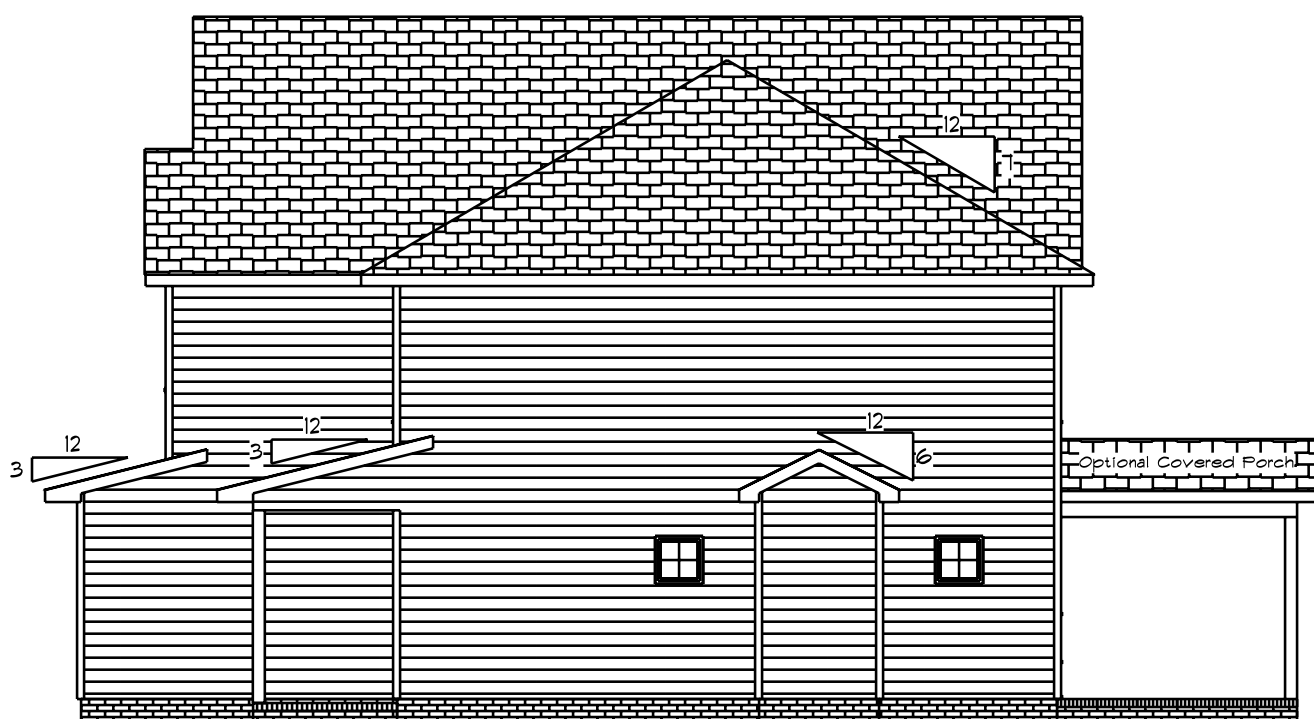
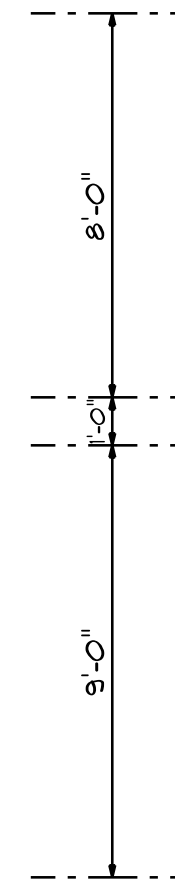




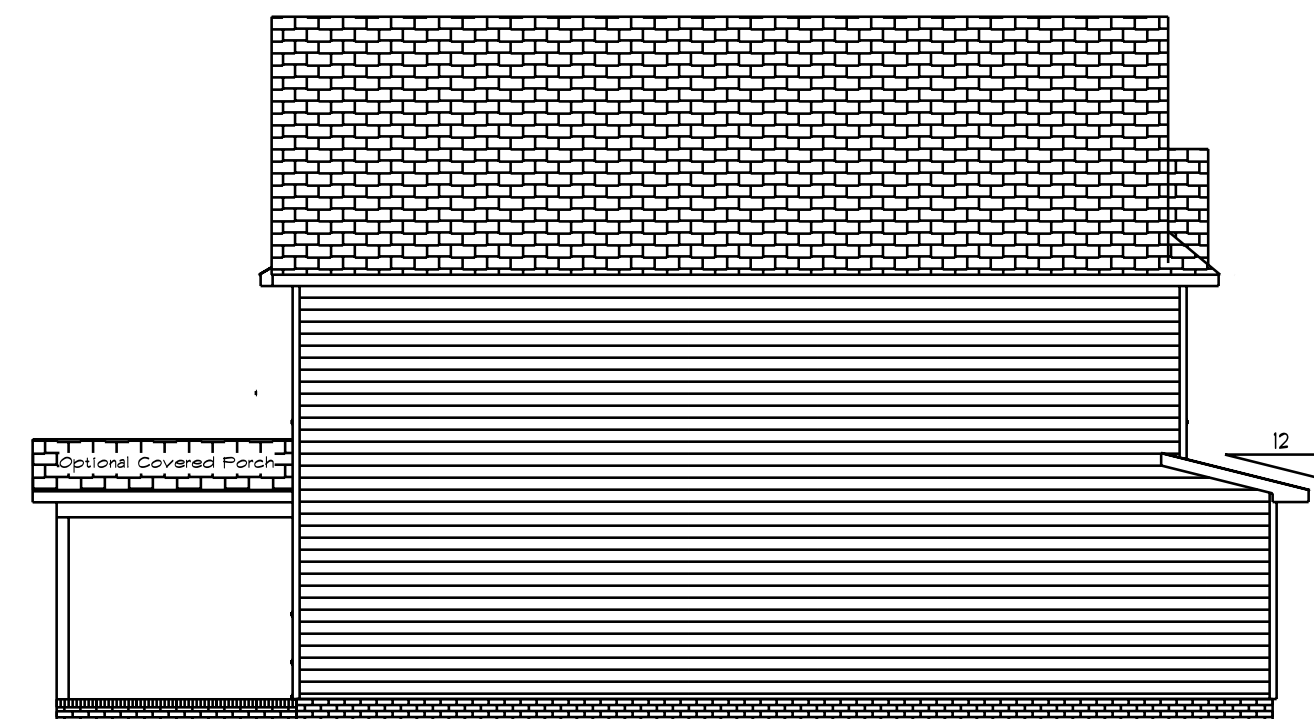
Front Elevation
Scale: 1/4" = 1'0"



Left Elevation
Scale: 1/8" = 1'0"



Rear Elevation
Scale: 1/8" = 1'0"



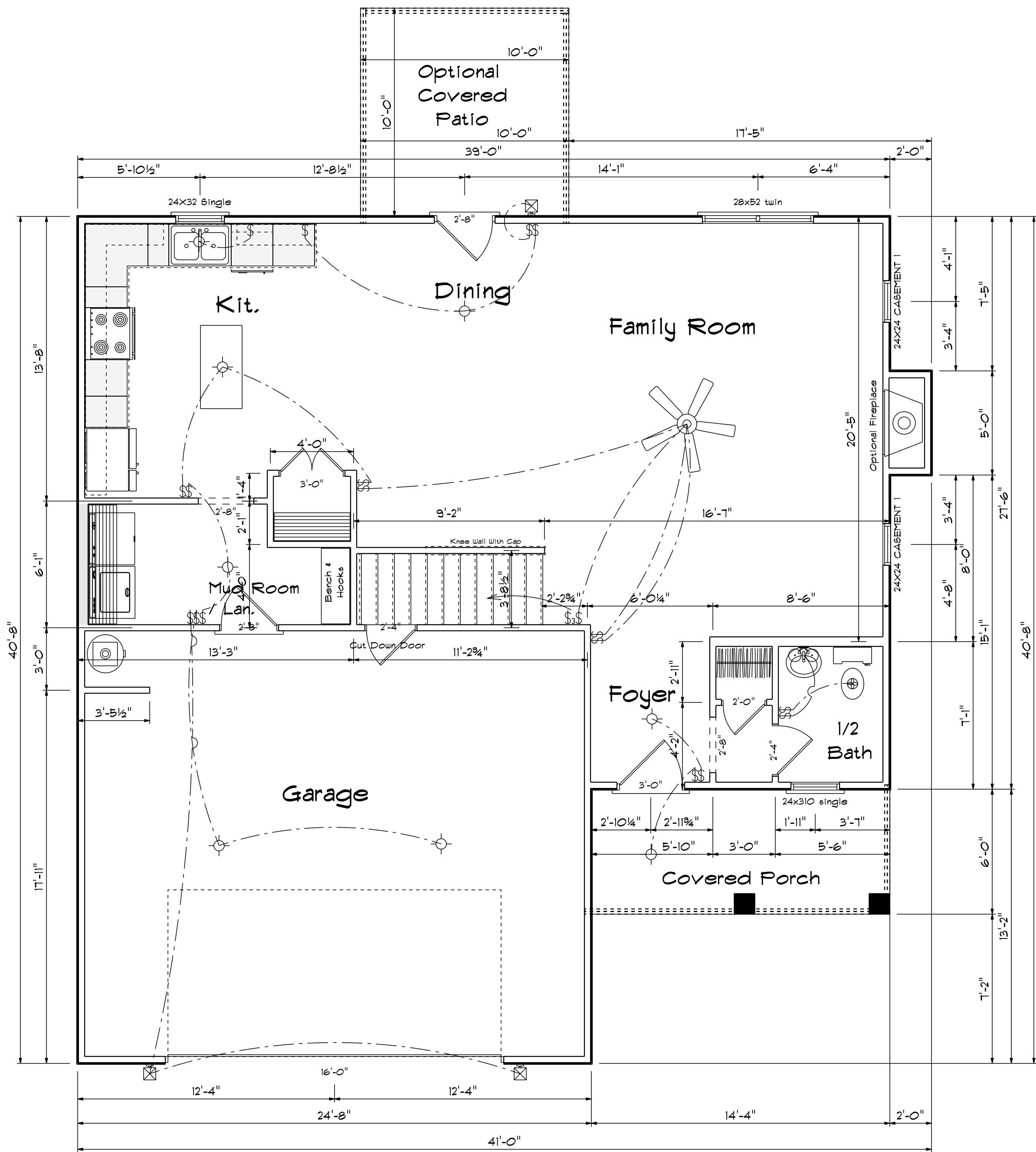
Right Elevation
Scale: 1/8" = 1'0"

DATE: 8/5/2021
REVISED
DRAWING#

EIV. A

SCALE: 1/4"
DRAWN BY
APPROVED

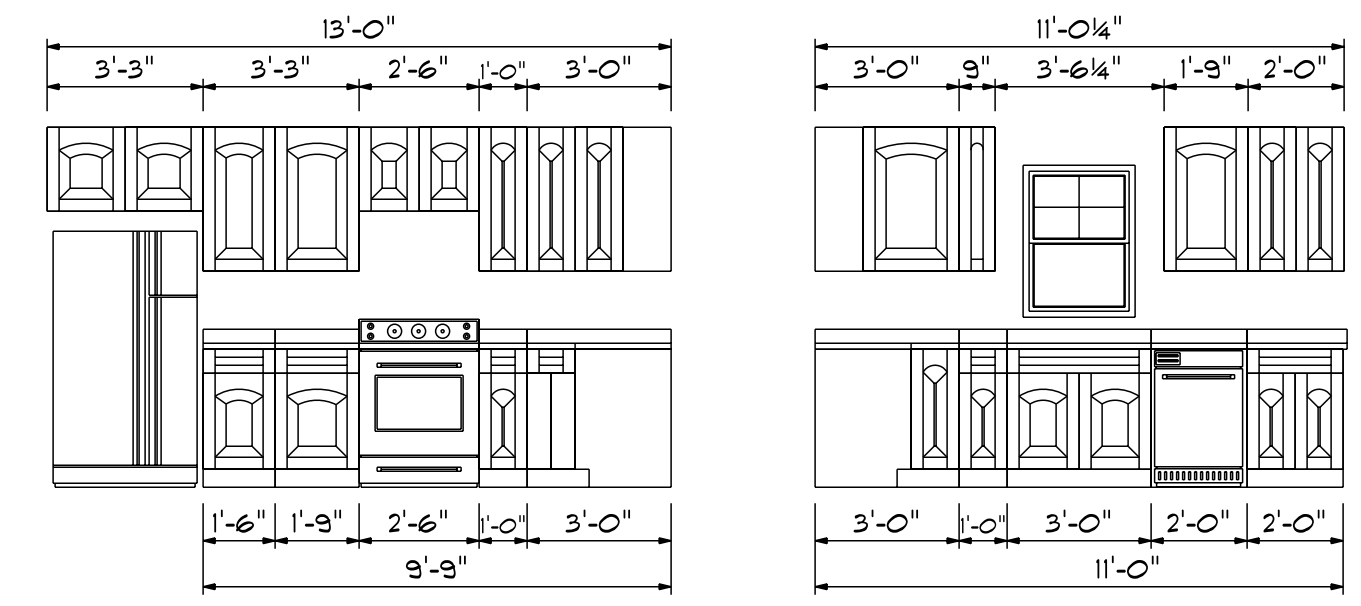
The Westbrook



First Floor Plan

Scale: 1/4" = 1'-0"

Kitchen Cabinets



FIRST FLOOR OPENING SCHEDULE			
PRODUCT CODE	SIZE	HINGE	COUNT
36X80 COLONIAL A 1	3'-0"	L	1
192X96 - 8 PANEL - 4 WINDOW	16'-0"	U	1
72X80 SLIDING FRENCH 2	6'-0"	NL	1
2-0 Door Unit	2'-0"	L	1
2-0 Door Unit	2'-0"	R	1
2-4 Door Unit	2'-4"	L	1
2-4 Door Unit	2'-4"	R	1
2-8 Door Unit	2'-8"	R	1
24X24 CASEMENT 1	2'-0" x 2'-0"	N	2
24X32 Single	2'-4" x 3'-2"	N	1
24x310 single	2'-4" x 3'-10"	N	1
28x52 twin	5'-4" x 5'-2"	NN	1

Areas

First Floor	892
Second Floor	1316
=====	
Total Heated	2208
Garage	512
Front Porch	85

DATE: 8/5/2021

REVISED

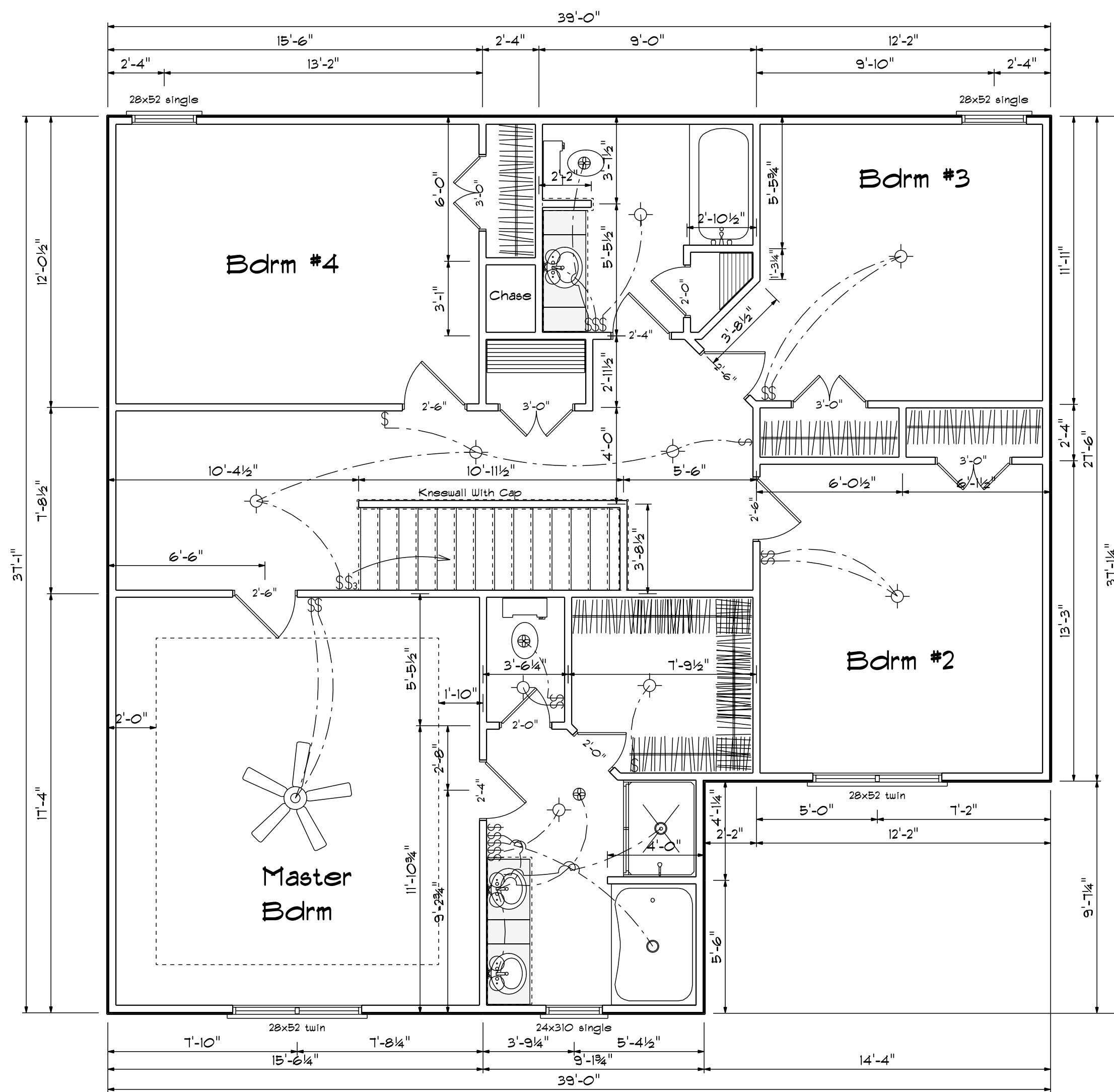
DRAWING#

SCALE: 1/4"

DRAWN BY

APPROVED

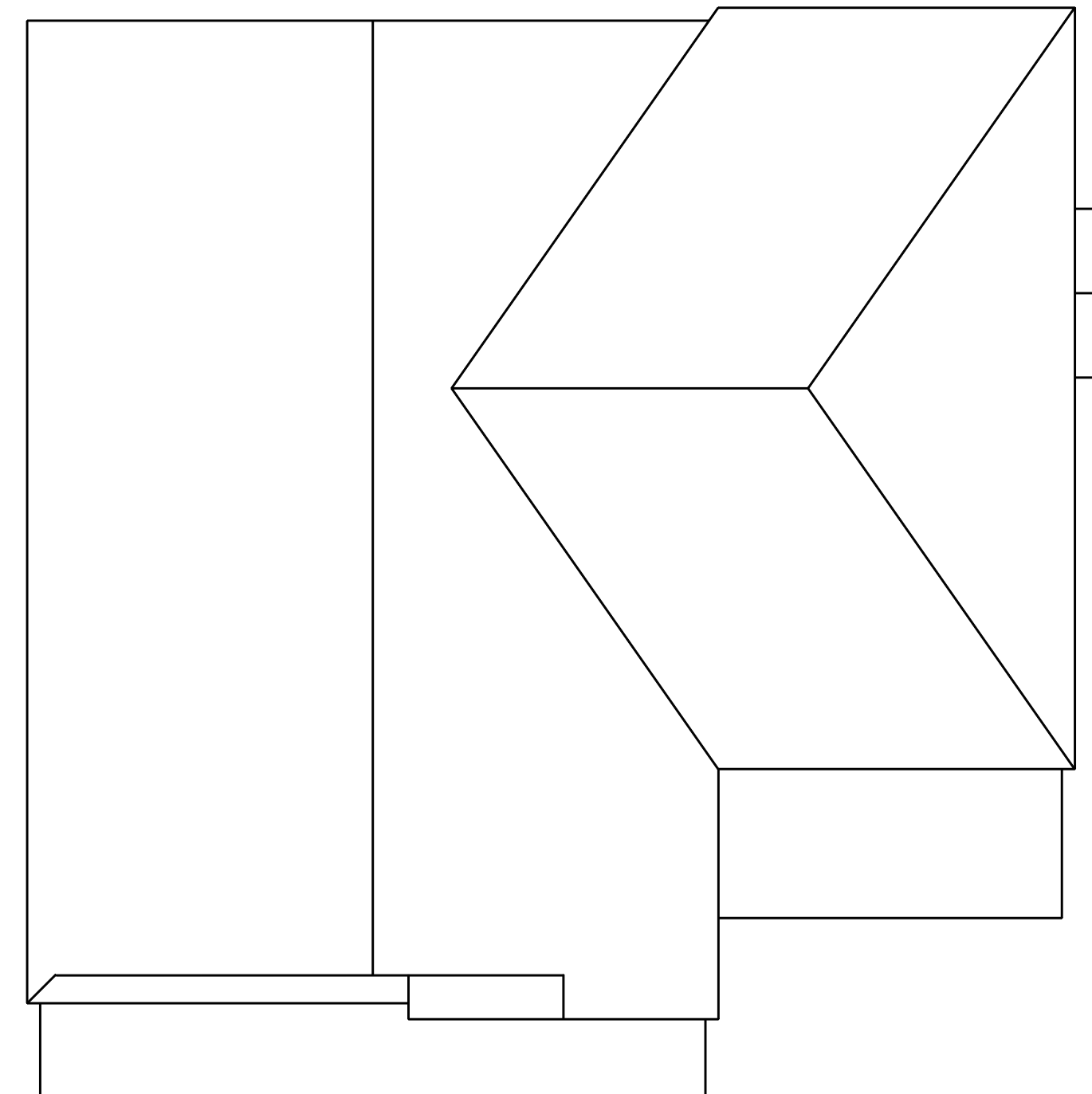
The Westbrook



Second Floor Plan

Scale: 1/4" = 1'-0"

SECOND FLOOR OPENING SCHEDULE			
PRODUCT CODE	SIZE	HINGE	COUNT
2-0 Door Unit	2'-0"	L	3
2-4 Door Unit	2'-4"	R	1
2-4 Door Unit	2'-4"	L	1
2-6 Door Unit	2'-6"	L	2
2-6 Door Unit	2'-6"	R	2
3-0 Doublehung Door Unit	3'-0"	LR	4
20x20 twin	4'-0" x 3'-2"	NA	1
24x310 single	2'-4" x 3'-10"	N	1
28x52 single	2'-8" x 5'-2"	N	2
28x52 twin	5'-4" x 5'-2"	NN	2



Roof Plan

DATE: 8/5/2021

REVISED

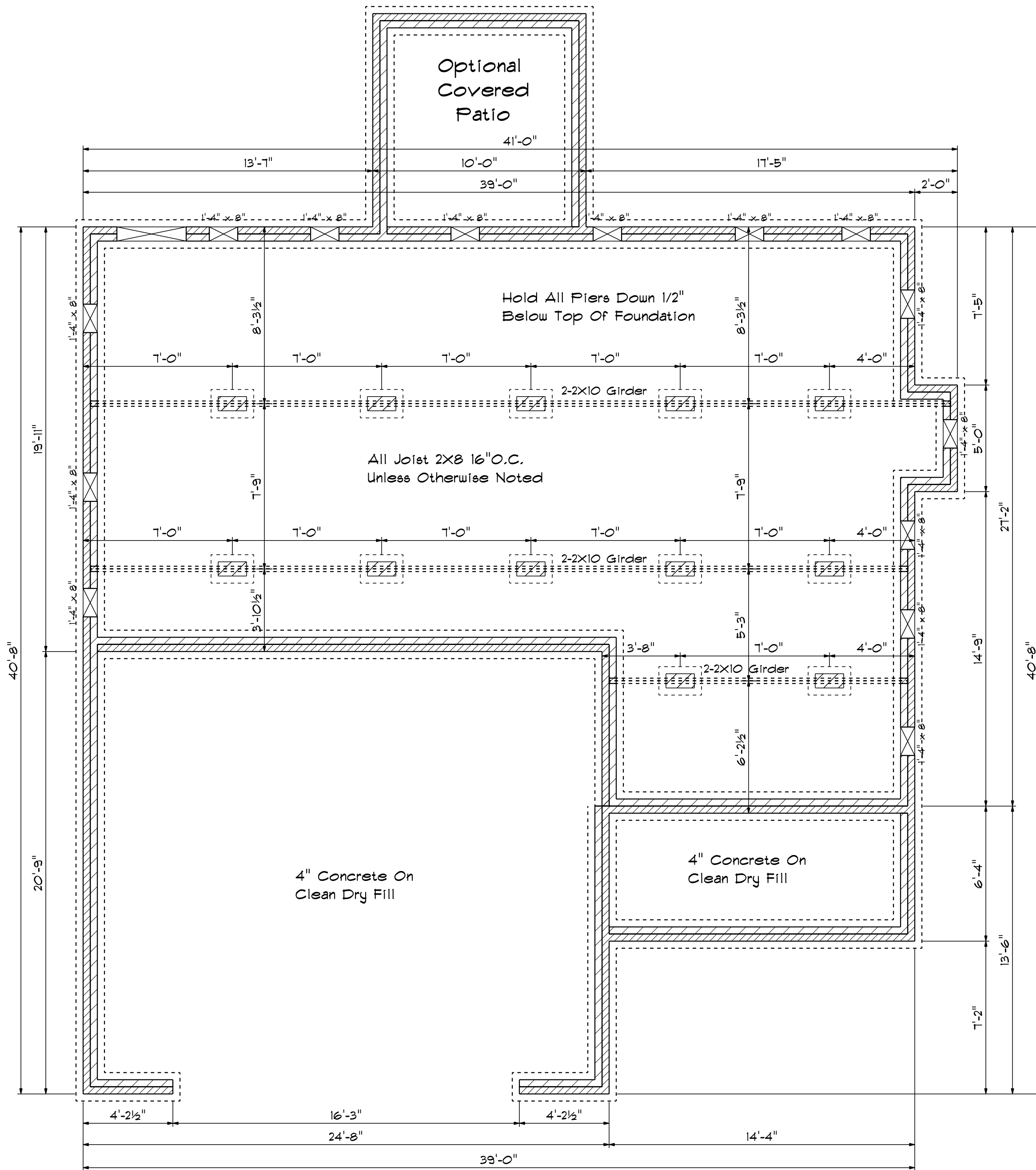
DRAWING#

SCALE: 1/4"

DRAWN BY

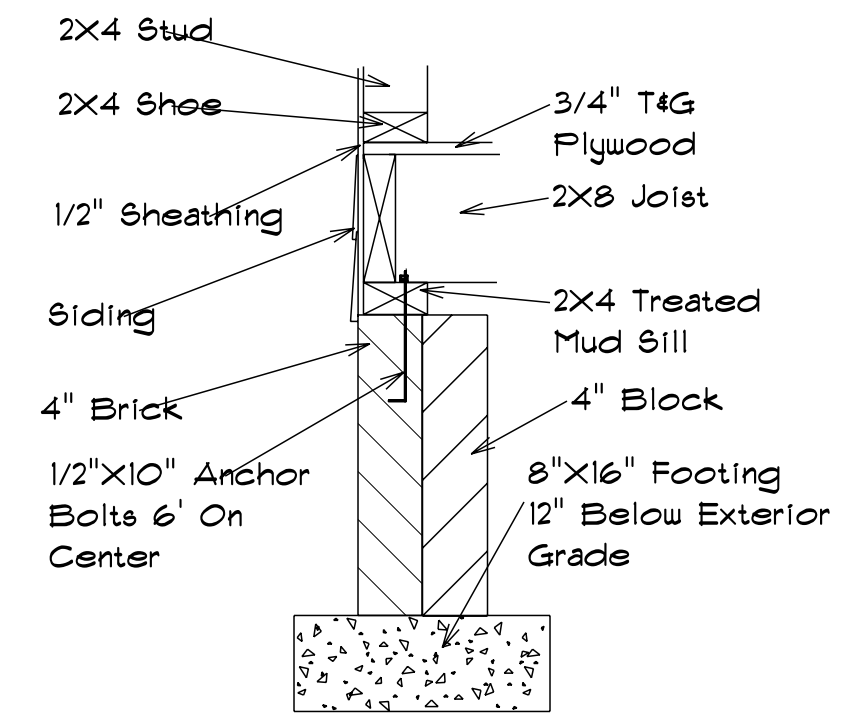
APPROVED

The Westbrook

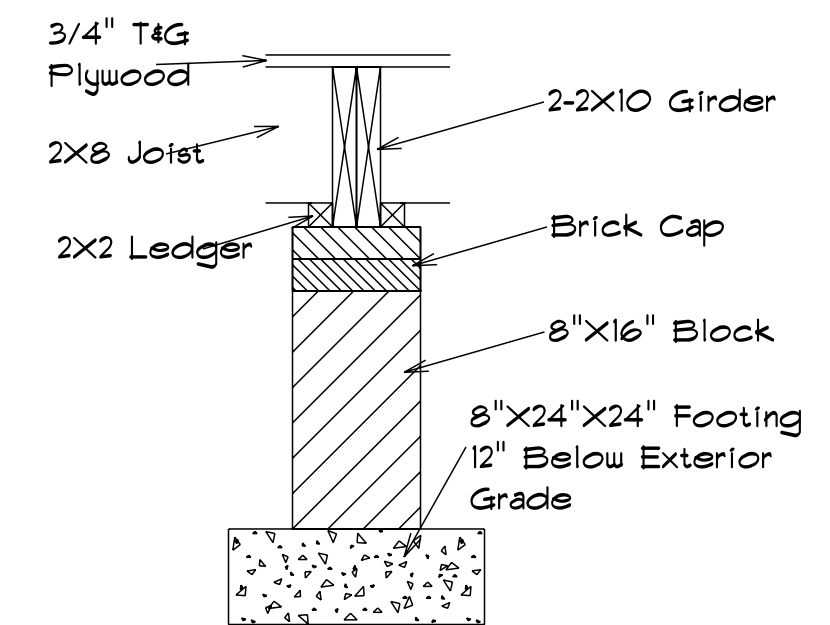


Foundation Plan

Foundation Detail



Footing & Pier Detail



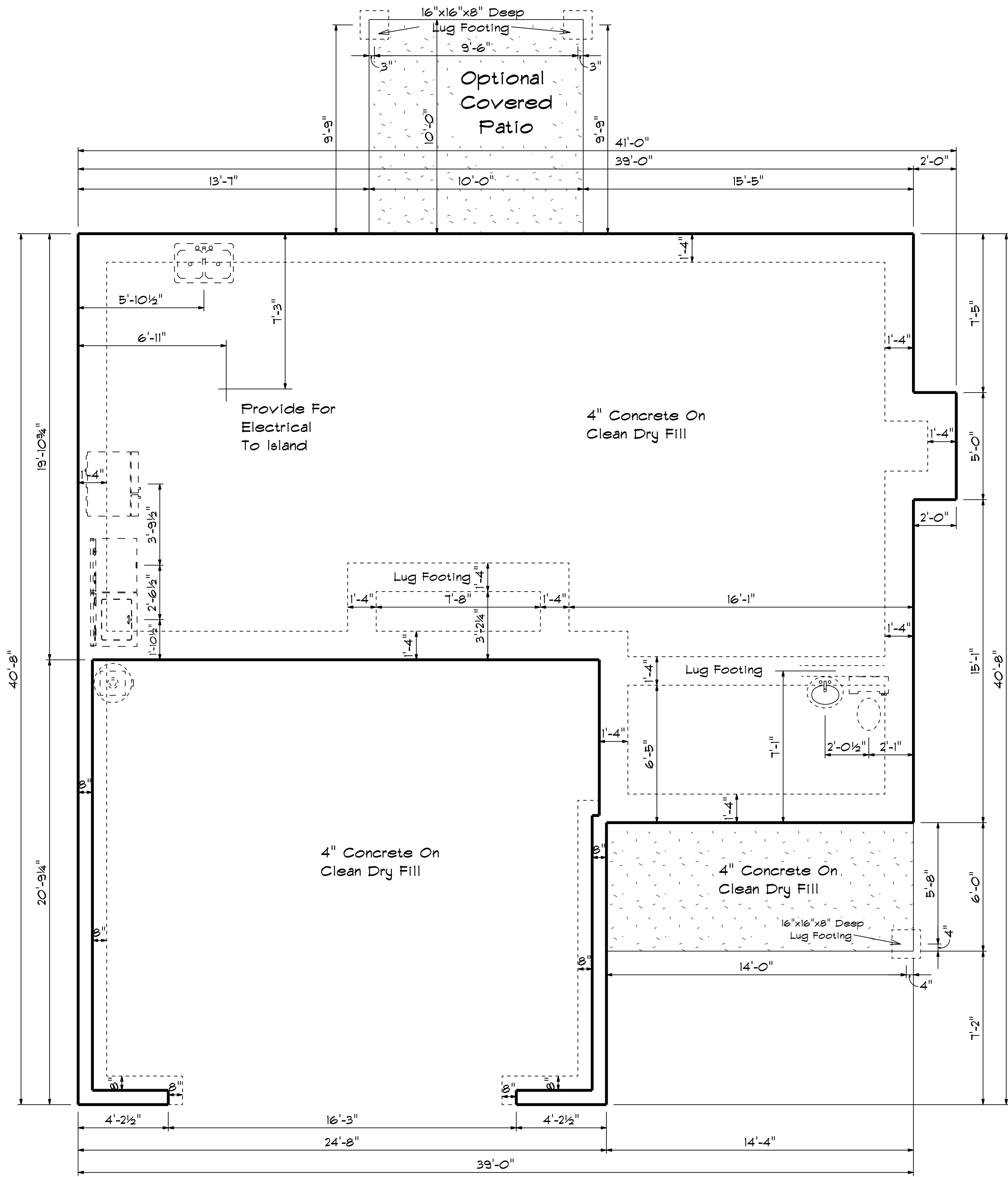
FOUNDATION VENTILATION

892 Sq.Ft. Foundation Area
 Requires 5.94 Sq.Ft. Ventilation.
 With 6 Mil. Poly, Plans Indicate
 Vents For Adequate Cross
 Ventilation.

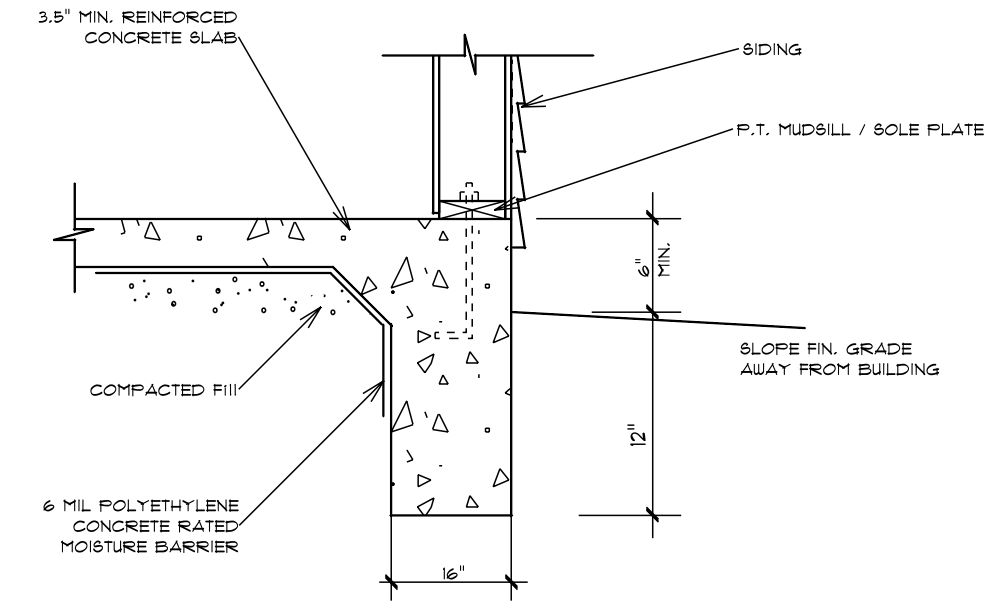
DATE: 8/5/2021
 REVISED
 DRAWING#

SCALE: 1/4"
 DRAWN BY
 APPROVED

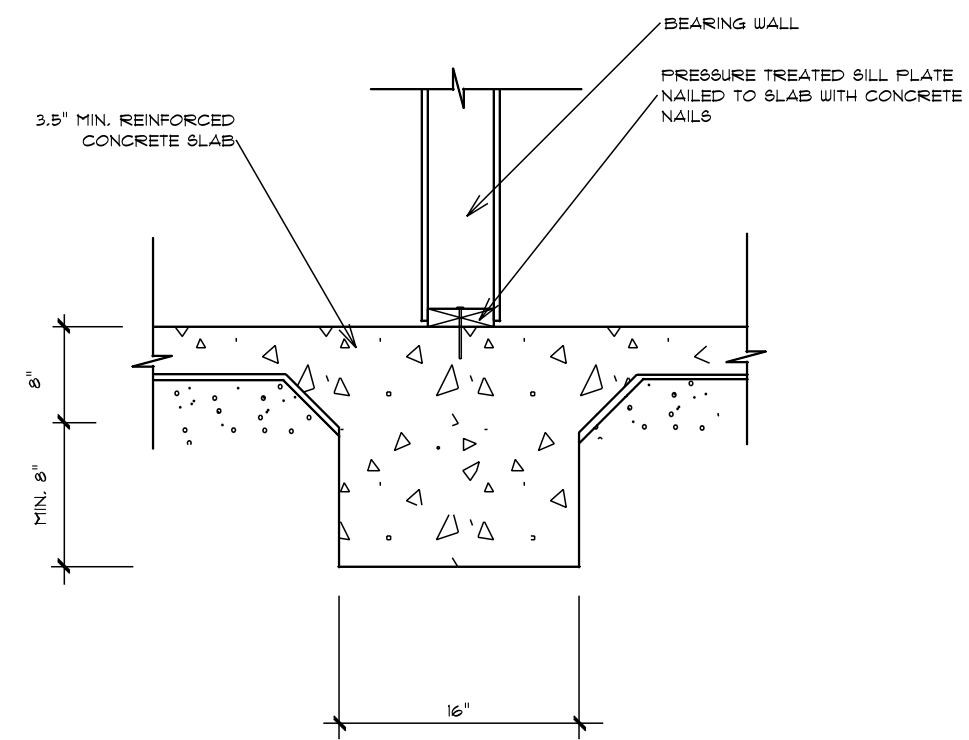
The Westbrook



Foundation Plan



TURN-DOWN FOOTING DETAIL

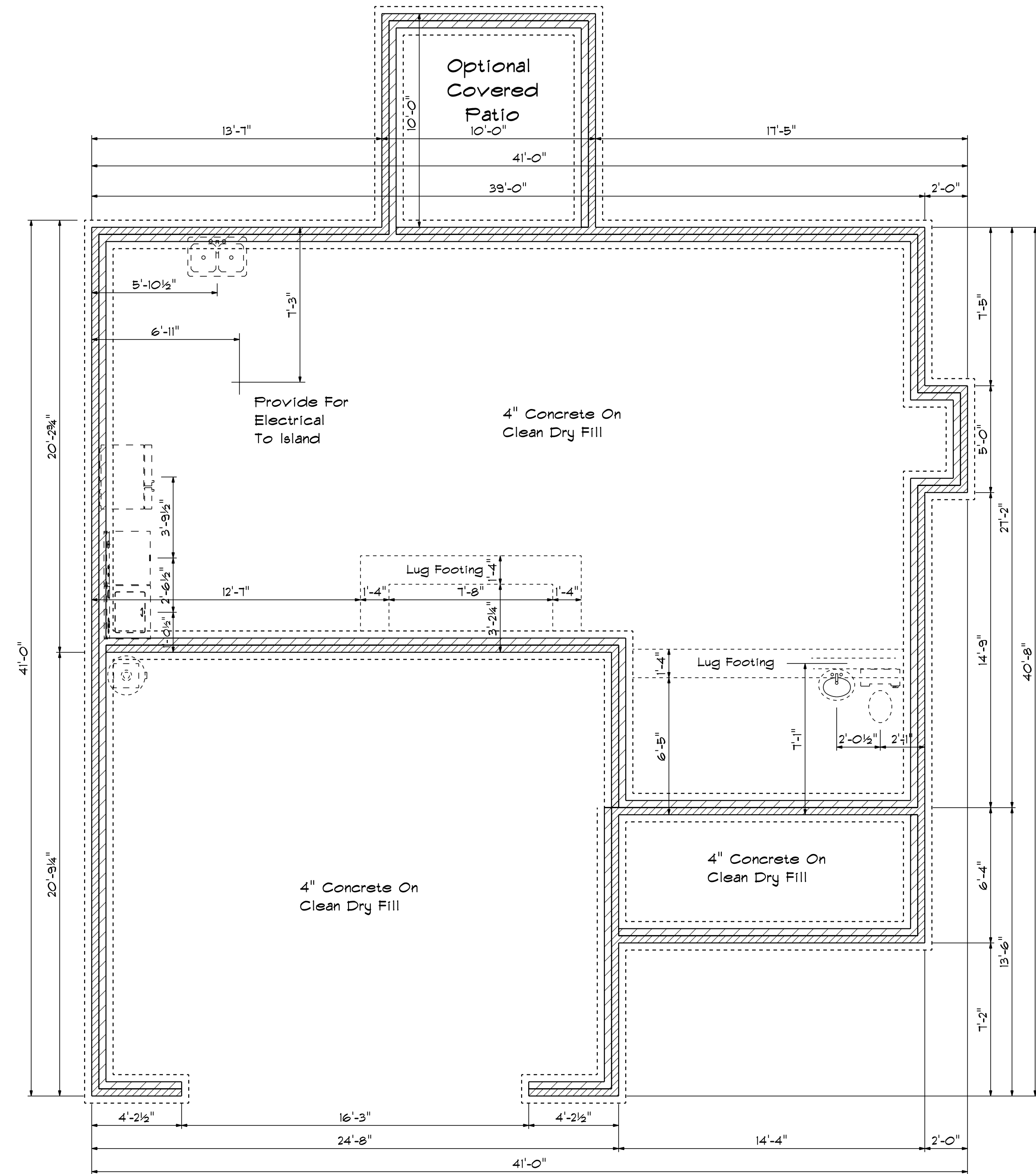


INTEGRAL SLAB FOOTING DETAIL AT BEARING WALL

DATE: 8/5/2021
 REVISIONS
 DRAWING#

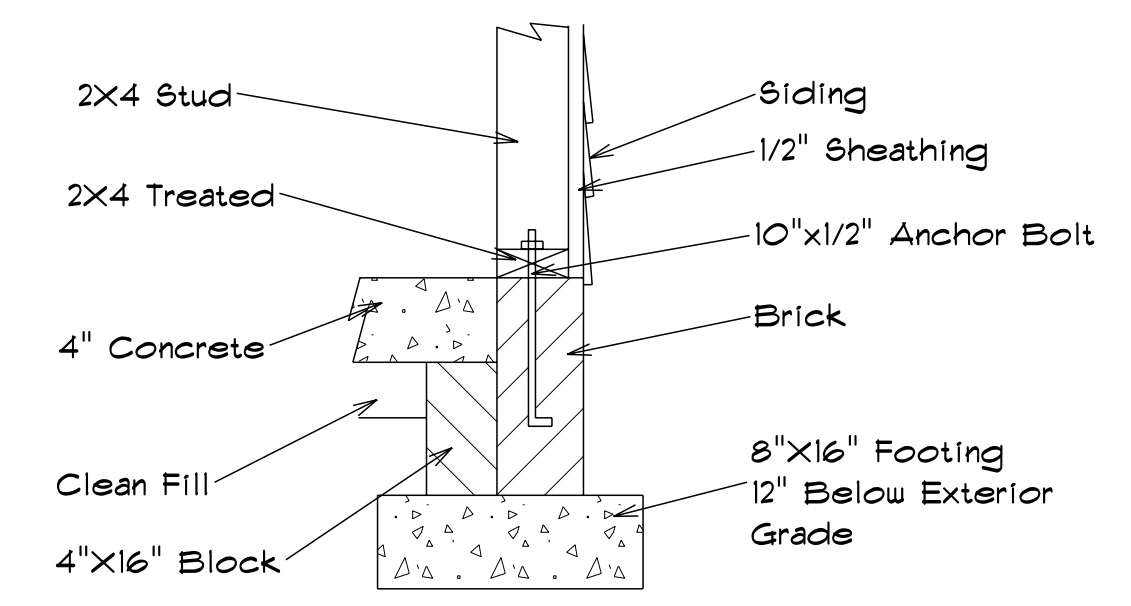
SCALE: 1/4"
 DRAWN BY
 APPROVED

The Westbrook

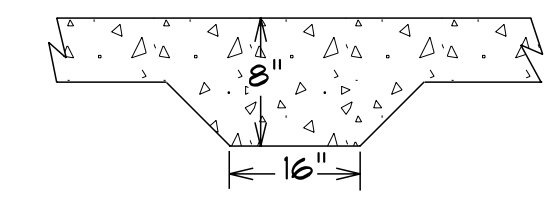


Foundation Plan

Foundation Detail Siding



Lug Footing Detail



DATE: 8/5/2021	REVISION	DRAWING#
SCALE: 1/4"	DRAWN BY	APPROVED
The Westbrook		



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

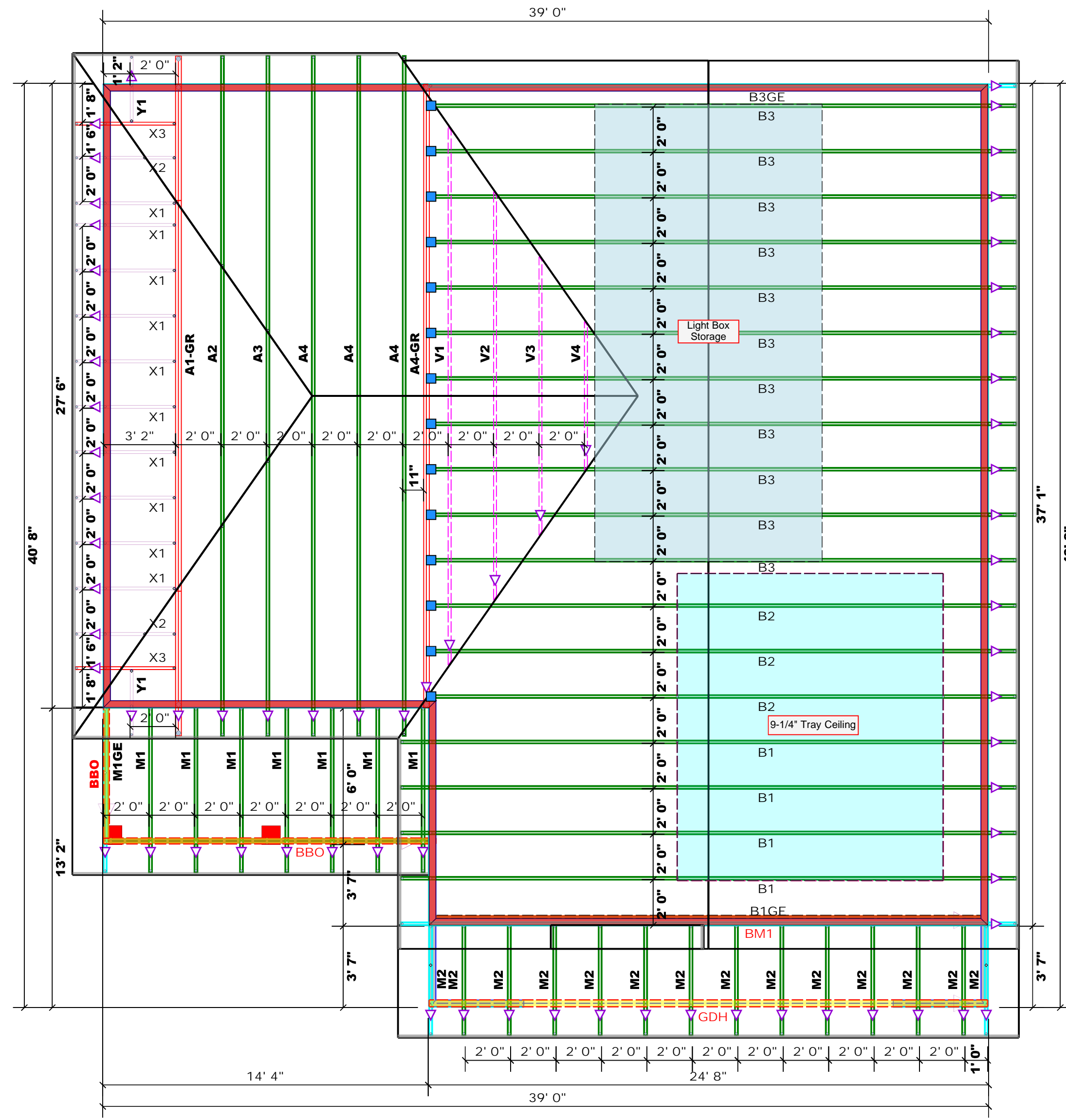
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature David Landry
David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES MODEL: S 03)

REACTION (UP/KN)	NUMBER OF JACK STUDS REQUIRED PER EACH END OF HEADERS/BEAMS	REQ'D STUDS FOR TRAY CEILING		REQ'D STUDS FOR 2ND FLOOR	
		REQ'D STUDS FOR TRAY CEILING	REQ'D STUDS FOR 2ND FLOOR	REQ'D STUDS FOR TRAY CEILING	REQ'D STUDS FOR 2ND FLOOR
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				



Dimension Notes

- All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
- All interior wall dimensions are to face of frame wall unless noted otherwise
- All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Roof Area = 2311.33 sq.ft.
Ridge Line = 65.83 ft.
Hip Line = 40.81 ft.
Horiz. OH = 178.13 ft.
Raked OH = 122.62 ft.
Decking = 79 sheets

Hatch Legend

[Blue Hatch]	Box Storage
[Light Blue Hatch]	Tray Ceiling
[Red Hatch]	2nd Floor Walls
[Yellow Hatch]	Drop Beam

Connector Information

Sym	Product	Manuf	Qty	Supported Member	Header	Truss
[Blue Box]	HUS26	USP	14	NA	16d/3-1/2"	16d/3-1/2"

Nail Information

Header	Truss
16d/3-1/2"	16d/3-1/2"

Products

PlotID	Length	Product	Plies	Net Qty
BM1	25' 0"	1-3/4"x 23-7/8" LVL Kerto-S	3	3
BM2	7' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	11' 0"	1-3/4"x 14" LVL Kerto-S	2	2
GDH	25' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
J-8	8' 0"	14" NI-40x	1	4
J-10	10' 0"	14" NI-40x	1	1
J-16	16' 0"	14" NI-40x	1	6
J-18	18' 0"	14" NI-40x	1	6
J-22	22' 0"	14" NI-40x	1	4
J-28	28' 0"	14" NI-40x	1	6
J-38	38' 0"	14" NI-40x	1	11
RB1	12' 0"	1 1/8" x 14" Rim Board	1	13

1 Truss Placement Plan
Scale: 1/4"=1'

CITY / CO.	Linden / Harnett
ADDRESS	724 Walker Road
MODEL	Roof
DATE REV.	03/18/22
DRAWN BY	David Landry
SALES REP.	Marshall Naylor
BUILDER	Ben Stout Real Estate
JOB NAME	Lot 3 Walker Rd.
PLAN	Westbrook
SEAL DATE	N/A
QUOTE #	
JOB #	J0821-5011

△ = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

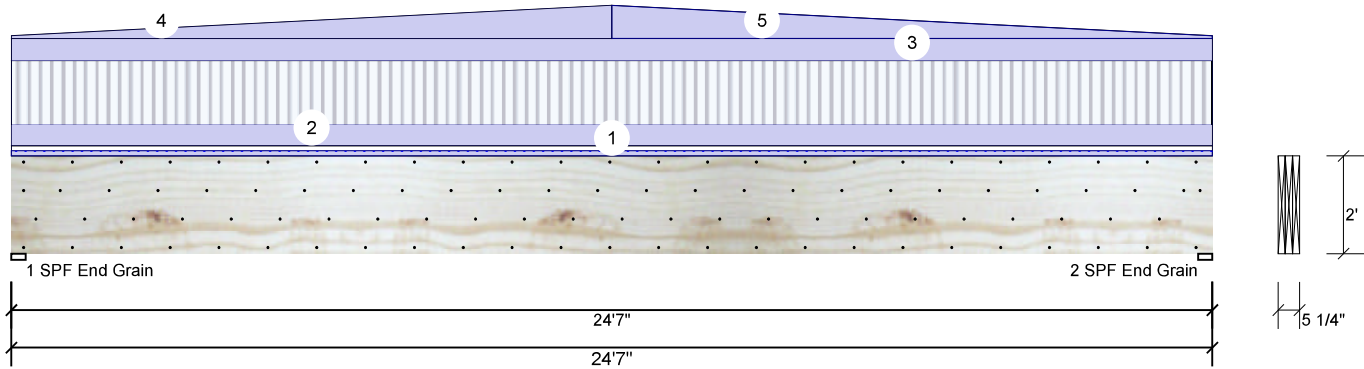


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

BM1 Kerto-S LVL 1.750" X 24.000" 3-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	3
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	Yes
Deck:	Not Checked
Ceiling:	Gypsum 1/2"

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	4253	4788	344	0	0
2	4253	4788	344	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	57%	4788 / 4253	9041	L	D+L
2 - SPF End Grain	3.500"	57%	4788 / 4253	9041	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	55677 ft-lb	12'3 1/2"	114169 ft-lb	0.488 (49%)	D+L	L
Unbraced	55677 ft-lb	12'3 1/2"	55925 ft-lb	0.996 (100%)	D+L	L
Shear	8539 lb	2'2 5/8"	26880 lb	0.318 (32%)	D+L	L
LL Defl inch	0.242 (L/1198)	12'3 9/16"	0.604 (L/480)	0.400 (40%)	L	L
TL Defl inch	0.531 (L/545)	12'3 9/16"	0.805 (L/360)	0.660 (66%)	D+L	L

Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 4'8 1/4" o.c.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Near Face	28 PLF	0 PLF	28 PLF	0 PLF	0 PLF	M2
2	Uniform			Far Face	116 PLF	346 PLF	0 PLF	0 PLF	0 PLF	F1, F3
3	Uniform			Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
4	Tapered Start	0-0-0		Top	15 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE
	End	12-3-8			180 PLF	0 PLF	0 PLF	0 PLF	0 PLF	
5	Tapered Start	12-3-8		Top	180 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE

Continued on page 2...

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.
Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation
 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding
 This design is valid until 4/24/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
 www.metsawood.com/us
 ICC-ES: ESR-3633

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS

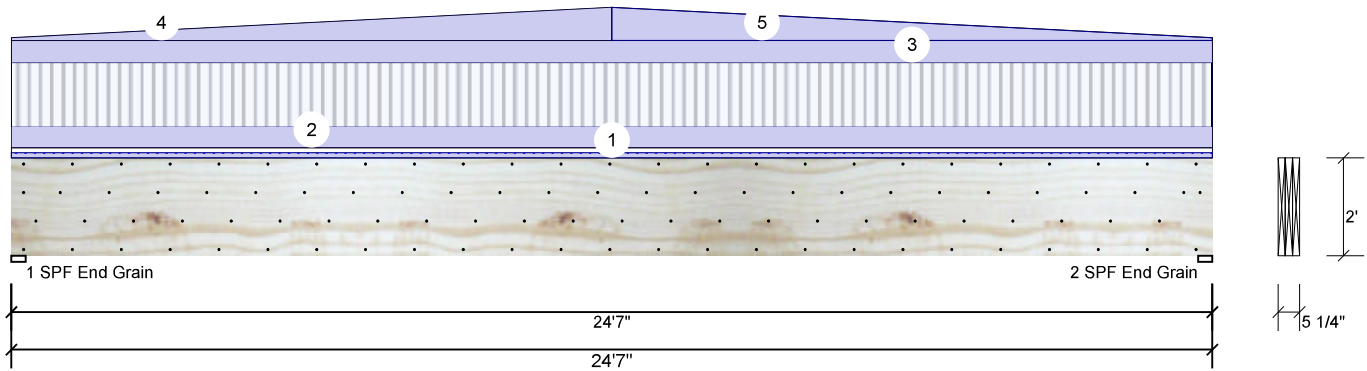


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

BM1 Kerto-S LVL 1.750" X 24.000" 3-Ply - PASSED

Level: Level



...Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
	End	24-7-0			15 PLF	0 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				28 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Manufacturer Info

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 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
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 1001 S. Reilly Road, Suite #639
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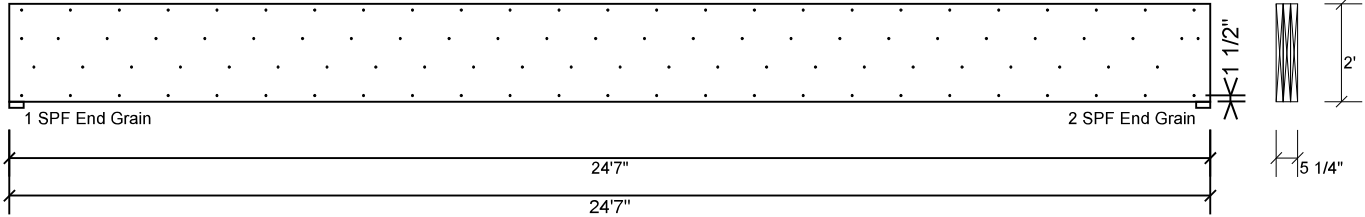


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

BM1 Kerto-S LVL 1.750" X 24.000" 3-Ply - PASSED


Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed 6"

Capacity	94.1 %
Load	308.0 PLF
Yield Limit per Foot	327.4 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+L
Duration Factor	1.00

<p>Notes</p> <p>Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.</p> <p>Lumber</p> <ol style="list-style-type: none"> Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosive chemicals 	<p>Handling & Installation</p> <ol style="list-style-type: none"> LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 	<p>6. For flat roofs provide proper drainage to prevent ponding</p> <p>This design is valid until 4/24/2023</p>	<p>Manufacturer Info</p> <p>Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633</p>	<p>Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS</p> 
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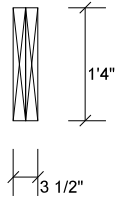
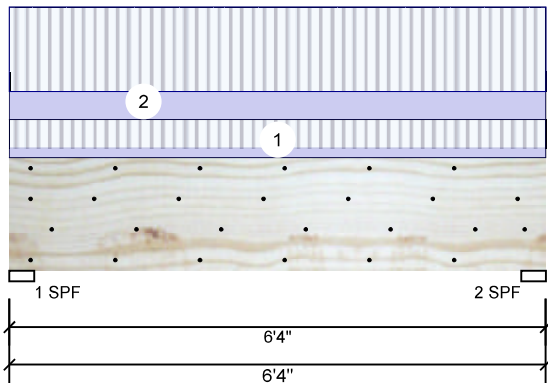


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
 Plies: 2
 Moisture Condition: Dry
 Deflection LL: 480
 Deflection TL: 360
 Importance: Normal
 Temperature: Temp <= 100°F

Application: Floor
 Design Method: ASD
 Building Code: IBC/IRC 2015
 Load Sharing: No
 Deck: Not Checked
 Ceiling: Gypsum 1/2"

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	1742	622	0	0	0
2	1742	622	0	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	45%	622 / 1742	2364	L	D+L
2 - SPF	3.500"	45%	622 / 1742	2364	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3243 ft-lb	3'2"	34565 ft-lb	0.094 (9%)	D+L	L
Unbraced	3243 ft-lb	3'2"	19457 ft-lb	0.167 (17%)	D+L	L
Shear	2184 lb	4'9 3/8"	11947 lb	0.183 (18%)	D+L	L
LL Defl inch	0.011 (L/6331)	3'2"	0.147 (L/480)	0.080 (8%)	L	L
TL Defl inch	0.015 (L/4665)	3'2"	0.197 (L/360)	0.080 (8%)	D+L	L

Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Near Face	47 PLF	139 PLF	0 PLF	0 PLF	0 PLF	F5
2	Uniform			Far Face	137 PLF	411 PLF	0 PLF	0 PLF	0 PLF	F4
	Self Weight				12 PLF					

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation
 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
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 28314
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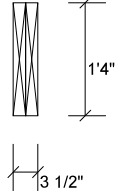
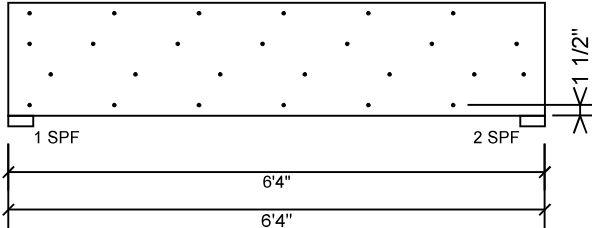


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	83.7 %
Load	274.0 PLF
Yield Limit per Foot	327.4 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+L
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Manufacturer Info

Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



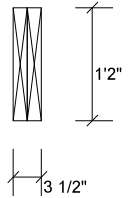
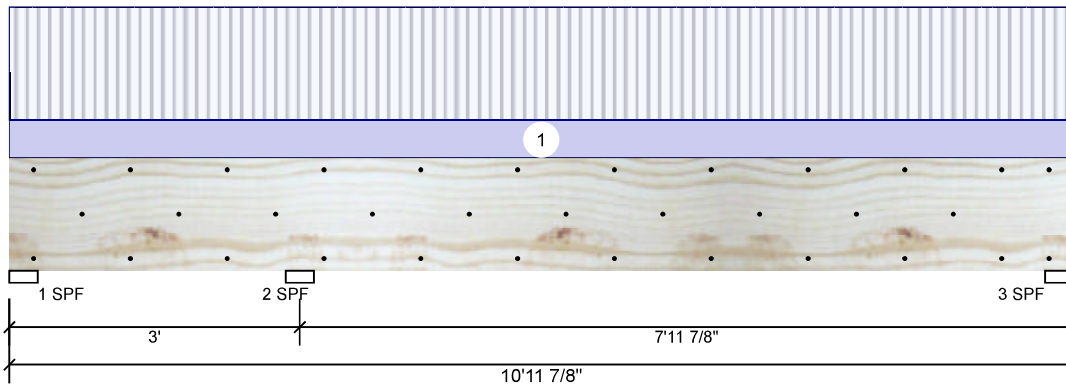


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

BM3 Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked
Ceiling:	Gypsum 1/2"

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	3	1	0	0	0
2	2419	892	0	0	0
3	1149	424	0	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	8%	-57 / 495	438 (-634)	L_	D+L(D+L)
2 - SPF	3.500"	69%	972 / 2635	3606	LL	D+L
3 - SPF	3.500"	29%	403 / 1103	1506	_L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-2580 ft-lb	3'	26999 ft-lb	0.096 (10%)	D+L	LL
Pos Moment	2216 ft-lb	7'7 1/4"	26999 ft-lb	0.082 (8%)	D+L	_L
Unbraced	2216 ft-lb	7'7 1/4"	13144 ft-lb	0.169 (17%)	D+L	_L
Shear	2046 lb	4'2"	10453 lb	0.196 (20%)	D+L	LL
LL Defl inch	0.017 (L/5561)	7' 11/16"	0.194 (L/480)	0.090 (9%)	L	_L
TL Defl inch	0.023 (L/4079)	7' 13/16"	0.259 (L/360)	0.090 (9%)	D+L	_L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Tie-down connection required at bearing 1 for uplift 634 lb (Combination D+L, Load Case _L).
- 5 Top braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	109 PLF	325 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				11 PLF					

Notes
 Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation
 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/24/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
 www.metsawood.com/us
 ICC-ES: ESR-3633

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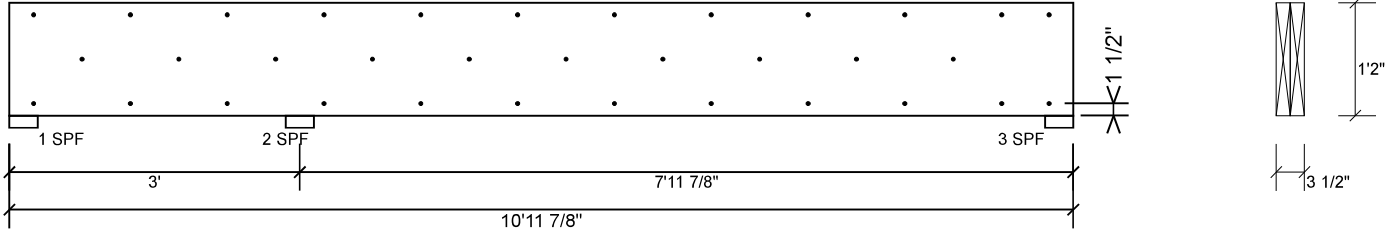


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

BM3 Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	88.4 %
Load	217.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+L
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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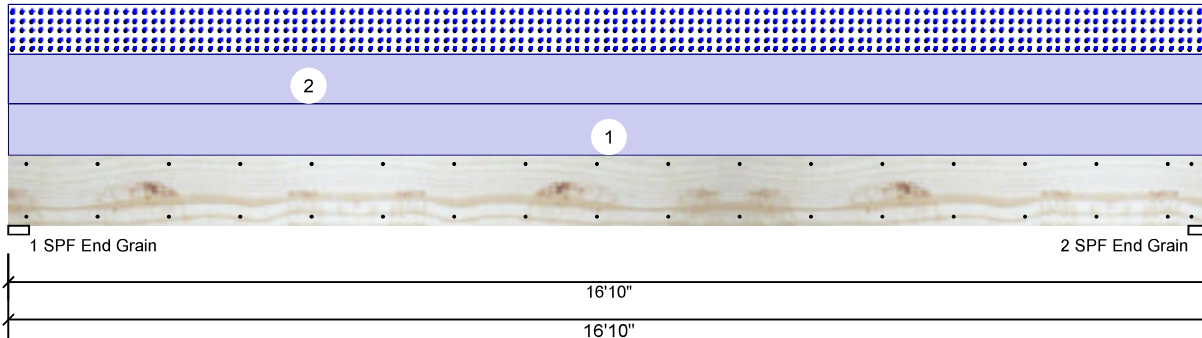


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked
Ceiling:	Gypsum 1/2"

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	1071	488	0	0
2	0	1071	488	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total Ld.	Case	Ld. Comb.
1 - SPF End Grain	3.500"	15%	1071 / 488	1559	L	D+S
2 - SPF End Grain	3.500"	15%	1071 / 488	1559	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6209 ft-lb	8'5"	22897 ft-lb	0.271 (27%)	D+S	L
Unbraced	6209 ft-lb	8'5"	6213 ft-lb	0.999 (100%)	D+S	L
Shear	1333 lb	1'2 5/8"	10197 lb	0.131 (13%)	D+S	L
LL Defl inch	0.101 (L/1937)	8'5 1/16"	0.409 (L/480)	0.250 (25%)	S	L
TL Defl inch	0.324 (L/607)	8'5 1/16"	0.546 (L/360)	0.590 (59%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 16' 3/8" o.c.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
2	Uniform			Top	58 PLF	0 PLF	58 PLF	0 PLF	0 PLF	M2
	Self Weight				9 PLF					

Notes
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Lumber
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Handling & Installation
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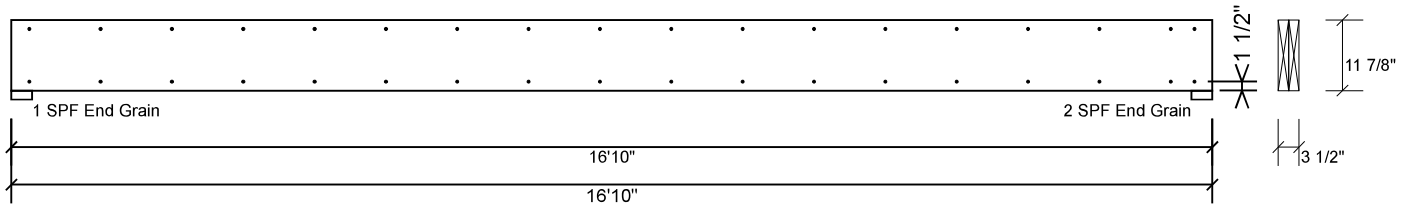


Client: Benjamin Stout Real Estate
 Project: The Westbrook
 Address: 724 Walker Road
 Linden, NC 28356

Date: 3/18/2022
 Input by: David Landry
 Job Name: Lot 3 Walker Rd.
 Project #: J0821-5011

GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

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RE: J0821-5011
Lot 3 Walker Rd.

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

Customer: Benjamin Stout Real Estate Project Name: J0821-5011
Lot/Block: 3 Model: Westbrook
Address: 724 Walker Road Subdivision: Walker Rd.
City: Linden State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 23 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E16171316	A1-GR	9/15/2021	21	E16171336	X2	9/15/2021
2	E16171317	A2	9/15/2021	22	E16171337	X3	9/15/2021
3	E16171318	A3	9/15/2021	23	E16171338	Y1	9/15/2021
4	E16171319	A4	9/15/2021				
5	E16171320	A4-GR	9/15/2021				
6	E16171321	B1	9/15/2021				
7	E16171322	B1GE	9/15/2021				
8	E16171323	B2	9/15/2021				
9	E16171324	B3	9/15/2021				
10	E16171325	B3GE	9/15/2021				
11	E16171326	C1	9/15/2021				
12	E16171327	C1GE	9/15/2021				
13	E16171328	M1	9/15/2021				
14	E16171329	M1GE	9/15/2021				
15	E16171330	M2	9/15/2021				
16	E16171331	V1	9/15/2021				
17	E16171332	V2	9/15/2021				
18	E16171333	V3	9/15/2021				
19	E16171334	V4	9/15/2021				
20	E16171335	X1	9/15/2021				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

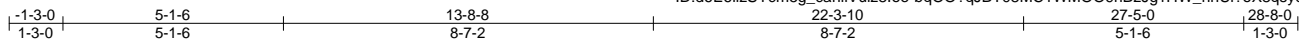


September 15, 2021

Job J0821-5011	Truss A1-GR	Truss Type HIP GIRDER	Qty 1	Ply 2	Lot 3 Walker Rd. Job Reference (optional)	E16171316
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:11:51 2021 Page 1
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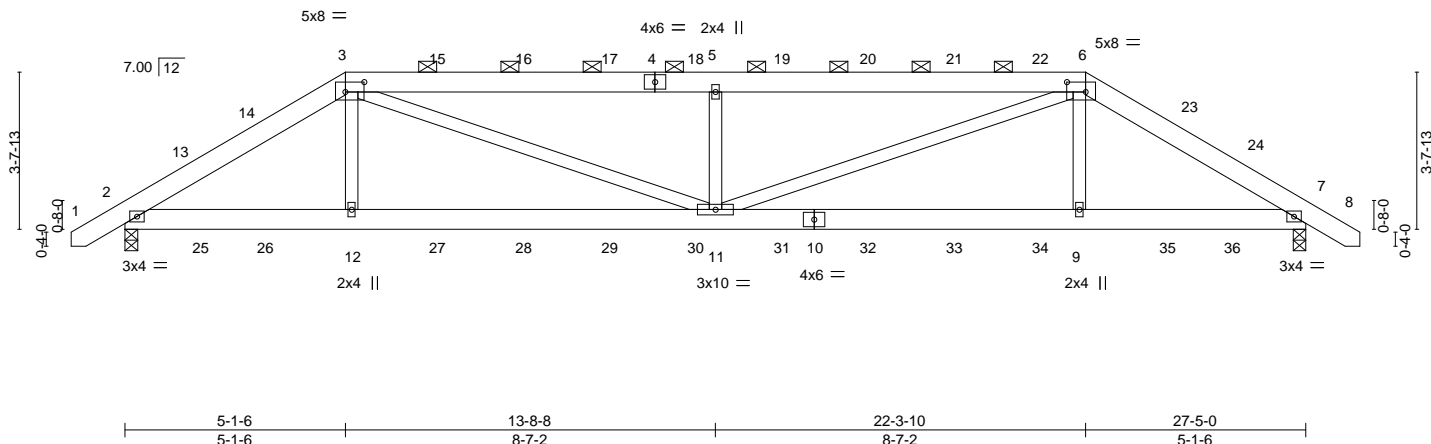


Plate Offsets (X,Y)--	[3:0-5-4,0-2-12], [6:0-5-4,0-2-12]
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.19	Vert(LL)	-0.05	11	>999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.17	Vert(CT)	-0.10	11	>999		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.16	Horz(CT)	0.02	7	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.07	11	>999	Weight: 354 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD 2x6 SP No.1	2-0-0 oc purlins (6-0-0 max.): 3-6.
WEBS 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 7=0-3-8
 Max Horz 2=87(LC 26)
 Max Uplift 2=345(LC 5), 7=361(LC 4)
 Max Grav 2=1321(LC 1), 7=1327(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2061/628, 3-5=-2793/1003, 5-6=-2793/1002, 6-7=-2075/658
 BOT CHORD 2-12=-564/1694, 11-12=-559/1703, 9-11=-531/1705, 7-9=-536/1695
 WEBS 3-12=0/376, 3-11=-525/1238, 5-11=-642/479, 6-11=-499/1208, 6-9=0/386

- NOTES-**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
 Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=345, 7=361.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 57 lb down and 58 lb up at 1-8-12, 53 lb down and 61 lb up at 3-2-12, 90 lb down and 88 lb up at 5-1-6, 94 lb down and 85 lb up at 7-2-12, 94 lb down and 85 lb up at 9-2-12, 94 lb down and 85 lb up at 11-2-12, 94 lb down and 85 lb up at 13-2-12, 94 lb down and 85 lb up at 15-2-12, 94 lb down and 85 lb up at 17-2-12, 94 lb down and 85 lb up at 19-2-12, 94 lb down and 85 lb up at 21-2-12, 90 lb down and 88 lb up at 22-3-10, and 53 lb down and 61 lb up at 24-2-4, and 57 lb down and 58 lb up at 25-8-4 on top chord, and 20 lb down at 1-8-12, 22 lb down at 3-2-12, 19 lb down at 5-2-12, 19 lb down at 7-2-12, 19 lb down at 9-2-12, 19 lb down at 11-2-12, 19 lb down at 13-2-12, 19 lb down at 15-2-12, 19 lb down at 17-2-12, 19 lb down at 19-2-12, 19 lb down at 21-2-12, 19 lb down at 22-2-4, and 22 lb down at 24-2-4, and 20 lb down at 25-8-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.



September 15, 2021

Continued on page 2

LOAD CASE(S), Standard

WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss A1-GR	Truss Type HIP GIRDER	Qty 1	Ply 2	Lot 3 Walker Rd. Job Reference (optional)	E16171316
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:11:51 2021 Page 2
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LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-6=-60, 6-8=-60, 2-7=-20

Concentrated Loads (lb)

Vert: 3=-13(F) 6=-13(F) 12=-10(F) 9=-10(F) 13=-17(F) 14=-13(F) 15=-13(F) 16=-13(F) 17=-13(F) 18=-13(F) 19=-13(F) 20=-13(F) 21=-13(F) 22=-13(F) 23=-13(F) 24=-17(F) 25=-13(F) 26=-10(F) 27=-10(F) 28=-10(F) 29=-10(F) 30=-10(F) 31=-10(F) 32=-10(F) 33=-10(F) 34=-10(F) 35=-10(F) 36=-13(F)

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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component**



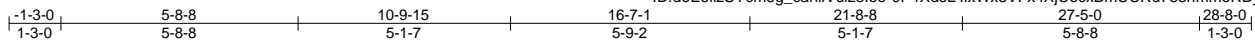
818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss A3	Truss Type HIP	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171318
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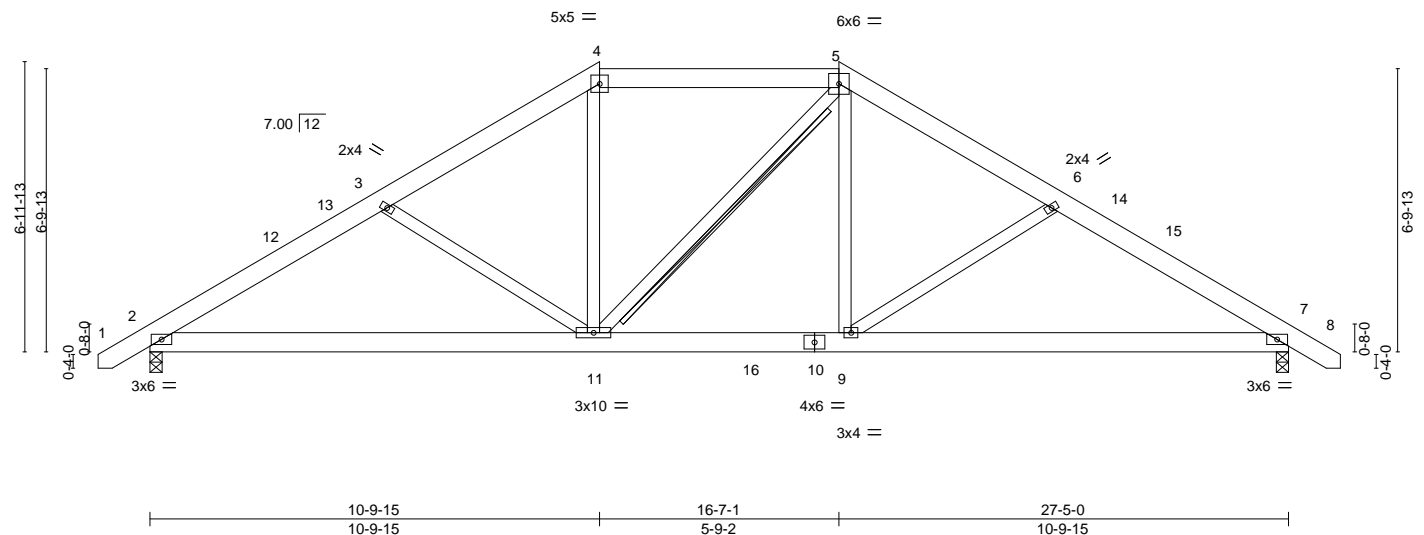
Comtech, Inc. Fayetteville, NC - 28314.

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:11:54 2021 Page 1

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LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.38	Vert(LL) -0.09 7-9 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.22	Vert(CT) -0.19 7-9 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.03 7 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.02 9 >999 240	Weight: 191 lb	FT = 20%

LUMBER-
 TOP CHORD 2x6 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 5-11-1 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
 Rigid ceiling directly applied or 10-0-0 oc bracing.
 BOT CHORD T-Brace: 2x4 SPF No.2 - 5-11
 WEBS Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. (size) 2=0-3-8, 7=0-3-8
 Max Horz 2=165(LC 11)
 Max Uplift 2=65(LC 12), 7=65(LC 13)
 Max Grav 2=1159(LC 1), 7=1159(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1660/451, 3-4=-1387/387, 4-5=-1123/386, 5-6=-1385/387, 6-7=-1660/451
 BOT CHORD 2-11=-287/1362, 9-11=-110/1122, 7-9=-292/1360
 WEBS 3-11=-361/217, 4-11=-18/397, 5-9=-17/433, 6-9=-362/217

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-0 to 3-3-13, Interior(1) 3-3-13 to 10-9-15, Exterior(2) 10-9-15 to 22-9-12, Interior(1) 22-9-12 to 28-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



September 15, 2021

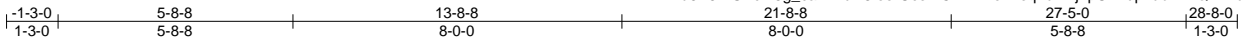
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



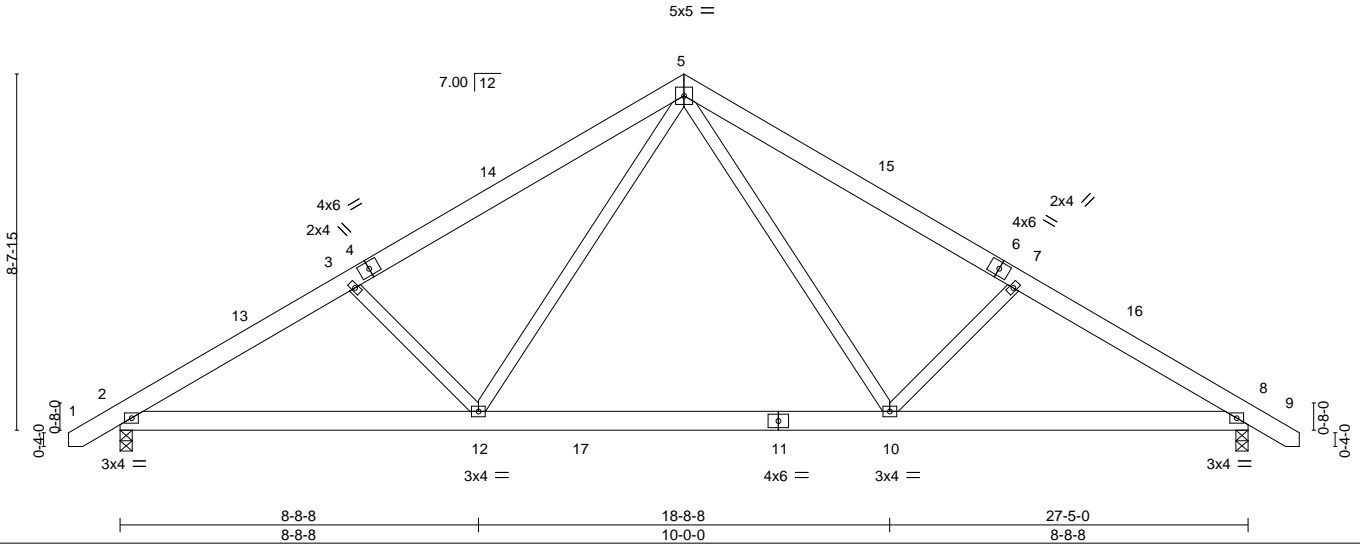
Job J0821-5011	Truss A4	Truss Type COMMON	Qty 3	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171319
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Comtech, Inc. Fayetteville, NC - 28314.

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:11:55 2021 Page 1
ID:d6E6lizSYcm5g_caniVuiZ8loe-UcevrCMIWFenheq7dEEjLpUKxupvdbRlwQVLzdydRpY



Scale = 1:52.7



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.26	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.43	Vert(LL) -0.16 10-12 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.16	Vert(CT) -0.23 10-12 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.03 8 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.03 10-12 >999 240	Weight: 186 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-9-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 8=0-3-8
Max Horz 2=207(LC 11)
Max Uplift 2=-78(LC 12), 8=-78(LC 13)
Max Grav 2=1194(LC 19), 8=1192(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1846/369, 3-5=-1624/369, 5-7=-1620/369, 7-8=-1842/369
BOT CHORD 2-12=-218/1656, 10-12=-13/1021, 8-10=-231/1497
WEBS 5-10=-81/711, 7-10=-430/257, 5-12=-81/718, 3-12=-430/257

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-0 to 3-3-13, Interior(1) 3-3-13 to 13-8-8, Exterior(2) 13-8-8 to 18-1-5, Interior(1) 18-1-5 to 28-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



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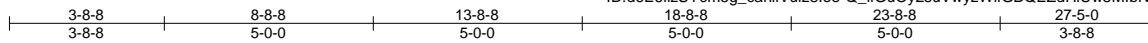
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road
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Job J0821-5011	Truss A4-GR	Truss Type Common Girder	Qty 1	Ply 2	Lot 3 Walker Rd. Job Reference (optional)	E16171320
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8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:11:57 2021 Page 1
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5x8 ||

Scale = 1:51.9

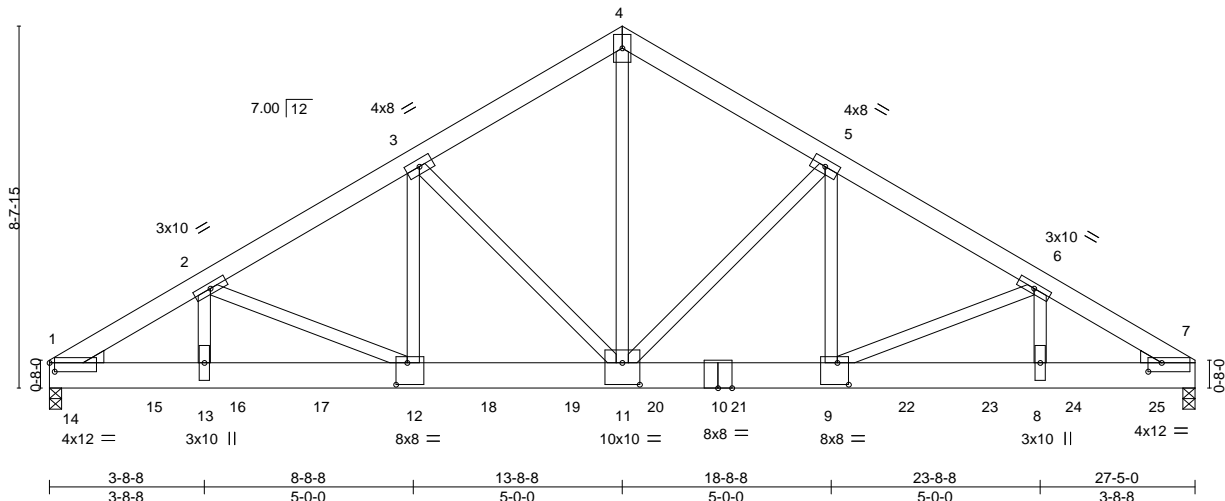


Plate Offsets (X,Y)-- [1:0-1-8,0-2-8], [7:0-3-14,0-2-8], [9:0-3-4,0-6-4], [11:0-5-0,0-6-4], [12:0-3-4,0-6-4]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.39	Vert(LL)	-0.16 11-12	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.46	Vert(CT)	-0.28 11-12	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.71	Horz(CT)	0.08 7	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.08 11-12	>999	240	Weight: 451 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x8 SP 2400F 2.0E
WEBS 2x4 SP No.2 *Except*
4-11: 2x4 SP No.1

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS.

(size) 1=0-3-8 (req. 0-3-10), 7=0-3-8 (req. 0-3-12)
Max Horz 1=194(LC 26)
Max Uplift 1=429(LC 8), 7=419(LC 9)
Max Grav 1=8714(LC 2), 7=9108(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-13724/666, 2-3=-11786/583, 3-4=-8895/491, 4-5=-8896/491, 5-6=-11961/583,
6-7=-14504/673
BOT CHORD 1-13=-616/11539, 12-13=-616/11539, 11-12=-469/10149, 9-11=-394/10303,
8-9=-526/12198, 7-8=-526/12198
WEBS 4-11=-403/8583, 5-11=-3809/280, 5-9=-143/4046, 3-11=-3588/279, 3-12=-142/3805,
6-9=-2093/169, 6-8=-60/2458, 2-13=-59/1895, 2-12=-1641/162

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-8-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- WARNING: Required bearing size at joint(s) 1, 7 greater than input bearing size.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=429, 7=419.



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Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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818 Soundside Road
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Job	Truss	Truss Type	Qty	Ply	Lot 3 Walker Rd.	E16171320
J0821-5011	A4-GR	Common Girder	1	2	Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:11:57 2021 Page 2
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NOTES-

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 960 lb down and 54 lb up at 0-5-12, 953 lb down and 61 lb up at 2-5-12, 953 lb down and 61 lb up at 4-5-12, 1192 lb down and 60 lb up at 6-5-12, 1192 lb down and 60 lb up at 8-5-12, 1192 lb down and 60 lb up at 10-5-12, 1192 lb down and 60 lb up at 12-5-12, 1192 lb down and 60 lb up at 14-5-12, 1192 lb down and 60 lb up at 16-5-12, 1192 lb down and 60 lb up at 18-5-12, 1192 lb down and 60 lb up at 20-5-12, 1192 lb down and 60 lb up at 22-5-12, and 1192 lb down and 60 lb up at 24-5-12, and 1194 lb down and 58 lb up at 26-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 9=-953(B) 12=-953(B) 14=-960(B) 15=-953(B) 16=-953(B) 17=-953(B) 18=-953(B) 19=-953(B) 20=-953(B) 21=-953(B) 22=-953(B) 23=-953(B) 24=-953(B) 25=-955(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component**

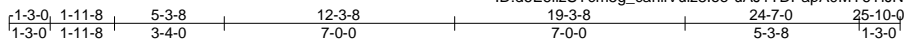


818 Soundside Road
 Edenton, NC 27932

Job J0821-5011	Truss B1	Truss Type COMMON	Qty 4	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171321
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Comtech, Inc. Fayetteville, NC - 28314.

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:11:58 2021 Page 1
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5x5 =

Scale = 1:65.9

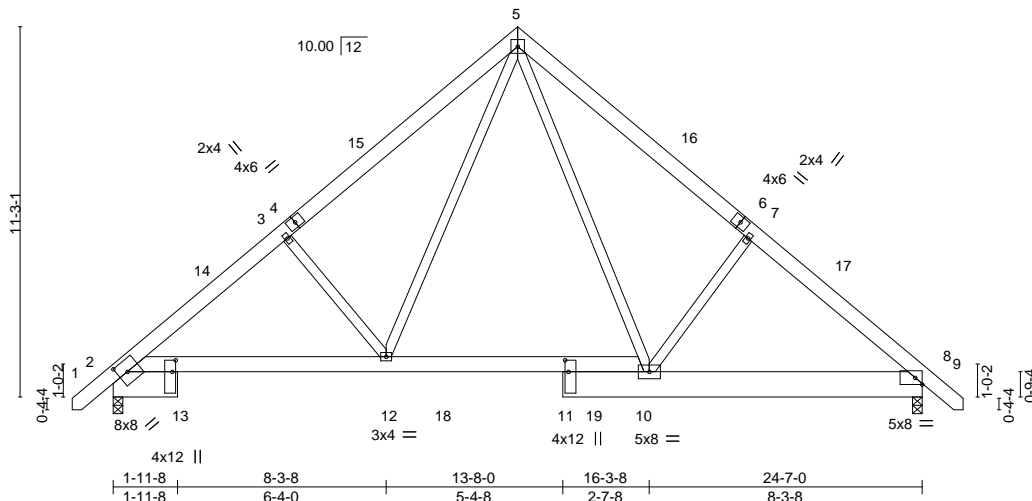


Plate Offsets (X,Y)-- [2:0-3-6,0-4-0], [11:0-4-4,0-1-4], [13:0-4-4,0-1-4]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.19	Vert(LL)	-0.06	10-12	>999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.44	Vert(CT)	-0.10	2-12	>999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.27	Horz(CT)	0.04	8	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.04	2-12	>999	Weight: 216 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x10 SP No.1 *Except*
2-10: 2x6 SP No.1
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 2=0-3-8, 8=0-3-8
Max Horz 2=268(LC 10)
Max Uplift 2=58(LC 12), 8=58(LC 13)
Max Grav 2=1062(LC 19), 8=1061(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1414/297, 3-5=-1279/365, 5-7=-1188/349, 7-8=-1351/278
BOT CHORD 2-12=-132/1207, 10-12=0/735, 8-10=-93/964
WEBS 3-12=-412/272, 5-12=-137/704, 5-10=-115/605, 7-10=-382/275

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-4 to 3-3-9, Interior(1) 3-3-9 to 12-3-8, Exterior(2) 12-3-8 to 16-8-5, Interior(1) 16-8-5 to 25-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



September 15, 2021

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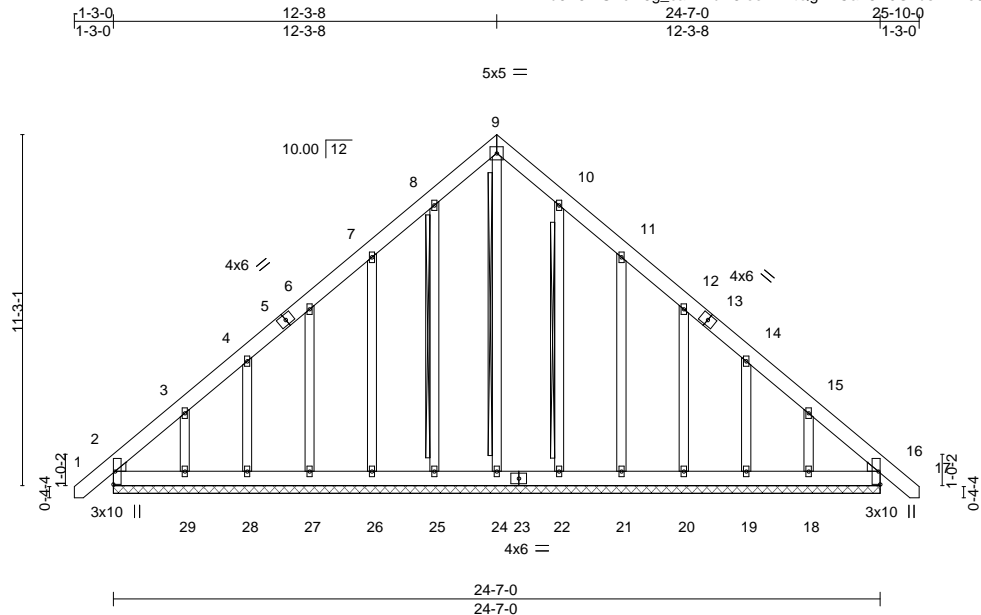


818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss B1GE	Truss Type COMMON SUPPORTED GAB	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171322
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:11:59 2021 Page 1
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Scale = 1:69.5

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.05	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.03	Vert(LL) -0.00 16 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.16	Vert(CT) -0.00 16 n/r 120		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 16 n/a n/a		
	Code IRC2015/TPI2014			Weight: 236 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
OTHERS 2x4 SP No.2
WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2x4 SPF No.2 - 9-24, 8-25, 10-22
Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.
Brace must cover 90% of web length.

REACTIONS.

All bearings 24-7-0.
(lb) - Max Horz 2=335(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 25, 22, 16 except 2=104(LC 8), 26=-124(LC 12), 27=-111(LC 12), 28=-107(LC 12), 29=-190(LC 12), 21=-127(LC 13), 20=-111(LC 13), 19=-106(LC 13), 18=-181(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 24, 25, 26, 27, 28, 29, 22, 21, 20, 19, 18, 16 except 2=264(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-397/265, 8-9=-231/253, 9-10=-231/253, 15-16=-334/227
BOT CHORD 2-29=-193/300, 28-29=-193/300, 27-28=-193/300, 26-27=-193/300, 25-26=-193/300, 24-25=-193/300, 22-24=-193/300, 21-22=-193/300, 20-21=-193/300, 19-20=-193/300, 18-19=-193/300, 16-18=-193/300

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 22, 16 except (jt=lb) 2=104, 26=124, 27=111, 28=107, 29=190, 21=127, 20=111, 19=106, 18=181.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



September 15, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

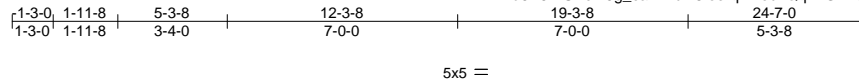


818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss B2	Truss Type COMMON	Qty 3	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171323
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:00 2021 Page 1
ID:d6E6lizSYcm5g_caniVuiZ8loe-qZRouvQqLnG4nQi5Qnpu2tBBvWrHqj14iD6erydRpT



Scale = 1:65.9

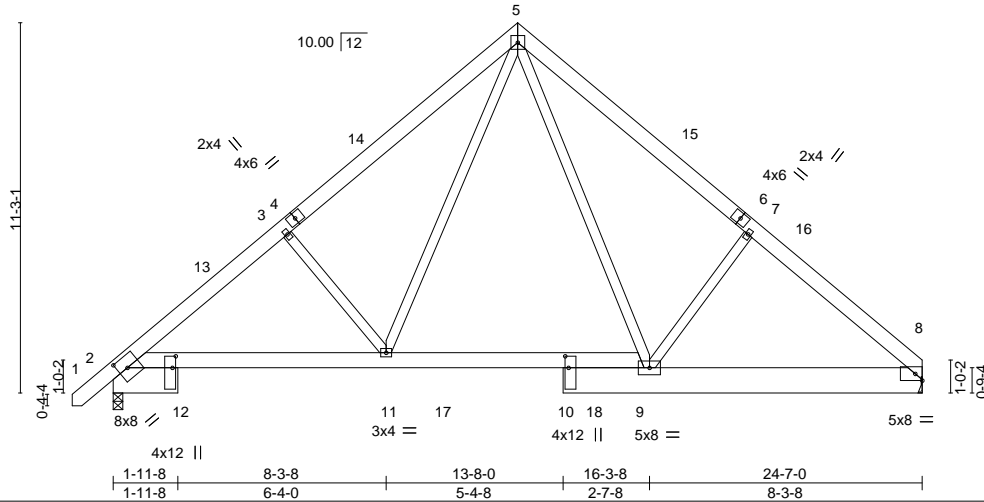


Plate Offsets (X,Y)-- [2:0-3-6,0-4-0], [10:0-4-4,0-1-4], [12:0-4-4,0-1-4]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.19	Vert(LL)	-0.06	9-11	>999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.44	Vert(CT)	-0.10	2-11	>999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.28	Horz(CT)	0.04	8	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.04	2-11	>999	Weight: 213 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
 BOT CHORD 2x10 SP No.1 *Except*
 2-9: 2x6 SP No.1
 WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 2=0-3-8, 8=Mechanical
 Max Horz 2=263(LC 9)
 Max Uplift 2=58(LC 12), 8=41(LC 13)
 Max Grav 2=1066(LC 19), 8=993(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1420/302, 3-5=-1285/370, 5-7=-1204/369, 7-8=-1365/298
 BOT CHORD 2-11=-142/1205, 9-11=0/733, 8-9=-120/977
 WEBS 3-11=-412/273, 5-11=-144/704, 5-9=-121/623, 7-9=-396/282

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-4 to 3-3-9, Interior(1) 3-3-9 to 12-3-8, Exterior(2) 12-3-8 to 16-8-5, Interior(1) 16-8-5 to 24-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



September 15, 2021

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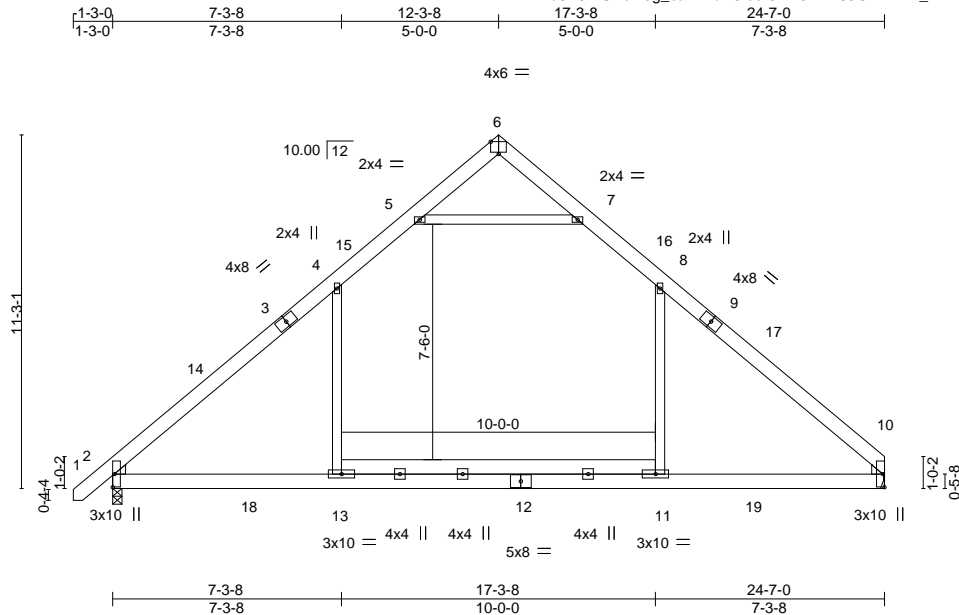


818 Soundside Road
 Edenton, NC 27932

Job J0821-5011	Truss B3	Truss Type COMMON	Qty 11	Ply 1	Lot 3 Walker Rd. E16171324
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:01 2021 Page 1
ID:d6E6lizSYcm5g_caniVuiZ8loe-JI?A5FR T65OxPZHH_VL7b4kENJri0AaAIMygbHydRpS



Scale = 1:69.1

Plate Offsets (X,Y)-- [6:0-3-0,Edge]

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.71	Vert(LL) -0.21 11 >999 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.47	Vert(CT) -0.30 11-13 >981 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.69	Horz(CT) 0.02 10 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.19 10-11 >999 240	Weight: 188 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2
WEDGE
Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-2-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 10=Mechanical
Max Horz 2=264(LC 9)
Max Uplift 2=57(LC 12), 10=-40(LC 13)
Max Grav 2=1324(LC 19), 10=1249(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-1644/234, 4-5=-979/309, 5-6=-74/332, 6-7=-66/333, 7-8=-979/317, 8-10=-1627/230
BOT CHORD 2-13=-1/1094, 11-13=-4/1095, 10-11=-1/1094
WEBS 4-13=0/691, 8-11=0/673, 5-7=-1413/478

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-4 to 3-3-9, Interior(1) 3-3-9 to 12-3-8, Exterior(2) 12-3-8 to 16-8-5, Interior(1) 16-8-5 to 24-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.



September 15, 2021

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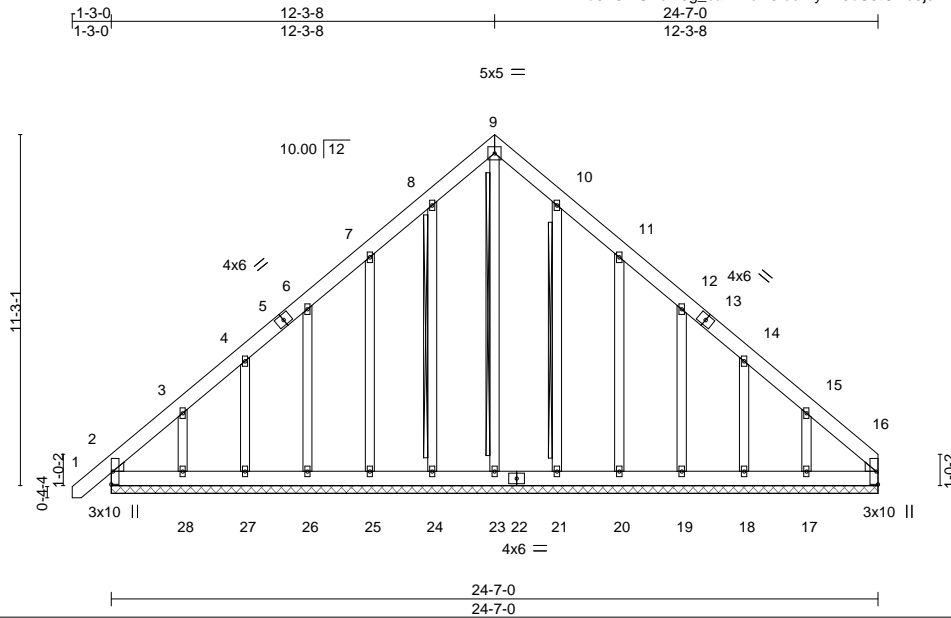


818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss B3GE	Truss Type COMMON SUPPORTED GAB	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171325
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:02 2021 Page 1
ID:d6E6lizSYcm5g_caniVuiZ8loe-nyZYJbS5tOXo0jsTYCsM7lGaSjklI7KX0iDjydRpR



Scale = 1:69.5

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.05	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.03	Vert(LL) -0.00 1 n/r 120		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.16	Vert(CT) -0.00 1 n/r 120		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 16 n/a n/a		
	Code IRC2015/TPI2014			Weight: 233 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
OTHERS 2x4 SP No.2
WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2x4 SPF No.2 - 9-23, 8-24, 10-21
Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.
Brace must cover 90% of web length.

REACTIONS.

All bearings 24-7-0.
(lb) - Max Horz 2=330(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 24, 21, 16 except 2=-102(LC 8), 25=-124(LC 12), 26=-111(LC 12), 27=-107(LC 12), 28=-190(LC 12), 20=-127(LC 13), 19=-111(LC 13), 18=-103(LC 13), 17=-197(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 23, 24, 25, 26, 27, 28, 21, 20, 19, 18, 17, 16 except 2=265(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-400/263, 15-16=-346/224
BOT CHORD 2-28=-183/286, 27-28=-183/286, 26-27=-183/286, 25-26=-183/286, 24-25=-183/286, 23-24=-183/286, 21-23=-183/286, 20-21=-183/286, 19-20=-183/286, 18-19=-183/286, 17-18=-183/286, 16-17=-183/286

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 21, 16 except (jt=lb) 2=102, 25=124, 26=111, 27=107, 28=190, 20=127, 19=111, 18=103, 17=197.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



September 15, 2021

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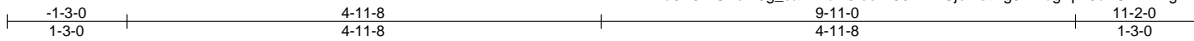


818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 3 Walker Rd.	E16171326
J0821-5011	C1	Common	4	1		

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:03 2021 Page 1
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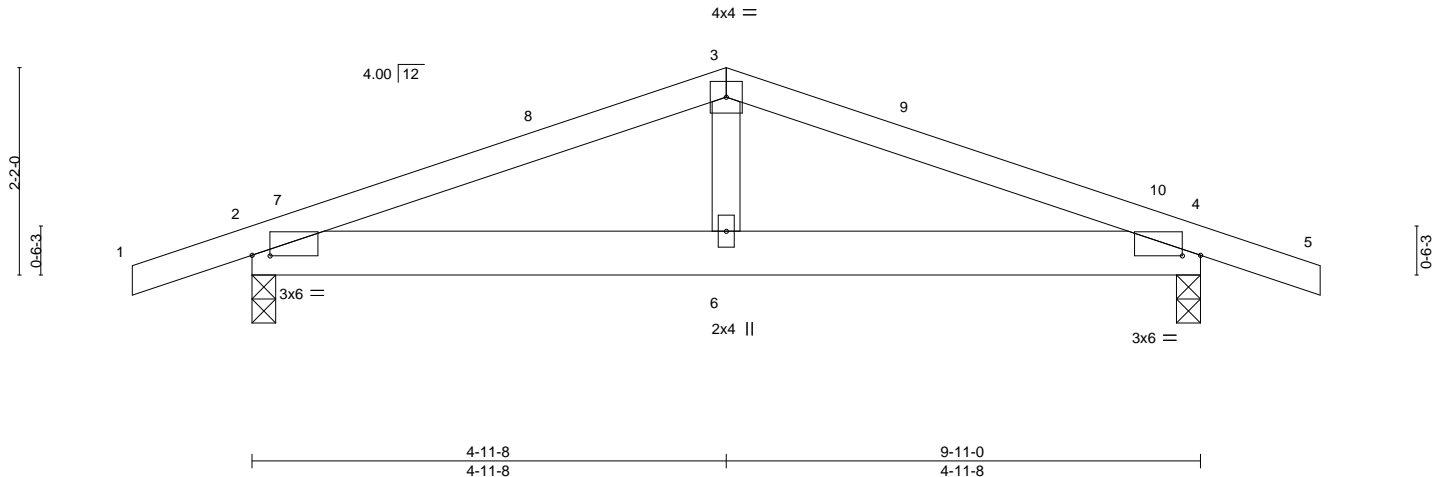


Plate Offsets (X,Y)-- [2:0-2-4,0-0-1], [4:0-2-4,0-0-1]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.21	Vert(LL) -0.01	6	>999	360	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.12	Vert(CT) -0.02	6	>999	240		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.05	Horz(CT) 0.00	4	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Wind(LL) 0.02	4-6	>999	240	Weight: 45 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-
 TOP CHORD 2x4 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 9-10-1 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-3-0
 Max Horz 2=-25(LC 13)
 Max Uplift 2=-191(LC 8), 4=-191(LC 9)
 Max Grav 2=469(LC 1), 4=469(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-638/730, 3-4=-638/730
 BOT CHORD 2-6=-601/548, 4-6=-601/548
 WEBS 3-6=-303/229

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 4-11-8, Exterior(2) 4-11-8 to 9-4-5, Interior(1) 9-4-5 to 11-2-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=191, 4=191.



September 15, 2021

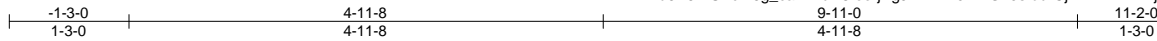
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
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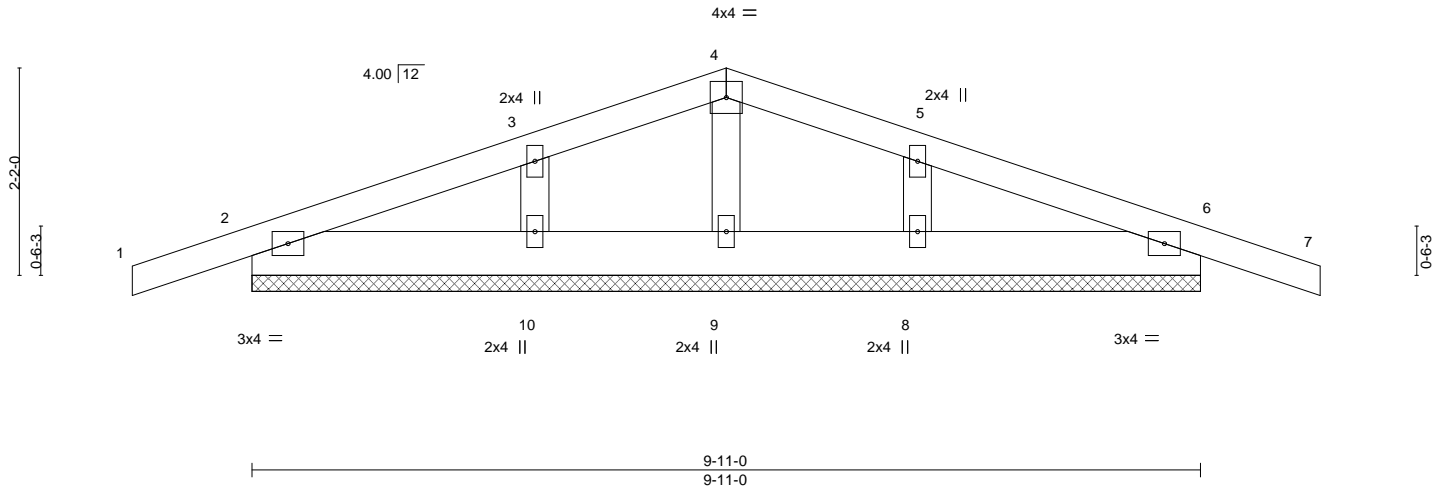
Job J0821-5011	Truss C1GE	Truss Type COMMON SUPPORTED GAB	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171327
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:04 2021 Page 1
ID:d6E6lizSYcm5g_caniVuiZ8l0e-jKgJkHTLP0nWG10sfducJmVvWzNDhjd_KBKncydRpP



Scale = 1:22.7



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.07	Vert(LL) -0.00 7 n/r 120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT) -0.00 7 n/r 120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT) 0.00 6 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 47 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x6 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 9-11-0.
(lb) - Max Horz 2=-42(LC 13)
Max Uplift All uplift 100 lb or less at joint(s) 9, 10, 8 except 2=-103(LC 8), 6=-107(LC 9)
Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 10, 8 except (jt=lb) 2=103, 6=107.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.



September 15, 2021

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818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss M1GE	Truss Type GABLE	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171329
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:07 2021 Page 1
ID:d6E6lizSYcm5g_caniIvuz8loe-7vMRMIVDhx947UkRkMRYqL_PpkzDQ2b3hHP_OxydRpM

-1-3-0
1-3-0

6-0-0
6-0-0

Scale = 1:14.0

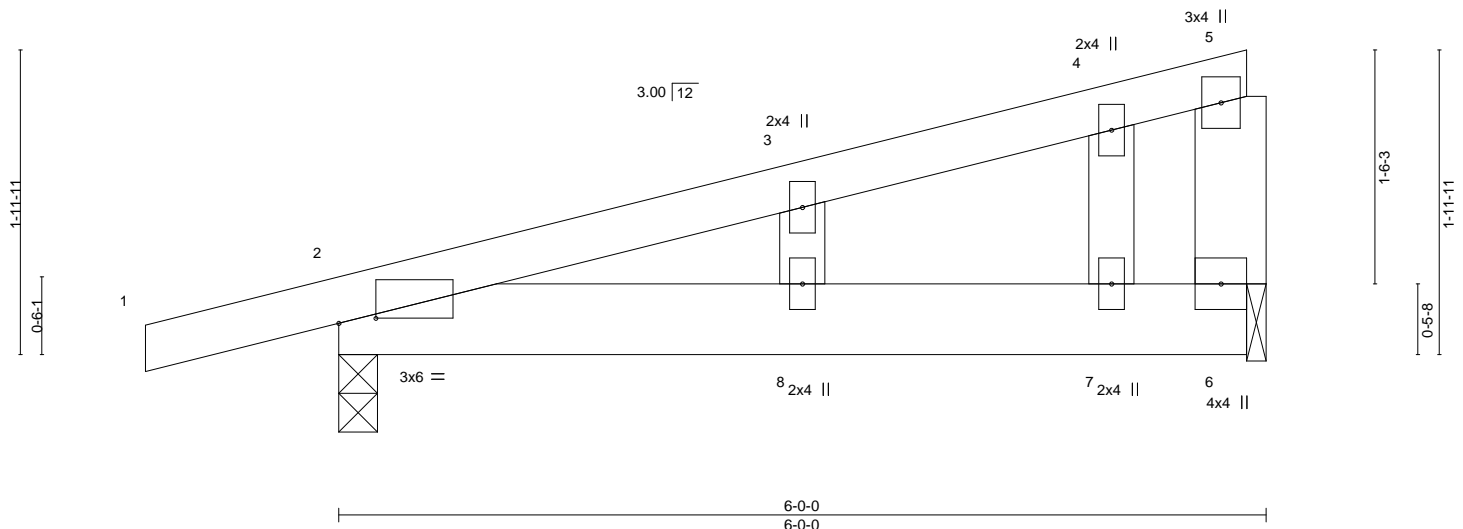


Plate Offsets (X,Y)-- [2:0-2-14,0-0-6]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.13	Vert(LL) 0.03	8	>999	240	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.14	Vert(CT) -0.02	8	>999	240		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.02	Horz(CT) -0.00	6	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S					Weight: 30 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x6 SP No.1
OTHERS 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8
Max Horz 2=85(LC 8)
Max Uplift 2=195(LC 8), 6=129(LC 8)
Max Grav 2=318(LC 1), 6=216(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 3) Gable studs spaced at 2-0-0 oc.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=195, 6=129.



September 15,2021

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Walker Rd.	E16171330
J0821-5011	M2	Monopitch	14	1	Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

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ID:d6E6lizSYcm5g_caniIvuiZ8loe-b6wpZeWsSEHxkeJduTznNZWbD7L49V6Cvx9XwNydRpL



Scale = 1:9.6

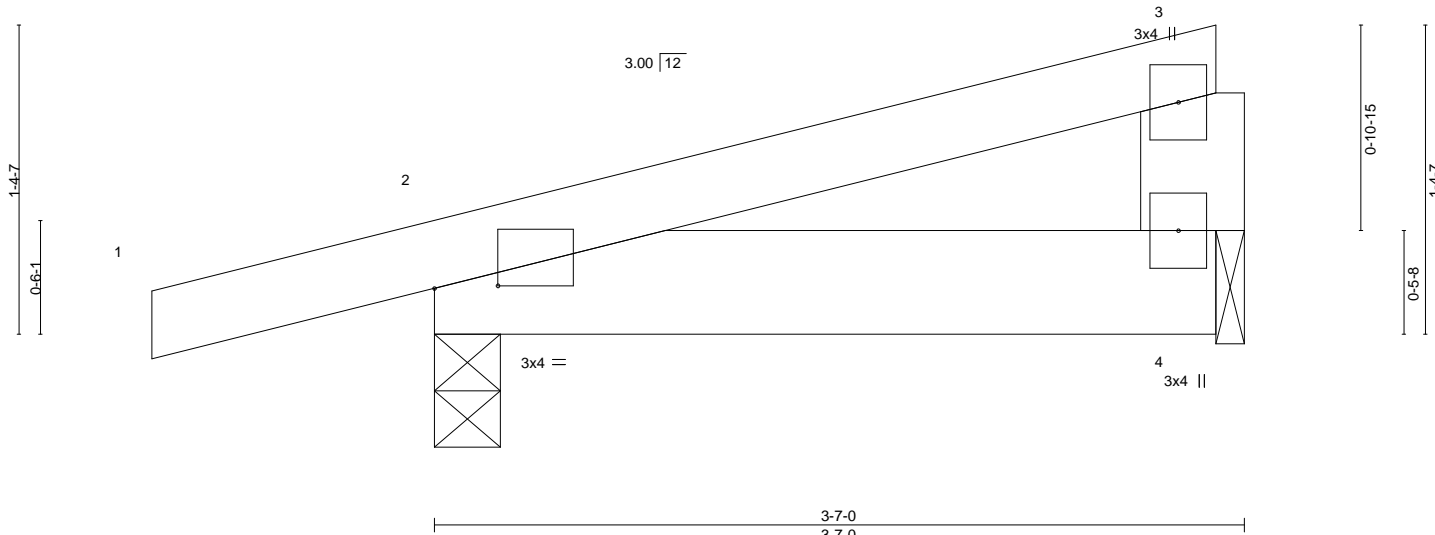


Plate Offsets (X,Y)-- [2:0-3-6,0-0-2]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.09	Vert(LL)	-0.00	2-4	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.04	Vert(CT)	-0.00	2-4	>999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.00	Horz(CT)	0.00	n/a	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Wind(LL)	0.00	2-4	>999	Weight: 17 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x6 SP No.1

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 4=0-1-8, 2=0-3-8
 Max Horz 2=57(LC 8)
 Max Uplift 4=-63(LC 8), 2=-152(LC 8)
 Max Grav 4=110(LC 1), 2=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=152.



September 15, 2021

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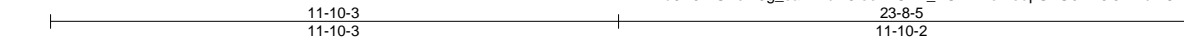


818 Soundside Road
 Edenton, NC 27932

Job J0821-5011	Truss V1	Truss Type VALLEY	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171331
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:09 2021 Page 1
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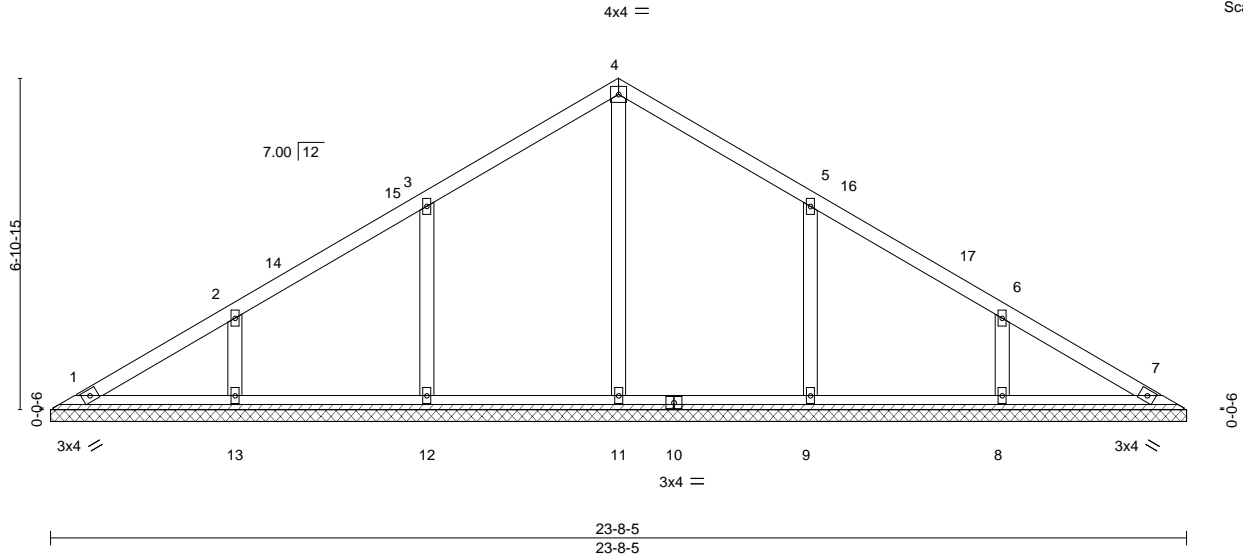


Plate Offsets (X,Y)-- [5:0-0-0,0-0-0], [6:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.19	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.14	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 7 n/a n/a		
	Code IRC2015/TPI2014			Weight: 100 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 23-8-5.
(lb) - Max Horz 1=158(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 12, 13, 9, 8
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=459(LC 19), 12=445(LC 19), 13=327(LC 19), 9=444(LC 20), 8=327(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-12=-294/186, 2-13=-271/174, 5-9=-294/186, 6-8=-271/174

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-6-12 to 4-11-9, Interior(1) 4-11-9 to 11-10-3, Exterior(2) 11-10-3 to 16-2-15, Interior(1) 16-2-15 to 23-1-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 12, 13, 9, 8.



September 15, 2021

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

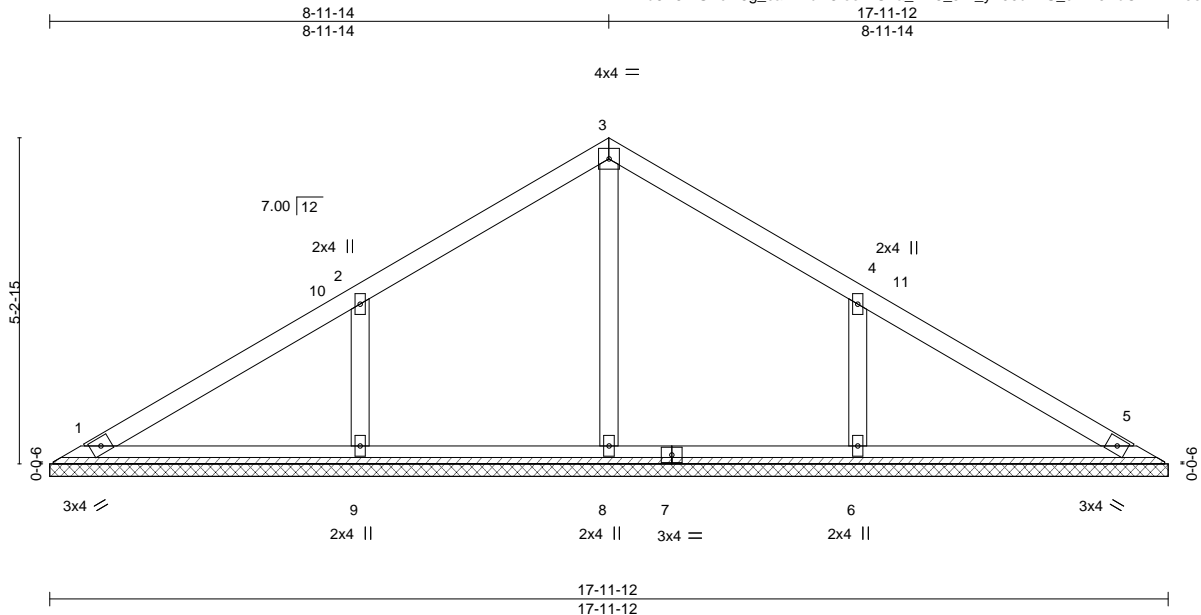


818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss V2	Truss Type VALLEY	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171332
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:10 2021 Page 1
ID:d6E6lizSYcm5g_caniilVuiZ8loe-YU2a_KY6_sXf_yT00u?FS_cv4x0KdOwVNFee?GydRpJ



Scale = 1:34.9

Plate Offsets (X,Y)-- [4:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.20	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.11	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.07	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S					Weight: 70 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 17-11-12.
(lb) - Max Horz 1=118(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=104(LC 12), 6=104(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 8 except 9=430(LC 19), 6=429(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-9=-342/208, 4-6=-342/208

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-6-12 to 4-11-14, Interior(1) 4-11-14 to 8-11-14, Exterior(2) 8-11-14 to 13-4-11, Interior(1) 13-4-11 to 17-5-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=104, 6=104.



September 15, 2021

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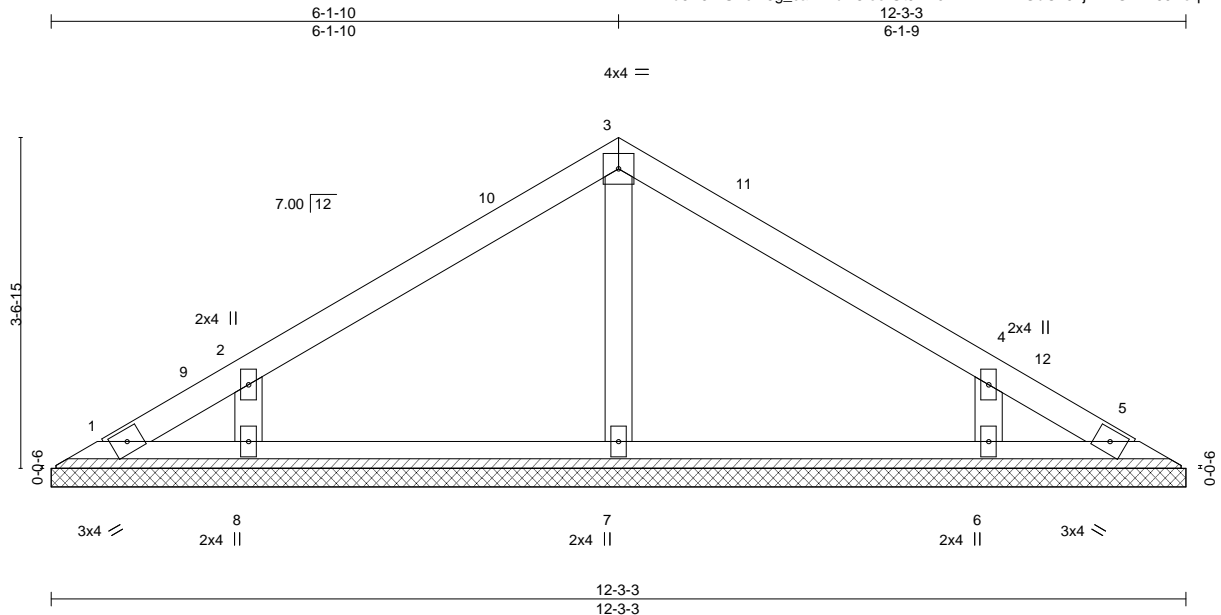


818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss V3	Truss Type VALLEY	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171333
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:12 2021 Page 1
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Scale = 1:23.4

Plate Offsets (X,Y)-- [4:0-0-0,0-0-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.13	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.09	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.04	Horz(CT)	0.00	5	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S					Weight: 44 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 12-3-3.
(lb) - Max Horz 1=78(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=275(LC 1), 8=309(LC 19), 6=309(LC 20)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-264/186, 4-6=-264/186

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-6-12 to 4-11-9, Interior(1) 4-11-9 to 6-1-10, Exterior(2) 6-1-10 to 10-6-6, Interior(1) 10-6-6 to 11-8-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.



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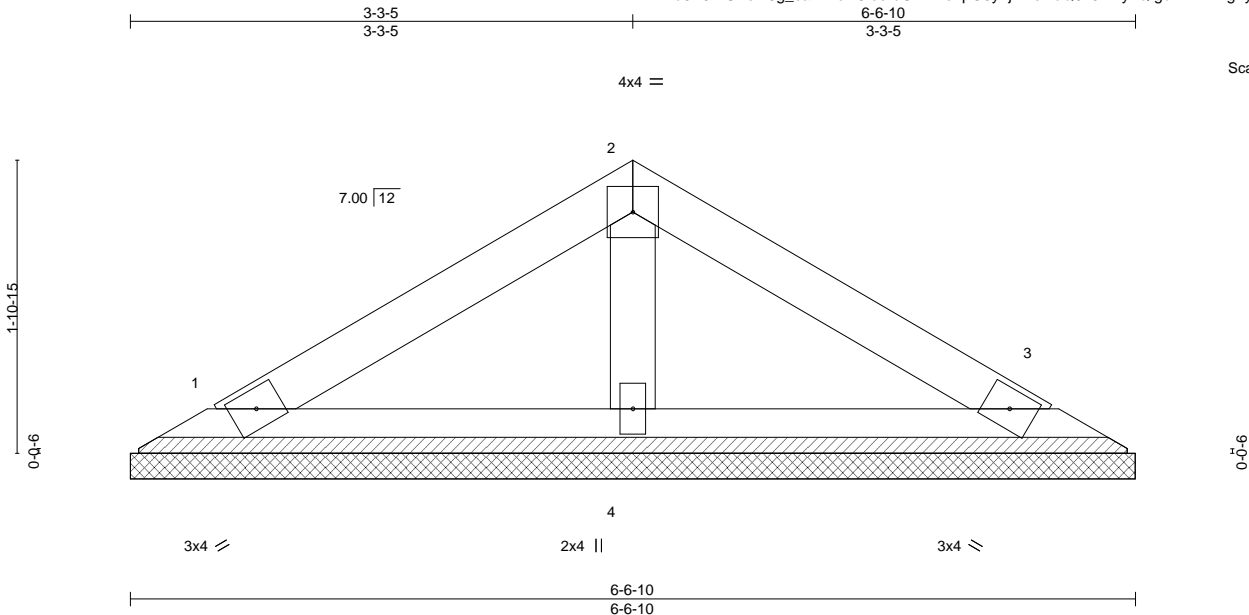
818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss V4	Truss Type VALLEY	Qty 1	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171334
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:15 2021 Page 1

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Scale = 1:14.1

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.10	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.05	Vert(LL) n/a - n/a 999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.02	Vert(CT) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
	Code IRC2015/TPI2014			Weight: 21 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=6-6-10, 3=6-6-10, 4=6-6-10
Max Horz 1=38(LC 9)
Max Uplift 1=18(LC 12), 3=22(LC 13)
Max Grav 1=114(LC 1), 3=114(LC 1), 4=206(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



September 15, 2021

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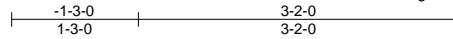


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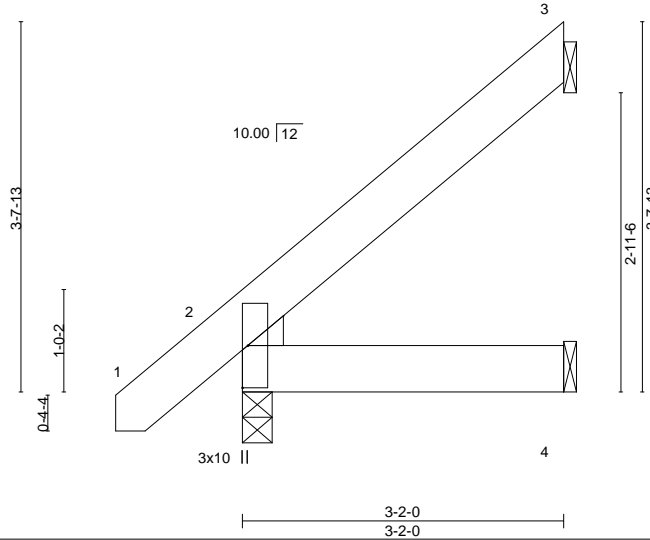
Job J0821-5011	Truss X1	Truss Type JACK-OPEN	Qty 10	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171335
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Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:15 2021 Page 1
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Scale = 1:21.4



LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.07	Vert(LL)	-0.00	2-4	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.03	Vert(CT)	-0.00	2-4	>999		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.00	Horz(CT)	-0.00	3	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Wind(LL)	0.00	2	****	Weight: 22 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEDGE
Left: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical
Max Horz 2=111(LC 12)
Max Uplift 3=78(LC 12)
Max Grav 3=93(LC 19), 2=209(LC 1), 4=59(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.



September 15, 2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

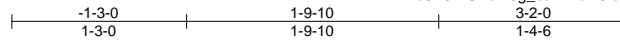


818 Soundside Road
Edenton, NC 27932

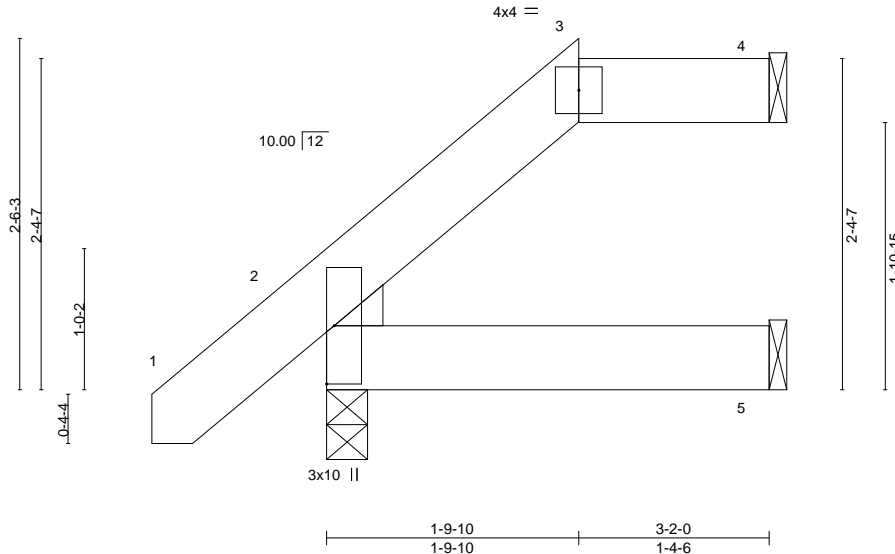
Job J0821-5011	Truss X2	Truss Type JACK-OPEN	Qty 2	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171336
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Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:16 2021 Page 1
ID:d6E6lizSYcm5g_caniIvuiZ8loe-MePrFNctaiHpitw9M96fiFszM40175OIB5yDvydRpD



Scale = 1:15.5



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.04	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.03	Vert(LL) -0.00 2 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.00	Vert(CT) -0.00 2-5 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 4 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) -0.00 2-5 >999 240	Weight: 21 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEDGE
Left: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-2-0 oc purlins, except 2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical
Max Horz 2=74(LC 12)
Max Uplift 4=30(LC 9), 2=-11(LC 12)
Max Grav 4=73(LC 1), 2=209(LC 1), 5=54(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



September 15, 2021

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818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss X3	Truss Type JACK-OPEN GIRDER	Qty 2	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171337
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Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:17 2021 Page 1
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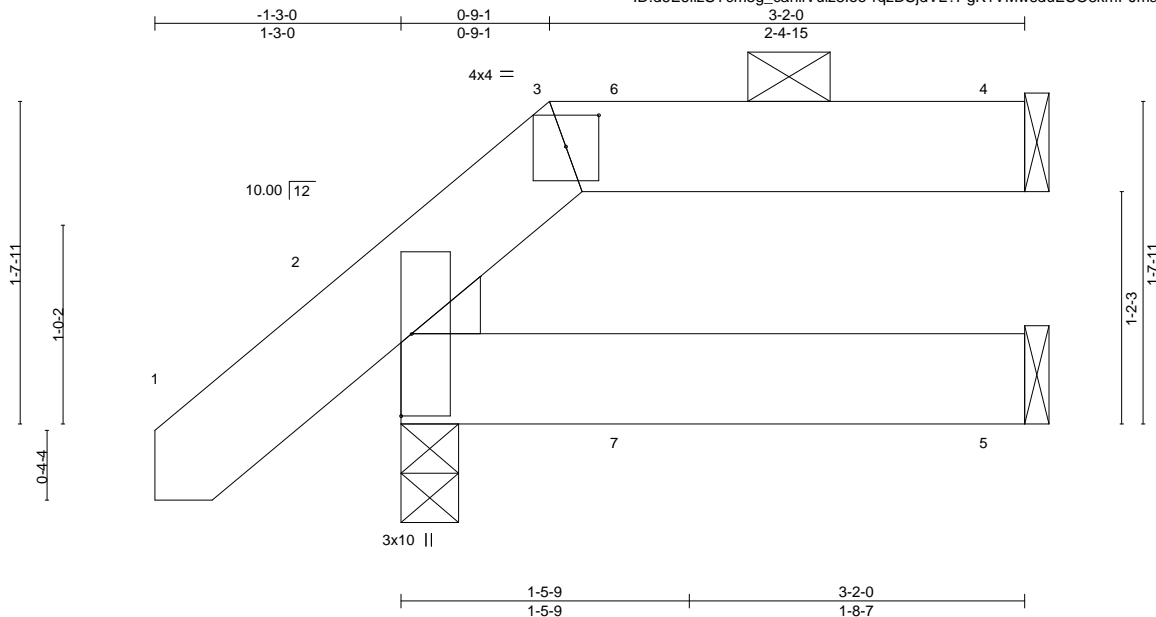


Plate Offsets (X,Y)-- [3:0-2-0,0-1-15]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0 Plate Grip DOL 1.15	TC 0.04	Vert(LL) -0.00	2	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) -0.00	2-5	>999	240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.00	Horz(CT) 0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.00	2	>999	240	Weight: 20 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEDGE
Left: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 3-2-0 oc purlins, except 2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical
Max Horz 2=50(LC 8)
Max Uplift 4=27(LC 5), 2=-19(LC 8)
Max Grav 4=77(LC 20), 2=209(LC 1), 5=54(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 54 lb down and 21 lb up at 1-2-12 on top chord, and 4 lb down at 1-2-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-60, 3-4=-60, 2-5=-20



September 15, 2021

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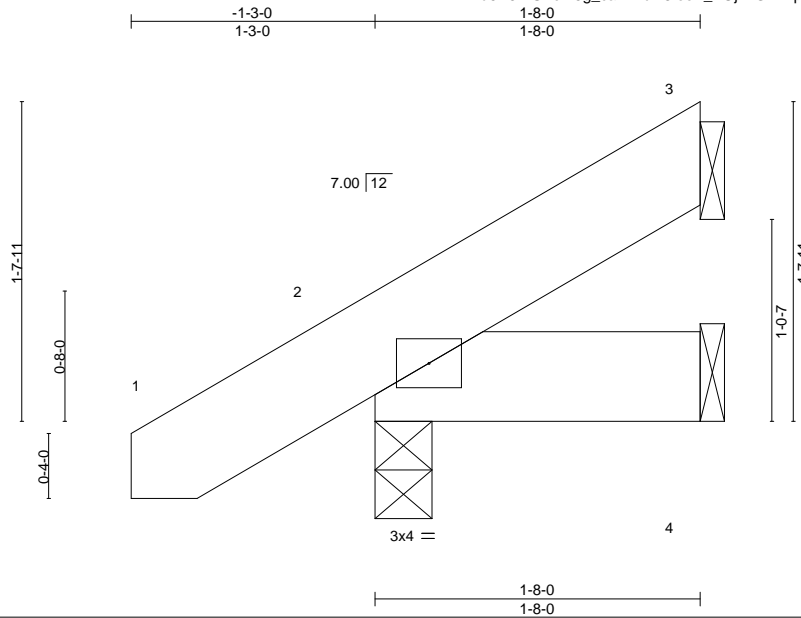


818 Soundside Road
Edenton, NC 27932

Job J0821-5011	Truss Y1	Truss Type JACK-OPEN	Qty 2	Ply 1	Lot 3 Walker Rd. Job Reference (optional)	E16171338
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Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Sep 15 08:12:23 2021 Page 1
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Scale = 1:11.1

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.03	Vert(LL) -0.00 2 >999 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.01	Vert(CT) -0.00 2 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.00 2 **** 240	Weight: 12 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1

BRACING-

TOP CHORD Structural wood sheathing directly applied or 1-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical
Max Horz 2=49(LC 12)
Max Uplift 3=23(LC 12), 2=-16(LC 12)
Max Grav 3=32(LC 19), 2=154(LC 1), 4=33(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



September 15,2021

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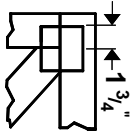
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



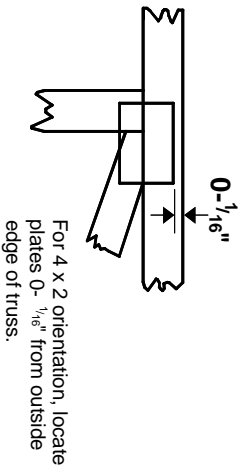
818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20** software or upon request.

PLATE SIZE

4 X 4

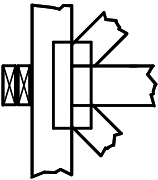
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



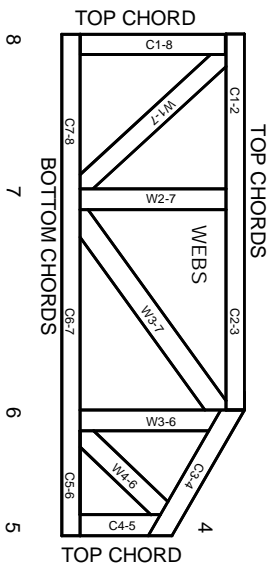
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/ITP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing, Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

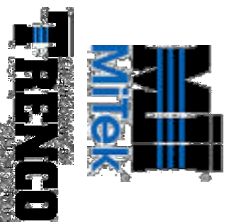
ICC-ES Reports:

ESR-1311, ESR-1352, ESR 1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPP 1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: Mill-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPP 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPP 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPP 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

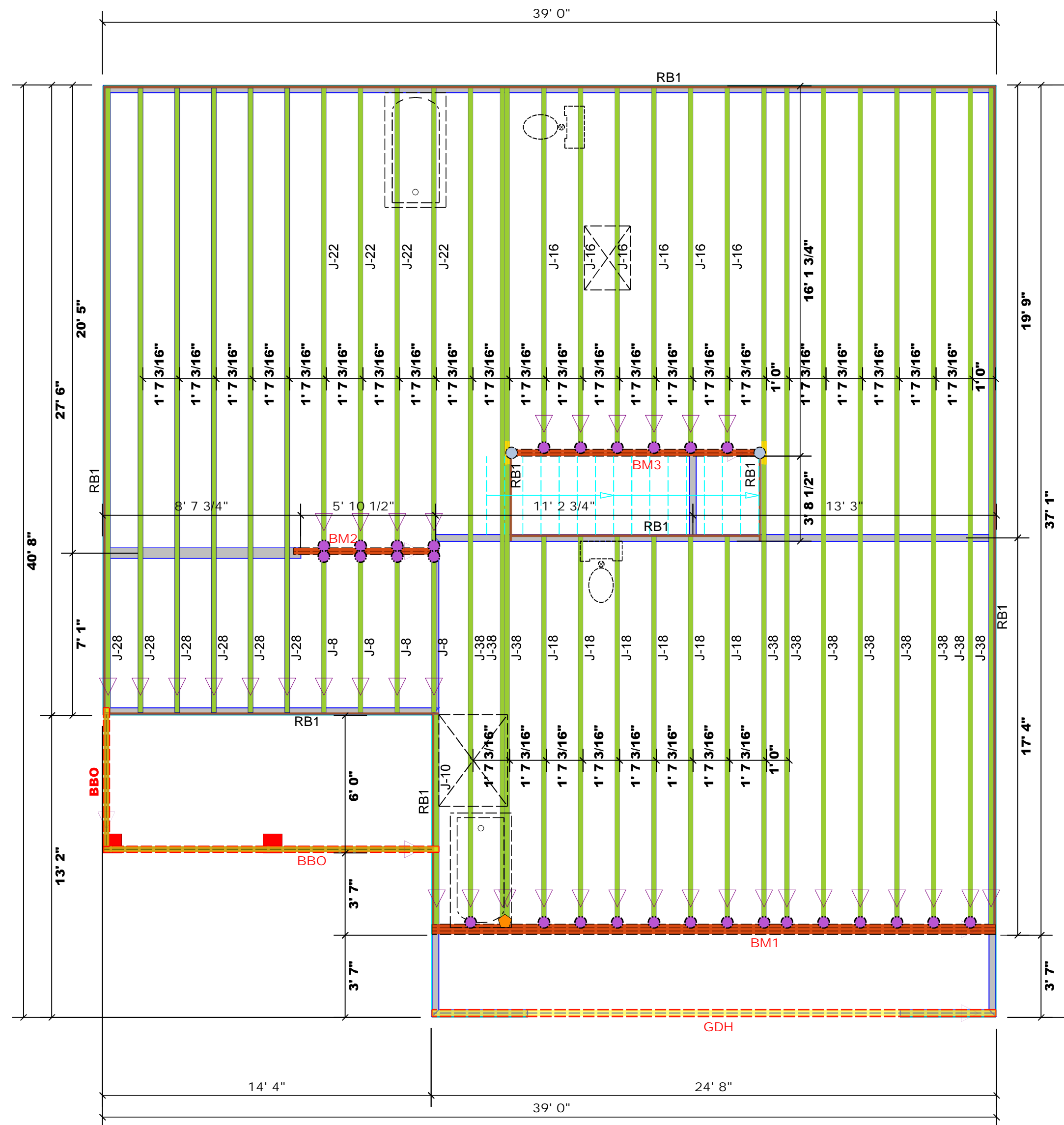
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature David Landry
David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES MODEL: S 03)

REAR REACTION (LBS)	NUMBER OF JACK STUDS REQUIRED PER EACH END OF HEADERS/BEAMS	REAR REACTION (LBS)	
		10 FT	15 FT
1700	1	2550	3400
3400	2	5100	6800
5100	3	7650	10200
6800	4	10200	13600
8500	5	12750	17000
10200	6	15300	
11900	7		
13600	8		
15300	9		



Dimension Notes

- All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
- All interior wall dimensions are to face of frame wall unless noted otherwise
- All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

- Plumbing drop locations shown are NOT exact.
- Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
- Adjust spacing as needed not to exceed 24"oc.

Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
●	THF25140	USP	28	NA	10d/3"	10d/3"
▲	THF25140-2	USP	1	NA	10d/3"	10d/3"
○	MSH422	USP	2	Varies	10d/3"	10d/3"

Products				
PlotID	Length	Product	Plies	Net Qty
BM1	25' 0"	1-3/4"x 23-7/8" LVL Kerto-S	3	3
BM2	7' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	11' 0"	1-3/4"x 14" LVL Kerto-S	2	2
GDH	25' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
J-8	8' 0"	14" NI-40x	1	4
J-10	10' 0"	14" NI-40x	1	1
J-16	16' 0"	14" NI-40x	1	6
J-18	18' 0"	14" NI-40x	1	6
J-22	22' 0"	14" NI-40x	1	4
J-28	28' 0"	14" NI-40x	1	6
J-38	38' 0"	14" NI-40x	1	11
RB1	12' 0"	1 1/8" x 14" Rim Board	1	13

1 Truss Placement Plan
Scale: 1/4"=1'

BUILDER	BENEFITARY	CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
	Ben Stout Real Estate	Linden / Harnett	724 Walker Road	2nd Floor I - Joist	03/18/22	David Landry	Marshall Naylor
JOB NAME	Lot 3 Walker Rd.	ADDRESS	724 Walker Road	MODEL	DATE REV.	DRAWN BY	SALES REP.
PLAN	Westbrook	ADDRESS	724 Walker Road	MODEL	DATE REV.	DRAWN BY	SALES REP.
SEAL DATE	N/A	ADDRESS	724 Walker Road	MODEL	DATE REV.	DRAWN BY	SALES REP.
QUOTE #		ADDRESS	724 Walker Road	MODEL	DATE REV.	DRAWN BY	SALES REP.
JOB #	J0821-5012	ADDRESS	724 Walker Road	MODEL	DATE REV.	DRAWN BY	SALES REP.

▲ = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

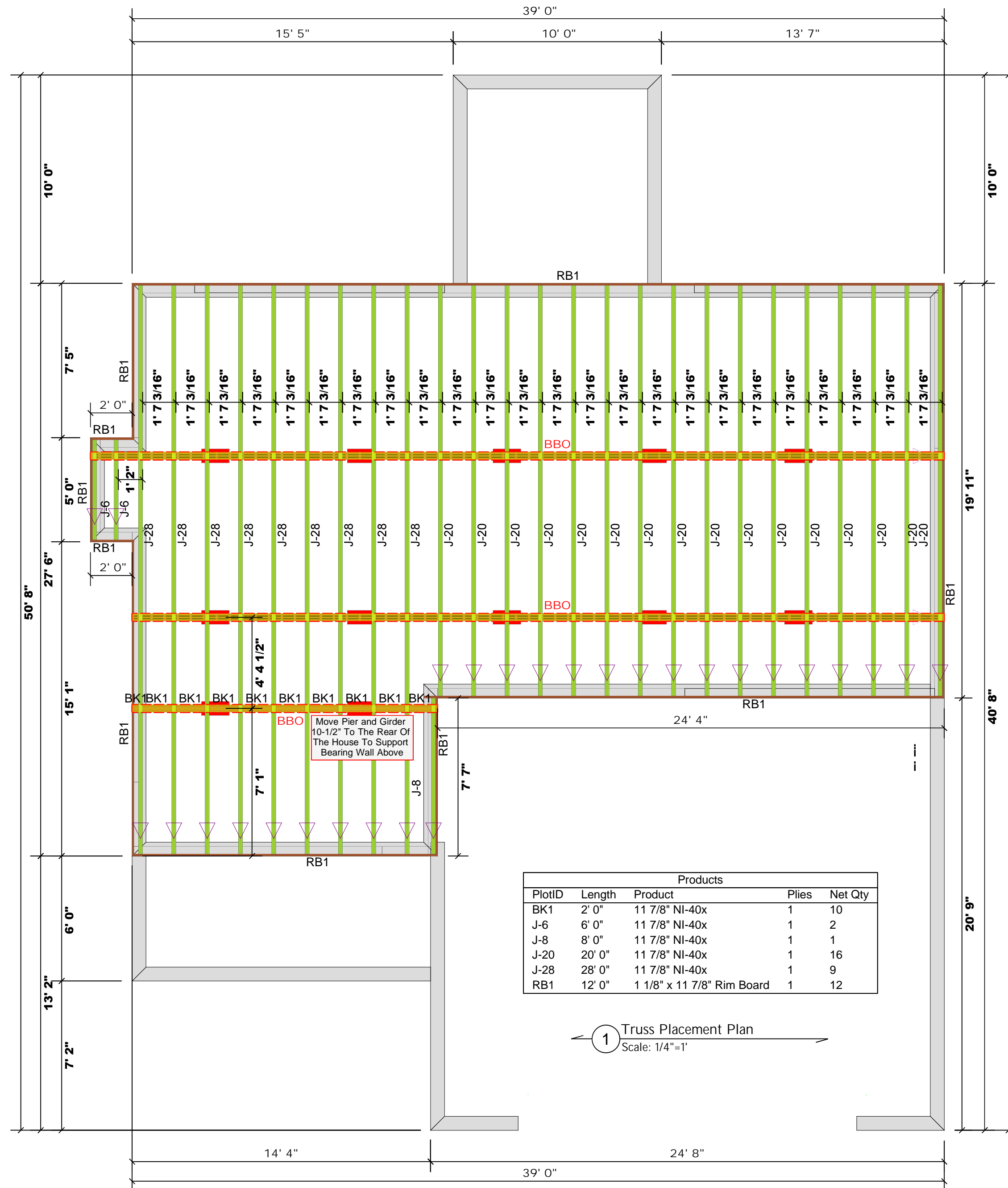
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature David Landry
David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES MODEL: S 103)

EVALUATION (LBS/FT)	NUMBER OF JACK STUDS REQUIRED PER BEARING REACTION	REAR REACTION (LBS/FT)		FRONT REACTION (LBS/FT)	
		REQ'D STUDS PER BEARING REACTION	REQ'D STUDS PER BEARING REACTION	REQ'D STUDS PER BEARING REACTION	REQ'D STUDS PER BEARING REACTION
1700	1	2550	1	3400	1
2400	2	5100	2	6800	2
3100	3	7650	3	10200	3
3800	4	10200	4	13600	4
4500	5	12750	5	17000	5
5200	6	15300	6		
5900	7				
6600	8				
7300	9				



PlotID	Length	Product	Plies	Net Qty
BK1	2' 0"	11 7/8" NI-40x	1	10
J-6	6' 0"	11 7/8" NI-40x	1	2
J-8	8' 0"	11 7/8" NI-40x	1	1
J-20	20' 0"	11 7/8" NI-40x	1	16
J-28	28' 0"	11 7/8" NI-40x	1	9
RB1	12' 0"	1 1/8" x 11 7/8" Rim Board	1	12

1 Truss Placement Plan
Scale: 1/4"=1'

BUILDER	BEN STOUT REAL ESTATE	CITY / CO.	LINDEN / HARNETT
JOB NAME	Lot 3 Walker Rd.	ADDRESS	724 Walker Road
PLAN	Westbrook	MODEL	I - Joists Over Crawl
SEAL DATE	N/A	DATE REV.	03/18/22
QUOTE #		DRAWN BY	David Landry
JOB #	J0322-1393	SALES REP.	Marshall Naylor

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

▲ = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards