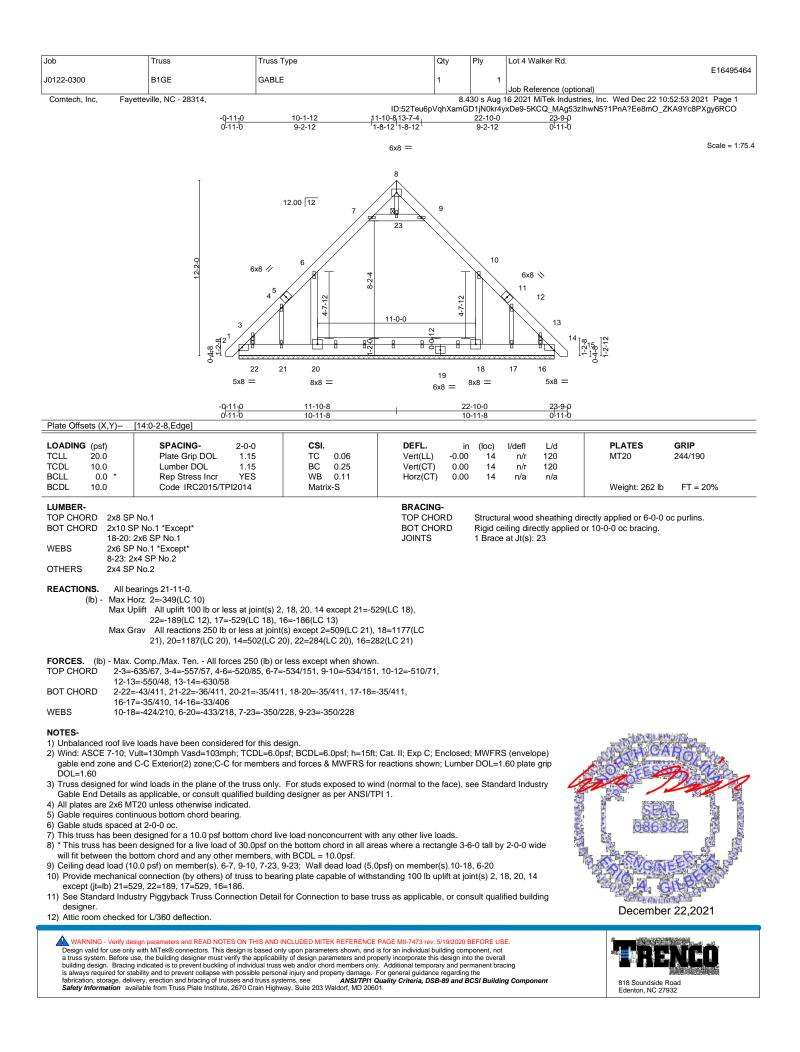
- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-2=-60, 2-3=-80, 3-4=-60, 4-5=-60, 5-6=-80, 6-7=-60, 1-10=-20, 8-10=-40, 7-8=-20, 3-5=-20

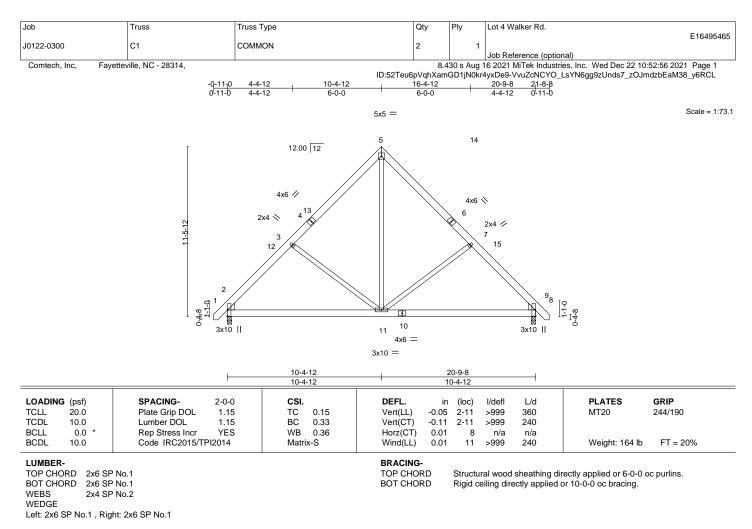
Drag: 6-8=-10, 2-10=-10 Concentrated Loads (lb)

Vert: 9=-60(B) 8=-1096(B) 10=-1096(B) 11=-469(B) 12=-469(B) 13=-60(B) 14=-60(B) 15=-60(B) 16=-60(B) 17=-60(B) 18=-469(B) 19=-469(B) 19=-469(B) 10=-1096(B) 11=-469(B) 12=-469(B) 13=-60(B) 14=-60(B) 15=-60(B) 15=-60(B)

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REACTIONS. (size) 8=0-3-8, 2=0-3-8 Max Horz 2=-268(LC 10) Max Uplift 8=-36(LC 13), 2=-36(LC 12) Max Grav 8=876(LC 1), 2=876(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-908/254, 3-5=-752/282, 5-7=-752/282, 7-8=-908/254

BOT CHORD 2-11=-122/675, 8-11=-60/568

WEBS 5-11=-179/668, 7-11=-354/267, 3-11=-354/267

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 10-4-12, Exterior(2) 10-4-12 to 14-9-9, Interior(1) 14-9-9 to 21-6-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

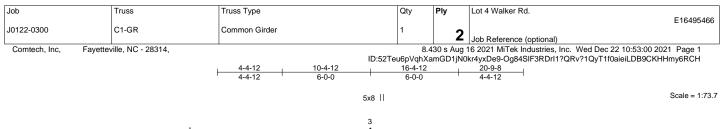
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

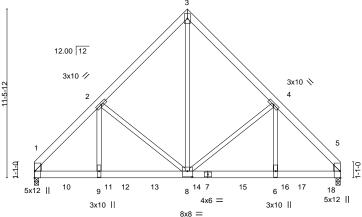
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2.









				4-4-12	10-4-12		6-4-12		20-9-	-		
				4-4-12	6-0-0	'	6-0-0		4-4-1	2 '		
Plate Offs	sets (X,Y)	[1:Edge,0-0-4], [5:Edge,0	-0-4], [8:0-4	4-0,0-4-12]								
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	TC	0.72	Vert(LL)	-0.09	8-9	>999	360	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.57	Vert(CT)	-0.16	8-9	>999	240		
SCLL	0.0 *	Rep Stress Incr	NO	WB	0.73	Horz(CT)	0.03	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	912014	Matri	x-S	Wind(LL)	0.05	8-9	>999	240	Weight: 344 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP 2400F 2.0E BOT CHORD 2x4 SP No.2 WEBS WEDGE

Left: 2x6 SP No.1, Right: 2x6 SP No.1

REACTIONS. (size) 1=0-3-8, 5=0-3-8 Max Horz 1=-262(LC 25) Max Uplift 1=-253(LC 9), 5=-280(LC 8) Max Grav 1=5843(LC 2), 5=6526(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1-2=-7016/344, 2-3=-4522/337, 3-4=-4523/337, 4-5=-6984/342 TOP CHORD
- 1-9=-278/4552, 8-9=-278/4562, 6-8=-168/4548, 5-6=-168/4538 BOT CHORD
- 3-8=-332/5948, 4-8=-1816/259, 4-6=-65/2983, 2-8=-1834/259, 2-9=-66/3027 WEBS

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=253, 5=280.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1097 lb down and 55 lb up at 2-0-12, 1097 lb down and 55 lb up at 4-0-12, 1097 lb down and 55 lb up at 6-0-12, 1089 lb down and 55 lb up at 8-0-12, 1089 lb down and 55 lb up at 10-0-12, 1089 lb down and 55 lb up at 12-0-12, 1089 lb down and 55 lb up at 14-0-12, 1089 lb down and 55 Ib up at 16-0-12, and 1097 lb down and 55 lb up at 18-0-12, and 1101 lb down and 51 lb up at 20-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

December 22,2021





Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Job	Truss	Truss Type	Qty	Ply	Lot 4 Walker Rd.
	04.05				E16495466
J0122-0300	C1-GR	Common Girder	1	2	Job Reference (optional)
Comtech. Inc. Favette	/ille. NC - 28314.		8.4		16 2021 MiTek Industries, Inc. Wed Dec 22 10:53:00 2021 Page 2

ID:52Teu6pVqhXamGD1jN0kr4yxDe9-Og84SIF3RDr11?QRv?1QyT1f0aieiLDB9CKHHmy6RCH

LOAD CASE(S) Standard

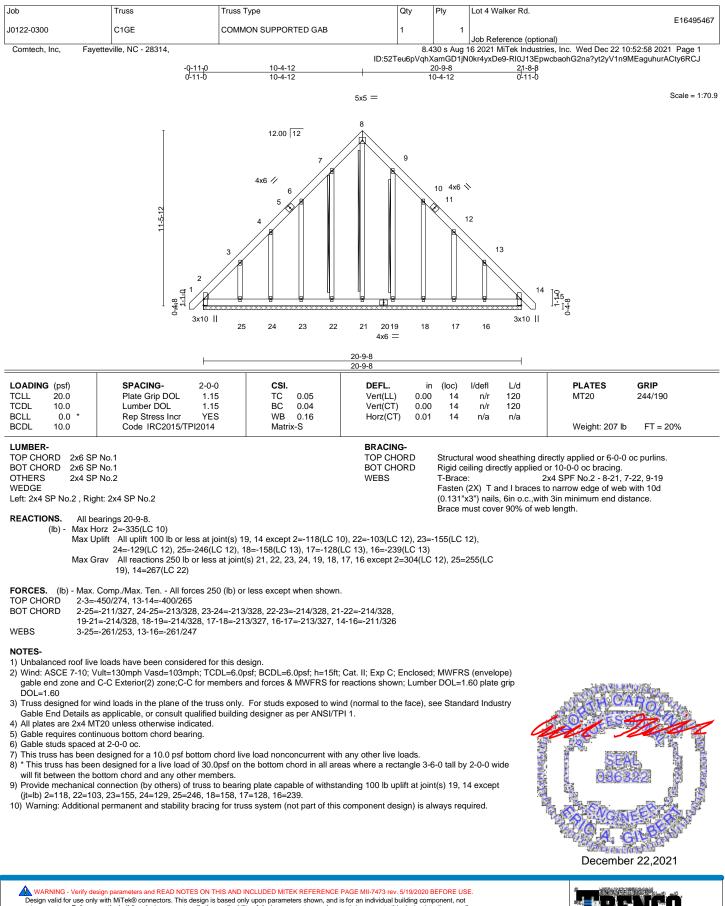
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

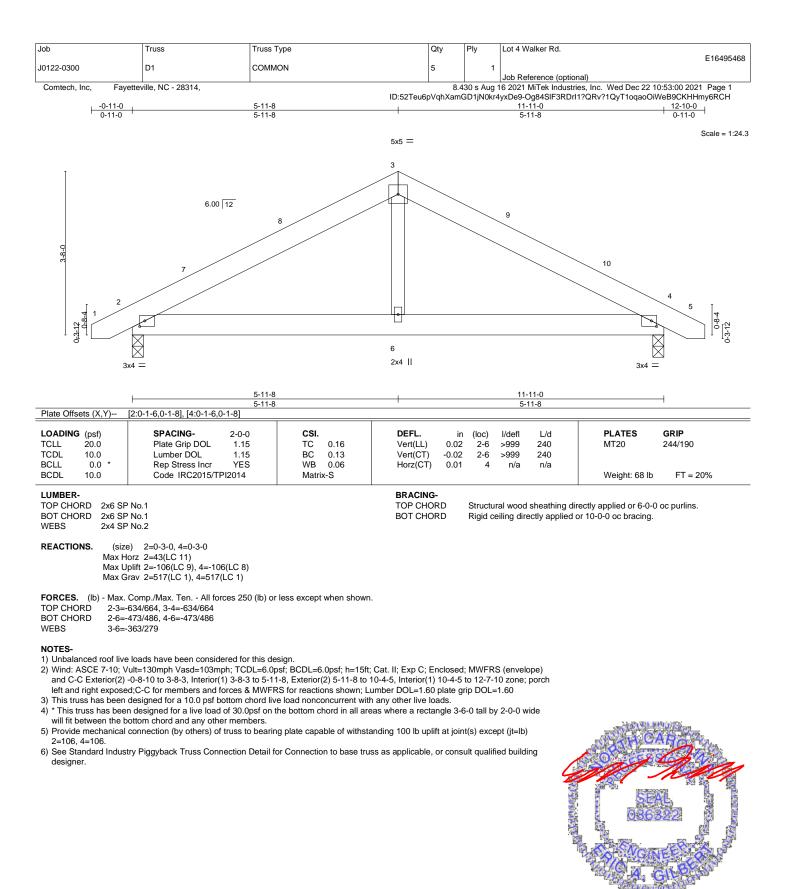
Concentrated Loads (lb)

Vert: 7=-967(B) 10=-967(B) 11=-967(B) 12=-967(B) 13=-967(B) 14=-967(B) 15=-967(B) 15=-967(B) 17=-967(B) 18=-971(B) 10=-967(B) 10=-96



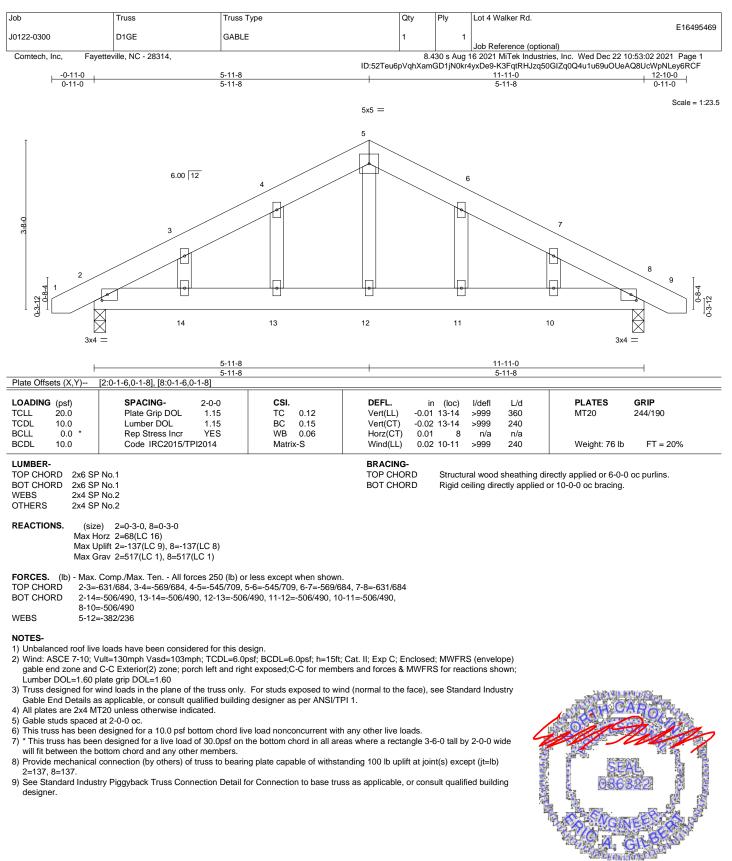






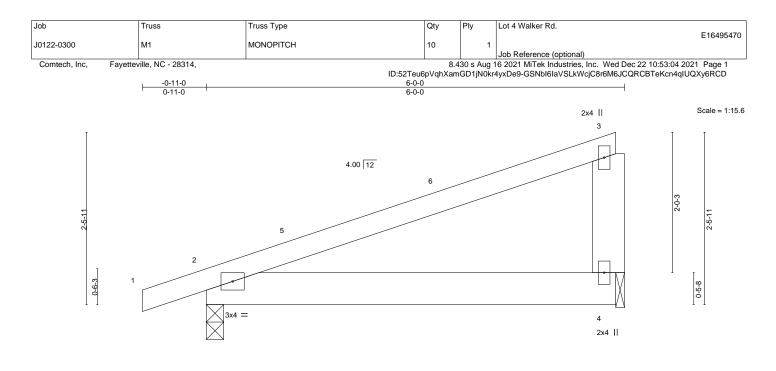
December 22,2021





December 22,2021





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.44 BC 0.12 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.03 0.00 0.03	(loc) 2-4 2-4 2-4	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 29 lb	GRIP 244/190 FT = 20%
LUMBER-			BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=75(LC 8) Max Uplift 2=-116(LC 8), 4=-96(LC 8)

Max Grav 2=294(LC 1), 4=220(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface.

- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=116.

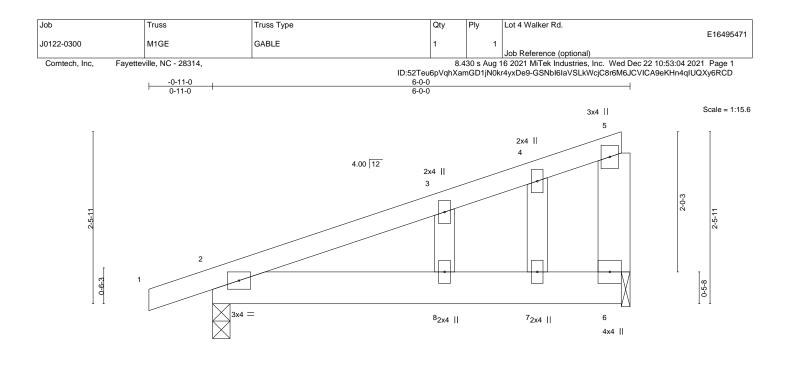


Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.13 BC 0.14 WB 0.02 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) 0.03 2-8 >999 240 Vert(CT) -0.02 2-8 >999 240 Horz(CT) -0.00 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 32 lb FT = 20%
LUMBER-			BRACING-	

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=107(LC 8) Max Uplift 2=-167(LC 8), 6=-140(LC 8)

Max Grav 2=294(LC 1), 6=220(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 1-4-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=167.6=140.





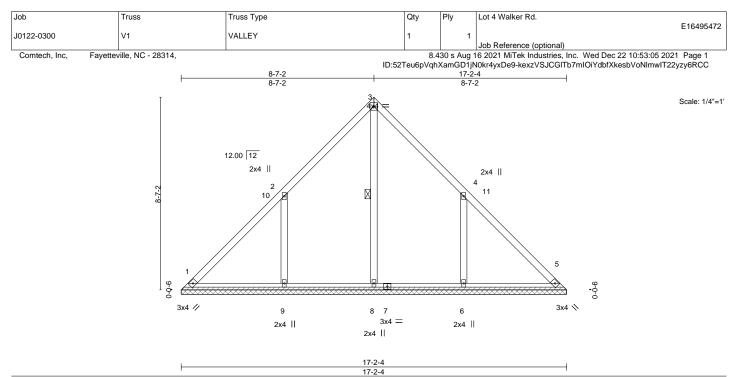


Plate Offsets (X,Y)	[4:0-0-0,0-0-0]				Γ
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.20 BC 0.18 WB 0.13	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	a - n/a 999	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S			Weight: 84 lb FT = 20%
BOT CHORD 2x4 S	P No.1 P No.1 P No.2		BRACING- TOP CHORD BOT CHORD WEBS	Rigid ceiling directly applied of	ectly applied or 6-0-0 oc purlins. or 10-0-0 oc bracing. -8

REACTIONS. All bearings 17-2-4.

(lb) - Max Horz 1=198(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-207(LC 12), 6=-207(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=415(LC 22), 9=537(LC 19), 6=537(LC 20)

WEBS 2-9=-445/331, 4-6=-445/331

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-7-2, Interior(1) 4-7-2 to 8-7-2, Exterior(2) 8-7-2 to 12-11-15, Interior(1) 12-11-15 to 16-10-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

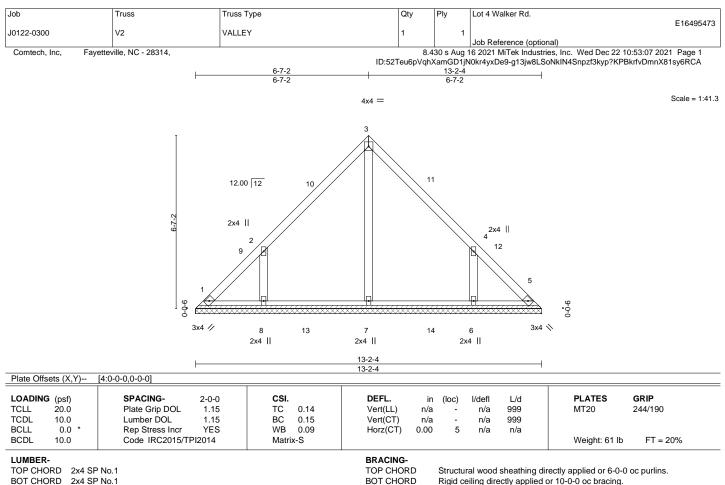
will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=207, 6=207.





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-2-4

(lb) - Max Horz 1=150(LC 9)

2x4 SP No.2

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-164(LC 12), 6=-163(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=388(LC 19), 8=378(LC 19), 6=378(LC 20)

WEBS 2-8=-359/290, 4-6=-359/290

NOTES-

BOT CHORD

OTHERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-7-2, Exterior(2) 6-7-2 to 10-11-15, Interior(1) 10-11-15 to 12-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

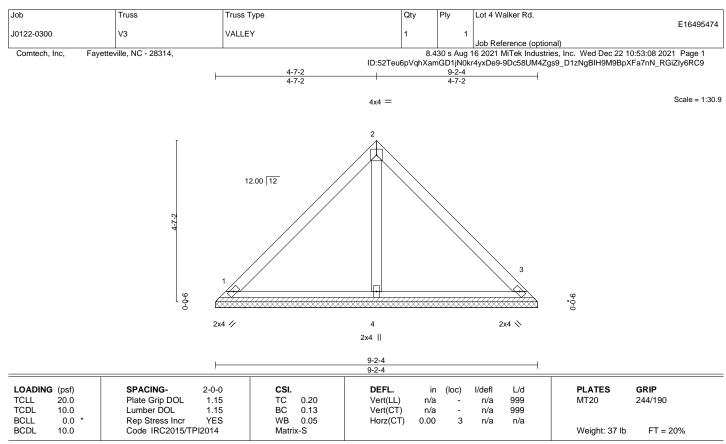
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=164. 6=163.





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=9-2-4, 3=9-2-4, 4=9-2-4

Max Horz 1=-102(LC 8)

Max Uplift 1=-25(LC 13), 3=-25(LC 13) Max Grav 1=192(LC 1), 3=192(LC 1), 4=294(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

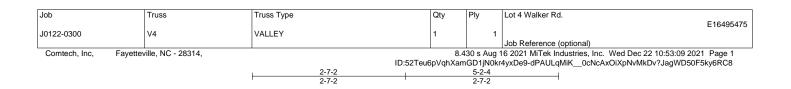
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





4x4 =

2 12.00 12 3 9-0-0



2x4	Ш
	5-2-4
	E 0 4

		1				5-2-4					1	
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-P						Weight: 20 lb	FT = 20%
		1		1							-	
LUMBEF	१-					BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=5-2-4, 3=5-2-4, 4=5-2-4 Max Horz 1=54(LC 9)

Max Uplift 1=-19(LC 13), 3=-19(LC 13) Max Grav 1=109(LC 1), 3=109(LC 1), 4=140(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



0-0-6

Structural wood sheathing directly applied or 5-2-4 oc purlins.

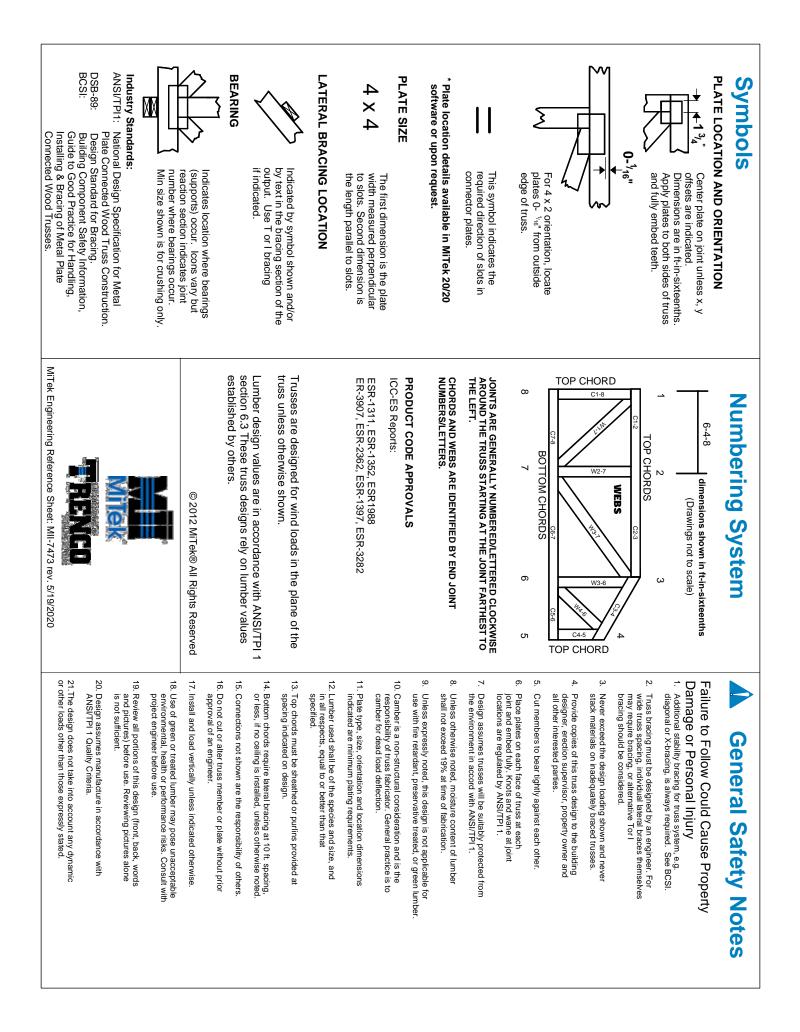
Rigid ceiling directly applied or 10-0-0 oc bracing.

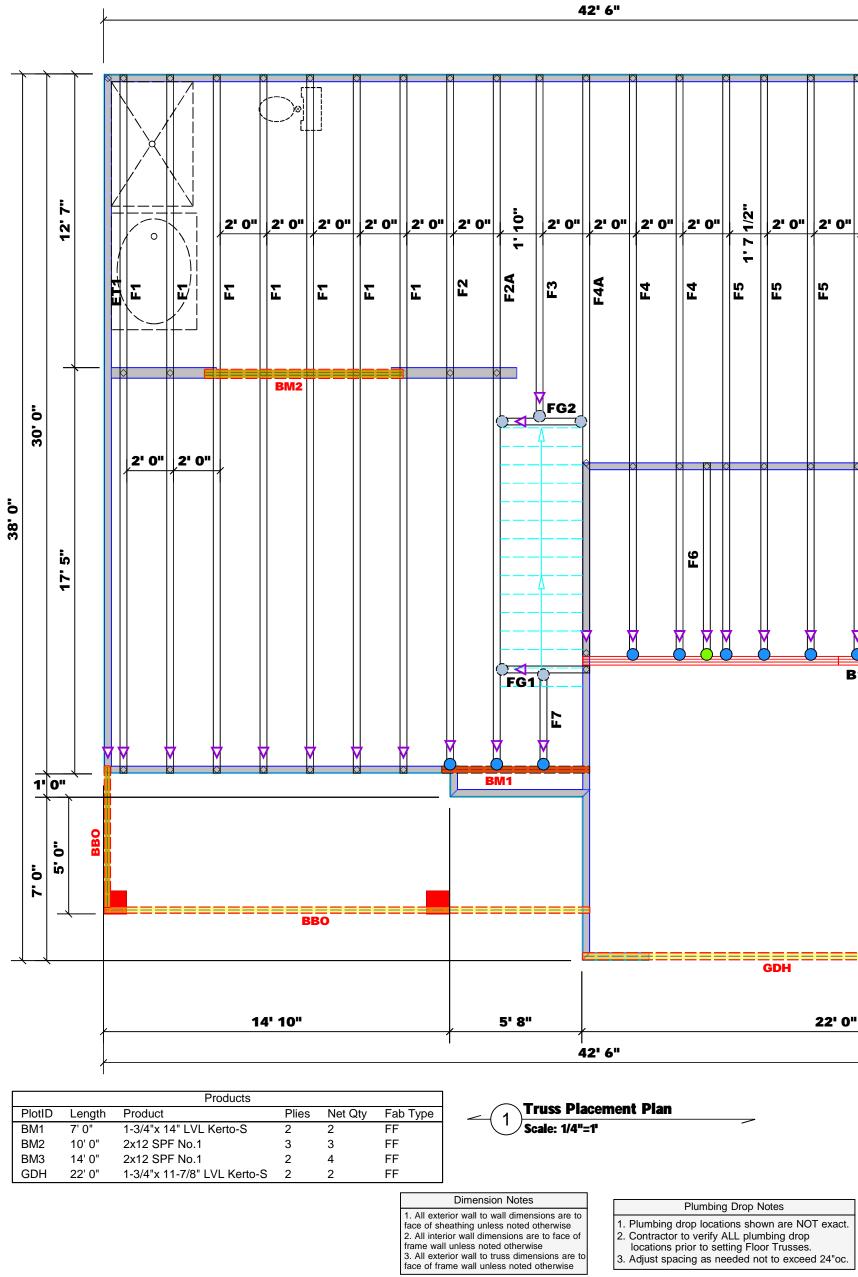
2x4 📏

Scale = 1:18.4

December 22,2021







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2' 0"	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"			
F5	F5	F5	F5	F5	4 4	F4 ET2	16' 8"		
						Õ			
								38' 0"	
				F6					
		1-GR					21. 4"		
3DH								<u> </u>	
	22' 0"					,			
			C	oppector	nformation		I Nail	Information	

	Conne	ctor Info	rmati	on	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	13	Varies	16d/3-1/2"	16d/3-1/2"
\bigcirc	MSH422	USP	5	Varies	10d/3"	10d/3"
\bigcirc	THD410	USP	2	Varies	16d/3-1/2"	10d/3"

Coontector Coonte	LOAD CHART FOR JACK ST (an SED ON TABLES RODEL(); a (b)) NUMBER OF LACK STUDS REQUIRE 0 E A C HEADSD/GTODER (a) 200 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	Linden / Harnett 708 Walker Road Floor 03/18/22 David Landry	COUNTYLinden / HADDRESS708 WalkMODELFloorDATE REV.03/18/22DRAWN BYDavid Lan	BUILDERBen Stout Real EstateJOB NAMELot 4 Walker Rd.JOB NAMEThe FawnbrookPLANThe FawnbrookSEAL DATEN/AQUOTE #
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Bearin deeme require attache Code r founda require but no profes suppo suppo suppo suppo exceed	NUA 2 2 2 2 2 2 2 2 2 2 2 2 2	Linden / Harnett		Ben Stout Real Estate

designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Design	Project: The		ite	In	put by:	-	Rd.		Page 1 of 11
<u> </u>	Lin	den, NC 28356			-				
Kerto-S LVL	1.750" X 1	4.000" 2-	Ply - P	ASSE	D Lev	el: Level			
6		2 SPF							1'2" 1'2" 3 1/2"
formation				Reaction	ns UNPA	TTERNED	lb (Unlift)	
Girder 2 Idition: Dry : 480 : 360 Normal	Building Coc	le: IBC/IRC 2015	5	Brg 1 2	Live 918 918	Dead 1014 1014	Snow 0 0	Wind	0 0
Temp <= 100°F				. .					
				Bearing 1 - SPF	Length 3.500"	37%	1014 / 918	1932 L	D+L
esults				2 - SPF	3.500"	37%	1014 / 918	1932 L	D+L
2472 ft-lb 2472 ft-lb 1509 lb 4'7 0.007 (L/9753) 0.014 (L/4635) tes plies using 3 rows of 10d Br 6". st page of calculations for fr e designed to be supported	3' 26999 ft-lb 0. 3' 26999 ft-lb 0. 1/4" 10453 lb 0. 3' 0.139 (L/480) 0. 3' 0.185 (L/360) 0. 50x nails (.128x3") at 12 asteners required for s on the bottom edge or	092 (9%) D+L 092 (9%) D+L 144 (14%) D+L 050 (5%) L 080 (8%) D+L " o.c. Maximum end c pecified loads.	Case L L L L L						
be continuously braced. aced at bearings.									
Load Type		Width Side	Dead 0.9	Live '	1 Snow 1	1.15 Wind	1.6 Cons	t. 1.25 Co	mments
Uniform		Тор	120 PLF						ll Above
Uniform		Top Far Face	102 PLF					0 PLF C10 0 PLF F2	± ال
Self Weight	chemicals			oper drainage to	prevent Ma	nufacturer Info	,	Comtech, 1001 S C	Inc. Telly Road, Suite #639
of this component based on the d loadings shown. It is the customer and/or the contractor to ment suitability of the intended prify the dimensions and loads.	andling & Installation LVL beams must not be cut or dr Refer to manufacturer's p regarding installation requi fastening details, beam strengt approvals Damaged Beams must not be us Design assumes top edge is late	pondi illed roduct information erments, multi-ply h values, and code ed rally restrained ing points to avoid			Me 30 ⁻ No (80 ww ICC	1 Merritt 7 Buildi rwalk, CT 0685 0) 622-5850 w.metsawood.c	om/us	Fayettevil USA 28314 910-864-	le, NC
		Address: 706 Kerto-S LVL 1.750" X 1 Image: Strate Strat	Address: 208 Walker Read Linden, NC 28356 Kerto-S LVL 1.750" X 14.000" 2 Image: Comparison of the state of t	Address YB Walker Road Linden, NC 28356 Kerto-S LVL 1.750" X 14.000" 2-Ply - P/ Image: State of the state of	Address: 208 Walker Road Linden, NC 28356 4.000 PR Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSEI Image: Comparison of the second of the sec	Address: 708 Walker Road Linden, NC 2835 ubb Name: I Project #** Kerto-S LVL 1.750" X 14.000" 2-Pily - PASSED Ive Image: Comparison of the second seco	Addes: Total Walker Frod Under, NC 28356 Job Name: Und V Walker Project 10/28467 Kerto-S LVL 1.750" X 14.000" 2-Pily - PASSED Level: Level Level: Level Image: Strate Strat	Address Tright Read Linden, No.23836 Project Million 2001 Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED Lone 1.150" X 14.000" 2-Ply - PASSED Lone 1.150" X 14.000 Second	Crieding Automass: 200 Walker Road Linden, NC 2005 We Name: Lot 4.Wukker Rd. Projects: Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED Invest. Units 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image: Not 2000 Image

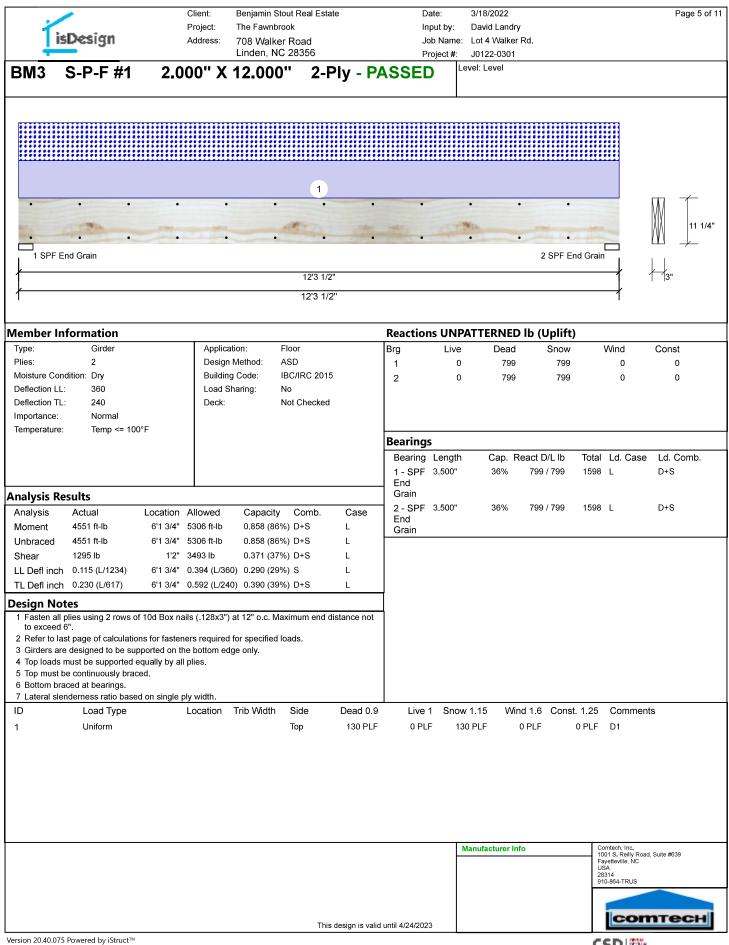
	/		Client:	Benjamin Stout Re	al Estate	Date	e:	3/18/2022	Page 2 of 11
			Project:	The Fawnbrook		Inpu	ut by:	David Landry	
	isDesign		Address:	708 Walker Roa			Name	Lot 4 Walker Rd.	
-				Linden, NC 283			ject #:	J0122-0301	
BM1	Kerto-S	LVL	1.750"	X 14.000"	2-Ply	- PASSED	L	evel: Level	
					-				
					_				
•	•	•	• •	• •	5.				NMA 1
	• •	•	•	• •	1/2				1'2"
					$\overline{\mathbf{v}}$				
Ŀ	•	•	• •	••·					
1 SP	PF			2 SPF	1				
<i>∤</i>		6	6'		1				3 1/2"
<i>↓</i>		6	s'		ł				
I		C C)		1				
Multi-Pl	y Analysis								
Fasten al	ll plies using 3 i	rows of ²	10d Box nails	(.128x3") at 12"	o.c Maxin	num end distan	ce no	t to exceed 6"	
Capacity		83.1							
Load Yield Limit p	por Foot		0 PLF 6 PLF						
	per Fastener	81.9							
Yield Mode		IV							
Edge Distar Min. End Di		1 1/2 3"	2"						
Load Combi		D+L							
Duration Fa		1.00							
Notes			chemicals		6. For flat roofs p	rovide proper drainage to pr	revent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Stru structural adeq	uctured Designs is responsible quacy of this component ba	ased on the 1	landling & Installa LVL beams must not be		ponding			Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA
design criteria responsibility o	a and loadings shown. of the customer and/or the c	It is the 2 contractor to	 Refer to manufacture regarding installation 	urer's product information n requirements, multi-ply				Norwalk, CT 06851	28314 910-864-TRUS
application, and	component suitability of th d to verify the dimensions and	l loads.	fastening details, bear approvals	n strength values, and code				(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service	conditions, unless noted othe	erwise 5	 Damaged Beams must Design assumes top ed Provide lateral support 	lge is laterally restrained				ICC-ES: ESR-3633	
2. LVL not to t	be treated with fire retardant	or corrosive	 Provide lateral suppor lateral displacement an 	t at bearing points to avoid d rotation	This design i	s valid until 4/24/2023			соттесн
Version 20.40	0.075 Powered by iStru	ict™							CSDI

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		Clie	nt: Beniamin	Stout Real Estate	e	D	ate:	3/18/2022			Pag	e 3 of 11
~		Proj					put by:	David Land	ry		5	
i	sDesign	Add	ress: 708 Wall	ker Road		J	b Name:	Lot 4 Walke	er Rd.			
			Linden, I	NC 28356		Р	roject #:	J0122-030	1			
BM2	S-P-F #1	2.000	" X 12.00	0" 3-P	ly - P	ASSE)	evel: Level				
			1									
-		•	•			•	•	•				\uparrow
											MMM	
	C.T.M.			ATT TO	-	-	100				MM	11 1.
	End Grain						SPF End					<u> </u>
			0141			2	SFF Ellu					ווכ
			8'1"								1 1/2	2"
1			8'1"					1				
Momhar	nformation					Postic		ATTERNE		<u> </u>		
Type:	nformation Girder		Application:	Floor		Brg	Live	Dea	D lb (Uplift)) Winc	l Const	
Plies:	3		Design Method:	ASD		ыу 1	2926	97		VVIIIC C		
Moisture Co			Building Code:	IBC/IRC 2015		2	2926	97		C		
Deflection L	L: 360		Load Sharing:	Yes								
Deflection T	L: 240		Deck:	Not Checked								
Importance:	Normal											
Temperature	e: Temp <= 100°F	:										
						Bearing	5					
						Bearing	Length	Cap.	React D/L lb	Total Ld.	Case Ld. Com	ıb.
						1 - SPF	3.500"	58%	978 / 2926	3904 L	D+L	
						End Grain						
Analysis R					0	2 - SPF	3.500"	58%	978 / 2926	3904 L	D+L	
Analysis		ocation Allo	•	-	Case	End						
Moment	7020 ft-lb	4' 1/2" 796		8%) D+L	L	Grain						
Unbraced	7020 ft-lb			8%) D+L	L							
Shear	2777 lb	1'2" 455		51%) D+L	L							
LL Defl inc		4'9/16" 0.25	4 (L/360) 0.290 (2	:9%) L	L							
TL Defl inc	h 0.098 (L/931)	4' 9/16" 0.38	1 (L/240) 0.260 (2	:6%) D+L	L							
Design No	otes											
	I plies using 2 rows of 10	0d Box nails (128x3") at 12" o.c. I	Maximum end di	stance not	1						
to exceed 2 Refer to l	ast page of calculations	for fasteners r	equired for specifie	d loads								
	re designed to be suppo											
	s must be supported equ		i.									
•	be continuously braced	•										
	raced at bearings. enderness ratio based o	on sinale plv wi	dth.									
ID	Load Type		ation Trib Width	n Side	Dead 0.9	Live	1 Snow	v 1.15 W	ind 1.6 Cons	t. 1.25 Cor	mments	
1	Uniform			Тор	242 PLF	724 PL	F	0 PLF	0 PLF	0 PLF F1		
•												
												ĺ
								Vanufacturer	nfo	Comtech,	Inc. eilly Road, Suite #639	
							-			Fayettevill	eilly Road, Suite #639 e, NC	
										USA 28314	2010	
										910-864-T	RUS	
										C	omtec	H
				This o	esign is valid	until 4/24/202	3					
/ersion 20.40.07	75 Powered by iStruct™									COD	L CREAK	

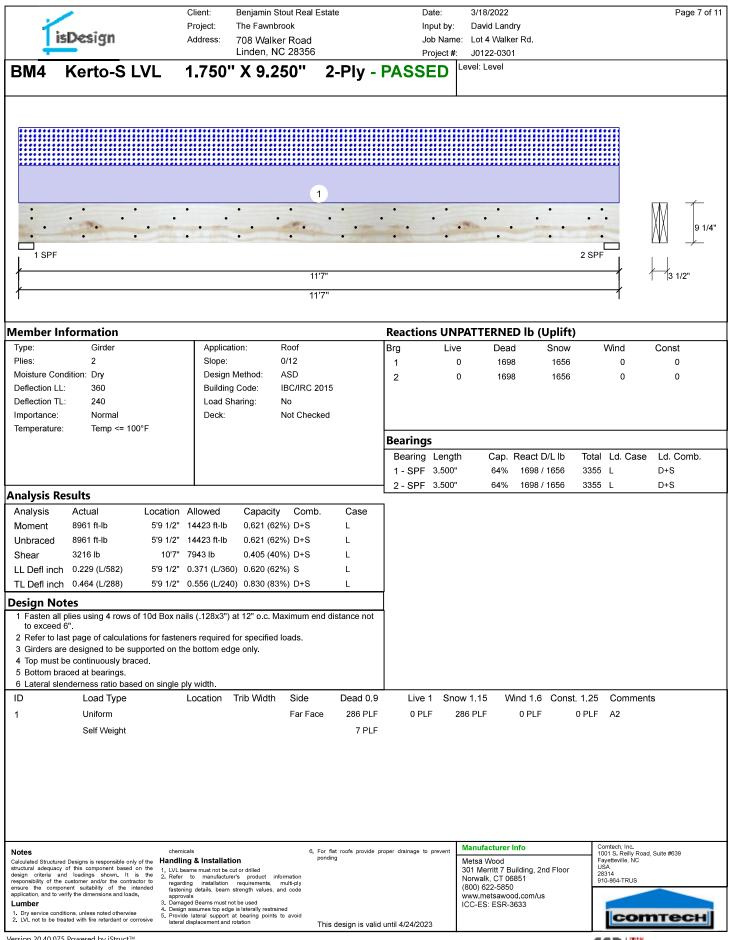
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-		ut Real Estate	Date: 3/18/2022	Page 4 of 11
isDesign	Project: The Fawnbro Address: 708 Walker		Input by: David Landry Job Name: Lot 4 Walker Rd.	
	Linden, NC	28356	Project #: J0122-0301	
BM2 S-P-F #1	2.000" X 12.000	' 3-Ply - PASSE		
	••••	• •		MM
			<11/2	
	• • •	• •		
1 SPF End Grain	8'1"		2 SPF End Grain	4 1/2"
	8'1"			
	01		'	
Multi-Ply Analysis				
	of 10d Box nails (.128x3") at	12" o.c Nail from both sid	des. Maximum end distance not to	exceed
6" Capacity 0	0.0 %	1		
Load 0	0.0 PLF			
Yield Limit per Fastener 7	57.4 PLF 78.7 lb			
	V □ 1/2"			
Min. End Distance 3 Load Combination				
	.00			
			Manufacturer Info	Comtech, Inc.
				Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayettexille, NC USA
				28314 910-864-TRUS
New 20 40 075 Design 1 he (See 17)		This design is valid until 4/24/2	2023	соттесн



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1	isDesign	Client: Project: Address:	Benjamin Stout Re The Fawnbrook 708 Walker Roa Linden, NC 283	ıd		Date: Input by: Job Name Project #:	3/18/2022 David Landry : Lot 4 Walker Rd. J0122-0301	Page 6 of
BM3	S-P-F #	1 2.000" >	K 12.000''	2-Ply	- PASSE	D ^L	_evel: Level	
						1		
	•	• •	• •	•	•	•	• •	• [2][[1] 1/4
	•	• •	• •	•	•	•	• •	
	End Grain		1	2'3 1/2"				2 SPF End Grain
				2'3 1/2"				
Multi-Ply								
Fasten all Capacity Load Yield Limit pe Yield Mode Edge Distand Min. End Dis Load Combir Duration Fac	er Foot er Fastener ce tance nation	ows of 10d Box nail	s (.128x3") at 12"	o.c Maxi	mum end dis	stance no	ot to exceed 6"	
						Г	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
						ľ		1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
				This desigr	n is valid until 4/24/2	2023		соттесн



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1	isDesign	Client: Project: Address:	Benjamin Stout Real The Fawnbrook 708 Walker Road Linden, NC 28356		Date: Input by: Job Nam Project #	e: Lot 4 Walker Rd.	Page 8 of 11
BM4	Kerto-S L	/L 1.750'	" X 9.250"		- PASSED	Level: Level	
				-			
_ ·	• •	•	• •	•	• •	• •	
	• •	•••	• •	•	• •	•••	9 1/4"
	=						
				1'7" 1'7"			
			I	17			I
Multi-Ply	-						
Fasten all Capacity	plies using 4 rows	of 10d Box nails 76.0 %	(.128x3") at 12" c	o.c Maximui	m end distance n	ot to exceed 6"	
Load Yield Limit pe	ar Foot	286.0 PLF 376.5 PLF					
Yield Limit pe		94.1 lb.					
Yield Mode Edge Distanc	e	IV 1 1/2"					
Min. End Dist		3"					
Load Combin Duration Fact		D+S 1.15					
Notes	und Designed 1	chemicals		. For flat roofs provide ponding	e proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequa design criteria responsibility of t	ured Designs is responsible only of acy of this component based on and loadings shown. It is the customer and/or the contracto nponent suitability of the inten	the 1. LVL beams must not be the 2. Refer to manufactur to regarding installation	cut or drilled rer's product information requirements, multi-ply			Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	Payeuevine, NC USA 28314 910-864-TRUS
application, and to Lumber 1. Dry service co	porting suitability of the finance overify the dimensions and loads.	approvals 3. Damaged Beams must r 4. Design assumes top edg 5. Provide lateral support	ge is laterally restrained at bearing points to avoid			www.metsawood.com/us ICC-ES: ESR-3633	соттесн
	075 Powered by iStruct™	lateral displacement and	Totation	This design is va	alid until 4/24/2023		CSDI

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1	Client: Benjamin Stout Real Estate Project: The Fawnbrook	Date: 3/18/2022 Page 9 of 11 Input by: David Landry
isDesign	Address: 708 Walker Road	Job Name: Lot 4 Walker Rd.
DME Karta CIVI		Project #: J0122-0301
BM5 Kerto-S LVL	1.750" X 9.250" 3-Ply	- PASSED
2		3
	1	
		X X X
the second second	and the second second	
1 SPF End Grain	714.01	2 SPF End Grain
,	7'10"	15 1/4"
I	7'10"	1
6		
Image: Member Information Type: Girder	Application: Floor	Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Snow Wind Const
Plies: 3	Design Method: ASD	1 5848 2463 0 0 0
Moisture Condition: Dry	Building Code: IBC/IRC 2015	2 5848 2463 0 0 0
Deflection LL: 480	Load Sharing: Yes	
Deflection TL: 240 Importance: Normal	Deck: Not Checked	
Temperature: Temp <= 100°F		
		Bearings
		Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb.
		1 - SPF 8.000" 23% 2463 / 5848 8310 L D+L End
nalysis Results		Grain
Analysis Actual Locatio	n Allowed Capacity Comb. Case	2 - SPF 8.000" 23% 2463 / 5848 8310 L D+L End
	I" 19565 ft-lb 0.595 (59%) D+L L	Grain
	l" 14536 ft-lb 0.801 (80%) D+L L 2" 10360 lb 0.521 (52%) D+L L	
	I" 0.166 (L/480) 0.680 (68%) L L	
	I" 0.331 (L/240) 0.480 (48%) D+L L	
Design Notes		
1 Girders are designed to be supported or	8 9	
2 Multiple plies must be fastened together 3 Top loads must be supported equally by	•	
4 Top braced at bearings.		
5 Bottom braced at bearings.6 Lateral slenderness ratio based on singl	e ply width.	
ID Load Type	Location Trib Width Side Dead	0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments
1 Uniform	Top 265	PLF 795 PLF 0 PLF 0 PLF 0 PLF J-32'
2 Uniform	Top 120	PLF 0 PLF 0 PLF 0 PLF Wall
3 Uniform	Top 233	PLF 698 PLF 0 PLF 0 PLF 52
Self Weight	11	PLF
		ide areast draining to provide Manufacturer Info Comtech, Inc.
		Metsä Wood Fayetteville, NC
Calculated Structured Designs is responsible only of the Han		301 Merritt 7 Building, 2nd Floor USA
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the 1 Ly design criteria and loadings shown. It is the 2 Reference of the structural structural adequacy of this component based on the structural adequacy of the structural a	/L beams must not be cut or drilled ofer to manufacturer's product information	20314
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the 1. Lucture and loadings shown. It is the stresponsibility of the customer and/or the contractor to resure the component suitability of the interded		Norwalk, CT 06851 910-864-TRUS (800) 622-5850
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the segonsibility of the customer and/or three contractor to measure the component suitability of the intended application, and to verify the dimensions and loads.	əfer to manufacturer's product information garding installation requirements, multi-ply stening details, beam strength values, and code	Norwalk, CT 06851 20014 910-864-TRUS

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	Design	Pr Ac	oject: T Idress: 7 L	enjamin Stou ne Fawnbrool 08 Walker F inden, NC 2	< Road 8356		Jol Pro	out by: o Name: oject #:	3/18/2022 David Land Lot 4 Walk J0122-030	er Rd.			Page 10
SDH H	Kerto-S LVI	_ 1.7	750'' X	11.875	" 2-1	Ply - P	ASSEE) Lev	el: Level				
								3					
		2			1								,
1 SPF End		-			top.	17		4.18		2 606	• End Grai		
	Grain									2 3 6 6		" J	
					16'10"								´ 3 1/2"
I					16'10"							1	
ember Inf	ormation						Peaction		TTEDNE	D lb (Uplif	+)		
ype:	Girder		Application	n: Flo	or		Brg	Live	Dea		-	Wind	Const
Plies:	2		Design Me				1	0	214	0 168	;	0	0
Ioisture Cond Deflection LL:	ition: Dry 360		Building C Load Shar		C/IRC 2015		2	0	214	0 168	;	0	0
effection TL:	240		Deck:	-	t Checked								
nportance:	Normal												
emperature:	Temp <= 100°F												
							Bearings						
							Bearing 1 - SPF	-	Cap. 22%	React D/L lb 2140 / 168	Total 2308	Ld. Case L	Ld. Comb. D+S
nalysis Res	ulte						End Grain						
nalysis		ocation Al	lowed	Capacity	Comb.	Case	2 - SPF	3.500"	22%	2140 / 168	2308	L	D+S
loment	8521 ft-lb	8'5" 17		0.476 (48%)	D	Uniform	End Grain						
Inbraced	8521 ft-lb	8'5" 17	'919 ft-lb	0.476 (48%)	D	Uniform							
hear				0.229 (23%)		Uniform							
			. ,	0.060 (6%)		L							
L Defl inch	0.480 (L/410) 8	"5 1/16" 0.8	319 (L/240)	0.590 (59%)	D+S	L							
esign Not							ļ						
to exceed 6	lies using 2 rows of 10 ".	d Box nails	(.128x3") at	12" o.c. Maxii	num end dis	stance not							
	page of calculations		•	•	ds.								
	designed to be suppo just be supported equi		-	oniy.									
•	continuously braced.												
	ed at bearings. derness ratio based o	n sinale ply v	width.										
)	Load Type			ib Width	Side	Dead 0.9	Live 1	Snow '	1.15 V	lind 1.6 Con	st. 1.25	Comments	6
	Uniform			-	Гор	45 PLF	0 PLF	0	PLF	0 PLF	0 PLF	Wall Above	
	Uniform			-	Гор	180 PLF	0 PLF	0	PLF	0 PLF	0 PLF	B1GE	
	Tie-In	0-0-0 to 1	6-10-0 1-0	D-0 -	Гор	20 PSF	0 PSF	20	PSF	0 PSF	0 PSF	Roof Load	
	Self Weight					9 PLF							
	oon worght												
		chemicals			6 Enc A-1	roofe previde	oper drainage to p	Ma	nufacturer	Info	Co	mtech, Inc.	
ructural adequacy o	Designs is responsible only of the first second on the third second on the third second on the second on the second secon	e Handling a	& Installation s must not be cut o	drilled	 For flat ponding 		oper orainage to p	Me	tsä Wood	uilding, 2nd Floor	10 Fa US	01 S. Reilly Road, yetteville, NC	Suite #639
sign criteria and sponsibility of the cr	loadings shown. It is the stomer and/or the contractor to	e 2 Refer to regarding	manufacturer's installation re	product informa quirements, mult	-ply			No	rwalk, CT 0	6851		314 0-864-TRUS	
plication, and to verif	nt suitability of the intende y the dimensions and loads.	d fastening approvals	details, beam stre Beams must not be	ngth values, and c	ode			ŵw	0) 622-5850 w.metsawo	od.com/us			
Umber Dry service condition	ns, unless noted otherwise	 Design ass 5. Provide la 	sumes top edge is l teral support at b	aterally restrained earing points to a	/oid			ICO	C-ES: ESR-	3633		leor	тесн
I VE NOT TO DE freat	ed with fire retardant or corrosiv	ateral disc	lacement and rotat	ion		lesign is valid	until 4/04/0000	1					

isDesi	gn	Client: Project: Address:	Benjamin Stout Rea The Fawnbrook 708 Walker Roa Linden, NC 2835	d	Date Input Job N Proje	: by: David Landry Name: Lot 4 Walker Rd.		Page 11 of 11
GDH Kert	o-S LVL	1.750"	X 11.875"	2-Ply	- PASSED	Level: Level		
							ىة	
	•••	• •	• •	• •	•••	• • •		11 7/8"
1 SPF End Grain	<u> </u>	•••	•••	<u> </u>	•••	· · ·	2 SPF End Grain	: [1]
				16'10"				3 1/2"
1				16'10"			1	
Multi-Ply Analysi Fasten all plies usi Capacity Load Yield Limit per Foot Yield Mode Edge Distance Min. End Distance Load Combination Duration Factor	ng 2 rows of 0.0 16 81 IV	0 % 0 PLF 33.7 PLF .9 lb. 1/2"	.128x3") at 12"	o.c Maxim	um end distanc	e not to exceed 6"		
Notes Calculated Structured Designs is structural adequacy of this com design criteria and loadings responsibility of the customer an ensure the component suitabi application, and to verify the dimer Lumber 1. Dry service conditions, unless 2. LVL not to be treated with fire Version 20.40.075 Powered	conent based on the shown. It is the d/or the contractor to ity of the intended usions and loads. noted otherwise retardant or corrosive	 LVL beams must not be of 2. Refer to manufacture regarding installation 	ut or drilled product information requirements, multi-ply strength values, and code of be used a is laterally restrained at bearing points to avoid	ponding	vide proper drainage to prev valid until 4/24/2023	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2 Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/u: ICC-ES: ESR-3633	s	ттесн

	CS	D	DESIGN
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RE: J0122-0301 Lot 4 Walker Rd. Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Benjamin Stout Real Estate Lot/Block: 4	Project Name: J0122-0301 Model: Fawnbrook
Address: 708 Walker Road	Subdivision: Walker Rd.
City: Linden	State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 13 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
		ET1	
1	E16494820	EII	12/22/2021
2	E16494821	ET2	12/22/2021
3	E16494822	F1	12/22/2021
4	E16494823	F2	12/22/2021
5	E16494824	F2A	12/22/2021
6	E16494825	F3	12/22/2021
7	E16494826	F4	12/22/2021
8	E16494827	F4A	12/22/2021
9	E16494828	F5	12/22/2021
10	E16494829	F6	12/22/2021
11	E16494830	F7	12/22/2021
12	E16494831	FG1	12/22/2021
13	E16494832	FG2	12/22/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

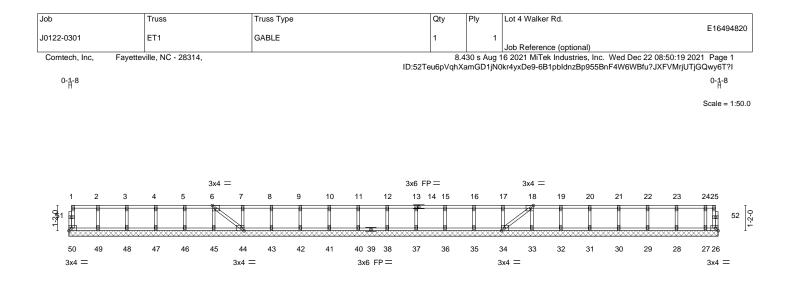
My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

December 22, 2021



29-11-0 1-4

	[0:0 1 0,Edg0]; [10:0 1 0,Edg0]; [04:0	1 0;Euge], [11:0 1 0;Euge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. i Vert(LL) n/a Vert(CT) n/a Horz(CT) -0.00	a - n/a 999	PLATES MT20 Weight: 128 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 S WEBS 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing of except end verticals. Rigid ceiling directly applied 10-0-0 oc bracing: 49-50,48	d or 6-0-0 oc bracing, E	xcept:

REACTIONS. All bearings 29-11-0.

(lb) - Max Uplift All uplift 100 lb or less at joint(s) 26

Max Grav All reactions 250 lb or less at joint(s) 50, 49, 48, 47, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 27

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26.

7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

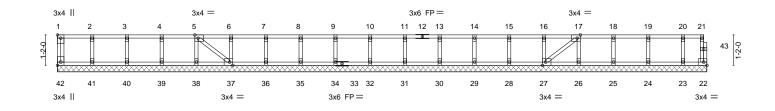




Job		Truss	Truss Type	Qty	Ply	Lot 4 Walker Rd.
J0122-0301		ET2	GABLE	1	1	E16494821
00122 0001						Job Reference (optional)
Comtech, Inc.	Favettev	/ille, NC - 28314.		8.4	30 s Aua	16 2021 MiTek Industries, Inc. Wed Dec 22 08:50:20 2021 Page 1

ID:52Teu6pVqhXamGD1jN0kr4yxDe9-aNaBoeePkVx0jFmzpn2L3PC3jjtUEp3sj7SpyMy6T?H 0-指名

Scale = 1:41.6



 1-4-0
 2-8-0
 4-0-0
 5-4-0
 6-8-0
 9-4-0
 10-8-0
 12-0-0
 13-4-0
 14-8-0
 16-0-0
 17-4-0
 18-8-0
 20-0-0
 21-4-0
 22-8-0
 24-0-0
 24-11-Q

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LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. Vert(LL) n/ Vert(CT) n/ Horz(CT) -0.0	'a -	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 108 lb	GRIP 244/190 FT = 20%F. 11%E
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	except e	end verticals.	g directly applied or 6-0-0 red or 6-0-0 oc bracing.	

REACTIONS. All bearings 24-11-0.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 42, 22, 41, 40, 39, 38, 37, 36, 35, 34, 32, 31, 30, 29, 28, 27, 26, 25, 24, 23

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

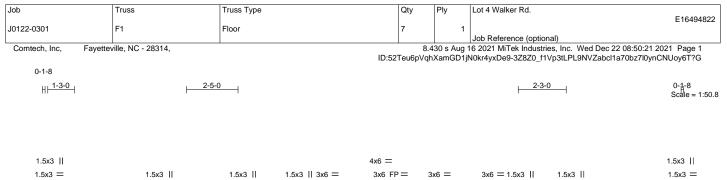
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

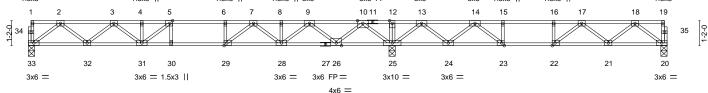
Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.









 	<u> </u>					29-11 12-10		
Plate Offsets (X,Y)	[5:0-1-8,Edge], [22:0-1-8,Edge], [23:0-1	-8,Edge], [29:0-1-8,Edge]						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.88 BC 0.85 WB 0.58 Matrix-S	Vert(CT)	in (loc) -0.19 29-30 -0.26 29-30 0.05 20	l/defl >999 >767 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 148 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD BOT CHORD WEBS 2x4 SP No.1(flat) BRACING- TOP CHORD 2x4 SP No.1(flat) Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals. BOT CHORD WEBS 2x4 SP No.3(flat) BOT CHORD Structural wood sheathing directly applied or 2-0 oc purlins, except end verticals. REACTIONS. (size) 33=0-3-8, 20=0-3-8, 25=0-3-8 BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc bracing.								
FORCES. (Ib) - Max TOP CHORD 2-3	Grav 33=822(LC 3), 20=614(LC 4), 25=1 (. Comp./Max. Ten All forces 250 (lb) oi =-1687/0, 3-4=-2703/0, 4-5=-2703/0, 5-6= =-2065/0, 9-10=-549/294, 10-12=0/2133,	less except when shown. -2936/0, 6-7=-2936/0, 7-8	=-2065/0,					
BOT CHORD 32- 26- 21- WEBS 2-3 10-	15=-1607/250, 15-16=-1607/250, 16-17=- 33=0/1025, 31-32=0/2317, 30-31=0/2936 28=-30/1421, 25-26=-830/0, 24-25=-1240 22=-9/1538, 20-21=0/756 3=-1283/0, 2-32=0/862, 3-32=-820/0, 3-3 3=-620/1220, 18-20=-945/0, 18-21=0/549, 5=-1182/0, 9-28=0/871, 7-28=-674/0, 7-2	, 29-30=0/2936, 28-29=0/2 /0, 23-24=-643/1160, 22-2 l=0/493, 5-31=-489/137, 1 7-21=-469/93, 17-22=-33	23=-250/1607, 0-25=-1646/0, 1/89,					

NOTES-

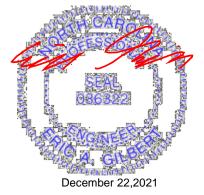
1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

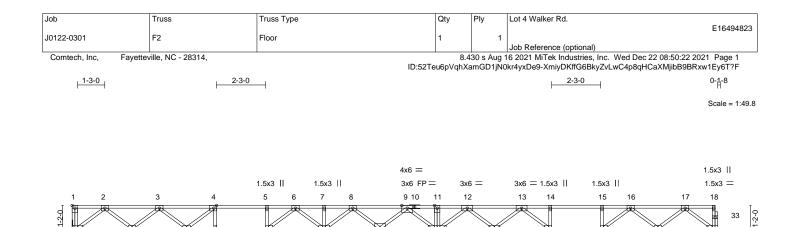
3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means. 5) CAUTION, Do not erect truss backwards.







27

3x6 =

26 25

3x6 FP =

4x6 =

Ř

24

3x10 =

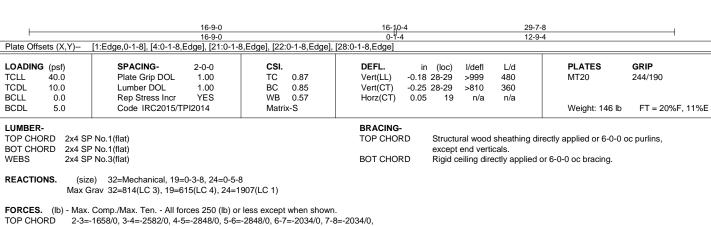
23

3x6 =

22

21

20



 8-9=-563/296, 9-11=0/2082, 11-12=0/2082, 12-13=-578/905, 13-14=-1611/221, 14-15=-1611/221, 15-16=-1611/221, 16-17=-1179/0

 BOT CHORD
 31-32=0/1002, 30-31=0/2282, 29-30=0/2848, 28-29=0/2848, 27-28=0/2500, 25-27=-36/1414, 24-25=-810/0, 23-24=-1188/0, 22-23=-602/1165, 21-22=-221/1611, 20-21=0/1540, 19-20=0/756

 WEBS
 2-32=-1257/0, 2-31=0/854, 3-31=-813/0, 3-30=0/392, 4-30=-431/27, 9-24=-1619/0, 9-25=0/1194, 17-19=-946/0, 17-20=0/550, 16-20=-470/84, 16-21=-317/90, 8-25=-1155/0, 8-27=0/843, 6-27=-652/0, 6-28=0/750, 5-28=-323/0, 12-24=-1316/0, 12-23=0/876, 13-23=-920/0, 13-22=0/36, 14-22=-417/0

29

1.5x3 ||

28

30

NOTES-

32

3x6 =

31

1) Unbalanced floor live loads have been considered for this design.

All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

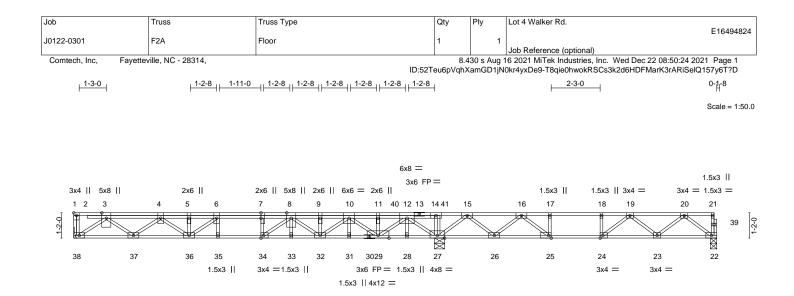


X

19

3x6 =

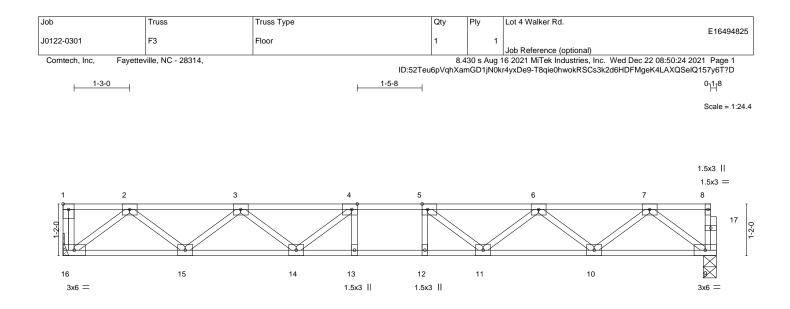




 	16-9-0 16-10-4 16-9-0 0-1-4			29-7-8 12-9-4				
Plate Offsets (X,Y)	[1:Edge,0-1-8], [7:0-3-0,Edge], [24:0-1-4	8,Edge], [25:0-1-8,Edge],	÷ · ·			12-9-4		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.76 BC 0.75 WB 0.82 Matrix-S	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) 0.	25 34-35	l/defl >999 >807 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 174 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	2x4 SP 2400F 2.0E(flat)		BRACING- TOP CHORD BOT CHORD	except	end vertic	als.	ctly applied or 6-0-0 c	oc purlins,
REACTIONS. (siz	e) 38=Mechanical, 27=0-5-8, 22=0-3-8 Grav 38=964(LC 3), 27=2406(LC 1), 22=							
TOP CHORD 3-4= 9-10 15-11 BOT CHORD 37-3 31-3 25-2 WEBS 3-38 12-2 16-2 7-34	Comp./Max. Ten All forces 250 (lb) or -2136/0, 4-5=-3370/0, 5-6=-3370/0, 6-7= =-2773/0, 10-11=-1237/0, 11-12=-1237// 6=-130/1235, 16-17=-1334/425, 17-18=- 8-0/1273, 36-37=0/2963, 35-36=0/3559 32=0/2048, 29-31=0/2048, 28-29=-674/0 6=-889/777, 24-25=-425/1334, 23-24=-1 =-1562/0, 3-37=0/1096, 4-37=-1051/0 9=0/1730, 11-29=-374/0, 15-27=-1531/0 6=-983/0, 16-25=0/1048, 17-25=-483/0, =-399/0, 6-36=-364/210, 20-22=-870/0, 2 4=-429/0	-3559/0, 7-8=-3559/0, 8-), 12-14=0/2680, 14-15=(1334/425, 18-19=-1334/4 34-35=0/3559, 33-34=0, , 27-28=-674/0, 26-27=-1 12/1373, 22-23=0/696 -36=0/508, 5-36=-262/11 , 10-29=-1078/0, 15-26=(10-32=0/953, 8-32=-591/	9=-2773/0,)/2687, 1425, 19-20=-1070/7 3203, 32-33=0/3203, 552/0, , 12-27=-2453/0,)/927, 0, 8-34=0/803,					
 2) All plates are 3x6 M 3) Plates checked for a 4) Refer to girder(s) fo 5) Recommend 2x6 st Strongbacks to be a 6) CAUTION, Do not e 7) Hanger(s) or other or Ib down at 14-9-4 o 8) In the LOAD CASE(LOAD CASE(S) Stan 1) Dead + Floor Live (I Uniform Loads (plf) Vert: 22-38 Concentrated Loads 	connection device(s) shall be provided so on top chord. The design/selection of sur (S) section, loads applied to the face of t dard balanced): Lumber Increase=1.00, Plate =-10, 1-21=-100	ts center. ts cand fastened to each tr strained by other means. ufficient to support concer ch connection device(s) is he truss are noted as fror	ntrated load(s) 241 lb c s the responsibility of o	lown at 4-		526	December	22,2021







 			14-8-8				
Plate Offsets (X,Y) [1	:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8,	Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.32 BC 0.65 WB 0.39 Matrix-S	Vert(LL) -0.13	n (loc) l/defl 3 12-13 >999 3 12-13 >947 4 9 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 75 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP N BOT CHORD 2x4 SP N WEBS 2x4 SP N REACTIONS. (size) Max Gra	lo.1(flat) lo.3(flat)		BRACING- TOP CHORD BOT CHORD	except end vertic	cals.	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1611/0, 3-4=-2484/0, 4-5=-2748/0, 5-6=-2484/0, 6-7=-1610/0 BOT CHORD 15-16=0/977, 14-15=0/2210, 13-14=0/2748, 12-13=0/2748, 11-12=0/2748, 10-11=0/2210, 9-10=0/976 WEBS 2-16=-1225/0, 2-15=0/825, 3-15=-780/0, 3-14=0/414, 4-14=-496/0, 7-9=-1221/0, 7-10=0/826, 6-10=-781/0, 6-11=0/414, 5-11=-496/0							
NOTES-							

Unbalanced floor live loads have been considered for this design.

All plates are 3x4 MT20 unless otherwise indicated.

Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

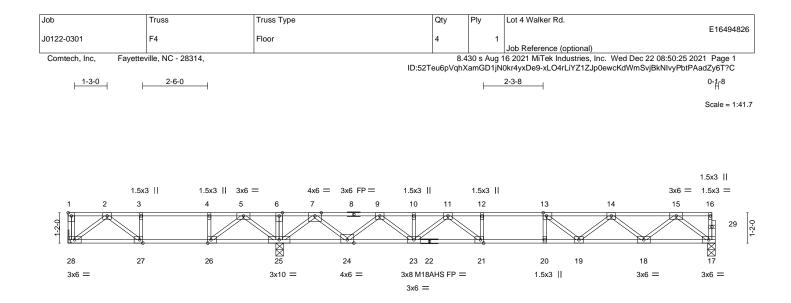
5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

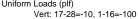
6) CAUTION, Do not erect truss backwards.







H	8-1-8 8- 8-1-8 0	1 ₁ 12 0-4		24-11-0 16-9-4			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [13:0-1-8,Edge], [21:0	1-8,Edge], [26:0-1-8,Edge	e], [27:0-1-8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.85 BC 0.86 WB 0.54 Matrix-S	Vert(LL) -0.21	(loc) l/defl 21-23 >960 21-23 >717 17 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 123 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S		1	BRACING- TOP CHORD BOT CHORD	except end verti	cals.	ctly applied or 6-0-0 o 6-0-0 oc bracing.	oc purlins,
FORCES. (Ib) - Max FOP CHORD 1-28 7-9= 13-1 30T CHORD 27-2 21-2 WEBS 2-22 9-24	Grav 28=1890(LC 3), 25=1573(LC 1), 1 3=-1554/0, 2-3=-611/299, 3-4=-611/299 3=-1554/0, 9-10=-2489/0, 10-11=-2489/0 14=-2795/0, 14-15=-1764/0 28=-60/417, 26-27=-299/611, 25-26=-73 23=0/2903, 20-21=0/3160, 19-20=0/316 3=-523/75, 2-27=-306/248, 5-25=-821/0 4=-1078/0, 9-23=0/756, 11-23=-561/0, 1 3=-428/0, 15-18=0/917, 14-18=-871/0, 1	r less except when show 4-5=-611/299, 5-6=0/122 11-12=-3160/0, 12-13=-3 4/169, 24-25=-45/267, 23 0, 18-19=0/2434, 17-18=0 5-26=0/874, 7-25=-1526 1-21=0/630, 12-21=-279//	5, 6-7=0/1225, 1160/0, -24=0/1921, //1060 0, 7-24=0/1130, 0, 15-17=-1326/0,				
 All plates are MT20 All plates are 3x4 N Plates checked for S Refer to girder(s) fr Recommend 2x6 s Strongbacks to be CAUTION, Do not LOAD CASE(S) Star 	ve loads have been considered for this) plates unless otherwise indicated. AT20 unless otherwise indicated. a plus or minus 1 degree rotation abour or truss to truss connections. trongbacks, on edge, spaced at 10-0-0 attached to walls at their outer ends or n erect truss backwards. mdard (balanced): Lumber Increase=1.00, Plat	its center. oc and fastened to each t estrained by other means		3") nails.		AND S	2000

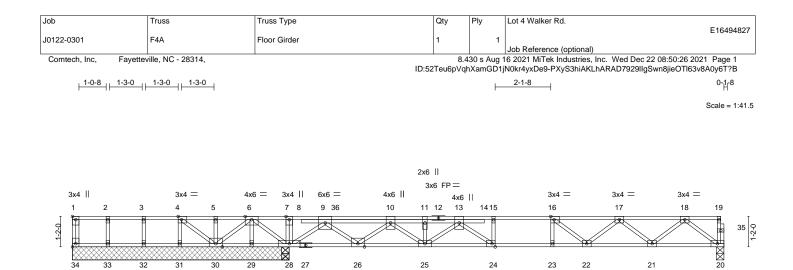


Concentrated Loads (lb)









3x6 =

3x4 =

3x4 =

3x4 =

3x6 =

L	8-0-0 8	3 ₇ 3 ₇ 8)-3-8		24-11-0			
Plate Offsets (X,Y)	8-0-0 ([1:Edge,0-1-8], [4:0-1-8,Edge], [16:0-1-		[34:Edge.0-1-8]	16-7-8			1
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO	CSI. TC 0.73 BC 0.85 WB 0.61	DEFL. ir Vert(LL) -0.16 Vert(CT) -0.23	n (loc) l/defl 5 22-23 >999 5 22-23 >879 5 20 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	Horz(CT) 0.03	3 20 n/a	n/a	Weight: 135 lb	FT = 20%F, 11%E
BOT CHORD 2x4 SI WEBS 2x4 SI REACTIONS. All b	P No.3(flat) earings 8-3-8 except (jt=length) 20=0-3-		BRACING- TOP CHORD BOT CHORD	except end vertica Rigid ceiling direc 6-0-0 oc bracing:	als. tly applied c	ectly applied or 6-0-0 c or 10-0-0 oc bracing, I 9,26-28.	
Max C FORCES. (lb) - Max TOP CHORD 4-5= 11-1 BOT CHORD 29-3 23-2 WEBS 6-28 10-2	Jplift All uplift 100 lb or less at joint(s) e Grav All reactions 250 lb or less at joint 20=764(LC 4) . Comp./Max. Ten All forces 250 (lb) or 0/366, 5-6=0/366, 6-7=0/2801, 7-9=0/28 3=-1711/0, 13-15=-2538/0, 15-16=-2533 0=-1208/0, 28-29=-1208/0, 26-28=-1053 4=0/2538, 22-23=0/2538, 21-22=0/2127 =-1978/0, 6-29=0/507, 6-30=0/1075, 4-3 6=-1254/0, 10-25=0/857, 13-25=-601/0, 1=-753/0, 17-22=0/364, 16-22=-390/0	t(s) 34, 30, 31, 32, 33 exc r less except when shown 301, 9-10=-252/176, 10-11 3/0, 16-17=-2364/0, 17-18 9/0, 25-26=0/1024, 24-25= 7, 20-21=0/941 30=-459/0, 9-28=-2246/0,	ept 28=2735(LC 1), 28=2 =-1711/0, =-1549/0 =0/2192, 9-26=0/1290,				
 All plates are 1.5x3 Plates checked for Provide mechanica joint 30 and 226 lb Recommend 2x6 st Strongbacks to be a CAUTION, Do not e Hanger(s) or other chord. The design/ 	rongbacks, on edge, spaced at 10-0-0 attached to walls at their outer ends or re	its center. Ing plate capable of withsta oc and fastened to each tr estrained by other means. strutticient to support concert the responsibility of othe	uss with 3-10d (0.131" X ntrated load(s) 491 lb doo rs.	3") nails.	4		

LOAD CASE(S) Standard

3x4 ||

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

4x6 =

3x6 FP =

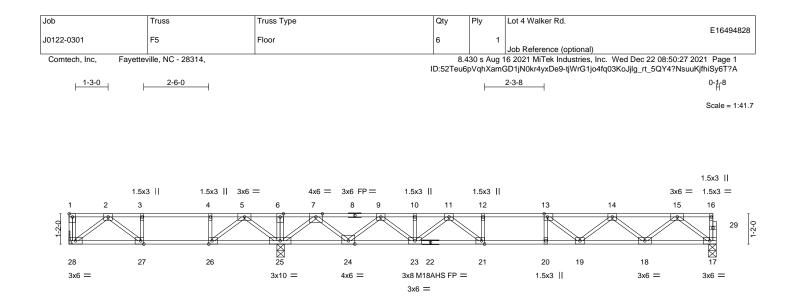
3x10 =

4x6 =

- Vert: 20-34=-10, 1-19=-100 Concentrated Loads (lb)
- Concentrated Loads (lb) Vert: 36=-411(B)







l		r12		<u>24-11-0</u> 16-9-4		
Plate Offsets (X,Y)	[1:Edge,0-1-8], [13:0-1-8,Edge], [21:0-	••	, [27:0-1-8,Edge]	10-9-4		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.73 BC 0.78 WB 0.54 Matrix-S	Vert(LL) -0.21	(loc) l/defl L/d 21-23 >960 480 21-23 >717 360 17 n/a n/a	PLATES MT20 M18AHS Weight: 123 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	2 11	oc purlins,
Max	ze) 28=Mechanical, 25=0-3-8, 17=0-3- Uplift 28=-16(LC 4) Grav 28=391(LC 3), 25=1573(LC 1), 17					
TOP CHORD 2-3 9-1	c. Comp./Max. Ten All forces 250 (lb) c =-611/299, 3-4=-611/299, 4-5=-611/299, D=-2489/0, 10-11=-2489/0, 11-12=-3160, 15=-1764/0	5-6=0/1225, 6-7=0/1225, 7	7-9=-1115/0,			
	28=-60/416, 26-27=-299/611, 25-26=-73- 23=0/2903, 20-21=0/3160, 19-20=0/3160					
WEBS 2-2 14	23=0/2905, 20-21=0/3180, 19-20=0/3180 8=-521/76, 2-27=-305/250, 5-25=-821/0, 19=0/501, 13-19=-603/0, 7-25=-1526/0, 4=-1078/0, 9-23=0/756, 11-23=-561/0, 1	15-17=-1327/0, 15-18=0/9 7-24=0/1130, 5-26=0/874,	917, 14-18=-871/0,			

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x4 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Refer to girder(s) for truss to truss connections.

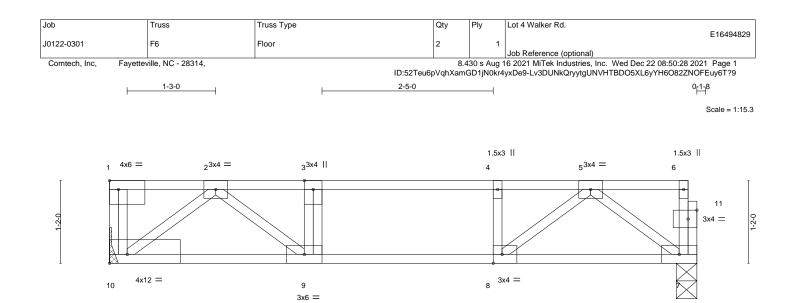
7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

8) CAUTION, Do not erect truss backwards.







L			8-3-8			
			8-3-8			1
Plate Offsets (X,Y)	[1:Edge,0-1-8], [8:0-1-8,Edge], [10:Edge	e,0-1-8], [11:0-1-8,0-1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO	CSI. TC 0.42 BC 0.27 WB 0.22	DEFL. in Vert(LL) -0.04 Vert(CT) -0.05 Horz(CT) 0.07	5 9-10 >999 360	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 43 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing din except end verticals. Rigid ceiling directly applied o	<i>y</i> 11	oc purlins,
WEBS 2x4 SI	P No.3(flat)		BOT CHORD	Rigid ceiling directly applied of	or 10-0-0 oc bracing.	

REACTIONS. (size) 10=Mechanical, 7=0-3-8 Max Grav 10=4092(LC 1), 7=436(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-10=-3709/0, 2-3=-810/0, 3-4=-810/0, 4-5=-810/0

BOT CHORD 9-10=0/489, 8-9=0/810, 7-8=0/485 WEBS

2-10=-614/0, 2-9=0/451, 5-7=-604/0, 5-8=0/460

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

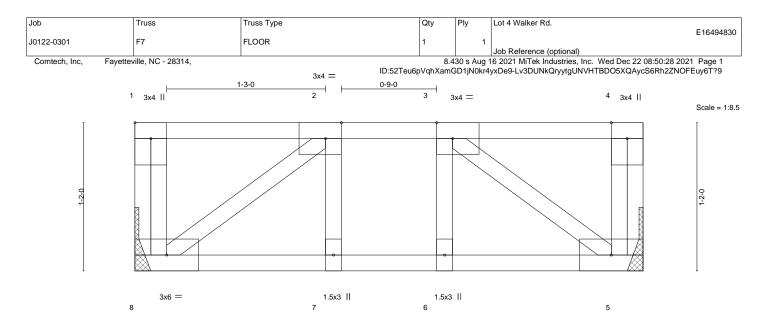
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 7-10=-10, 1-6=-100 Concentrated Loads (lb) Vert: 1=-3650

3x6 =

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3x6 =

Plate Offsets (X,Y)	[1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,	Edge]	4-0-0 4-0-0			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.09 BC 0.06 WB 0.06 Matrix-S	DEFL. in Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	7 >999 360	PLATES MT20 Weight: 25 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	<i>y</i>) oc purlins,

REACTIONS. (size) 8=Mechanical, 5=Mechanical Max Grav 8=206(LC 1), 5=206(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

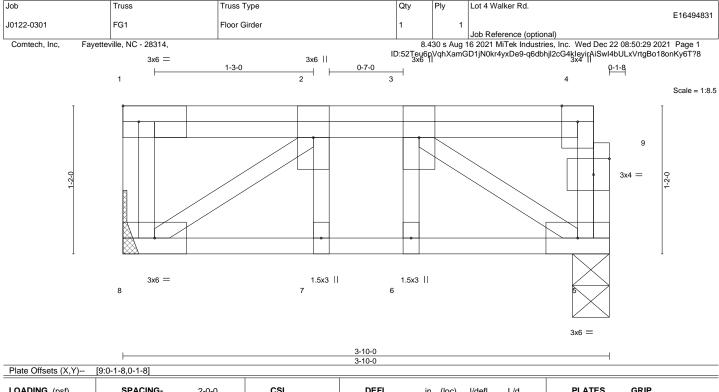
1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

a) Refer to girder(s) for truss to truss connections.
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

A designed to be December 22,2021





LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.08 WB 0.07 Matrix-S	DEFL. in Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	0 7 >999 480 0 7 >999 360	PLATES MT20 Weight: 28 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	<i>y</i> 11	-0 oc purlins,	
REACTIONS. (size Max G	e) 8=Mechanical, 5=0-3-8 Brav 8=261(LC 1), 5=234(LC 1)					

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-257/0

BOT CHORD 7-8=0/257, 6-7=0/257, 5-6=0/257

WEBS 3-5=-307/0, 2-8=-311/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 135 lb down at 1-10-4 on top

chord. The design/selection of such connection device(s) is the responsibility of others.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

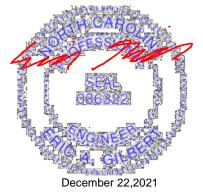
LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

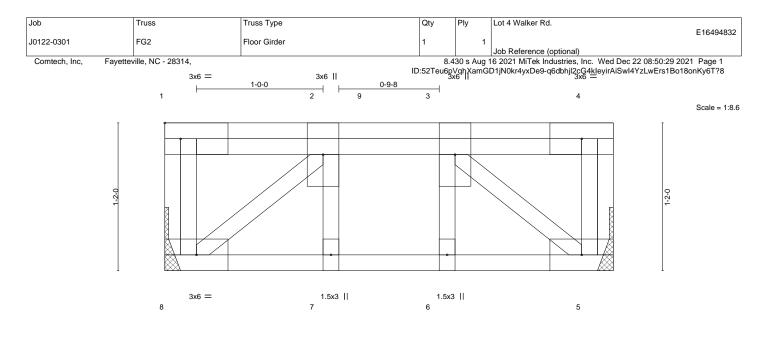
Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb)









except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

3-6-8 3-6-8												
LOADING TCLL TCDL	(psf) 40.0 10.0	SPACING- Plate Grip DOL	2-0-0 1.00	CSI. TC	0.22	DEFL. Vert(LL)	in -0.00 -0.01	(loc) 7	l/defl >999	L/d 480	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 5.0	Lumber DOL Rep Stress Incr Code IRC2015/TP	1.00 NO 12014	BC WB Matri	0.16 0.18 x-S	Vert(CT) Horz(CT)	0.00	5	>999 n/a	360 n/a	Weight: 27 lb	FT = 20%F, 11%E
LUMBER-	RD 2x4 S	P No.1(flat)				BRACING- TOP CHOR		Structu	ral wood	sheathing di	rectly applied or 3-6-8	oc purlins,

BOT CHORD

TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat) 2x4 SP No.3(flat) WFBS

REACTIONS. (size) 8=Mechanical, 5=Mechanical Max Grav 8=546(LC 1), 5=511(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-590/0

BOT CHORD 7-8=0/590, 6-7=0/590, 5-6=0/590

WEBS 2-8=-765/0, 3-5=-765/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 720 lb down at 1-8-4 on top

chord. The design/selection of such connection device(s) is the responsibility of others.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

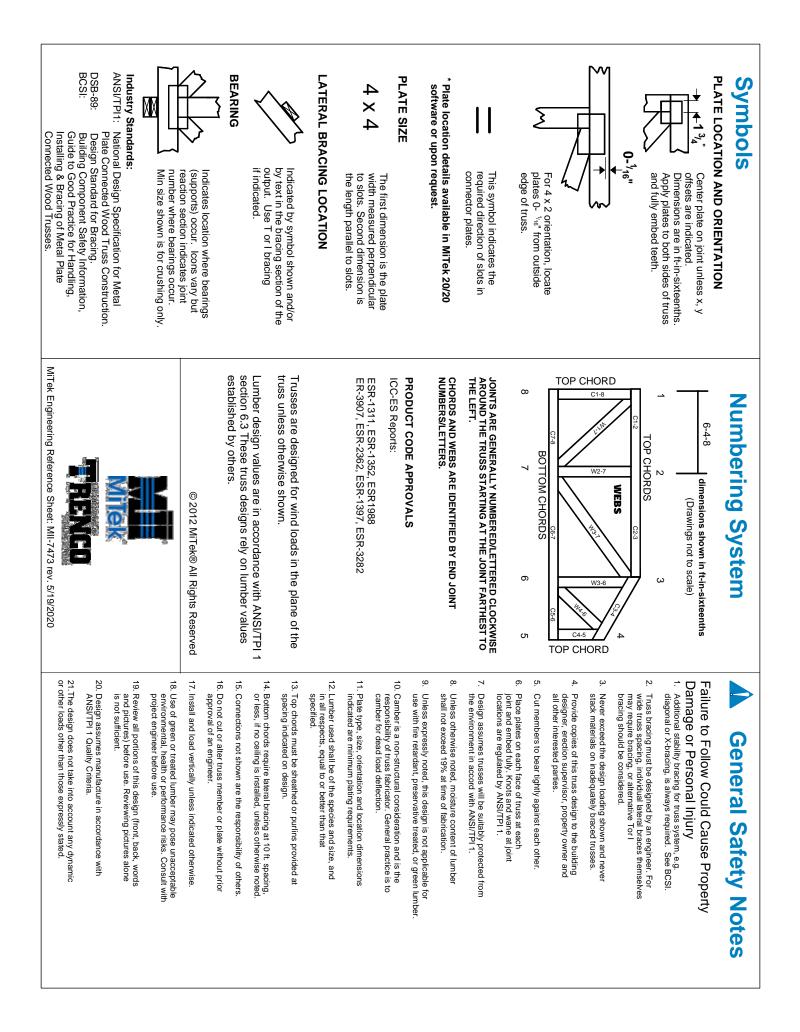
Uniform Loads (plf)

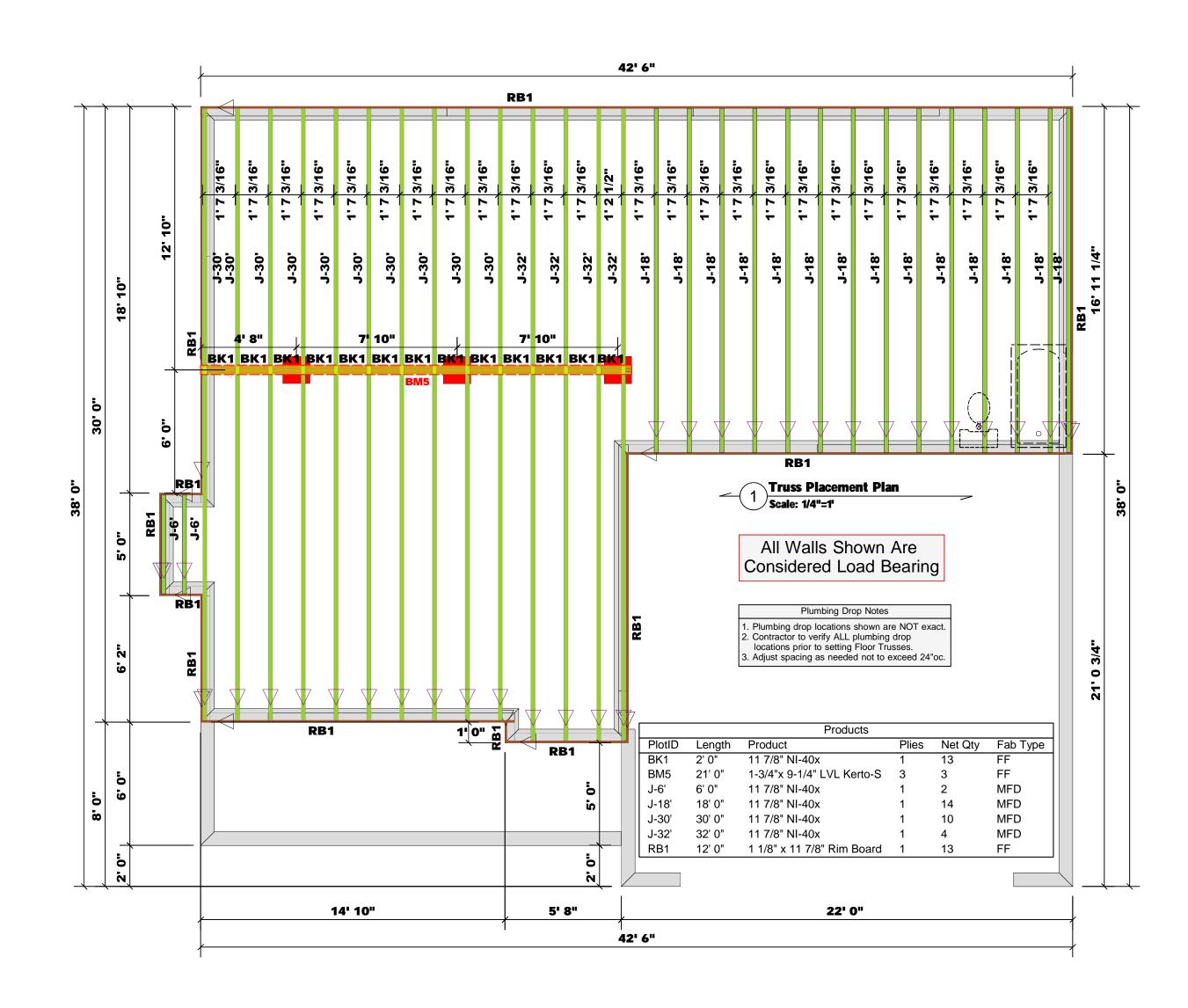
Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb)











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Ben Stout Real Estate COUNTY Lind	Lot 4 Walker Rd. ADDRESS 708	The Fawnbrook MODEL I-Jo	N/A DATE REV. 03/	DRAWN BY David Landry	J0322-1390 SALESMAN Marshall Naylor			
THIS IS These tr comport design a See indi identifie designe consult	A TRUSS USSES AT TRUSS TUSSES AT THE SECOND A TRUSS TUSSES AT THE SECOND A TRUSS TUSSES AT THE SECOND A TRUSS TUSSES A TRUST TUSSES A TRUST T	S PLACEN e designe e incorpo accification ssign she placemer place	JUDENTIFY OF CONTRACT OF CONT	FAM ON vidual buto o the build uilding de ch truss of s, The buil ary and floor systen n of the truss floor systen arding bra ided with sbcindus	# BOC Ly. ilding signer. design Iding em and uss wwalls, ding accing, the			