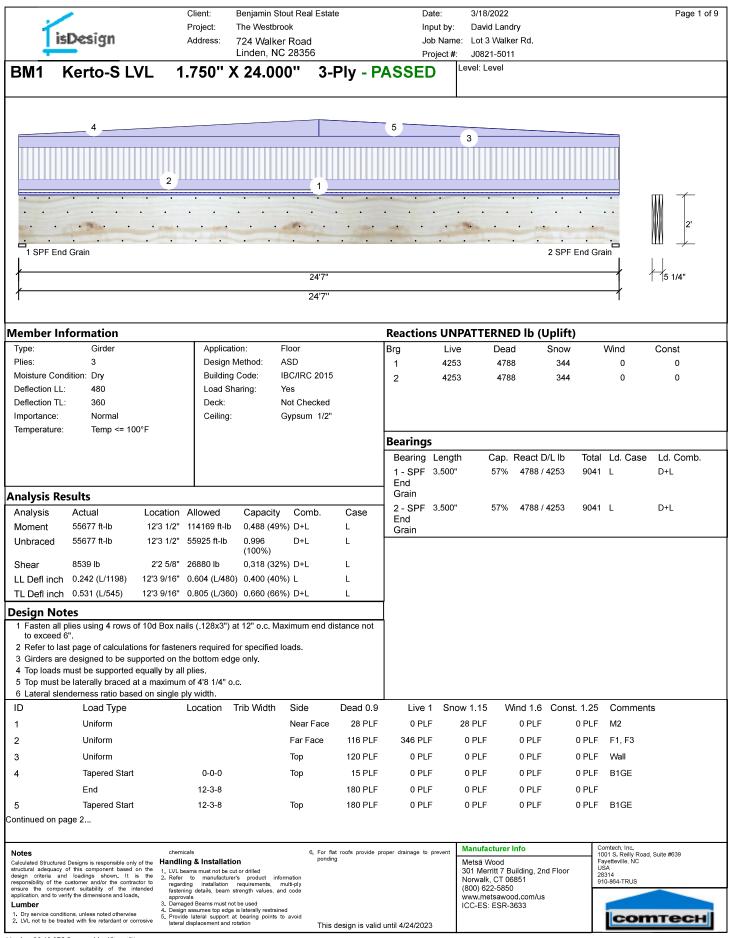


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THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



(Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards



Version 20.40.075 Powered by iStruct™

-	Client: Benjamin Stout Re	al Estate	Date:	3/18/2022	Page 2 of 9
isDesign	Project: The Westbrook Address: 724 Walker Roa			David Landry Lot 3 Walker Rd.	
BM1 Kerto-S LVL	Linden, NC 283	³⁻ Ply - PASSE	Project #:	J0821-5011 evel: Level	
	1.750 X 24.000	5-FTy - FASSE			
4		5		3	
	2	1			
		· · · · · ·	· · ·	· · · · · ·	·. M 1
· · · · · · · ·		7. · · ·		· · · · ·	2'
1 SPF End Grain			•	2 SPF End	I Grain
 		24'7"			5 1/4"
<i>†</i>		24'7"			
Continued from page 1					
ID Load Type End	Location Trib Width Side 24-7-0			v 1.15 Wind 1.6 Const. 1. 0 PLF 0 PLF 0 F	
Self Weight	24-7-0	28 PLF	~LF	UPLF UPLF UF	LF
Notos	chemicals	6. For flat roofs provide proper drainage	to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the	Handling & Installation 1. LVL beams must not be cut or drilled	 For flat roots provide proper drainage ponding 		Metsä Wood 301 Merritt 7 Building, 2nd Floor	1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314
responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.	regarding installation requirements multi-ply			Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	910-864-TRUS
Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	 Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid 			CC-ES: ESR-3633	соттесн
Version 20.40.075 Powered by iStruct™	lateral displacement and rotation	This design is valid until 4/24/2	023		CSD

isDesign	Client: Benjamin Stout Re Project: The Westbrook Address: 724 Walker Roz Linden, NC 283	Input by ad Job Nan	ne: Lot 3 Walker Rd.	Page 3 of 9
BM1 Kerto-S LVL	1.750" X 24.000"	56 Project # 3-Ply - PASSED	Level: Level	
1 SPF End Grain	· · · · · · · ·	· · · · · · · ·	2 SPF En	
		24'7"		5 1/4"
<u>/</u>		24'7"		
Multi-Ply Analysis				
Fasten all plies using 4 rows of 10 6" Capacity 94.1 % Load 308.01 Yield Limit per Foot 327.41 Yield Limit per Fastener 81.9 lb Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination D+L Duration Factor 1.00	5 PLF PLF	' o.c Nail from both sides. Ma	ximum end distance not to	o exceed
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and localings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise	hemicals Inding & Installation VL beams must not be out or drilled feer to manufacturer's product information gearding installation requirements, multi-ply setning details, beam strength values, and component provals. Beams must not be used amaged Jeams must not be used amaged Jeams and the strength restrained rowide lateral support at bearing points to avoid ateral displacement and rotation	 For flat roofs provide proper drainage to prevent ponding This design is valid until 4/24/2023 	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Comtech, Inc. 1001 S. Relly Road, Suite #639 Fayetterille, NC USA 28314 910-864-TRUS

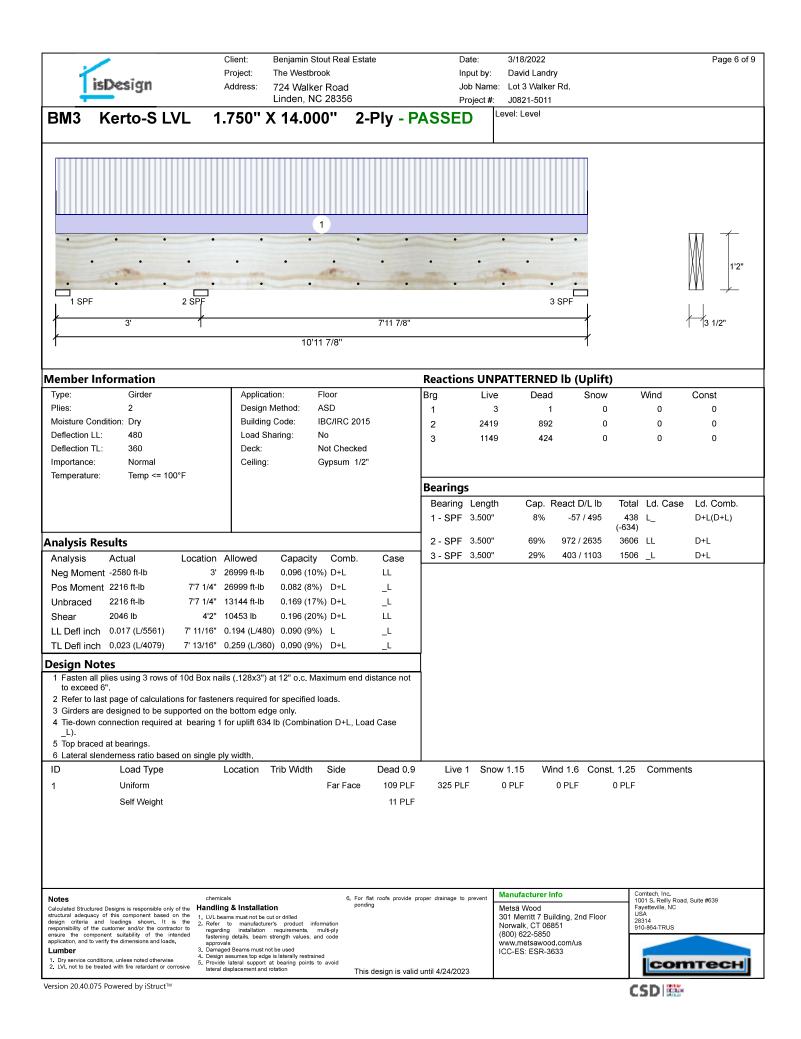
CSD 🗱

				njamin Stou e Westbroo	ıt Real Estate k	9		ate: put by:	3/18/2022 David Lan				Page 4 of
i	sDesign	A		4 Walker					Lot 3 Wall				
				iden, NC				oject #:	J0821-501 evel: Level	11			
BM2	Kerto-S L	VL 1.7	750" X 1	16.000)" 2-F	Ply - P	ASSE	ן נ	evel. Level				
	2	1											
•	· · ·	· · ·	• •	•									
	170	-		2 SPF									∭1'4"
		6'4"											3 1/2"
ł		6'4''		{									
/lember l	nformation							s UNP	ATTERN	ED lb (Upli	ft)		
Type: Plies:	Girder 2		Application: Design Met		oor SD		Brg	Live	Dea			Wind	Const
Moisture Co			Building Co		C/IRC 2015		1	1742 1742	62 62	22 22	0 0	0 0	0 0
Deflection L			Load Sharin				2	1742	02	-2	0	0	Ū
Deflection T	L: 360		Deck:	No	ot Checked								
Importance:	Normal		Ceiling:	Gy	ypsum 1/2"								
Temperature	e: Temp <= 10)0°F											
							Bearings	;					
							Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	3.500"	45%	622 / 1742	2364	L	D+L
			L				2 - SPF	3.500"	45%	622 / 1742	2364	L	D+L
nalysis R					<u> </u>		1						
Analysis	Actual	Location A		Capacity	Comb.	Case							
Moment	3243 ft-lb			.094 (9%)		L							
Unbraced	3243 ft-lb			.167 (17%)		L							
Shear	2184 lb			.183 (18%)		L							
LL Defl incl			.147 (L/480) 0			L							
TL Defl incl	h 0.015 (L/4665)	3'2" 0	.197 (L/360) 0	.080 (8%)	D+L	L							
esign No		(1010	(100, 01) - (1	011 · · · · · · · ·			4						
	l plies using 4 rows o l 6".	of 10d Box halls	(.128x3") at 12	2" o.c. Maxi	imum ena ais	stance not							
1 Fasten all to exceed		ons for fastener	s required for s	pecified loa	ads.								
to exceed 2 Refer to la	1 0	innorted on the	bottom edge of	nly.									
to exceed 2 Refer to la 3 Girders a	re designed to be su	ipported on the											
to exceed 2 Refer to la 3 Girders at 4 Top brace	re designed to be su ed at bearings.		width.										
to exceed 2 Refer to la 3 Girders at 4 Top brace 5 Lateral sle	re designed to be su	ed on single ply	width. ocation Trib	Width	Side	Dead 0.9	Live 1	1 Snow	/1.15 V	Vind 1.6 Cor	nst. 1.25	Comment	S
to exceed 2 Refer to la 3 Girders an 4 Top brace 5 Lateral slo ID	re designed to be su ed at bearings. enderness ratio base	ed on single ply			Side Near Face	Dead 0.9 47 PLF	Live 1 139 PLF		v 1.15 V 0 PLF	Vind 1.6 Cor 0 PLF	nst. 1.25 0 PLF	Comment F5	S
to exceed 2 Refer to la 3 Girders at 4 Top brace 5 Lateral slo 1D	re designed to be su ed at bearings. enderness ratio base Load Type Uniform	ed on single ply			Near Face	47 PLF	139 PLF	=	0 PLF	0 PLF	0 PLF	F5	s
to exceed 2 Refer to la 3 Girders at 4 Top brace	re designed to be su ed at bearings. enderness ratio base Load Type	ed on single ply						=				F5	S

CSD 🗱

seten all plies using 4 rows of 10d Box nails (128x3*) at 12* o.c. Maximum end distance not to exceed 6* pachy 274 PUF bid limit puF fada 927.4 PUF bid limit puF fadarer 918.0 m. ad Combnation 0.1 ad Combnation 0.0	K.	Client: Benjamin Stout Re Project: The Westbrook		te: 3/18/2022 ut by: David Landry	Page 5 of 9
BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED Land Land Image: State of the state of	IsDesign				
Sint Keine-S Lot Insort A 10,000 Z-HIJ-FRASED Image: Sint P and piles using 4 rows of 10d Box nais (128x3°) at 12° oct. Maximum end distance not to exceed 6° Image: Sint P and piles using 4 rows of 10d Box nais (128x3°) at 12° oct. Maximum end distance not to exceed 6° Image: Sint P and piles using 4 rows of 10d Box nais (128x3°) at 12° oct. Maximum end distance not to exceed 6° Image: Sint P address 10 B 10 B 10°					
Image: Protect in the second of the secon	BM2 Kerto-S LV	L 1.750" X 16.000"	2-Ply - PASSED		
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Image: set in a s		· · · · · · · · · · · · · · · · · · ·	_		W L
Image: set in a s	1 SPF				
Image: set of the set of 10d Box nails (128x3") at 12" o.c Maximum end distance not to exceed 6" party 393.7 % party 327.4 PLF bit integer Faste 1, 12° n. ft of Datance 3° p. ft of Datacc 3° p. ft of D	<u>,</u>				
Interpretation ³ / ₂ × 1/ ₂	4				3 1/2"
sector all plies using 4 rows of 10d Box nails (128x3*) at 12* o.c. Maximum end distance not to exceed 6* PACY Start PL pack of the production of the	1	6'4"			
sector all plies using 4 rows of 10d Box nails (128x3*) at 12* o.c. Maximum end distance not to exceed 6* PACY Start PL pack of the production of the					
paratry 83.7 % ad 27.4 0 µ F ki Linto per Sono 31.9 h, ki Linto per Sono 31.9 h, n. End Distance 3* ad Combination 0-L, radio Fector 1.00	Multi-Ply Analysis				
paratry 83.7 % ad 27.4 0 µ F ki Linto per Sono 31.9 h, ki Linto per Sono 31.9 h, n. End Distance 3* ad Combination 0-L, radio Fector 1.00	asten all plies using 4 rows		o.c Maximum end distan	ce not to exceed 6"	
Material control 327.4 PLF Material control Name Material control	apacity				
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Includition 1/2" Includition 0-1 Includition 0-1 <td< td=""><td>ield Limit per Fastener</td><td>81.9 lb.</td><td></td><td></td><td></td></td<>	ield Limit per Fastener	81.9 lb.			
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Dote Main definition of the composed profile methods and definition of the composed profile methods of the co	oad Combination				
Interview Chemicals Chemicals Chemicals Chemicals Chemicals Chemicals Chemicals Display alculated Structured Designs is responsible only of the segin criteria and loadings shown. It is the sponsibility of the customer and/or the contraction requirements, multiply fastening deliate, beam strength values, and code approvals Iourts. Relify Road, Suite #639 Fastening Umber 1. UL beams must not be customer and/or the contraction requirements, multiply fastening deliate, beam strength values, and code approvals Metsa Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 USA Umber 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid Series to avoid Series to avoid Series to avoid	Juration Factor	1.00			
Interview Chemicals Chemicals Chemicals Chemicals Chemicals Chemicals Chemicals Display alculated Structured Designs is responsible only of the segin criteria and loadings shown. It is the sponsibility of the customer and/or the contraction requirements, multiply fastening deliate, beam strength values, and code approvals Iourts. Relify Road, Suite #639 Fastening Umber 1. UL beams must not be customer and/or the contraction requirements, multiply fastening deliate, beam strength values, and code approvals Metsa Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 USA Umber 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid Series to avoid Series to avoid Series to avoid					
	structural adequacy of this component based on design criteria and loadings shown. It is responsibility of the customer and/or the contracto ensure the component suitability of the inten application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise	the Handling & Installation 1. UV beams must not be cut or dilled 4. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral europart at beams priors to avoid	6. For flat roofs provide proper drainage to p ponding	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	1001 S. Rellly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
	2. LVL not to be treated with fire retardant or corror	sive lateral displacement and rotation	This design is valid until 4/24/2023		соптесн

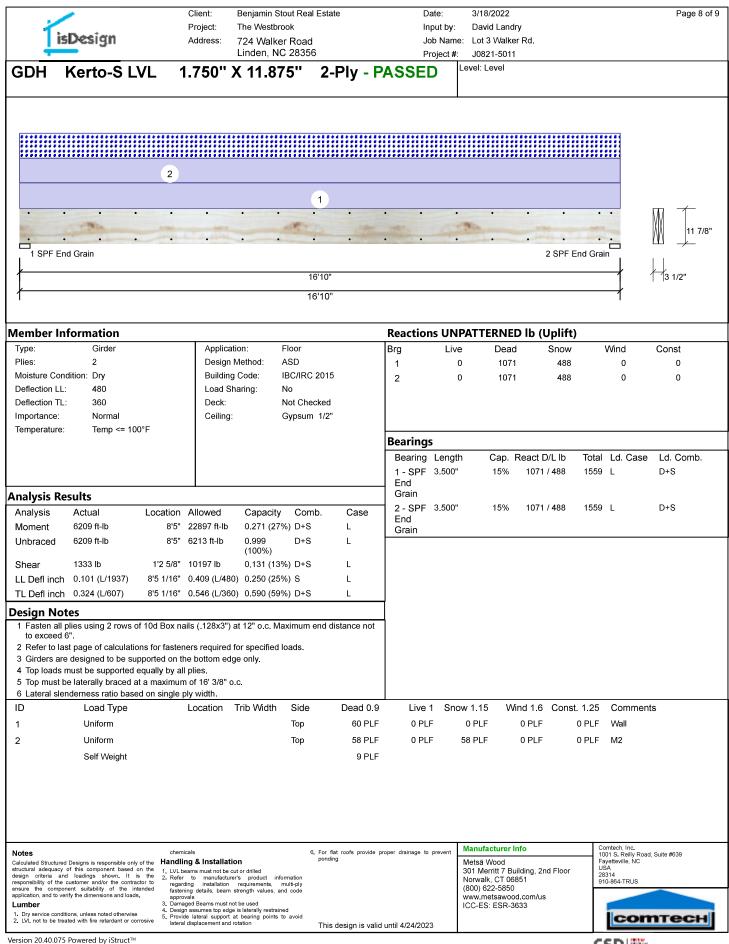
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/	_	Client: Project:	Benjamin Stout Rea The Westbrook	al Estate	Date:	3/18/2022 y: David Landry	Page 7 of 9
isDesi	gn	Address:	724 Walker Roa		Input b Job Na	me: Lot 3 Walker Rd.	
BM3 Kerte	o-S LVL	1 750"	Linden, NC 2835 X 14.000''		Project	t #: J0821-5011 Level: Level	
		11750	X 14.000	2-i iy			
							,
•••	•	• •	٠	•	• •	••••	\overline{M} 1
•	• •	•	• •	•	• •		1'2"
	•	•••	•	•	• •	····	
1 SPF	2 SPF	-				3 SPF	
3'	1			7'11 7	7/8"		13 1/2"
			10'11 7/8"			1	
Multi-Ply Analysi	5						
Fasten all plies usi		0d Box nails	(.128x3") at 12"	o.c Maxim	um end distance	not to exceed 6"	
Capacity Load	88.4 % 217.0						
Yield Limit per Foot	245.6	PLF					
Yield Limit per Fastener Yield Mode	81.9 ll IV	D.					
Edge Distance	1 1/2"						
Min. End Distance Load Combination	3" D+L						
Duration Factor	1.00						
Notes		chemicals		6. For flat roofs pro	ovide proper drainage to preven	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is r structural adequacy of this comp design criteria and loadings	shown. It is the 2	ndling & Installa LVL beams must not be Refer to manufactu	cut or drilled ırer's product information	ponding		Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	Fayetteville, NC USA 28314 910-864-TRUS
responsibility of the customer an ensure the component suitabil application, and to verify the dimen	d/or the contractor to ity of the intended sions and loads.	regarding installation fastening details, bean approvals	requirements, multi-ply n strength values, and code			(800) 622-5850 www.metsawood.com/us	510-004-1100
Lumber 1. Dry service conditions, unless 2. LVL not to be treated with fire	noted otherwise 5.	Damaged Beams must Design assumes top ed Provide lateral support lateral displacement and	ge is laterally restrained t at bearing points to avoid	This design in	s valid until 4/24/2023	ICC-ES: ESR-3633	соттесн
				rins design is	vallu until 4/24/2023	ļ	

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CSD 🚟

1	isDesign		Client: Project: Address	The Westbroo	Road		Date: Input by: Job Name Project #:	3/18/2022 David Landry e: Lot 3 Walker Rd. J0821-5011		Page 9 of 9
GDH	Kerto-S	LVL	1.750	" X 11.87	5" 2-Ply	/ - PAS	-	Level: Level		
_{[•}	• •	•	• •	• •	• •	•	• •	• •		$\overline{\mathbf{M}}$ \mathbf{T}
	• •	•				•	• •		<u>.</u> .	11 7/8"
1 SPF	End Grain								2 SPF End Grain	
					16'10"					1/2 "
					16'10"				I	
Multi-Ply	-									
Fasten all Capacity	plies using 2	0.0	%	ils (.128x3") at	12" o.c Maxi]	imum end	distance n	ot to exceed 6"		
Load Yield Limit pe		163	PLF 3.7 PLF							
Yield Limit pe Yield Mode	er Fastener	81. IV	9 lb.							
Edge Distand Min. End Dist		1 1 3"	/2"							
Load Combin	nation									
Duration Fact		1.0			J					
Notes Calculated Struct	tured Designs is responsible	e only of the	chemicals Handling & Insta	allation	6. For flat roofs ponding	provide proper dra	iinage to prevent	Manufacturer Info Metsä Wood	Comtech, Inc. 1001 S. Reilly R Fayetteville, NC	oad, Suite #639
structural adequa design criteria responsibility of f	acy of this component ba and loadings shown. the customer and/or the c	ised on the It is the contractor to	1. LVL beams must n 2. Refer to manu regarding install	ot be cut or drilled facturer's product infon ation requirements, m	ulti-ply			301 Merritt 7 Building, 2 Norwalk, CT 06851		
application, and to Lumber	mponent suitability of the overify the dimensions and	loads.	fastening details, approvals 3. Damaged Beams r 4. Design assumes to	beam strength values, and nust not be used op edge is laterally restrained	l code			(800) 622-5850 www.metsawood.com/u ICC-ES: ESR-3633		
2. LVL not to be	onditions, unless noted othe	or corrosive	 Provide lateral su lateral displacement 	pport at bearing points to	avoid	n is valid until 4,	24/2023			тесн
version 20.40.0	075 Powered by iStru	C('''							CSD 🐻	C 44

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Trenco RE: J0821-5011 Lot 3 Walker Rd. Site Information: Customer: Benjamin Stout Real Estate Project Name: J0821-5011 Lot/Block: 3 Model: Westbrook Address: 724 Walker Road Subdivision: Walker Rd. State: NC City: Linden General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf This package includes 23 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name
1	E16171316	A1-GR	9/15/2021	21	E16171336	X2
2	E16171317	A2	9/15/2021	22	E16171337	X3
3	E16171318	A3	9/15/2021	23	E16171338	Y1
4	E16171319	A4	9/15/2021			
5	E16171320	A4-GR	9/15/2021			
6	E16171321	B1	9/15/2021			
7	E16171322	B1GE	9/15/2021			
8	E16171323	B2	9/15/2021			
9	E16171324	B3	9/15/2021			
10	E16171325	B3GE	9/15/2021			
11	E16171326	C1	9/15/2021			
12	E16171327	C1GE	9/15/2021			
13	E16171328	M1	9/15/2021			
14	E16171329	M1GE	9/15/2021			
15	E16171330	M2	9/15/2021			
16	E16171331	V1	9/15/2021			
17	E16171332	V2	9/15/2021			
18	E16171333	V3	9/15/2021			
19	E16171334	V4	9/15/2021			
20	E16171335	X1	9/15/2021			

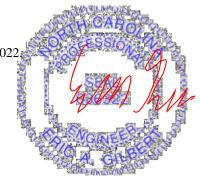
The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



818 Soundside Rd Edenton, NC 27932

> Date 9/15/2021

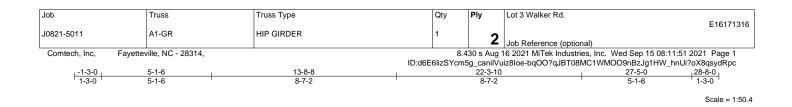
9/15/2021

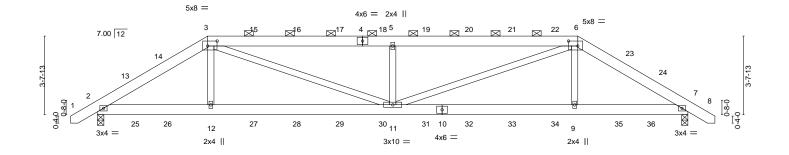
9/15/2021

1 of 1

Gilbert, Eric

September 15, 2021





 	5-1-6 5-1-6		<u>13-8-8</u> 8-7-2		22-3-10 8-7-2		27-5-0	
Plate Offsets (X,Y)		4,0-2-12]	8-7-2		8-7-2		5-1-6	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.15 1.15 NO PI2014	CSI. TC 0.19 BC 0.17 WB 0.16 Matrix-S	DEFL. in Vert(LL) -0.05 Vert(CT) -0.10 Horz(CT) 0.02 Wind(LL) 0.07	11 >999 11 >999 7 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 354 lb	GRIP 244/190 FT = 20%
Max H	No.1 No.1			BRACING- TOP CHORD BOT CHORD	2-0-0 oc purlins	(6-0-0 max.):	ectly applied or 6-0-0 o	oc purlins, except
FORCES. (lb) - Max. TOP CHORD 2-3=- BOT CHORD 2-12= WEBS 3-12= NOTES- 1) 2-ply truss to be conn Top chords connected Bottom chords connected as	2061/628, 3-5=-2793/10 564/1694, 11-12=-559, -0/376, 3-11=-525/1238, nected together with 100 ed as follows: 2x6 - 2 rov ected as follows: 2x6 - 2 follows: 2x4 - 1 row at 0	orces 250 (lb) or 103, 5-6=-2793/1 (1703, 9-11=-53 5-11=-642/479, d (0.131"x3") na ws staggered at rows staggered -9-0 oc.	1/1705, 7-9=-536/1695 , 6-11=-499/1208, 6-9=0/3 ils as follows: 0-9-0 oc.	386	ASE(S) section.	Plv to		
 ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; W 4) Lumber DOL=1.60 p 5) Provide adequate dr 6) This truss has been 7) * This truss has been will fit between the b 8) Provide mechanical 2=345, 7=361. 	e been provided to distril loads have been consit ult=130mph Vasd=103m late grip DOL=1.60 ainage to prevent water designed for a 10.0 psf n designed for a live load ottom chord and any oth connection (by others) c	bute only loads i dered for this de nph; TCDL=6.0p ponding. bottom chord liv d of 30.0psf on t her members. f truss to bearin	noted as (F) or (B), unless	s otherwise indicated. Cat. II; Exp C; Enclosed any other live loads. as where a rectangle 3-0 inding 100 lb uplift at joir	l; MWFRS (envel 6-0 tall by 2-0-0 v nt(s) except (jt=lb)	ope); vide		
 Hanger(s) or other 1-8-12, 53 lb down 85 lb up at 9-2-12, 94 lb down and 85 up at 22-3-10, and 1-8-12, 22 lb down down at 13-2-12, 1 	connection device(s) sh and 61 lb up at 3-2-12, 94 lb down and 85 lb up lb up at 17-2-12, 94 lb d 53 lb down and 61 lb up at 3-2-12, 19 lb down at 9 lb down at 15-2-12, 1 own at 24-2-4, and 20 l	all be provided s 90 lb down and o at 11-2-12, 94 down and 85 lb o at 24-2-4, and t 5-2-12, 19 lb 9 lb down at 17	sufficient to support conce 88 lb up at 5-1-6, 94 lb c lb down and 85 lb up at up at 19-2-12, 94 lb dowr 57 lb down and 58 lb up 57 lb down and 58 lb up jown at 7-2-12, 19 lb dow '-2-12, 19 lb down at 19- 4 on bottom chord. The	entrated load(s) 57 lb dov lown and 85 lb up at 7-2 13-2-12, 94 lb down and n and 85 lb up at 21-2-1: at 25-8-4 on top chord, wn at 9-2-12, 19 lb down 2-12, 19 lb down at 21-2	wn and 58 lb up a 2-12, 94 lb down a 1 85 lb up at 15-2 2, 90 lb down and and 20 lb down a 1 at 11-2-12, 19 l 2-12, 19 lb down a	and ?-12, 2-12, 1 88 lb at b at	Septembe	15,2021
LOAD WARNEN Vestant Design valid for use only a truss system. Before u building design. Bracing is always required for sta fabrication, storage, deli	with MiTek® connectors. This se, the building designer must indicated is to prevent bucklin ability and to prevent collapse v very, erection and bracing of tr	design is based onl verify the applicabili g of individual truss with possible persona usses and truss syst	NCLUDED MITEK REFERENCE y upon parameters shown, and i ty of design parameters and proy web and/or chord members only al injury and property damage. I ems, see ANSI/THI C ay, Suite 203 Waldorf, MD 2060	s for an individual building com perly incorporate this design int . Additional temporary and per for general guidance regarding uality Criteria, DSB-89 and B	ponent, not o the overall manent bracing the	onent	818 Soundside R Edenton, NC 275	

Job	Truss	Truss Type	Qty	Ply	Lot 3 Walker Rd.
					E16171316
J0821-5011	A1-GR	HIP GIRDER	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8.4	30 s Aug 1	6 2021 MiTek Industries, Inc. Wed Sep 15 08:11:51 2021 Page 2

ID:d6E6lizSYcm5g_canilVuiz8loe-bqOO?qJBT08MC1WMOO9nBzJg1HW_hnUi?oX8qsydRpc

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

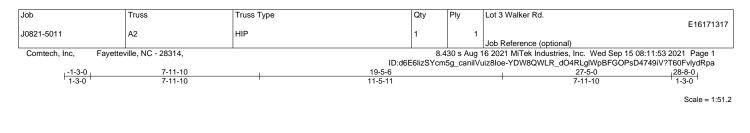
Uniform Loads (plf) Vert: 1-3=-60, 3-6=-60, 6-8=-60, 2-7=-20

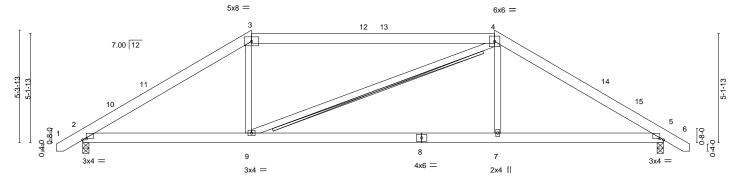
Concentrated Loads (lb)

Vert: 3=-13(F) 6=-13(F) 12=-10(F) 9=-10(F) 13=-17(F) 14=-13(F) 15=-13(F) 16=-13(F) 17=-13(F) 18=-13(F) 19=-13(F) 20=-13(F) 20=-13(F) 21=-13(F) 22=-13(F) 23=-13(F) 24=-17(F) 25=-13(F) 26=-10(F) 27=-10(F) 28=-10(F) 29=-10(F) 31=-10(F) 31=-10(F) 32=-10(F) 32=-10(F) 35=-10(F) 35=-10(F) 36=-13(F) 36=-13(F) 35=-10(F) 36=-13(F) 36=

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling of individual truss expletens, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	7-11-10	1	19-5-6	1	27-5-0	1
	7-11-10		11-5-11		7-11-10	
Plate Offsets (X,Y)	[2:0-2-0,0-0-11], [5:0-2-0,0-0-11]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.78 BC 0.39 WB 0.12 Matrix-S	DEFL. ir Vert(LL) -0.10 Vert(CT) -0.22 Horz(CT) 0.03 Wind(LL) 0.03	7-9 >999 360 7-9 >999 240 5 n/a n/a	PLATES MT20 Weight: 173 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP	No.1		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing except 2-0-0 oc purlins (4-2-6 max Rigid ceiling directly applier T-Brace: Fasten (2X) T and I braces (0.131"x3") nails, 6in o.c.,w Brace must cover 90% of w	.): 3-4. d or 10-0-0 oc bracing. 2x4 SPF No.2 - 4-9 s to narrow edge of web v ith 3in minimum end dist	with 10d
Max U Max G	 a) 2=0-3-8, 5=0-3-8 b) 2=125(LC 11) b) 11 2=-47(LC 12), 5=-47(LC 13) c) 1159(LC 1), 5=1159(LC 1) Comp./Max. Ten All forces 250 (lb) oi 	less excent when shown			ee longui.	
TOP CHORD 2-3=- BOT CHORD 2-9=-	1720/387, 3-4=-1371/421, 4-5=-1720/3 197/1378, 7-9=-205/1371, 5-7=-202/13 0/440, 4-7=0/440					
2) Wind: ASCE 7-10; V and C-C Exterior(2) Exterior(2) 19-5-6 to DOL=1.60 plate grip	loads have been considered for this de ult=130mph Vasd=103mph; TCDL=6.0 -1-1-0 to 3-3-13, Interior(1) 3-3-13 to 7- 25-8-0, Interior(1) 25-8-0 to 28-6-0 zon DOL=1.60 ainage to prevent water ponding.	osf; BCDL=6.0psf; h=15ft; 11-10, Exterior(2) 7-11-10	to 14-2-5, Interior(1) 14	-2-5 to 19-5-6,	A STAR	Roza

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

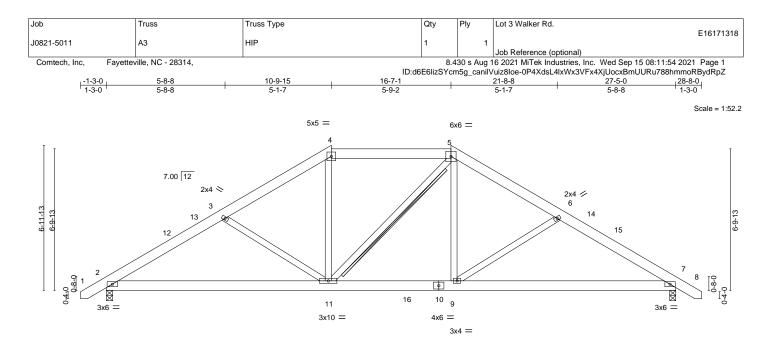
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





<u> </u>					16-7-1 5-9-2				27-5-0 10-9-15				
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP		
TCLL 20.0	Plate Grip DOL	1.15	тс	0.15	Vert(LL)	-0.09	7-9	>999	360	MT20	244/190		
TCDL 10.0	Lumber DOL	1.15	BC	0.38	Vert(CT)	-0.19	7-9	>999	240				
3CLL 0.0 *	Rep Stress Incr	YES	WB	0.22	Horz(CT)	0.03	7	n/a	n/a				
BCDL 10.0	Code IRC2015/T	PI2014	Matrix	x-S	Wind(LL)	0.02	9	>999	240	Weight: 191 lb	FT = 20%		

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 WFBS

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 5-11-1 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.

Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SPF No.2 - 5-11 T-Brace: Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. (size) 2=0-3-8, 7=0-3-8 Max Horz 2=165(LC 11) Max Uplift 2=-65(LC 12), 7=-65(LC 13) Max Grav 2=1159(LC 1), 7=1159(LC 1)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1660/451, 3-4=-1387/387, 4-5=-1123/386, 5-6=-1385/387, 6-7=-1660/451
- BOT CHORD 2-11=-287/1362, 9-11=-110/1122, 7-9=-292/1360
- WEBS 3-11=-361/217, 4-11=-18/397, 5-9=-17/433, 6-9=-362/217

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-0 to 3-3-13, Interior(1) 3-3-13 to 10-9-15, Exterior(2) 10-9-15 to 22-9-12, Interior(1) 22-9-12 to 28-6-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

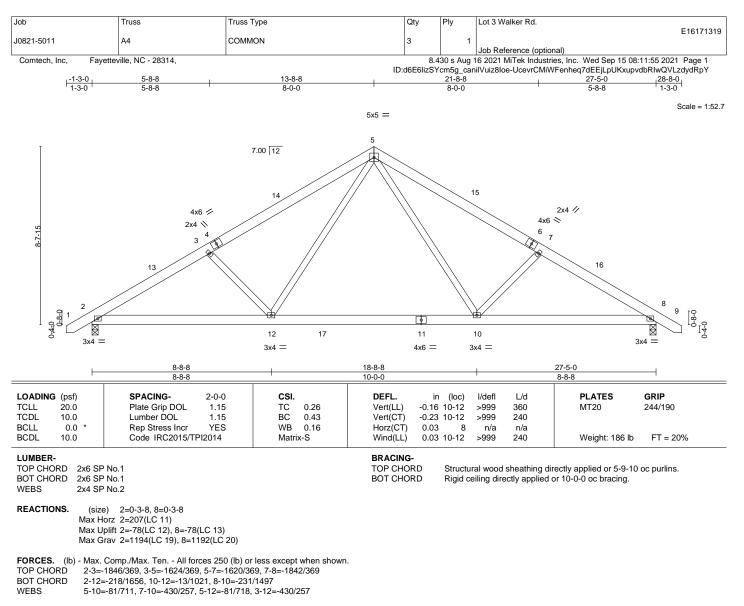
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.
 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-0 to 3-3-13, Interior(1) 3-3-13 to 13-8-8, Exterior(2) 13-8-8 to 18-1-5, Interior(1) 18-1-5 to 28-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

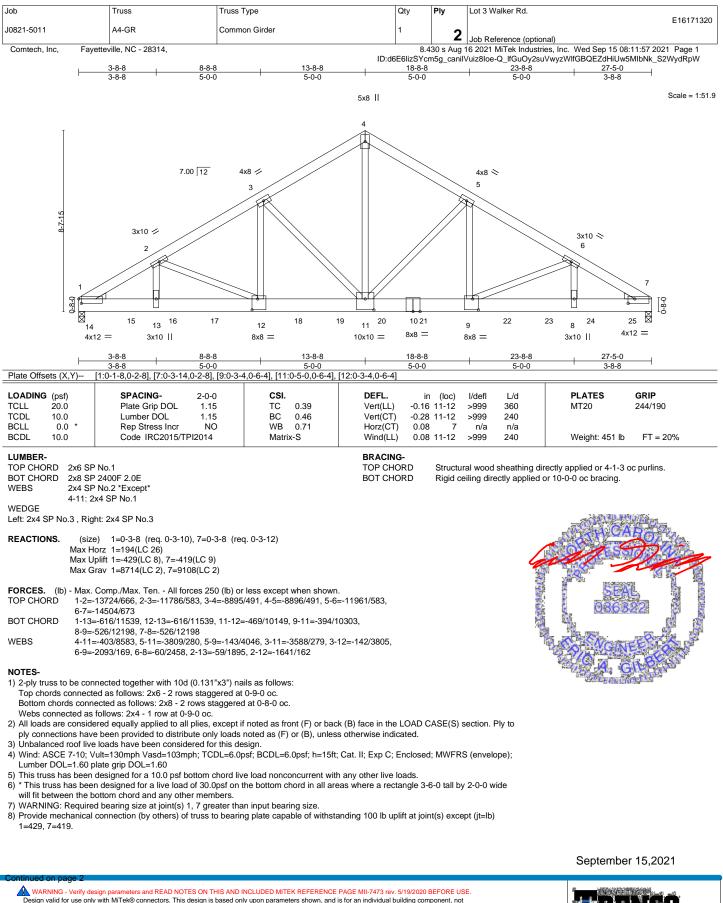
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



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ſ	Job	Truss	Truss Type	Qty	Ply	Lot 3 Walker Rd.
	10004 5044					E16171320
	J0821-5011	A4-GR	Common Girder	1	2	Job Reference (optional)
					_	Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8.4	30 s Aug 1	6 2021 MiTek Industries, Inc. Wed Sep 15 08:11:57 2021 Page 2
			ID:d6	E6lizSYcr	n5g_canil\	/uiz8loe-Q_lfGuOy2suVwyzWlfGBQEZdHiUw5MlbNk_S2WydRpW

NOTES-

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 960 lb down and 54 lb up at 0-5-12, 953 lb down and 61 lb up at 2-5-12, 953 lb down and 61 lb up at 4-5-12, 1192 lb down and 60 lb up at 6-5-12, 1192 lb down and 60 lb up at 8-5-12, 1192 lb down and 60 lb up at 10-5-12, 1192 lb down and 58 lb up at 20-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

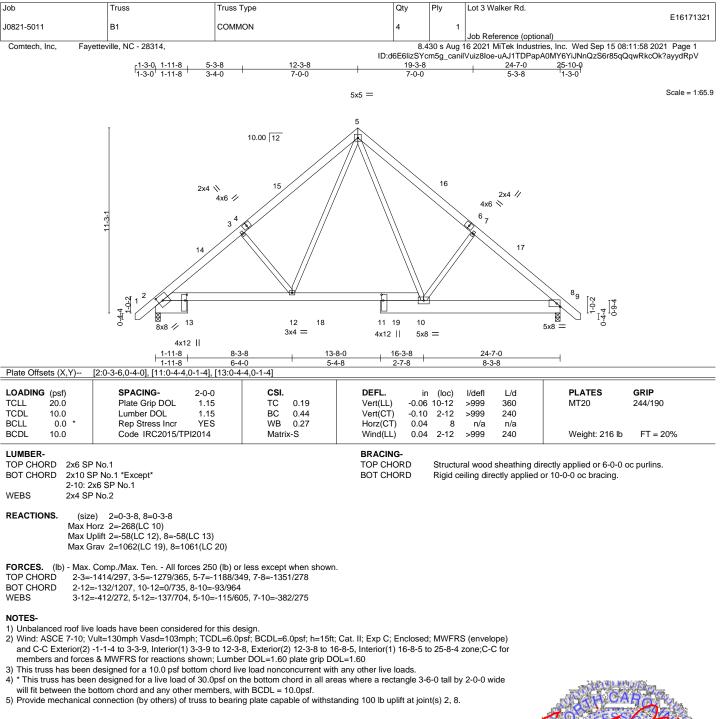
Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 9=-953(B) 12=-953(B) 14=-960(B) 15=-953(B) 16=-953(B) 17=-953(B) 18=-953(B) 19=-953(B) 20=-953(B) 21=-953(B) 22=-953(B) 23=-953(B) 24=-953(B) 25=-955(B) 25=-95(B) 25=-95(B) 25=-95(B) 25=-95(B) 25=-95(B) 25=-95(B) 25=-95(B) 25=-95(B) 25=-95(B) 25=-

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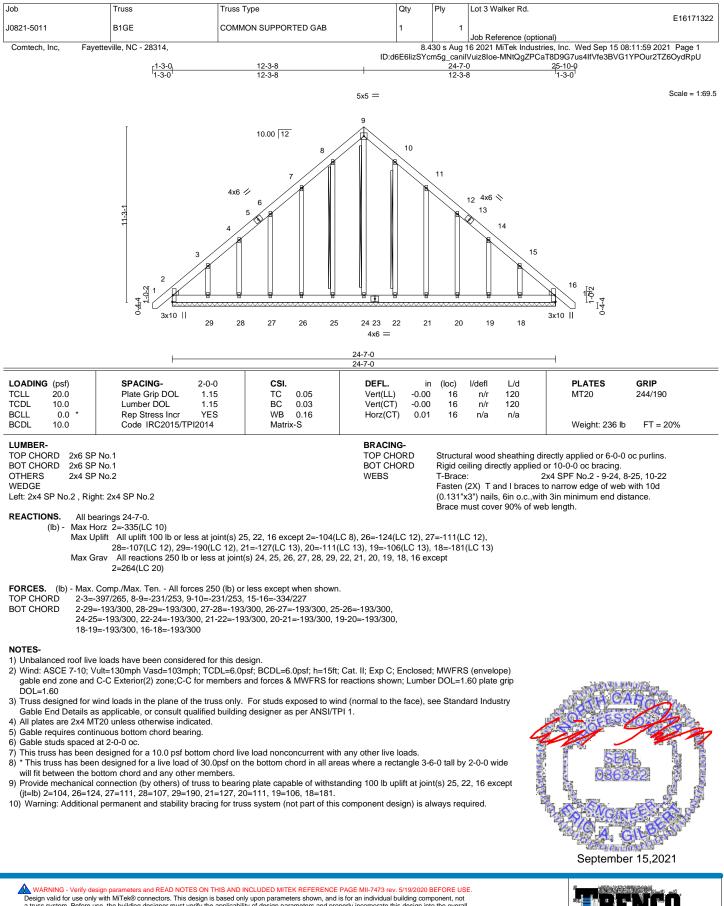






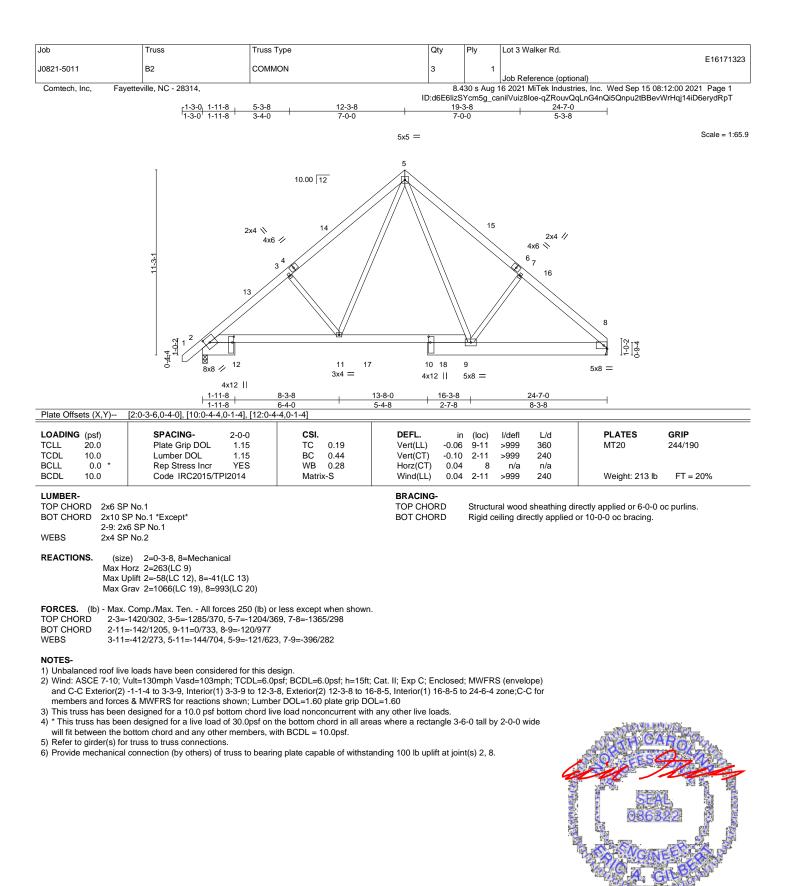
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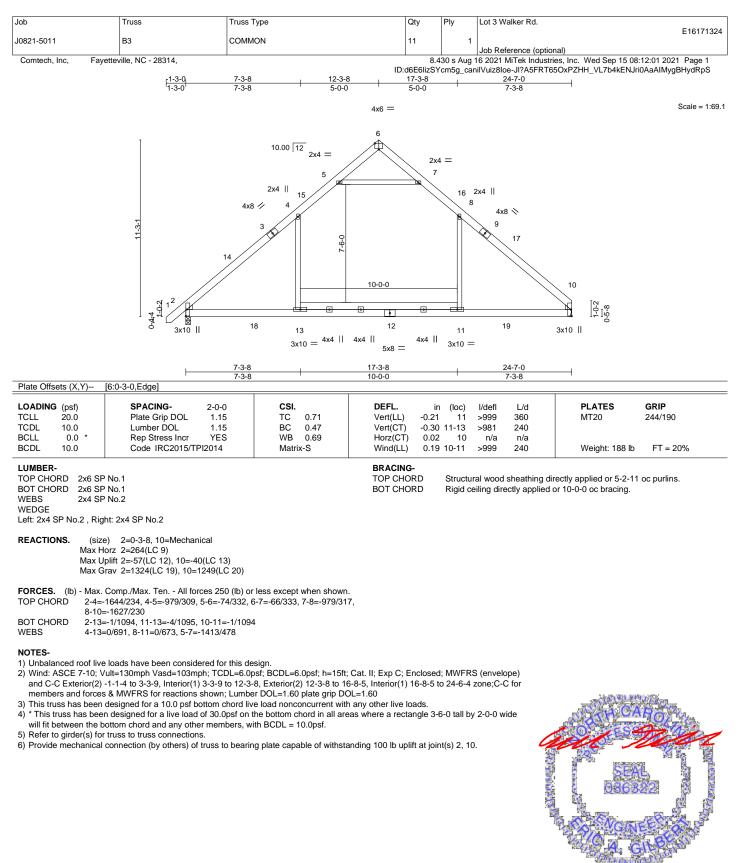
BIS Soundside Road Edenton, NC 27932



September 15,2021

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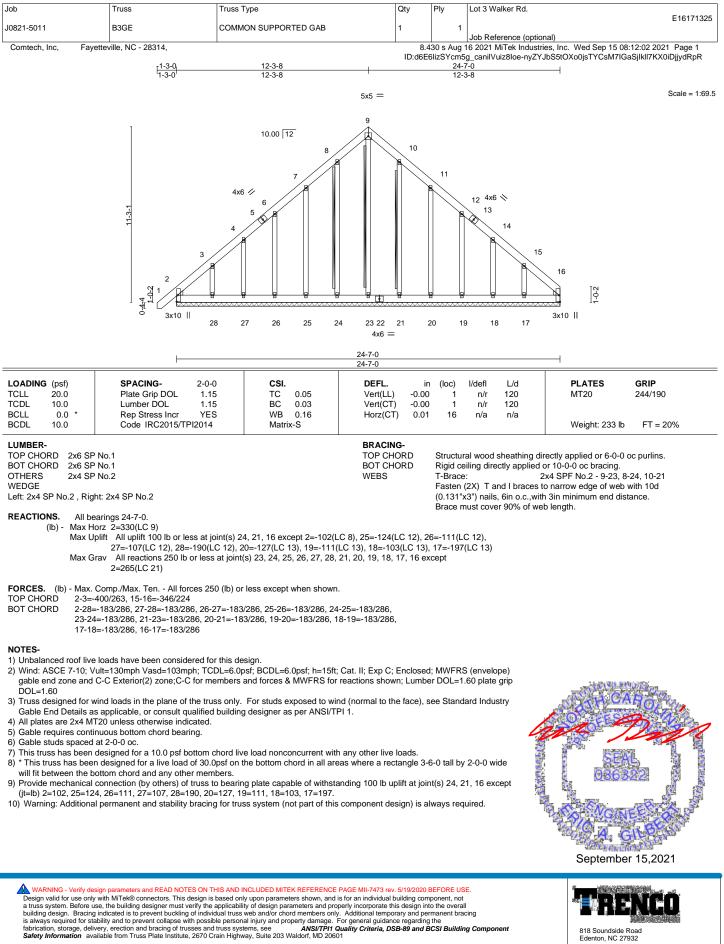




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818 Soundside Road Edenton, NC 27932

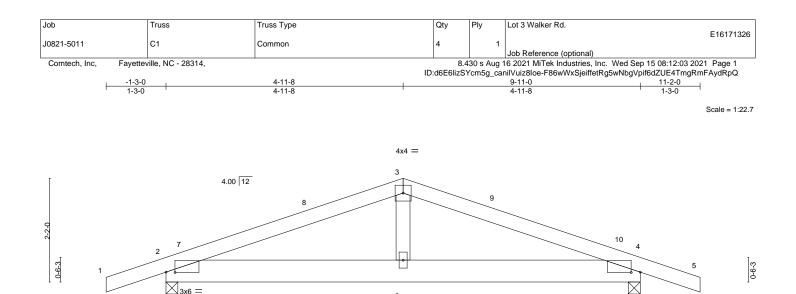


Plate Offsets (X,Y)	[2:0-2-4,0-0-1], [4:0-2-4,0-0-1]	4-11-8 4-11-8	ŀ	9-11-0 4-11-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.21 BC 0.12 WB 0.05 Matrix-S	DEFL. in Vert(LL) -0.01 Vert(CT) -0.02 Horz(CT) 0.00 Wind(LL) 0.02	(loc) l/defl L/d 6 >999 360 6 >999 240 4 n/a n/a 4-6 >999 240	PLATES GRIP MT20 244/190 Weight: 45 lb FT = 20%
	SP No.1 SP No.1			Structural wood sheathing dir Rigid ceiling directly applied c	ectly applied or 6-0-0 oc purlins. or 9-10-1 oc bracing.

6 2x4 ||

BOT CHORD 2x6 SP No.1 WFBS 2x4 SP No 2 REACTIONS. (size) 2=0-3-0, 4=0-3-0

Max Horz 2=-25(LC 13) Max Uplift 2=-191(LC 8), 4=-191(LC 9) Max Grav 2=469(LC 1), 4=469(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-638/730, 3-4=-638/730 BOT CHORD 2-6=-601/548, 4-6=-601/548

WEBS 3-6=-303/229

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 4-11-8, Exterior(2) 4-11-8 to 9-4-5, Interior(1) 9-4-5 to 11-2-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

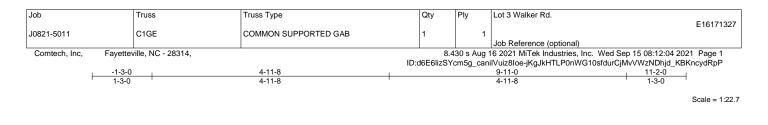
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=191, 4=191.

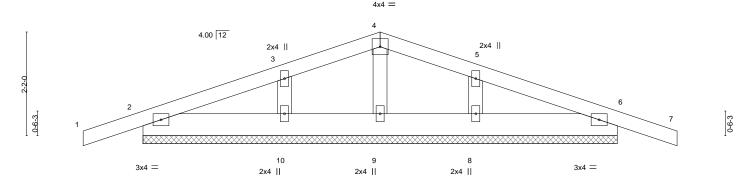
3x6 =

🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLUDED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



September 15,2021





			9-11-0 9-11-0	
OADING (psf) CLL 20.0 CDL 10.0 CLL 0.0 * CDL CDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.07 BC 0.02 WB 0.03 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 7 n/r 120 Vert(CT) -0.00 7 n/r 120 Horz(CT) 0.00 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 47 lb FT = 20%
UMBER-	No.1		BRACING- TOP CHORD Structural wood sheathing d	irectly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 9-11-0.

(lb) - Max Horz 2=-42(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 9, 10, 8 except 2=-103(LC 8), 6=-107(LC 9) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable End Details as applicable, or consult qualified building designer as per AN
 Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 10, 8 except (jt=lb) 2=103, 6=107.
- 9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.



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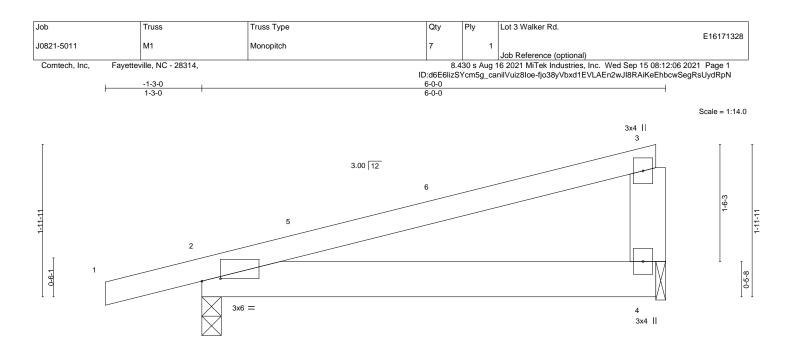


Plate Offsets (X,Y) [2	2:0-2-14,0-0-6]				6-0- 6-0-						
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.41	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	-0.03	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL 10.0	Code IRC2015/TF	12014	Matrix	k-P	Wind(LL)	0.03	2-4	>999	240	Weight: 28 lb	FT = 20%
LUMBER-					BRACING-						
TOP CHORD 2x4 SP 1 BOT CHORD 2x6 SP 1					TOP CHOR			ral wood end verti	0	rectly applied or 6-0-0	oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=60(LC 8) Max Uplift 2=-136(LC 8), 4=-88(LC 8) Max Grav 2=318(LC 1), 4=216(LC 1)

2x6 SP No 1

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

WFBS

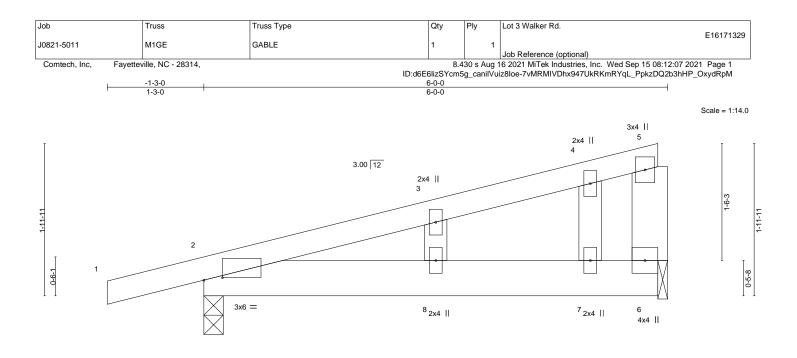
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=136.



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			6-0-0	
Plate Offsets (X,Y)	[2:0-2-14,0-0-6]			1
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
FCLL 20.0	Plate Grip DOL 1.15	TC 0.13	Vert(LL) 0.03 8 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.14	Vert(CT) -0.02 8 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02	Horz(CT) -0.00 6 n/a n/a	
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 30 lb FT = 20%
UMBER-			BRACING-	
FOP CHORD 2x4 SP	No.1		TOP CHORD Structural wood sheathing	g directly applied or 6-0-0 oc purlins,
BOT CHORD 2x6 SP	No.1		except end verticals.	
VEBS 2x6 SP	No.1		BOT CHORD Rigid ceiling directly appli	ed or 10-0-0 oc bracing.
OTHERS 2x4 SP	No.2			

6-0-0

REACTIONS. (size) 2=0-3-0, 6=0-1-8 Max Horz 2=85(LC 8) Max Uplift 2=-195(LC 8), 6=-129(LC 8) Max Grav 2=318(LC 1), 6=216(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

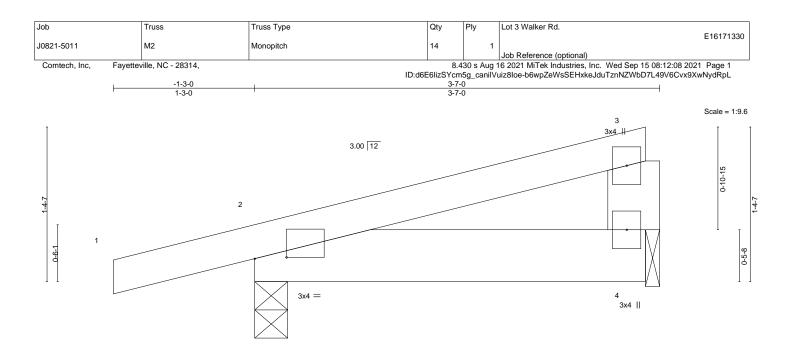
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=195. 6=129.



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			⊢				3-7-0					4
Plate Offse	ets (X,Y)	[2:0-3-6,0-0-2]				1					I	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	-0.00	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	k-P	Wind(LL)	0.00	2-4	>999	240	Weight: 17 lb	FT = 20%
LUMBER-						BRACING-					1	

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1

WFBS 2x6 SP No 1

TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins, except end verticals. BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=0-1-8, 2=0-3-8 Max Horz 2=57(LC 8) Max Uplift 4=-63(LC 8), 2=-152(LC 8)

Max Grav 4=110(LC 1), 2=230(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

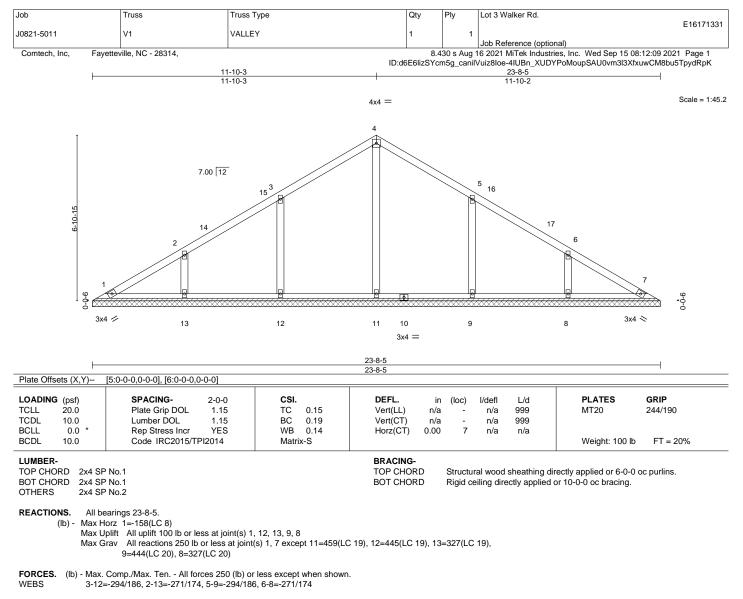
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=152.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-6-12 to 4-11-9, Interior(1) 4-11-9 to 11-10-3, Exterior(2) 11-10-3 to 16-2-15, Interior(1) 16-2-15 to 23-1-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

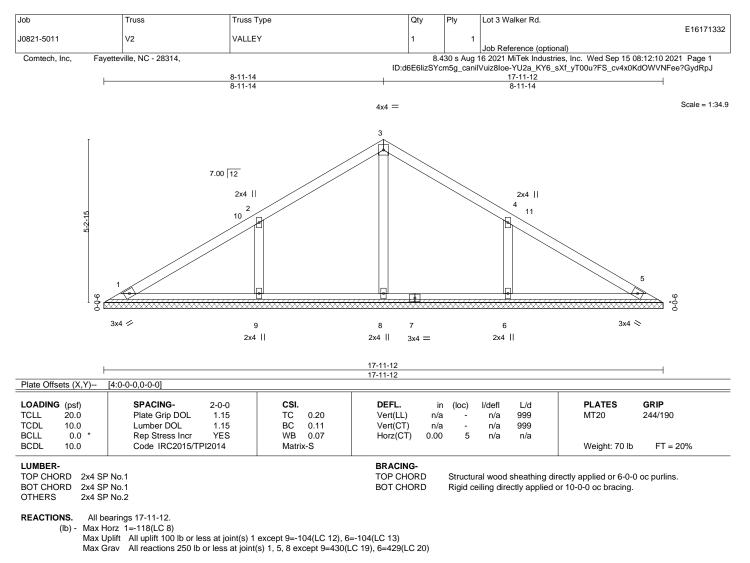
6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 12, 13, 9, 8.



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- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- WEBS 2-9=-342/208, 4-6=-342/208

NOTES-

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-6-12 to 4-11-14, Interior(1) 4-11-14 to 8-11-14, Exterior(2) 8-11-14 to 13-4-11, Interior(1) 13-4-11 to 17-5-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

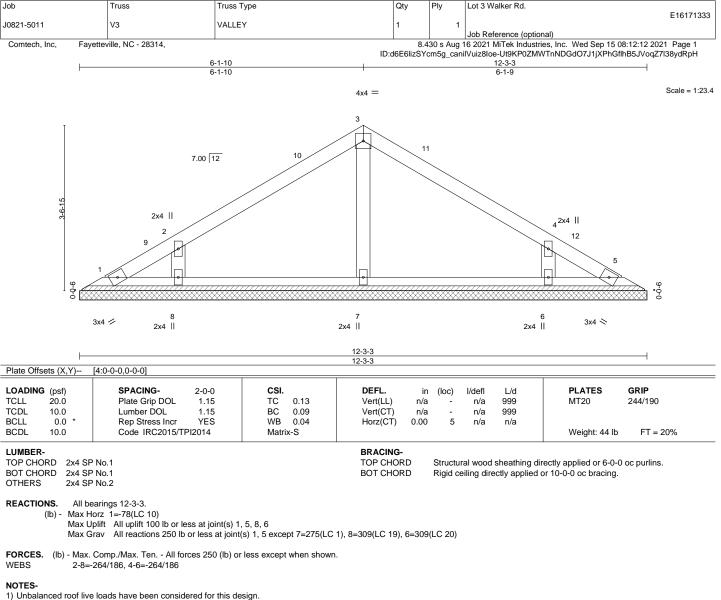
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=104, 6=104.



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¹⁾ Unbalanced roof live loads have been considered for this design.



Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-6-12 to 4-11-9, Interior(1) 4-11-9 to 6-1-10, Exterior(2) 6-1-10 to 10-6-6, Interior(1) 10-6-6 to 11-8-7 zone; C-C for enveloped and C-C Exterior(2) 0-6-12 to 4-11-9, Interior(1) 4-11-9 to 6-1-10, Exterior(2) 6-1-10 to 10-6-6, Interior(1) 10-6-6 to 11-8-7 zone; C-C for enveloped and C-C Exterior(2) 0-6-12 to 4-11-9, Interior(1) 4-11-9 to 6-1-10, Exterior(2) 6-1-10 to 10-6-6, Interior(1) 10-6-6 to 11-8-7 zone; C-C for enveloped and C-C Exterior(2) 0-6-12 to 4-11-9.

for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

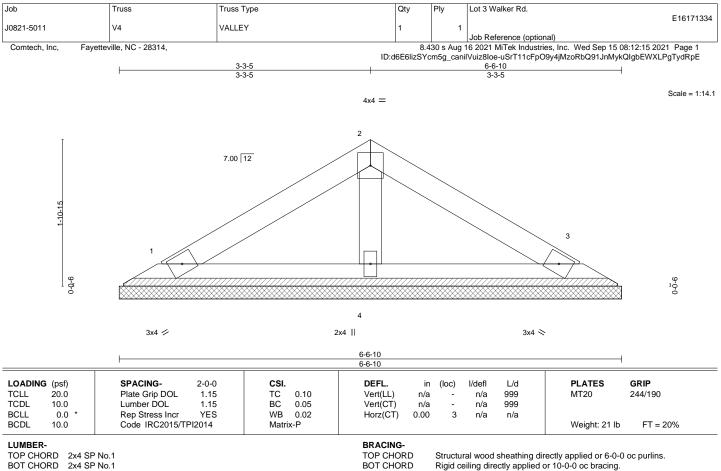
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.



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818 Soundside Road Edenton, NC 27932



BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=6-6-10, 3=6-6-10, 4=6-6-10

Max Horz 1=38(LC 9)

Max Uplift 1=-18(LC 12), 3=-22(LC 13)

Max Grav 1=114(LC 1), 3=114(LC 1), 4=206(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

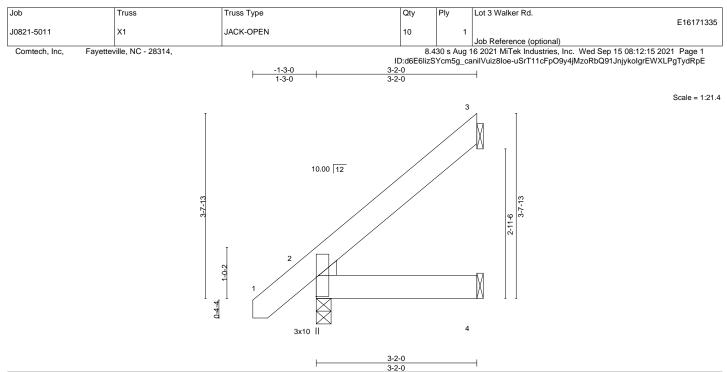
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

- and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plate Grip DOL	1.15	TC	0.07	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL 10.0		Lumber DOL	1.15	BC	0.03	Vert(CT)	-0.00	2-4	>999	240		
BCLL 0.0	*	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL 10.0		Code IRC2015/TP	12014	Matri	k-P	Wind(LL)	0.00	2	****	240	Weight: 22 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEDGE Left: 2x4 SP No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-2-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=111(LC 12) Max Uplift 3=-78(LC 12)

Max Grav 3=93(LC 19), 2=209(LC 1), 4=59(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

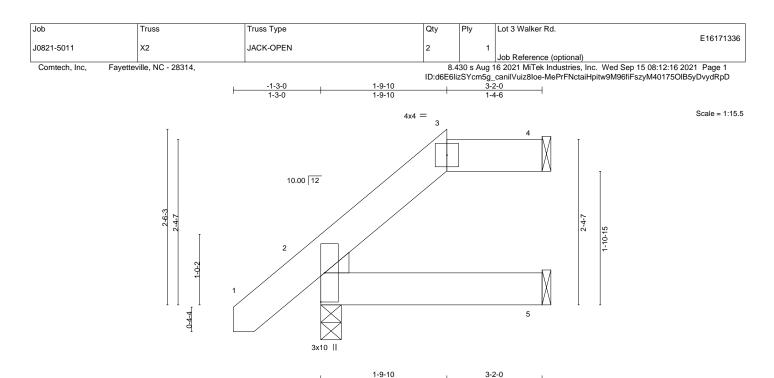
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.04	Vert(LL) -0.	00 2	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.03		00 2-5	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.	00 4	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) -0.	00 2-5	>999	240	Weight: 21 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEDGE Left: 2x4 SP No 2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 3-2-0 oc purlins, except 2-0-0 oc purlins: 3-4. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=74(LC 12) Max Uplift 4=-30(LC 9), 2=-11(LC 12)

Max Grav 4=73(LC 1), 2=209(LC 1), 5=54(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- a) Provide adequate drainage to prevent water ponding.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

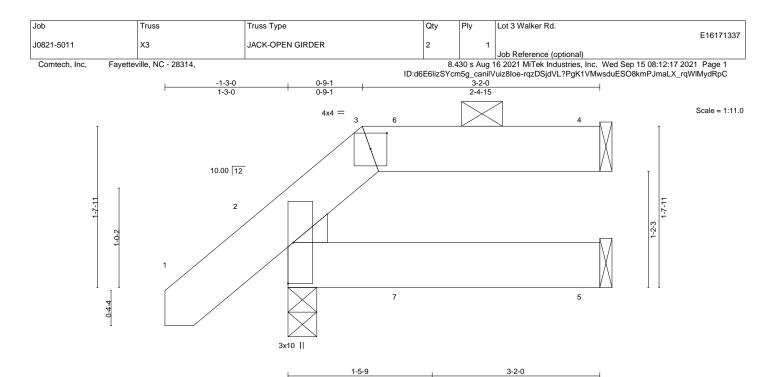
6) Refer to girder(s) for truss to truss connections.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	· •							
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.04	Vert(LL) -0.00	2	>999	360	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) -0.00	2-5	>999	240		
CLL 0.0 *	Rep Stress Incr NO	WB 0.00	Horz(CT) 0.00	4	n/a	n/a		
CDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.00	2	>999	240	Weight: 20 lb	FT = 20%

TOP CHORD

BOT CHORD

2-0-0 oc purlins: 3-4.

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEDGE

Left: 2x4 SP No.2

REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical Max Horz 2=50(LC 8) Max Uplift 4=-27(LC 5), 2=-19(LC 8) Max Grav 4=77(LC 20), 2=209(LC 1), 5=54(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 54 lb down and 21 lb up at 1-2-12 on top chord, and 4 lb down at 1-2-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 2-5=-20 AT A R A R A R

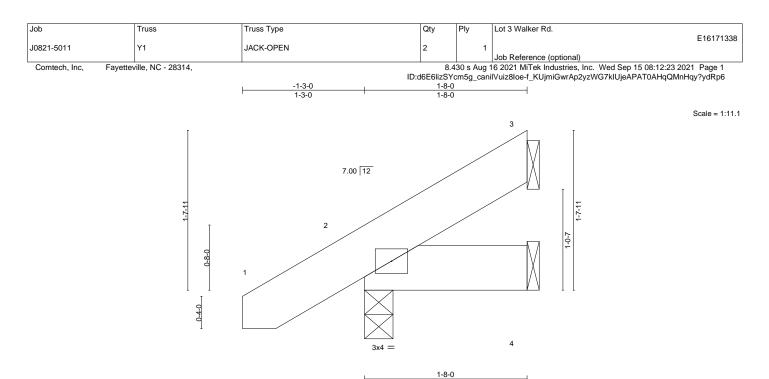
Structural wood sheathing directly applied or 3-2-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

September 15,2021



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						-	1-8-0					
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.03	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	-0.00	2	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	k-P	Wind(LL)	0.00	2	****	240	Weight: 12 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=49(LC 12)

Max Uplift 3=-23(LC 12), 2=-16(LC 12)

Max Grav 3=32(LC 19), 2=154(LC 1), 4=33(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.

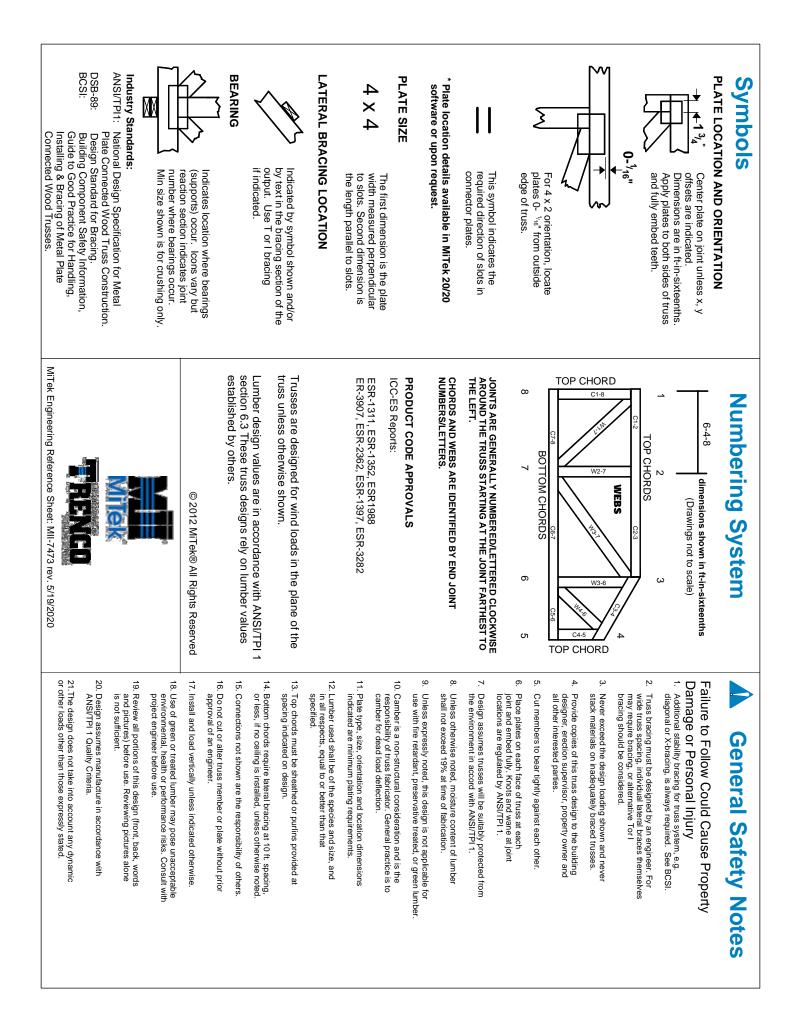


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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-8-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



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í í	<u>(1</u>																	RB1								
									J-22	J-22	J-22 0	J-22 [:		\langle	J-16	<u>J</u> -16	<u>1-16-</u>	J-16	J-16	J-16		3/4"				
	27' 6"	20' 5"		1.7 ³ /16"	1. 7 3/16"	1. 7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16" J-	1.7 3/16" J-	1. 7 3/16"	1. 7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1,0"	1.73/16" 16'1 3	1.7 3/16"	1. 7 3/16"	1. 7 3/16"	\
40' 8"	_	RB1			8' 7 3	/4"		┥	5' 1 ,BM2	0 1/2'	V V				1' 2 3	/4"		3 13 13				3' 8 1/2"	13' 3			
		7.1"	J-28	J-28	J-28	J-28)-28	J-28	8- 7- 7	8-f	8 <mark>-</mark> V V	8-f	J-38 J-38	J-38	J-18	J-18	J-18	J-18	J-18	J-18	J-38	J-38	J-38	J-38	J-38	J-38
	,	BBO	7					RB1			RB1		0	1.7 3/16"	1.7 3/16"	1. 7 3/16"	1. 7 3/16"	1. 7 3/16"	1. 7 3/16"	1. 7 3/16"	1. 0	ł				
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David Landry Marshall Naylor	03/18/22	2nd Floor I-Joist	724 Walker Road	Linden / Harnett	
DRAWN BY SALES REP.	DATE REV.	MODEL	ADDRESS	CITY / CO.	
J0821-5012	N/A	Westbrook	Lot 3 Walker Rd.	Ben Stout Real Estate	
-	SEAL DATE		JOB NAME		

19' 9"

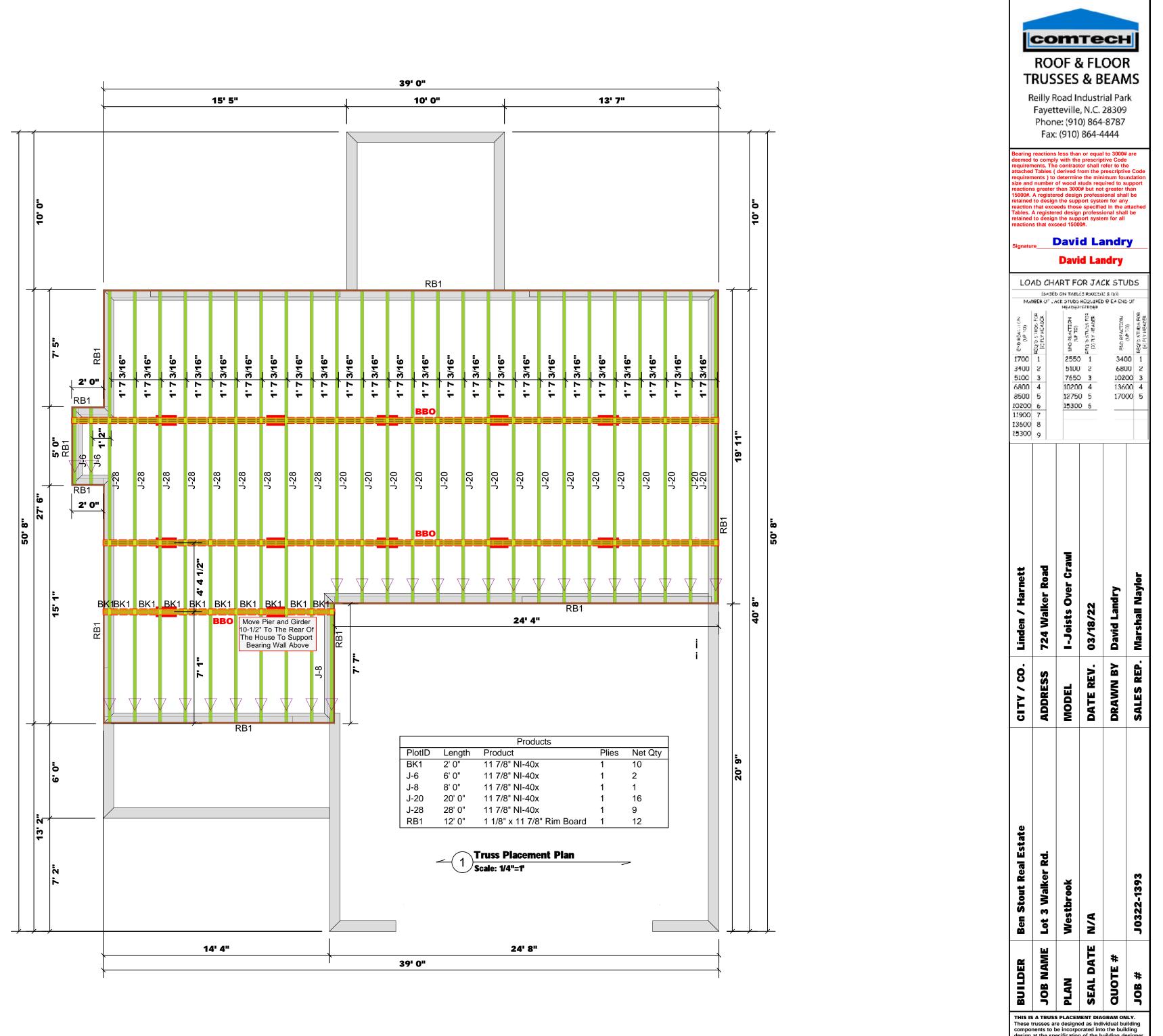
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17' 4"

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components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the root and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
 These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the root and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards