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Marshall Naylor	David Landry	03/18/22	Roof	754 Walker Road	
SALESMAN	DRAWN BY	DATE REV.	MODEL	ADDRESS	
J0821-5013		N/A	The Fawnbrook	Lot 1 Walker Rd.	
JOB #	QUOTE #	SEAL DATE	PLAN	JOB NAME	

		Cor
-	Sym	Product
		HUS26

support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



RE: J0821-5013 Lot 1 Walker Rd. **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Benjamin Stout Real Estate Lot/Block: 1	Project Name: J0821-5013 Model: Fawnbrook
Address: 754 Walker Road	Subdivision: Walker Rd.
City: Linden	State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 20 individual, dated Truss Design Drawings and 0 Additional Drawings.

	A 14		
No.	Seal#	Truss Name	Date
1	E16495456	A1	12/22/2021
2	E16495457	A1GE	12/22/2021
3	E16495458	A2	12/22/2021
4	E16495459	A3	12/22/2021
5	E16495460	A3A	12/22/2021
6	E16495461	A3GE	12/22/2021
7	E16495462	B1	12/22/2021
8	E16495463	B1-GR	12/22/2021
9	E16495464	B1GE	12/22/2021
10	E16495465	C1	12/22/2021
11	E16495466	C1-GR	12/22/2021
12	E16495467	C1GE	12/22/2021
13	E16495468	D1	12/22/2021
14	E16495469	D1GE	12/22/2021
15	E16495470	M1	12/22/2021
16	E16495471	M1GE	12/22/2021
17	E16495472	V1	12/22/2021
18	E16495473	V2	12/22/2021
19	E16495474	V3	12/22/2021
20	E16495475	V4	12/22/2021

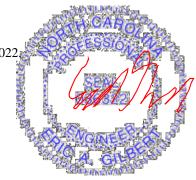
The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

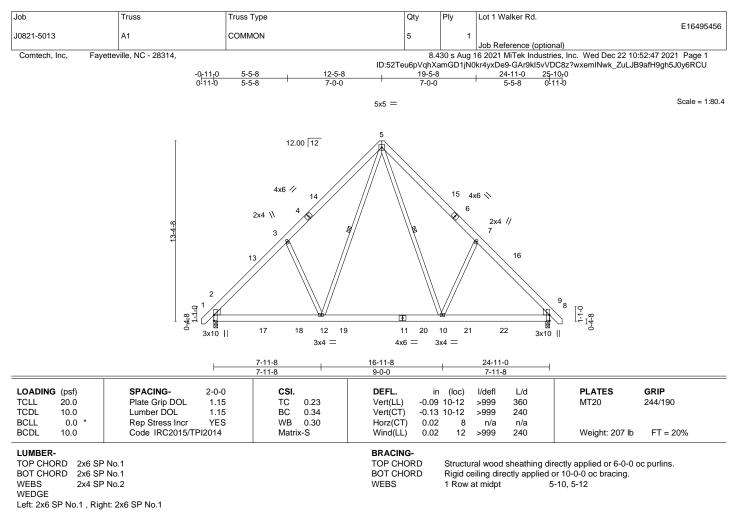
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



December 22, 2021



REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-317(LC 10) Max Uplift 2=-41(LC 12), 8=-41(LC 13) Max Grav 2=1185(LC 19), 8=1185(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1356/272, 3-5=-1240/456, 5-7=-1240/456, 7-8=-1356/272 TOP CHORD

2-12=-123/1040, 10-12=-10/672, 8-10=-51/884 BOT CHORD

5-10=-217/739, 7-10=-443/340, 5-12=-217/738, 3-12=-443/340 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 12-5-8, Exterior(2) 12-5-8 to 16-10-5, Interior(1) 16-10-5 to 25-8-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

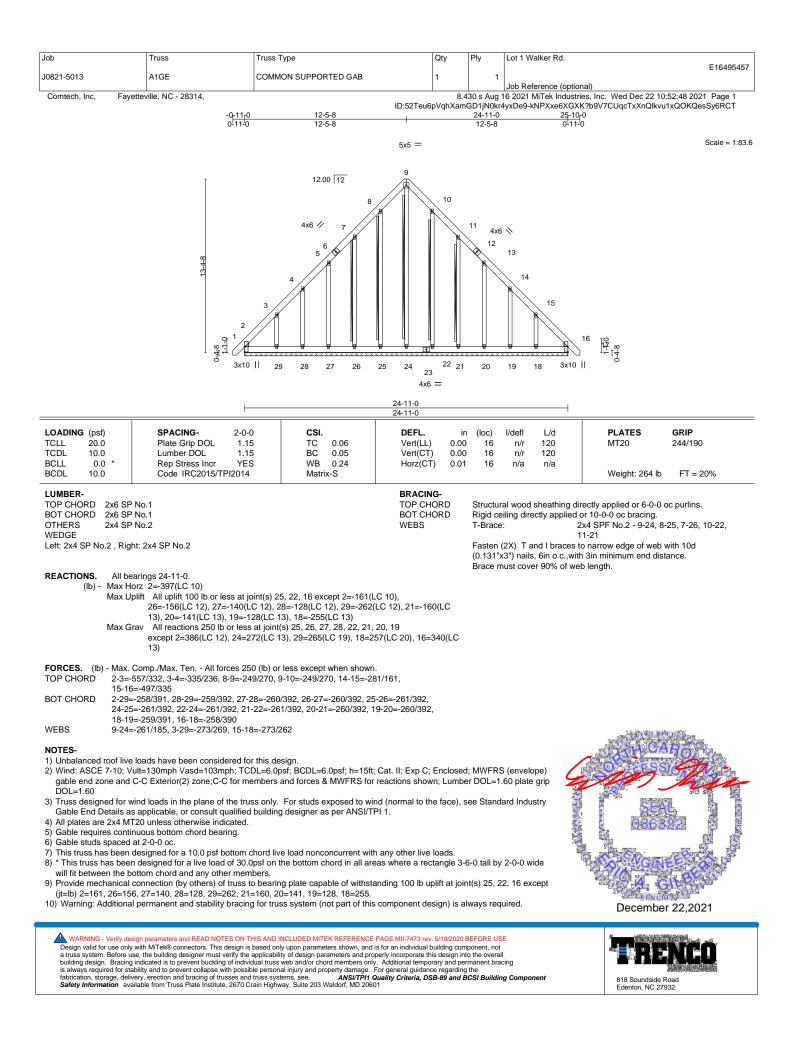
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

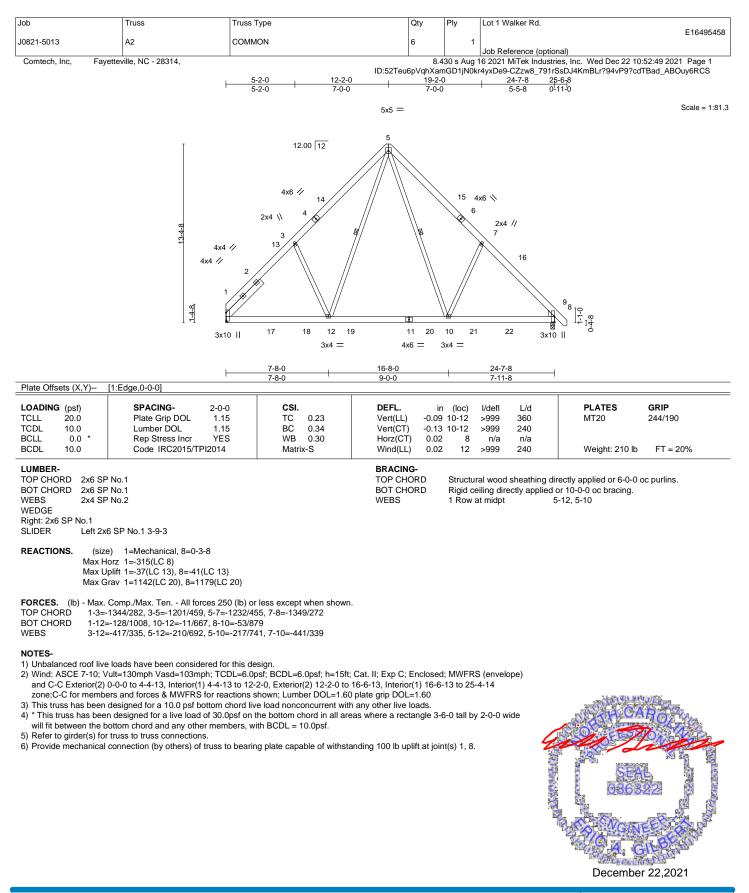
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

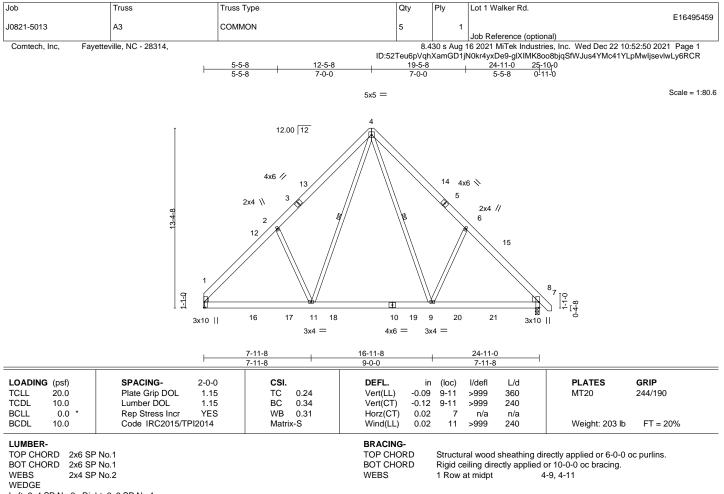






WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Left: 2x4 SP No.2 , Right: 2x6 SP No.1

REACTIONS. (size) 1=Mechanical, 7=0-3-8 Max Horz 1=-315(LC 8) Max Uplift 1=-35(LC 13), 7=-41(LC 13) Max Grav 1=1146(LC 20), 7=1190(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-1367/280, 2-4=-1255/470, 4-6=-1246/458, 6-7=-1362/274 TOP CHORD

BOT CHORD WEBS

1-11=-122/1056, 9-11=-9/677, 7-9=-55/888 4-9=-217/737, 6-9=-443/340, 4-11=-223/756, 2-11=-454/346

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 12-5-8, Exterior(2) 12-5-8 to 16-10-5, Interior(1) 16-10-5 to 25-8-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

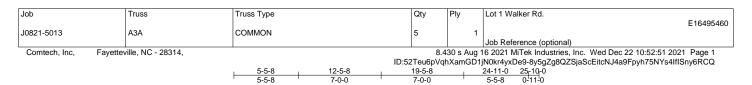
5) Refer to girder(s) for truss to truss connections.

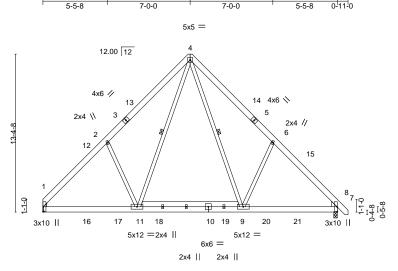
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. 🛕 WARNING - Verify design pa Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







		7-11-8 7-11-8	+ <u>16-11-8</u> 9-0-0	24-1 7-1	11-0 1-8		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0- Plate Grip DOL 1.1: Lumber DOL 1.1: Rep Stress Incr YES Code IRC2015/TPI2014	5 TC 0.24 5 BC 0.34 6 WB 0.3	4 Vert(CT)	in (loc) -0.09 9-11 -0.12 9-11 0.02 7 0.02 11	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 223 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 BOT CHORD 2x4 SP No.2 WFBS WEDGE

BRACING-TOP CHORD BOT CHORD WFBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 4-9, 4-11

Left: 2x4 SP No.2 , Right: 2x6 SP No.1

REACTIONS. (size) 1=Mechanical, 7=0-3-8 Max Horz 1=-315(LC 8) Max Uplift 1=-35(LC 13), 7=-41(LC 13) Max Grav 1=1139(LC 20), 7=1183(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-1357/280, 2-4=-1245/470, 4-6=-1236/458, 6-7=-1352/274 TOP CHORD

BOT CHORD 1-11=-122/1049, 9-11=-9/672, 7-9=-55/881 WEBS

4-9=-217/730, 6-9=-443/340, 4-11=-223/748, 2-11=-454/346

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 12-5-8, Exterior(2) 12-5-8 to 16-10-5, Interior(1) 16-10-5 to 25-8-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

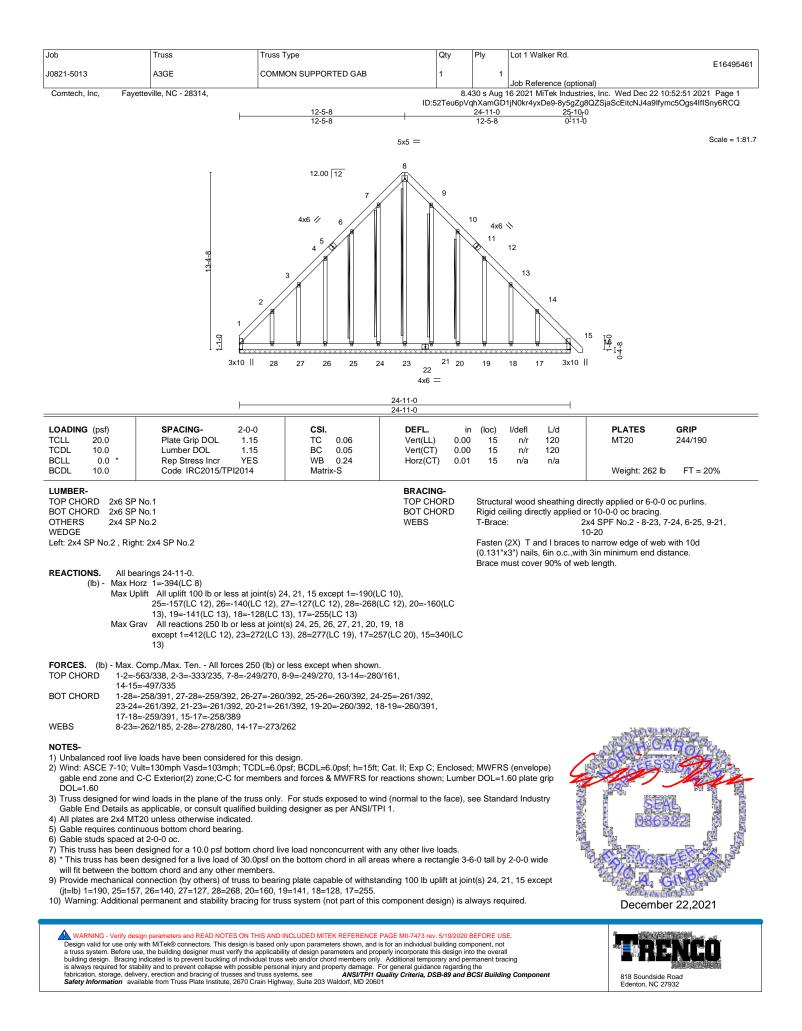
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

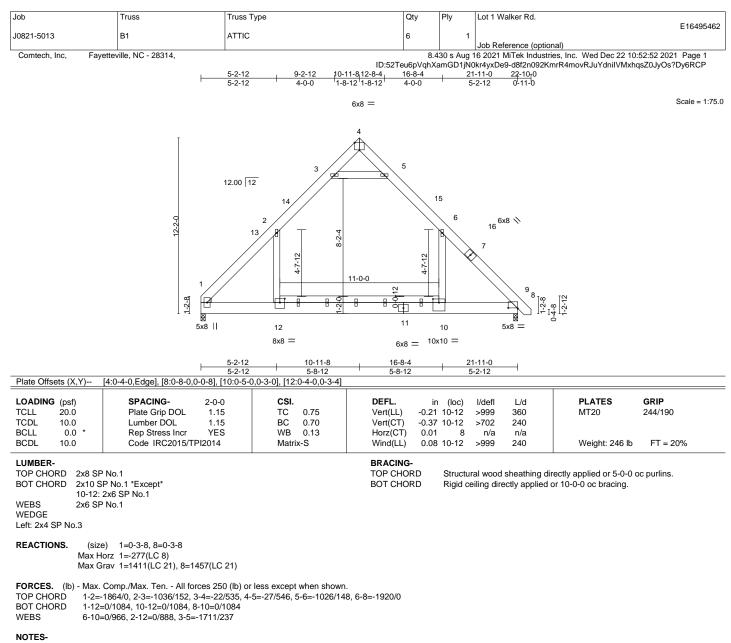


Scale = 1:91.8

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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-6 to 4-5-3, Interior(1) 4-5-3 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-7-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x6 MT20 unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-5; Wall dead load (5.0psf) on member(s).6-10, 2-12

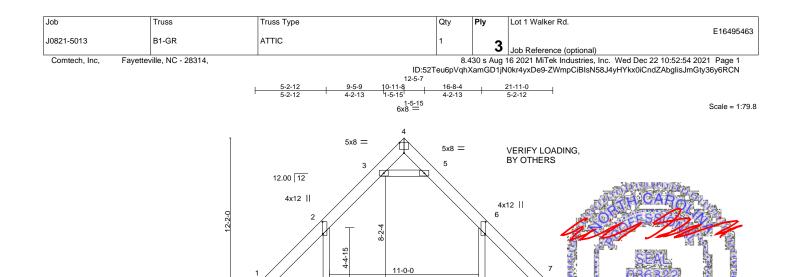
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12

8) Attic room checked for L/360 deflection.



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2x6 || 2x6 || 2x6 ||

10-11-8 5-8-12

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13 14

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2x6 || 2x6 ||

16-8-4 5-8-12

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6x8 = 10x10 =

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21-11-0 5-2-12

1-2-8

∑ 5x8 ∥

11

5-2-12 5-2-12 12

10

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10x10 =

Plate Offsets (X,Y)	[1:0-2-3,0-3-8], [2:0-10-12,0-0-8], [4:0-4	0,Edge], [6:0-10-12,0-0-8	B], [7:Edge,0-3-0], [8:0-4	-8,0-2-0]	, [10:0-4-8	,0-2-0]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.78 BC 0.79 WB 0.40 Matrix-S	Vert(LL) -0.31 Vert(CT) -0.42 Horz(CT) 0.02	n (loc) 8-10 2 8-10 2 7 8-10	l/defl >835 >614 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 801 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x10 SI	P 2400F 2.0E P No.1 *Except* x6 SP No.1 No.1		BRACING- TOP CHORD BOT CHORD				rectly applied or 6-0-0 o or 10-0-0 oc bracing.	oc purlins.
Max Ho	e) 1=0-4-0, 7=0-4-0 orz 1=-269(LC 4) rav 1=10108(LC 14), 7=10091(LC 14)							
TOP CHORD 1-2=- BOT CHORD 1-10=	Comp./Max. Ten All forces 250 (lb) or 10697/0, 2-3=-4448/35, 3-4=0/4719, 4-5 -0/5996, 8-10=0/6065, 7-8=0/5996 //8663, 2-10=0/8686, 3-5=-12734/0							
Top chords connecte Bottom chords conner Webs connected as 2) All loads are conside ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; V Lumber DOL=1.60 p 5) Concentrated loads I MWFRS Wind (Pos. Left; #7 Dead + 0.6 N MWFRS Wind (Pos. Left; #7 Dead + 0.6 N MWFRS Wind (Pos. Left; #20 De Live (bal.) + 0.75 Atti	nected together with 10d (0.131"x3") na ad as follows: 2x10 - 2 rows staggered a ected as follows: 2x10 - 5 rows staggered follows: 2x6 - 2 rows staggered at 0-9-0 red equally applied to all plies, except ii a been provided to distribute only loads loads have been considered for this de ult=130mph Vasd=103mph; TCDL=6.0p late grip DOL=1.60 from layout are not present in Load Cas Internal) Left; #5 Dead + 0.6 MWFRS W WWFRS Wind (Neg. Internal) Right; #8 Internal) 2nd Parallel; #10 Dead + 0.6 N arallel; #12 Dead + 0.6 MWFRS Wind (f ad + 0.75 Roof Live (bal.) + 0.75 Attic F ic Floor + 0.75(0.6 MWFRS Wind (Neg. ind (Neg. Int) 1st Parallel); #23 Dead + 0.6	t 0-9-0 oc. d at 0-4-0 oc. oc. noted as front (F) or bac noted as (F) or (B), unless sign. sf; BCDL=6.0psf; h=15ft; e(s): #3 Dead + Uninhabi /ind (Pos. Internal) Right; Dead + 0.6 MWFRS Wind (WFRS Wind (Pos. Interr leg. Internal) 1st Parallel; loor + 0.75(0.6 MWFRS V Int) Right); #22 Dead + 0	s otherwise indicated. Cat. II; Exp C; Enclosed table Attic Without Stora #6 Dead + 0.6 MWFRS I (Pos. Internal) 1st Para Ial) 3rd Parallel; #11 De #13 Dead + 0.6 MWFR Vind (Neg. Int) Left); #2 75 Roof Live (bal.) + 0.	d; MWFF ge; #4 D Wind (N Illel; #9 D ad + 0.6 S Wind (I Dead + 75 Attic F	RS (envelo Dead + 0.6 Neg. Intern Dead + 0.6 MWFRS V (Neg. Inter • 0.75 Root Floor +	pe); al) Vind nal) f	Decembe	r 22,2021
WARNING - Verify des Design valid for use only a truss system. Before u building design. Bracing is always required for sta fabrication, storage, deliv	sign parameters and READ NOTES ON THIS AND I with MiTek® connectors. This design is based on se, the building designer must verify the applicabil indicated is to prevent buckling of individual truss ability and to prevent collapse with possible person very, erection and bracing of trusses and truss sys aliable from Truss Plate Institute, 2670 Crain High	y upon parameters shown, and i ty of design parameters and proj web and/or chord members only al injury and property damage. I ems, see ANSI/TPI 1	s for an individual building con perly incorporate this design in . Additional temporary and pe for general guidance regarding uality Criteria, DSB-89 and I	nponent, no to the over rmanent br g the	ot all racing	ent	818 Soundside F Edenton, NC 275	

Job	Tr	russ	Truss Type	Qty	Ply	Lot 1 Walker Rd.
J0821-5013	B1	1-GR	ATTIC	1	2	E16495463
						Job Reference (optional)
Comtech, Inc,	Fayetteville	e, NC - 28314,		8.4	30 s Aug	16 2021 MiTek Industries, Inc. Wed Dec 22 10:52:54 2021 Page 2
			ID:52T	[eu6pVqh>	(amGD1jN	I0kr4yxDe9-ZWmpCiBIsN58J4yHYkx0iCndZAbglisJmGty36y6RCN

NOTES-

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-5; Wall dead load (5.0psf) on member(s).6-8, 2-10
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-10
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1870 lb down at 2-1-12, 1870 lb down at 4-1-12, 4072 lb down at 5-3-12, 371 lb down and 36 lb up at 7-9-4, 371 lb down and 36 lb up at 9-9-4, 371 lb down and 36 lb up at 11-9-4, 371 lb down and 36 lb up at 13-9-4, 371 lb down and 36 lb up at 13-9-4, 371 lb down and 36 lb up at 13-9-4, 371 lb down and 36 lb up at 15-9-4, 4072 lb down at 16-7-4, and 1870 lb down at 17-9-4, and 1870 lb down at 19-9-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 11) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

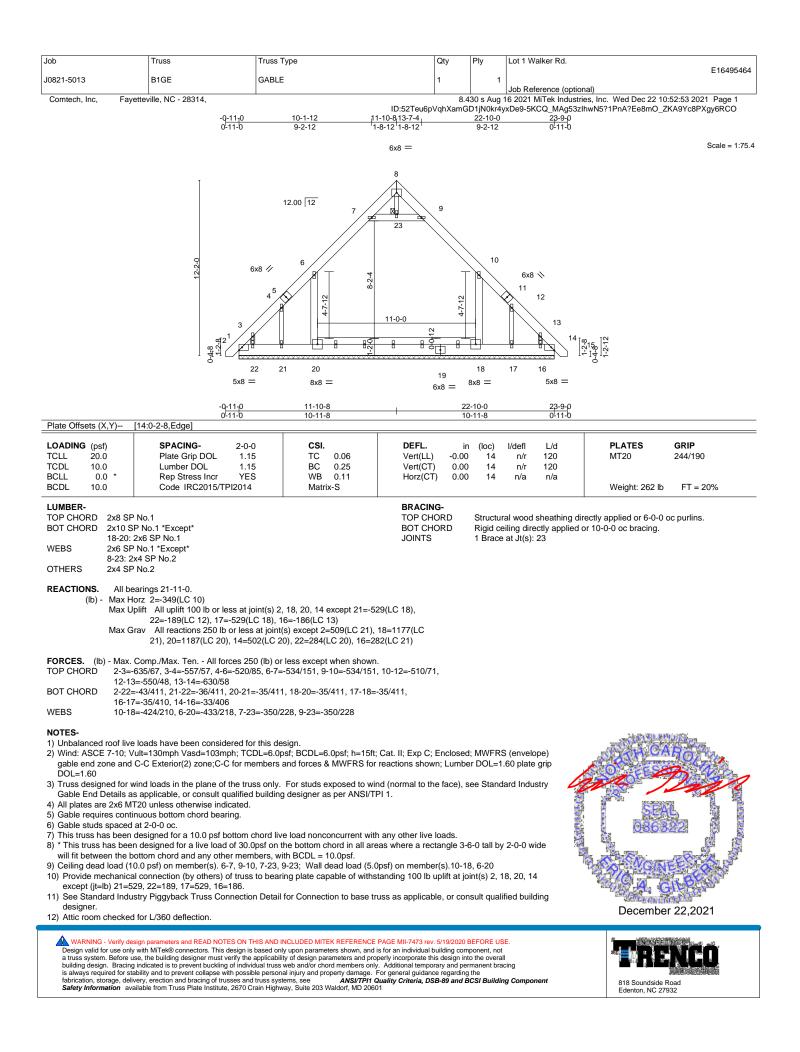
- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-2=-60, 2-3=-80, 3-4=-60, 4-5=-60, 5-6=-80, 6-7=-60, 1-10=-20, 8-10=-40, 7-8=-20, 3-5=-20

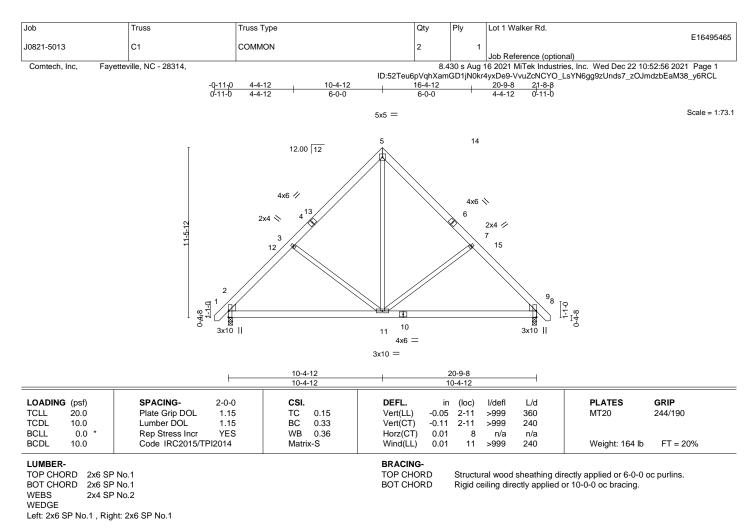
Drag: 6-8=-10, 2-10=-10 Concentrated Loads (lb)

Vert: 9=-60(B) 8=-1096(B) 10=-1096(B) 11=-469(B) 12=-469(B) 13=-60(B) 14=-60(B) 15=-60(B) 16=-60(B) 17=-60(B) 18=-469(B) 19=-469(B) 19=-469(B) 10=-1096(B) 11=-469(B) 12=-469(B) 13=-60(B) 14=-60(B) 15=-60(B) 15=-60(B)

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REACTIONS. (size) 8=0-3-8, 2=0-3-8 Max Horz 2=-268(LC 10) Max Uplift 8=-36(LC 13), 2=-36(LC 12) Max Grav 8=876(LC 1), 2=876(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-908/254, 3-5=-752/282, 5-7=-752/282, 7-8=-908/254

BOT CHORD 2-11=-122/675, 8-11=-60/568

WEBS 5-11=-179/668, 7-11=-354/267, 3-11=-354/267

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 10-4-12, Exterior(2) 10-4-12 to 14-9-9, Interior(1) 14-9-9 to 21-6-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

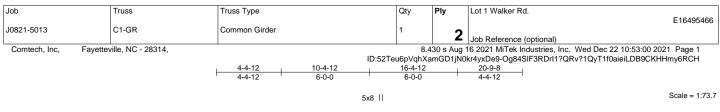
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

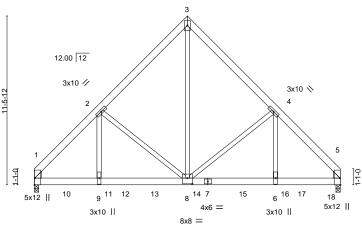
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2.



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				4-4-12	10-4-12		5-4-12		20-9-	-		
Plate Offsets ((X,Y)	[1:Edge,0-0-4], [5:Edge,0	-0-4], [8:0-4	4-4-12 ' 4-0,0-4-12]	6-0-0		6-0-0		4-4-1	2		
LOADING (ps	sf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.15	TC	0.72	Vert(LL)	-0.09	8-9	>999	360	MT20	244/190
ICDL 10	0.0	Lumber DOL	1.15	BC	0.57	Vert(CT)	-0.16	8-9	>999	240		
BCLL 0).0 *	Rep Stress Incr	NO	WB	0.73	Horz(CT)	0.03	5	n/a	n/a		
BCDL 10	0.0	Code IRC2015/TF	912014	Matri	x-S	Wind(LL)	0.05	8-9	>999	240	Weight: 344 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP 2400F 2.0E BOT CHORD 2x4 SP No.2 WEBS WEDGE

Left: 2x6 SP No.1, Right: 2x6 SP No.1

REACTIONS. (size) 1=0-3-8, 5=0-3-8 Max Horz 1=-262(LC 25) Max Uplift 1=-253(LC 9), 5=-280(LC 8) Max Grav 1=5843(LC 2), 5=6526(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1-2=-7016/344, 2-3=-4522/337, 3-4=-4523/337, 4-5=-6984/342 TOP CHORD
- 1-9=-278/4552, 8-9=-278/4562, 6-8=-168/4548, 5-6=-168/4538 BOT CHORD
- 3-8=-332/5948, 4-8=-1816/259, 4-6=-65/2983, 2-8=-1834/259, 2-9=-66/3027 WEBS

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=253, 5=280.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1097 lb down and 55 lb up at 2-0-12, 1097 lb down and 55 lb up at 4-0-12, 1097 lb down and 55 lb up at 6-0-12, 1089 lb down and 55 lb up at 8-0-12, 1089 lb down and 55 lb up at 10-0-12, 1089 lb down and 55 lb up at 12-0-12, 1089 lb down and 55 lb up at 14-0-12, 1089 lb down and 55 Ib up at 16-0-12, and 1097 lb down and 55 lb up at 18-0-12, and 1101 lb down and 51 lb up at 20-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

December 22,2021



🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Job	Truss	Truss Type	Qty	Ply	Lot 1 Walker Rd.
					E16495466
J0821-5013	C1-GR	Common Girder	1	2	Job Reference (optional)
Comtech. Inc. Favett	eville. NC - 28314.				16 2021 MiTek Industries, Inc. Wed Dec 22 10:53:00 2021 Page 2

ID:52Teu6pVqhXamGD1jN0kr4yxDe9-Og84SIF3RDr11?QRv?1QyT1f0aieiLDB9CKHHmy6RCH

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

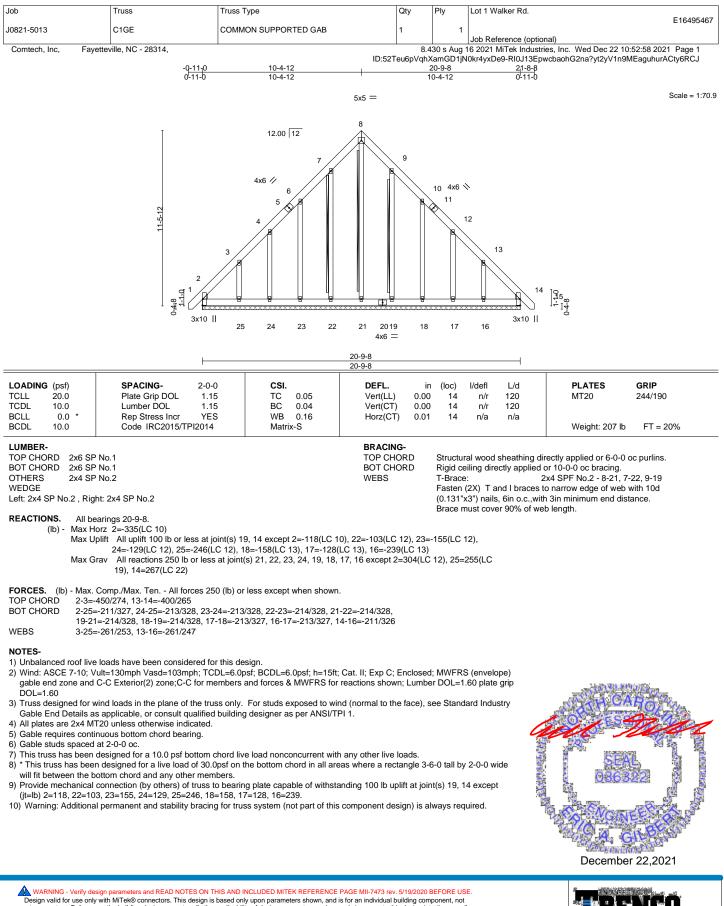
Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-967(B) 10=-967(B) 11=-967(B) 12=-967(B) 13=-967(B) 14=-967(B) 15=-967(B) 15=-967(B) 17=-967(B) 18=-971(B) 10=-967(B) 10=-96

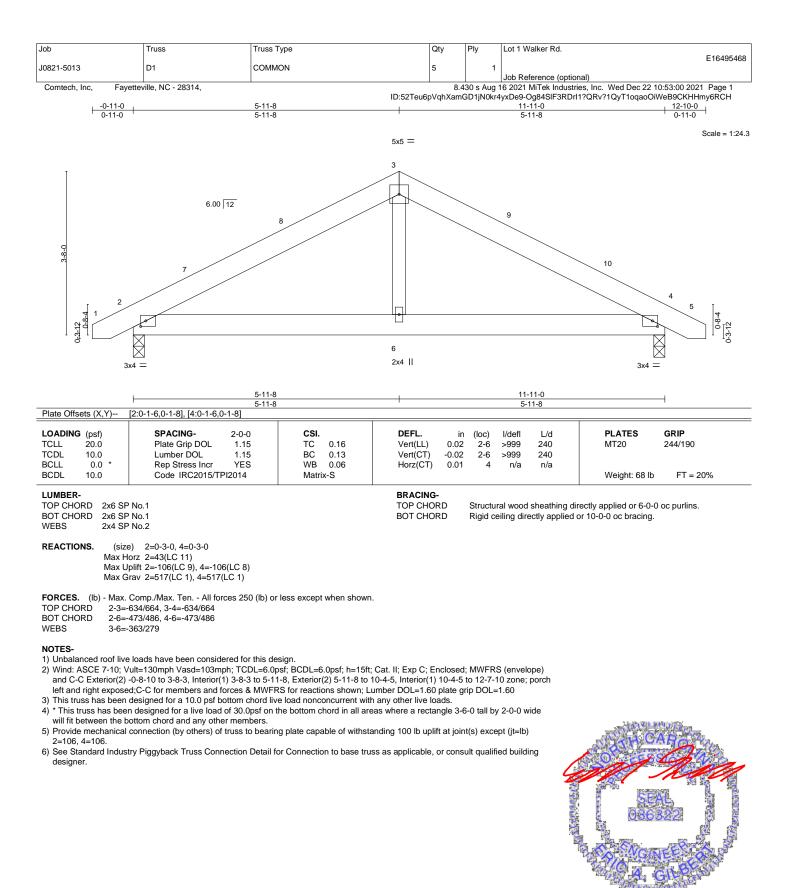
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design properly incorporet his design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 TRENCO

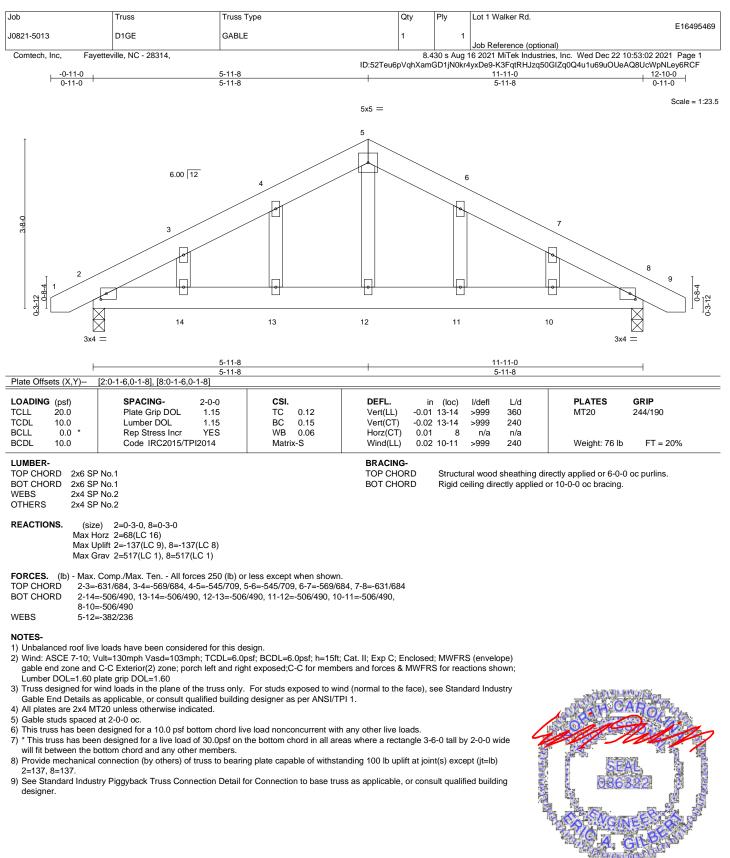
818 Soundside Road Edenton, NC 27932



December 22,2021

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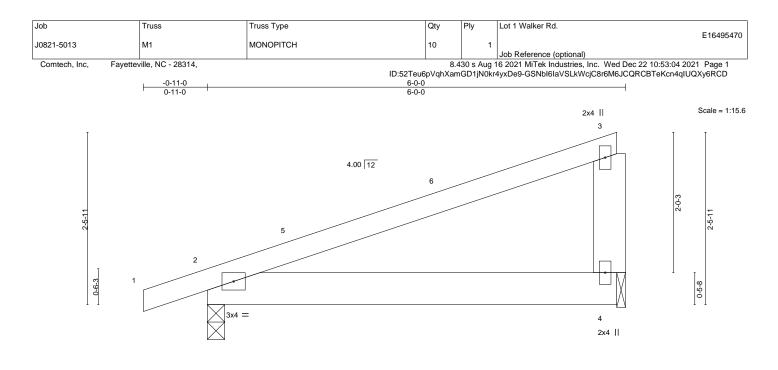




December 22,2021

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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.44 BC 0.12 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.03 0.00 0.03	(loc) 2-4 2-4 2-4	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 29 lb	GRIP 244/190 FT = 20%
LUMBER-			BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=75(LC 8) Max Uplift 2=-116(LC 8), 4=-96(LC 8)

Max Grav 2=294(LC 1), 4=220(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=116.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

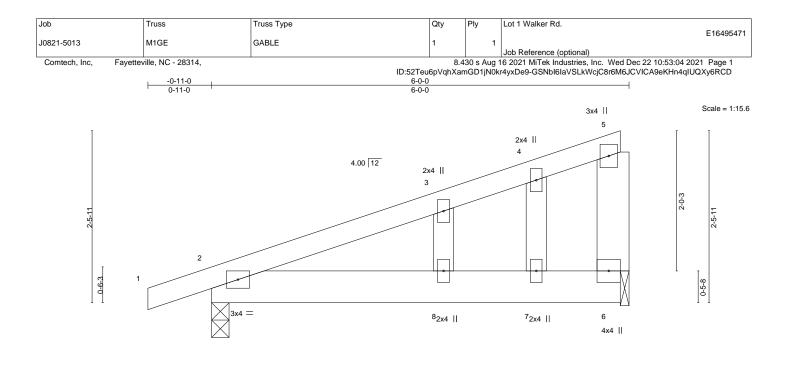
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

December 22,2021

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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.13 BC 0.14 WB 0.02 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) 0.03 2-8 >999 240 Vert(CT) -0.02 2-8 >999 240 Horz(CT) -0.00 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 32 lb FT = 20%
LUMBER-			BRACING-	

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WFBS OTHERS 2x4 SP No 2

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=107(LC 8) Max Uplift 2=-167(LC 8), 6=-140(LC 8)

Max Grav 2=294(LC 1), 6=220(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 1-4-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=167.6=140.



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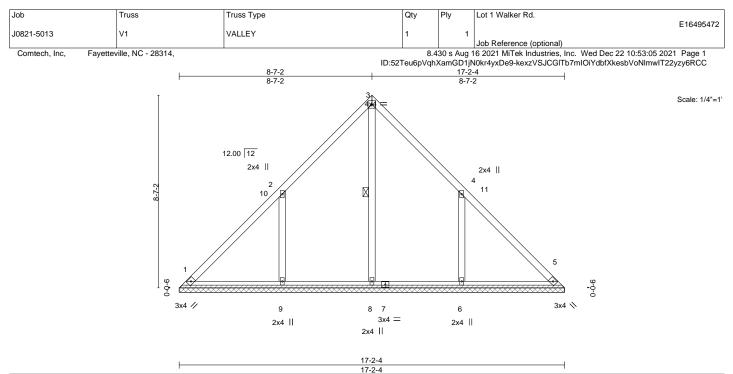


Plate Offsets (X,Y)	[4:0-0-0,0-0-0]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.20 BC 0.18 WB 0.13	DEFL. Vert(LL) n/ Vert(CT) n/ Horz(CT) 0.0	a - n/a 999	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S			Weight: 84 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF 3OT CHORD 2x4 SF DTHERS 2x4 SF	° No.1	11	BRACING- TOP CHORD BOT CHORD WEBS	Rigid ceiling directly applied	rectly applied or 6-0-0 oc purlins. or 10-0-0 oc bracing. 3-8

REACTIONS. All bearings 17-2-4.

(lb) - Max Horz 1=198(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-207(LC 12), 6=-207(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=415(LC 22), 9=537(LC 19), 6=537(LC 20)

WEBS 2-9=-445/331, 4-6=-445/331

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=60.pgf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-7-2, Interior(1) 4-7-2 to 8-7-2, Exterior(2) 8-7-2 to 12-11-15, Interior(1) 12-11-15 to 16-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

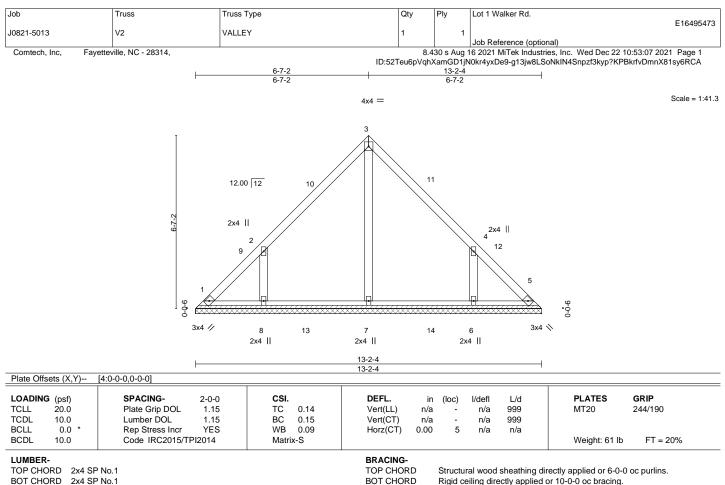
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=207, 6=207.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-2-4

(lb) - Max Horz 1=150(LC 9)

2x4 SP No.2

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-164(LC 12), 6=-163(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=388(LC 19), 8=378(LC 19), 6=378(LC 20)

WEBS 2-8=-359/290, 4-6=-359/290

NOTES-

OTHERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-7-2, Exterior(2) 6-7-2 to 10-11-15, Interior(1) 10-11-15 to 12-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

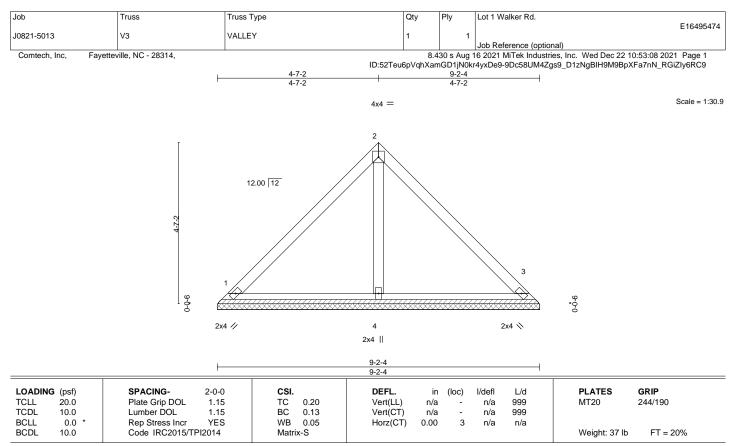
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=164. 6=163.



meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. 🛕 WARNING - Verify design pa Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=9-2-4, 3=9-2-4, 4=9-2-4

Max Horz 1=-102(LC 8)

Max Uplift 1=-25(LC 13), 3=-25(LC 13) Max Grav 1=192(LC 1), 3=192(LC 1), 4=294(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

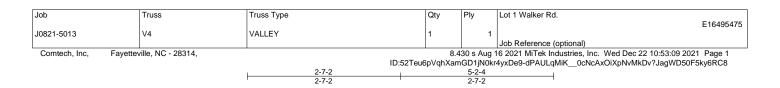


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





4x4 =

2 12.00 12 3 9-0-0



2x4

	1		5-2-4
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.08 BC 0.04 WB 0.01	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) n/a - n/a 999 MT20 244/190 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 3 n/a
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Weight: 20 lb FT = 20%
LUMBER-			BRACING-

TOP CHORD

BOT CHORD

5-2-4

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=5-2-4, 3=5-2-4, 4=5-2-4 Max Horz 1=54(LC 9)

Max Uplift 1=-19(LC 13), 3=-19(LC 13) Max Grav 1=109(LC 1), 3=109(LC 1), 4=140(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Scale = 1:18.4

December 22,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

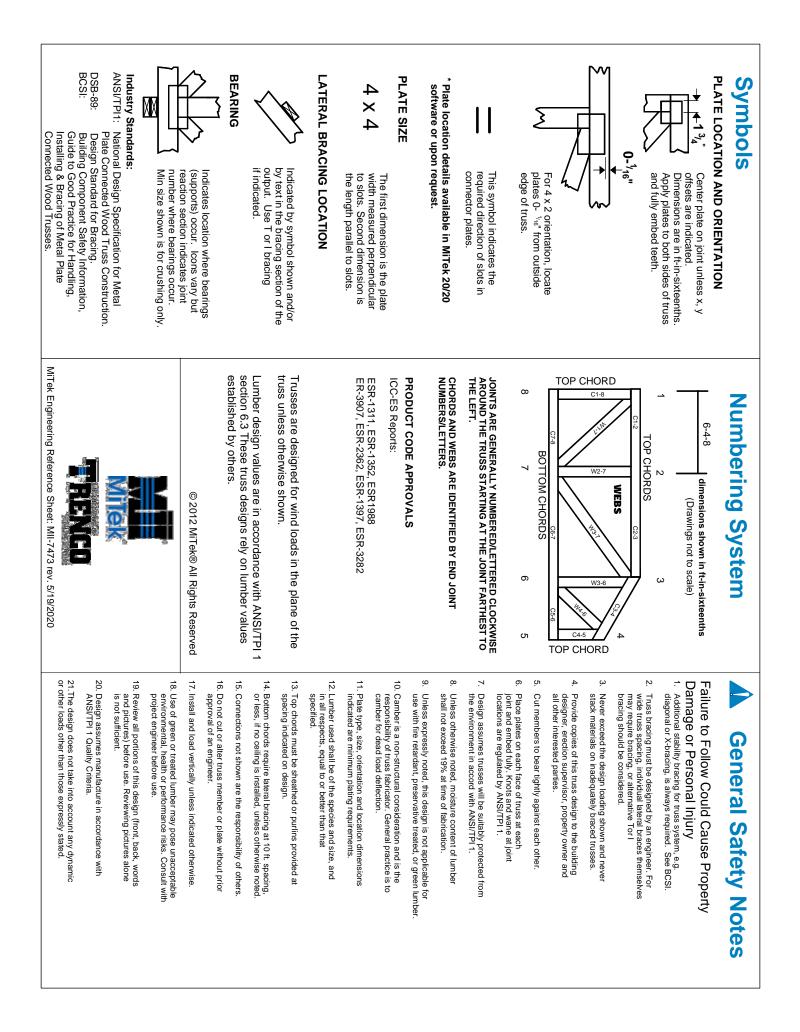


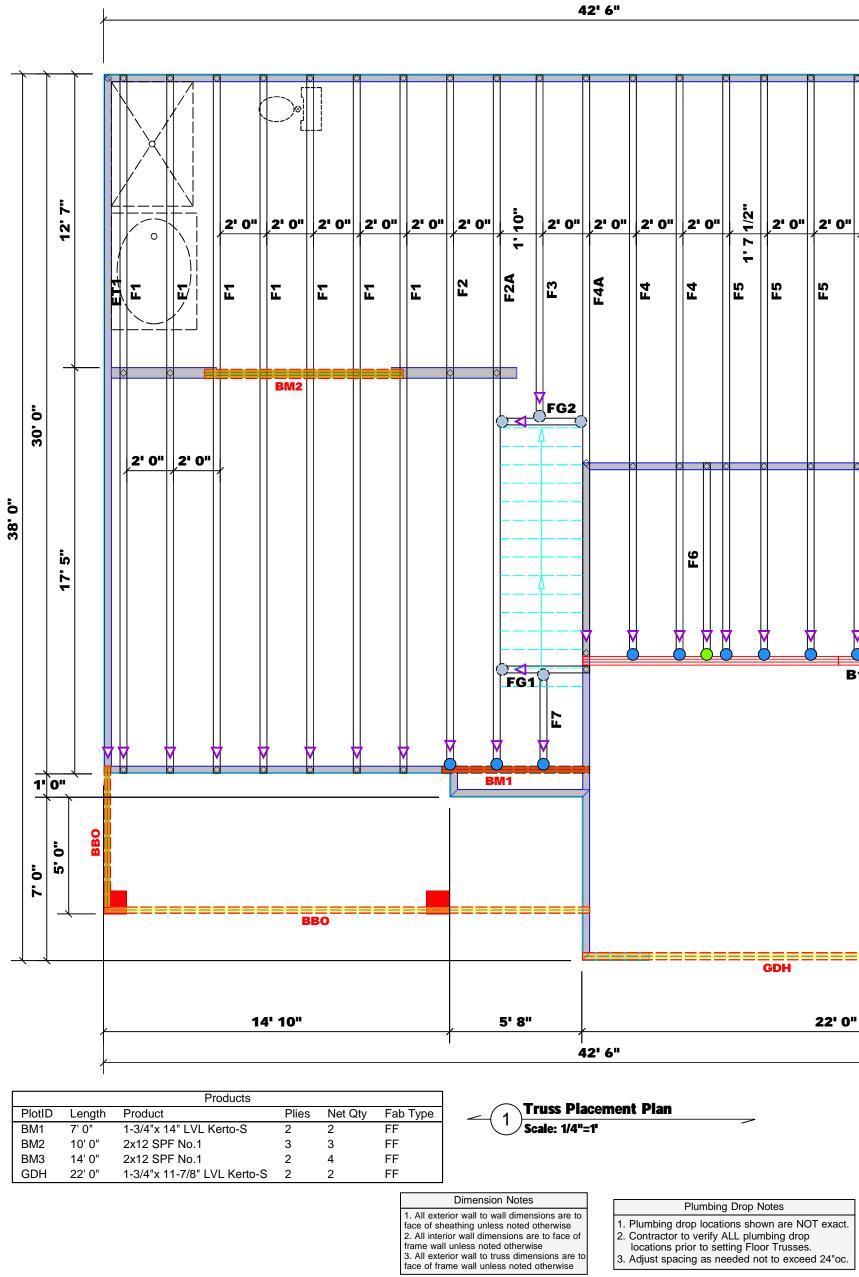
2x4 🚿

0-0-6

Structural wood sheathing directly applied or 5-2-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





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				X			 	~ `	
<u>,2' 0"</u>	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"			
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				/ ⁻ / / <u>/</u> <u>/</u> 				38' 0"	
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							21' 4"		
GDH		====					ا <u></u> م	└── ┥	
	22' 0"					,			
			C	onnector I	Informatio		Nail I	Information	

	Conne	ctor Info	rmati	on	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	13	Varies	16d/3-1/2"	16d/3-1/2"
\bigcirc	MSH422	USP	5	Varies	10d/3"	10d/3"
\bigcirc	THD410	USP	2	Varies	16d/3-1/2"	10d/3"

These to comport design a See ind identified designed perman for the of support and coll designed consult	BUILDER	Ben Stout Real Estate COUNTY	Linden / Harnett		deeme require attache Code r founda require but no profes suppoi those registe design exceed	
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	/	Client:	Benjamir	n Stout Real Esta	te		Date:	3/18/2022				Page 1 of 11
ŕ	isDesign	Projec					Input by:	David Land	-			
- +	Isizesigii	Addre		lker Road NC 28356			Job Name Project #:	: Lot 1 Walk J0821-501				
BM1	Kerto-S LVL	1.750)" X 14.	000" 2-	Ply - P	ASS		Level: Level				
					- - y							
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remperatu	re. 1emp <= 100 P					Bearii	ngs					
						Beari 1 - SI	ing Length PF 3.500"	37%	React D/L lb 1014 / 918	1932		Ld. Comb. D+L
Analysis	Results	I				2 - SI	PF 3.500"	37%	1014 / 918	1932	L	D+L
Analysis		cation Allow		-	Case							
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Unbraced		3' 26999		(9%) D+L	L							
Shear LL Defl in		4'7 1/4" 10453 3' 0 139	(L/480) 0.050 ((14%) D+L	L 1							
	ch 0.014 (L/4635)		(L/480) 0.030 ((L/360) 0.080 (L							
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2	Uniform			Тор	105 PLF	0	PLF	0 PLF	0 PLF	0 PLF	C1GE	
3	Uniform			Far Face	102 PLF	306	PLF	0 PLF	0 PLF	0 PLF	F2	
	Self Weight				11 PLF							
structural adequa design criteria responsibility of ensure the cor	ured Designs is responsible only of the acy of this component based on the and loadings shown. It is the the customer and/or the contractor to mponent suitability of the intended verify the dimensions and loads.	 LVL beams must Refer to ma regarding inst 		pondir information s, multi-ply	at roofs provide p ng	roper drainag	je to prevent	Manufacturer Metsä Wood 301 Merritt 7 B Norwalk, CT 06 (800) 622-5850 www.metsawo	uilding, 2nd Floor 6851)	10 Fi U 28	omtech, Inc. 001 S. Reilly Road ayetteville, NC SA 3314 10-864-TRUS	I, Suite #639
Lumber	onditions, unless noted otherwise	 Damaged Beams Design assumes 	top edge is laterally res	strained				ICC-ES: ESR-				
2. LVL not to be	onditions, unless noted otherwise treated with fire retardant or corrosive	 Provide lateral lateral displacem 	support at bearing po	ints to avoid	design is valid	until 4/24/	/2023				con	птесн
/ersion 20.40.0	075 Powered by iStruct™									C	SD 🚟	

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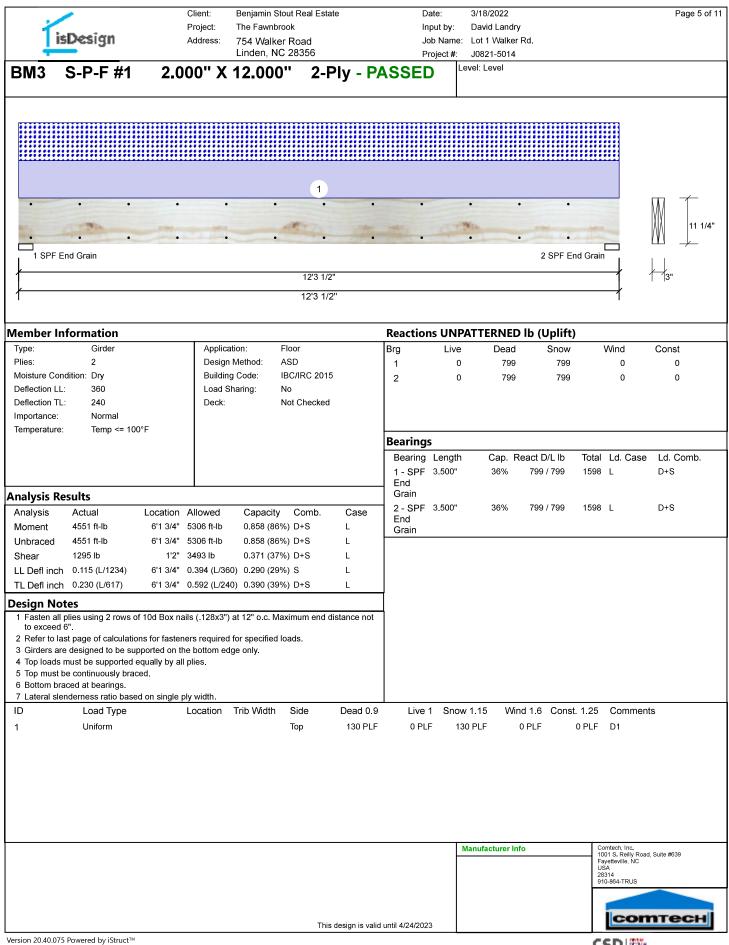
-	Client: Benjamin Stout Re	al Estate Date:	3/18/2022	Page 2 of 11
	Project: The Fawnbrook	Input by	r: David Landry	
isDesign	Address: 754 Walker Roa			
	Linden, NC 283	,		
BM1 Kerto-S LVL	1.750" X 14.000"	2-Ply - PASSED	Level: Level	
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		N		NAA I
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1 SPF	2 SPF			
(<u>1</u>	6'	1.		1 13 1/2"
Γ Γ e	6'	1		
Multi-Ply Analysis				
			ant to successf C"	
Fasten all plies using 3 rows of 7 Capacity 83.1		o.c Maximum end distance	not to exceed 6"	
	% 0 PLF			
Yield Limit per Foot 245.	6 PLF			
Yield Limit per Fastener 81.9 Yield Mode IV	lb.			
Edge Distance 1 1/2	2"			
Min. End Distance 3"				
Load Combination D+L Duration Factor 1.00				
Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is responsible only of the	landling & Installation	ponding	Metsä Wood	
structural adequacy of this component based on the 1 design criteria and loadings shown. It is the 2 responsibility of the customer and/or the contractor to	LVL beams must not be cut or drilled Refer to manufacturer's product information requirements multiply		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the component suitability of the intended application, and to verify the dimensions and loads.	regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals		(800) 622-5850 www.metsawood.com/us	
Lumber 3	 Damaged Beams must not be used Design assumes top edge is laterally restrained 		ICC-ES: ESR-3633	
Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosive	 Provide lateral support at bearing points to avoid lateral displacement and rotation 	This design is valid until 4/24/2023		соттесн
Version 20.40.075 Powered by iStruct™				

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	1	Client:	Benjamin	Stout Real Estate	e	D	ate:	3/18/2022				Page 3	3 of 11
~		Project:	The Fawn				put by:	David Land	Iry			0	
i	sDesign	Address:	754 Wal	ker Road		J	b Name:	Lot 1 Walk	er Rd.				
			Linden, I	NC 28356		Р	roject #:	J0821-501	4				
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I			8'1"					.1					
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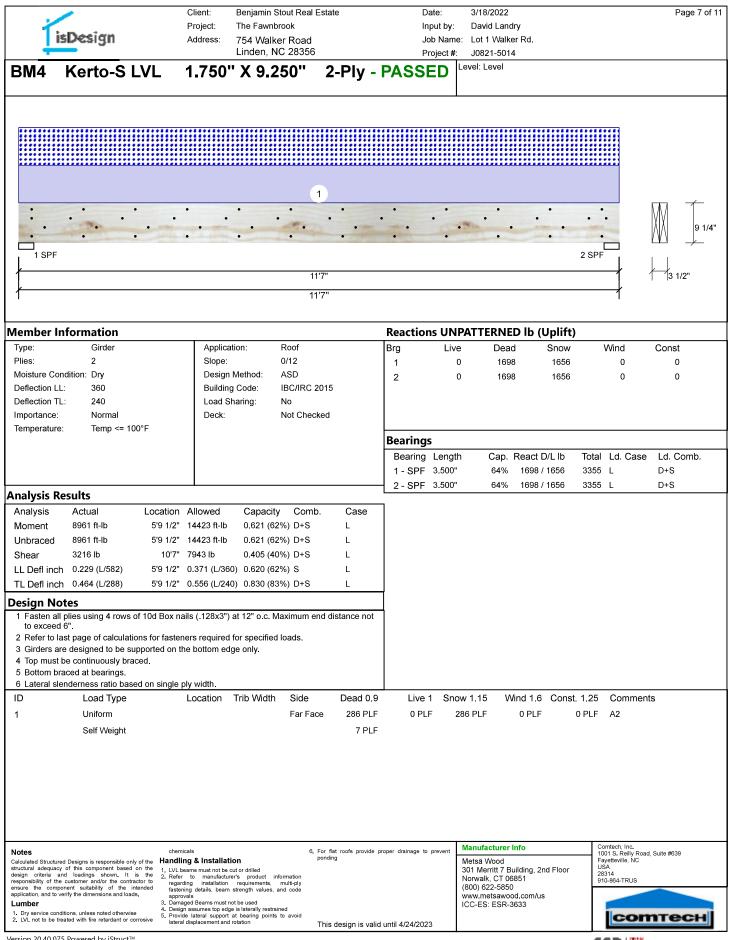
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Address: 754 Walker Road Linden, NC 28356 Project #: J0821-5014 BM2 S-P-F #1 2.000" X 12.000" 3-Ply - PASSED		-	Client:	Benjamin Stout F	Real Estate		Date:	3/18/2022	Page 4 of 11
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28314 910-864-TRUS							F		Fayetteville, NC USA
This design is valid until 4/24/2023									28314
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1	isDesign	Client: Project: Address:	Benjamin Stout Real Estate The Fawnbrook 754 Walker Road Linden, NC 28356	Date: Input by Job Nar Project i	ne: Lot 1 Walker Rd.	Page 6 of 11
BM3	S-P-F #1	2.000" X	12.000" 2-Ply	- PASSED	Level: Level	
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	• •	• •	• •	•••	• •	
	End Grain		12'3 1/2"		2	SPF End Grain
			12'3 1/2"			
Multi-Ply		ws of 10d Box nails ((.128x3") at 12" o.c Maxi	mum end distance i	not to exceed 6"	
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					Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314
L			This design	n is valid until 4/24/2023	1	



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Í	isDesign	Client: Project: Address:	Benjamin Stout Rea The Fawnbrook 754 Walker Road Linden, NC 28356		Date: Input by: Job Name		Page 8 of 11
BM4	Kerto-S L	VL 1.750	X 9.250 "		Project #:	J0821-5014 Level: Level	
				,			
							=
	· · ·	••••	· · ·	• •	• • • • • •	· · · ·	
				4 4 1711			2 SPF 73 1/2"
				11'7" 11'7"			3 1/2"
							·
Multi-Ply	Analysis						
Fasten all Capacity	plies using 4 row	s of 10d Box nails 76.0 %	(.128x3") at 12" o	o.c Maximum	end distance no	ot to exceed 6"	
Load		286.0 PLF					
Yield Limit pe Yield Limit pe		376.5 PLF 94.1 lb.					
Yield Mode Edge Distanc	e	IV 1 1/2"					
Min. End Dist	ance	3"					
Load Combin Duration Fact		D+S 1.15					
Notes	rod Decigne in many	chemicals		 For flat roofs provide ponding 	proper drainage to prevent	Manufacturer Info Metsä Wood	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequa design criteria responsibility of tl	ured Designs is responsible only of this component based of and loadings shown. It is he customer and/or the contract	n the 1. LVL beams must not be the 2. Refer to manufactu tor to regarding installation				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the com application, and to Lumber 1. Dry service co	ponent suitability of the inte overify the dimensions and loads, nditions, unless noted otherwise treated with fire retardant or corr	inded fastening details, beam approvals 3. Damaged Beams must i 4. Design assumes top edg 5. Browide Lateral exponent	strength values, and code tot be used te is laterally restrained at bearing points to avoid	This design is valio	1 until 4/24/2023	(800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	соттесн
L				acorgin io valit			

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Version 20.40.075 Powered by iStruct™

	Client: Benjamin Stout Real Estate Project: The Fawnbrook	Date: 3/18/2022 Page 9 of 1' Input by: David Landry
isDesign	Address: 754 Walker Road	Job Name: Lot 1 Walker Rd.
	Linden, NC 28356	Project #: J0821-5014
BM5 Kerto-S LVL	1.750" X 9.250" 3-Pl	
(11111111111111111111111111111111111111		
2		3
2		
		X X X
C. The second	and the second	I WW
1 SPF End Grain		2 SPF End Grain
ļ	7'10"	5 1/4"
<u>/</u>	7'10"	
	710	l l
lember Information		Reactions UNPATTERNED lb (Uplift)
Type: Girder	Application: Floor	Brg Live Dead Snow Wind Const
Plies: 3 Moisture Condition: Dry	Design Method: ASD Building Code: IBC/IRC 2015	1 5848 2463 0 0 0 2 5848 2463 0 0 0
Deflection LL: 480	Load Sharing: Yes	2 5848 2463 0 0 0
Deflection TL: 240	Deck: Not Checked	
Importance: Normal		
Temperature: Temp <= 100°F		Boarings
		Bearings
		Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 8.000" 23% 2463 / 5848 8310 L D+L
		End End
nalysis Results		Grain
•		ase 2 - SPF 8.000" 23% 2463 / 5848 8310 L D+L End
	1" 19565 ft-lb 0.595 (59%) D+L L	Grain
	1" 14536 ft-lb 0.801 (80%) D+L L 2" 10360 lb 0.521 (52%) D+L L	
	1" 0.166 (L/480) 0.680 (68%) L L	
	1" 0.331 (L/240) 0.480 (48%) D+L L	
Design Notes 1 Girders are designed to be supported or		
Design Notes 1 Girders are designed to be supported or 2 Multiple plies must be fastened together	r as per manufacturer's details.	
Design Notes 1 Girders are designed to be supported or	r as per manufacturer's details.	
Pesign Notes 1 Girders are designed to be supported or 2 Multiple plies must be fastened together 3 Top loads must be supported equally by 4 Top braced at bearings. 5 Bottom braced at bearings.	r as per manufacturer's details. r all plies.	
Pesign Notes 1 Girders are designed to be supported or 2 Multiple plies must be fastened together 3 Top loads must be supported equally by 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on single	r as per manufacturer's details. r all plies. le ply width.	ad 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments
Pesign Notes 1 Girders are designed to be supported or 2 Multiple plies must be fastened together 3 Top loads must be supported equally by 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on singl ID Load Type	r as per manufacturer's details. [,] all plies. <u>le ply width.</u> Location Trib Width Side Dea	ad 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 35 PLF 795 PLF 0 PLF 0 PLF 0 PLF J-32'
Design Notes 1 Girders are designed to be supported or 2 Multiple plies must be fastened together 3 Top loads must be supported equally by 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on singl ID Load Type	r as per manufacturer's details. ^y all plies. <u>le ply width.</u> Location Trib Width Side Dea Top 265	
Design Notes 1 Girders are designed to be supported or 2 Multiple plies must be fastened together 3 Top loads must be supported equally by 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on singl ID Load Type 1 Uniform 2 Uniform	r as per manufacturer's details. r all plies. le ply width. Location Trib Width Side Dea Top 265 Top 120	35 PLF 795 PLF 0 PLF 0 PLF 0 PLF J-32'
Design Notes 1 Girders are designed to be supported or 2 Multiple plies must be fastened together 3 Top loads must be supported equally by 4 Top braced at bearings. 5 Bottom braced at bearings. 6 Lateral slenderness ratio based on singl ID Load Type 1 Uniform 2 Uniform	r as per manufacturer's details. r all plies. <u>le ply width.</u> Location Trib Width Side Dea Top 265 Top 120 Top 233	35 PLF 795 PLF 0 PLF 0 PLF 0 PLF J-32' 20 PLF 0 PLF 0 PLF 0 PLF 0 PLF Wall

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	Design	Pro Ad	oject: Idress:	The Fawnt 754 Walk Linden, N	er Road IC 28356		Inp Job Pro	ut by: D Name: L ject #: J	/18/2022 David Lanc ot 1 Walke 0821-501 el: Level	er Rd.			Page 10
GDH H	Kerto-S LVL	_ 1./	′50" <i>)</i>	(11.8	/5" 2-	Ply - P	ASSED						
		2			1								
1 SPF End	Grain	-	· [7]		atop	4		4.77		• • •	• End Grai		
<u> </u>					16'10"								3 1/2"
/					16'10"							\rightarrow	
ember Inf	ormation						Reactions	UNPAT	TERNE	D lb (Uplift	:)		
Гуре:	Girder		Applicati		Floor		Brg	Live	Dea			Wind	Const
Plies: Moisture Cond	2 ition: Dry		Design M Building		ASD IBC/IRC 2015		1 2	0 0	214 214			0 0	0 0
Deflection LL:	360		Load Sh		No		~	v	214	- 100		5	U
eflection TL:	240		Deck:		Not Checked								
nportance:	Normal												
emperature:	Temp <= 100°F						Bearings						
							Bearing	enath	Can	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	-	22%	2140 / 168	2308		D+S
							End			21107100	2000	-	
nalysis Res	sults						Grain		000/	2140 / 168	0000		D+S
nalysis		ocation All		Capacit		Case	2 - SPF 3 End	5.500	22%	2140/100	2308	L	D+3
Moment	8521 ft-lb	8'5" 17		0.476 (48		Uniform	Grain						
Jnbraced	8521 ft-lb 1830 lb 1		919 ft-lb	0.476 (48	,	Uniform Uniform							
Shear		5'7 3/8" 79 \$'5 1/16" 0.5	80 lb	0.229 (23	,	L							
	. ,	5 1/16 0.8			·	L							
		0.0	010 (L/240) 0.000 (0.) D 0	L							
esign Not	es lies using 2 rows of 10		(100,021)	+ 12" o o N	lovimum and di	ctanco not							
to exceed 6			(.120,0) 8	n 12 0.c. N		stance not							
	page of calculations f designed to be support		•	•	loads.								
	ust be supported equa		-	e only.									
•	continuously braced.												
	ed at bearings. derness ratio based or	n sinale nlv v	width										
D	Load Type			Frib Width	Side	Dead 0.9	Live 1	Snow 1.	.15 W	ind 1.6 Cons	st. 1.25	Comment	S
	Uniform				Тор	45 PLF	0 PLF	0 F		0 PLF	0 PLF	Wall Above	
2	Uniform				Тор	180 PLF	0 PLF	0 F		0 PLF	0 PLF	B1GE	
-	Tie-In	0-0-0 to 1	6-10-0	1-0-0	Тор	20 PSF	0 PSF	20 P		0 PSF	0 PSF	Roof Load	
	Self Weight		-			9 PLF	51			-			
otoo		chemicals			6 Eor 4o	t roofs provide ~	oper drainage to p	Man	ufacturer	Info	Co	omtech, Inc.	o 11 11005
	Designs is responsible only of the	e Handling 8			6. For fla pondin		орог чтаппаде то р	Met	sä Wood		Fa	01 S. Reilly Road, yetteville, NC	Suite #639
esign criteria and	f this component based on the loadings shown. It is the astomer and/or the contractor to	e 2 Refer to	must not be cu manufacturer	's product ir	formation				Merritt 7 B walk, CT 06	uilding, 2nd Floor 851	28	5A 314 0-864-TRUS	
sure the compone	int suitability of the intended y the dimensions and loads.	d fastening c approvals	letails, beam s	requirements, trength values,	and code			(800	0) 622-5850 v.metsawoo	1			
		approvais		he could									
umber	ns, unless noted otherwise	 Damaged E Design ass 	umes top edae	is laterally restra t bearing points	ined			ICC-	-ES: ESR-3	633			

1	isDesign		Client: Project: Address:	Benjamin Stout Re The Fawnbrook 754 Walker Roa Linden, NC 2835	d		Date: Input by: Job Name: Project #:	3/18/2022 David Landry : Lot 1 Walker Rd. J0821-5014	Page 11	of 11
GDH	Kerto-S	LVL	1.750"	X 11.875"		- PASSE		evel: Level		
									à	
	•••	••••	•	· ·	· ·	•	· ·			3"
	End Grain							2	SPF End Grain	
					16'10"				3 1/2"	
					16'10''				I	
Capacity Load Yield Limit pe Yield Mode Edge Distanc Min. End Dist Load Combin Duration Fact	plies using 2 r Foot r Fastener e ance ation	0.0 % 0.0 PL 163.7 I 81.9 Ib IV 1 1/2" 3" 1.00	F PLF	(.128x3") at 12"				t to exceed 6"	Contech. Inc.	
structural adequa design criteria responsibility of t ensure the com	ured Designs is responsible cy of this component be and loadings shown, he customer and/or the c oponent suitability of th overify the dimensions and	e only of the sed on the 1. L It is the 2. F contractor to n e intended fa	VL beams must not be lefer to manufactu agarding installatior		 For flat roofs pro ponding 	ovide proper drainage	b prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	1001 S. Reilly Road, Suite #639 Fayetteville, NC	
Lumber 1. Dry service co 2. LVL not to be	nditions, unless noted othe treated with fire retardant	3. E 4. E orwise 5. F or corrosive la	lamaged Beams must lesign assumes top eo	ge is laterally restrained t at bearing points to avoid	This design is	valid until 4/24/2		ICC-ES: ESR-3633	соттесн	J
Version 20.40.0	75 Powered by iStru	ct™							CSD 🚟	

r 5	D	DC3
	~	NULL



RE: J0821-5014 Lot 1 Walker Rd. **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Benjamin Stout Real Estate Lot/Block: 1	Project Name: J0821-5014 Model: Fawnbrook
Address: 754 Walker Road	Subdivision: Walker Rd.
City: Linden	State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 13 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
		ET1	
1	E16494820	EII	12/22/2021
2	E16494821	ET2	12/22/2021
3	E16494822	F1	12/22/2021
4	E16494823	F2	12/22/2021
5	E16494824	F2A	12/22/2021
6	E16494825	F3	12/22/2021
7	E16494826	F4	12/22/2021
8	E16494827	F4A	12/22/2021
9	E16494828	F5	12/22/2021
10	E16494829	F6	12/22/2021
11	E16494830	F7	12/22/2021
12	E16494831	FG1	12/22/2021
13	E16494832	FG2	12/22/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

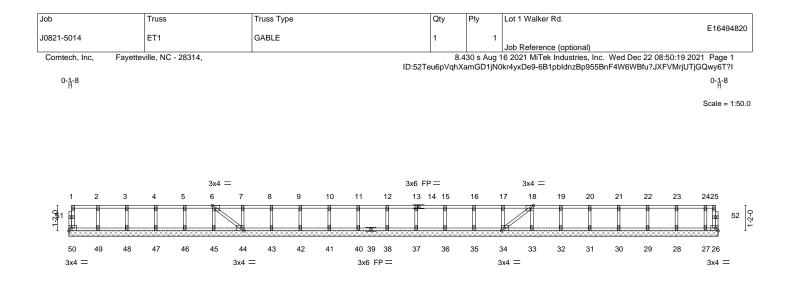
My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

December 22, 2021



29-11-0 1-4

	010 (71,17		[04:0 1 0,Edg0], [44:0 1 0,							
LOADING	(psf)	SPACING- 2-0-	csi.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL 1.0	TC 0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL 1.0	BC 0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr YES	S WB 0.03	Horz(CT)	-0.00	26	n/a	n/a		
BCDL	5.0	Code IRC2015/TPI2014	Matrix-S						Weight: 128 lb	FT = 20%F, 11%E
LUMBER-				BRACING	-					
TOP CHO	RD 2x4 SF	P No.1(flat)		TOP CHO	RD	Structu	ral wood	sheathing di	rectly applied or 6-0-0 of	oc purlins,
BOT CHO	RD 2x4 SF	P No.1(flat)				except	end vert	icals.		
WEBS	2x4 SF	P No.3(flat)		BOT CHO	RD	Rigid c	eiling dir	ectly applied	or 6-0-0 oc bracing, E	xcept:

Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 49-50,48-49,47-48,46-47,45-46,44-45.

REACTIONS. All bearings 29-11-0.

2x4 SP No.3(flat)

(lb) - Max Uplift All uplift 100 lb or less at joint(s) 26

Max Grav All reactions 250 lb or less at joint(s) 50, 49, 48, 47, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 27

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26.

7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

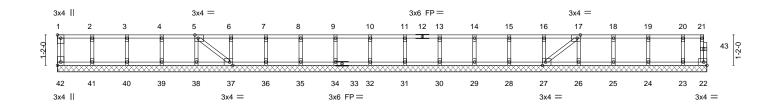




	Job	Truss	Truss Type	Qty	Ply	Lot 1 Walker Rd.
	J0821-5014	ET2	GABLE	1	1	E16494821
	30021-3014		GABLE			Job Reference (optional)
Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Aug 16 2021 MiTek Industries, Inc. Wed Dec 22 08:50:20 2021 Page						

ID:52Teu6pVqhXamGD1jN0kr4yxDe9-aNaBoeePkVx0jFmzpn2L3PC3jjtUEp3sj7SpyMy6T?H 0-<u>1</u>-8

Scale = 1:41.6



 1-4-0
 2-8-0
 4-0-0
 5-4-0
 6-8-0
 8-0-0
 9-4-0
 10-8-0
 12-0-0
 13-4-0
 14-8-0
 16-0-0
 17-4-0
 18-8-0
 20-0-0
 21-4-0
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LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. i Vert(LL) n/. Vert(CT) n/. Horz(CT) -0.00	a - n/a 999	PLATES MT20 Weight: 108 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing d except end verticals. Rigid ceiling directly applied	2 11	oc purlins,

REACTIONS.

2x4 SP No.3(flat)

 All bearings 24-11-0.
 (lb) - Max Grav All reactions 250 lb or less at joint(s) 42, 22, 41, 40, 39, 38, 37, 36, 35, 34, 32, 31, 30, 29, 28, 27, 26, 25, 24, 23

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

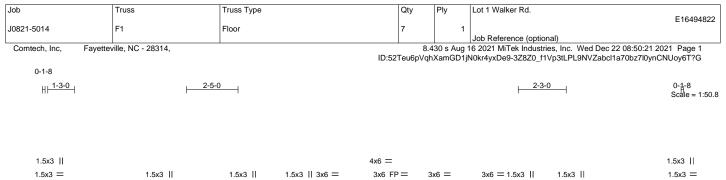
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

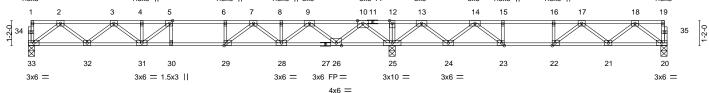
Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.









 	<u> </u>					29-11		
Plate Offsets (X,Y)		-8,Edge], [29:0-1-8,Edge]				12-10	J-0	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.88 BC 0.85 WB 0.58 Matrix-S		in (loc) -0.19 29-30 -0.26 29-30 0.05 20	l/defl >999 >767 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 148 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 WEBS 2x4 REACTIONS.	4 SP No.1(flat) 4 SP No.1(flat) 4 SP No.3(flat) (size) 33=0-3-8, 20=0-3-8, 25=0-3-8 ax Grav 33=822(LC 3), 20=614(LC 4), 25=1	929(LC 1)	BRACING- TOP CHORI BOT CHORI	except	end vert	icals.	rectly applied or 2-2-0 c or 6-0-0 oc bracing.	oc purlins,
TOP CHORD 2 8 1 BOT CHORD 3	Max. Comp./Max. Ten All forces 250 (lb) of -3=-1687/0, 3-4=-2703/0, 4-5=-2703/0, 5-6 -9=-2065/0, 9-10=-549/294, 10-12=0/2133, 4-15=-1607/250, 15-16=-1607/250, 16-17=- 12-33=0/1025, 31-32=0/2317, 30-31=0/2936 6-28=-30/1421, 25-26=-830/0, 24-25=-1240	-2936/0, 6-7=-2936/0, 7-8=-2 12-13=0/2133, 13-14=-572/95 1607/250, 17-18=-1177/0 , 29-30=0/2936, 28-29=0/2556	52, 0,					
2 WEBS 2 1 9	1-22=-9/1538, 20-21=0/756 1-33=-1283/0, 2-32=0/862, 3-32=-820/0, 3-3 0-26=0/1220, 18-20=-945/0, 18-21=0/549, 1 -26=-1182/0, 9-28=0/871, 7-28=-674/0, 7-2 3-24=0/882, 14-24=-928/0, 14+23=0/951, 1	1=0/493, 5-31=-489/137, 10-2 7-21=-469/93, 17-22=-331/89 9=0/794, 6-29=-359/0, 13-25=	5=-1646/0, 9,					

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

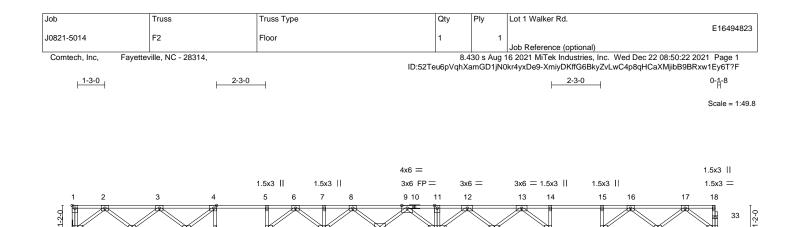
3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means. 5) CAUTION, Do not erect truss backwards.







27

3x6 =

26 25

3x6 FP =

4x6 =

Ř

24

3x10 =

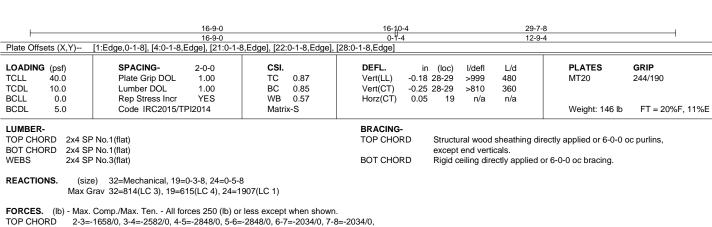
23

3x6 =

22

21

20



 8-9=-563/296, 9-11=0/2082, 11-12=0/2082, 12-13=-578/905, 13-14=-1611/221, 14-15=-1611/221, 15-16=-1611/221, 16-17=-1179/0

 BOT CHORD
 31-32=0/1002, 30-31=0/2282, 29-30=0/2848, 28-29=0/2848, 27-28=0/2500, 25-27=-36/1414, 24-25=-810/0, 23-24=-1188/0, 22-23=-602/1165, 21-22=-221/1611, 20-21=0/1540, 19-20=0/756

 WEBS
 2-32=-1257/0, 2-31=0/854, 3-31=-813/0, 3-30=0/392, 4-30=-431/27, 9-24=-1619/0, 9-25=0/1194, 17-19=-946/0, 17-20=0/550, 16-20=-470/84, 16-21=-317/90, 8-25=-1155/0, 8-27=0/843, 6-27=-652/0, 6-28=0/750, 5-28=-323/0, 12-24=-1316/0, 12-23=0/876, 13-23=-920/0, 13-22=0/36, 14-22=-417/0

29

1.5x3 ||

28

30

NOTES-

32

3x6 =

31

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

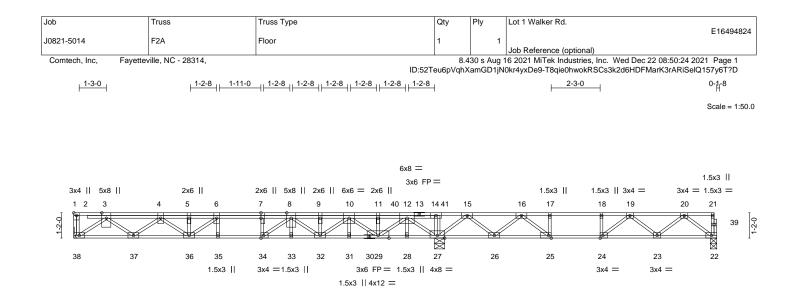


X

19

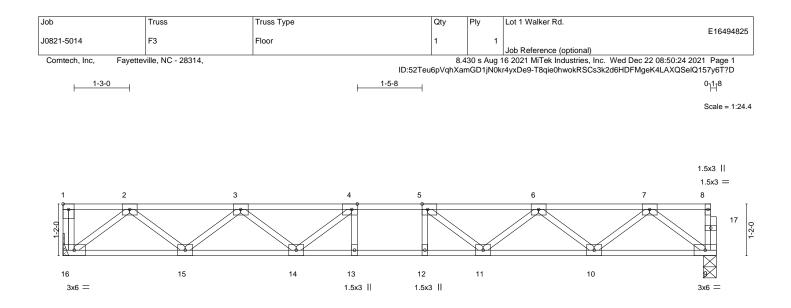
3x6 =





 	16-9-0		16-10-4 0- ¹ -4			29-7-8		
Plate Offsets (X,Y)	16-9-0 [1:Edge,0-1-8], [7:0-3-0,Edge], [24:0-1-8	3,Edge], [25:0-1-8,Edge],				12-9-4		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.76 BC 0.75 WB 0.82 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.18 34 -0.25 34-35 0.05 22	>999 4 >807 3	L/d 180 360 n/a	PLATES MT20 Weight: 174 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	2x4 SP 2400F 2.0E(flat)		BRACING- TOP CHORI BOT CHORI	except	end vertical	s.	tly applied or 6-0-0 c 3-0-0 oc bracing.	oc purlins,
REACTIONS. (siz	e) 38=Mechanical, 27=0-5-8, 22=0-3-8 Grav 38=964(LC 3), 27=2406(LC 1), 22=							
TOP CHORD 3-4= 9-10 15-11 BOT CHORD 37-3 31-3 25-2 WEBS 3-38 12-2 16-2 16-2 7-34	Comp./Max. Ten All forces 250 (lb) or -2136/0, 4-5=-3370/0, 5-6=-3370/0, 6-7= =-2773/0, 10-11=-1237/0, 11-12=-1237/0 6=-130/1235, 16-17=-1334/425, 17-18=- 8-0/1273, 36-37=0/2963, 35-36=0/3559, 32=0/2048, 29-31=0/2048, 28-29=-674/0, 6=-889/777, 24-25=-425/1334, 23-24=-1 =-1562/0, 3-37=0/1096, 4-37=-1051/0 9=0/1730, 11-29=-374/0, 15-27=-1531/0 6=-983/0, 16-25=0/1048, 17-25=-483/0, =-399/0, 6-36=-364/210, 20-22=-870/0, 2 4=-429/0	-3559/0, 7-8=-3559/0, 8- 5, 12-14=0/2680, 14-15=C 1334/425, 18-19=-1334/4 34-35=0/3559, 33-34=0/ 27-28=-674/0, 26-27=-1: 12/1373, 22-23=0/696 -36=0/508, 5-36=-262/11 10-29=-1078/0, 15-26=C 10-32=0/953, 8-32=-591/	9=-2773/0, //2687, I25, 19-20=-1070/7 3203, 32-33=0/320 552/0, , 12-27=-2453/0, //927, 0, 8-34=0/803,					
 2) All plates are 3x6 M 3) Plates checked for a 4) Refer to girder(s) fo 5) Recommend 2x6 st Strongbacks to be a 6) CAUTION, Do not e 7) Hanger(s) or other of Ib down at 14-94 co 8) In the LOAD CASE(LOAD CASE(S) Stan 1) Dead + Floor Live (I Uniform Loads (plf) Vert: 22-38 Concentrated Loads 	connection device(s) shall be provided su on top chord. The design/selection of suc (S) section, loads applied to the face of the dard balanced): Lumber Increase=1.00, Plate =-10, 1-21=-100	is center. c and fastened to each tr strained by other means. ifficient to support concer ch connection device(s) is ne truss are noted as fron	ntrated load(s) 241 s the responsibility of	lb down at 4-		6	December	22,2021





H			14-8-8				I
Plate Offsets (X,Y)	[1:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8	,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.32 BC 0.65 WB 0.39 Matrix-S	Vert(LL) -0.13	n (loc) l/defl 12-13 >999 12-13 >947 9 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 75 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF WEBS 2x4 SF REACTIONS. (siz	 No.1(flat) No.1(flat) No.3(flat) a) a) a) b) a) b) b) a) b) b) b) b) b) c) c)		BRACING- TOP CHORD BOT CHORD	except end verti	cals.	ectly applied or 6-0-0 or 10-0-0 oc bracing.) oc purlins,
TOP CHORD 2-3= BOT CHORD 15-10 9-10 9-10 WEBS 2-16	Comp./Max. Ten All forces 250 (lb) of -1611/0, 3-4=-2484/0, 4-5=-2748/0, 5-6= 6=0/977, 14-15=0/2210, 13-14=0/2748, =0/976 =-1225/0, 2-15=0/825, 3-15=-780/0, 3-1- =0/826, 6-10=-781/0, 6-11=0/414, 5-11=	2484/0, 6-7=-1610/0 12-13=0/2748, 11-12=0/2 4=0/414, 4-14=-496/0, 7-5	748, 10-11=0/2210,				
NOTES-							

Unbalanced floor live loads have been considered for this design.

All plates are 3x4 MT20 unless otherwise indicated.

Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

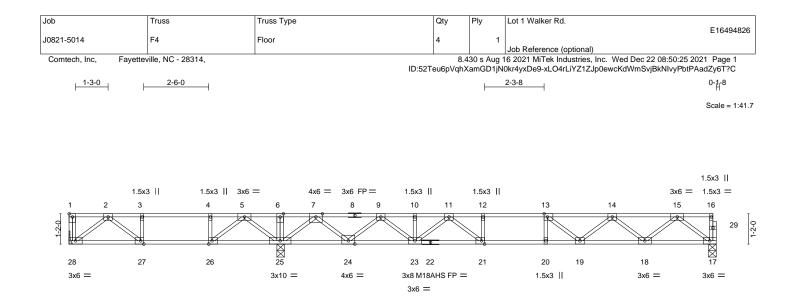
5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

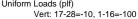
6) CAUTION, Do not erect truss backwards.







H	8-1-8 8- 8-1-8 0	1 ₁ 12 0-4		24-11-0 16-9-4			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [13:0-1-8,Edge], [21:0	1-8,Edge], [26:0-1-8,Edge	e], [27:0-1-8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.85 BC 0.86 WB 0.54 Matrix-S	Vert(LL) -0.21	(loc) l/defl 21-23 >960 21-23 >717 17 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS Weight: 123 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S		1	BRACING- TOP CHORD BOT CHORD	except end verti	cals.	ctly applied or 6-0-0 o 6-0-0 oc bracing.	oc purlins,
FORCES. (Ib) - Max FOP CHORD 1-28 7-9= 13-1 30T CHORD 27-2 21-2 WEBS 2-22 9-24	Grav 28=1890(LC 3), 25=1573(LC 1), 1 3=-1554/0, 2-3=-611/299, 3-4=-611/299 3=-1554/0, 9-10=-2489/0, 10-11=-2489/0 14=-2795/0, 14-15=-1764/0 28=-60/417, 26-27=-299/611, 25-26=-73 23=0/2903, 20-21=0/3160, 19-20=0/316 3=-523/75, 2-27=-306/248, 5-25=-821/0 4=-1078/0, 9-23=0/756, 11-23=-561/0, 1 3=-428/0, 15-18=0/917, 14-18=-871/0, 1	r less except when show 4-5=-611/299, 5-6=0/122 11-12=-3160/0, 12-13=-3 4/169, 24-25=-45/267, 23 0, 18-19=0/2434, 17-18=0 5-26=0/874, 7-25=-1526 1-21=0/630, 12-21=-279//	5, 6-7=0/1225, 1160/0, -24=0/1921, //1060 0, 7-24=0/1130, 0, 15-17=-1326/0,				
 All plates are MT20 All plates are 3x4 N Plates checked for S Refer to girder(s) fr Recommend 2x6 s Strongbacks to be CAUTION, Do not LOAD CASE(S) Star 	ve loads have been considered for this) plates unless otherwise indicated. AT20 unless otherwise indicated. a plus or minus 1 degree rotation abour or truss to truss connections. trongbacks, on edge, spaced at 10-0-0 attached to walls at their outer ends or n erect truss backwards. mdard (balanced): Lumber Increase=1.00, Plat	its center. oc and fastened to each t estrained by other means		3") nails.		AND S	2000

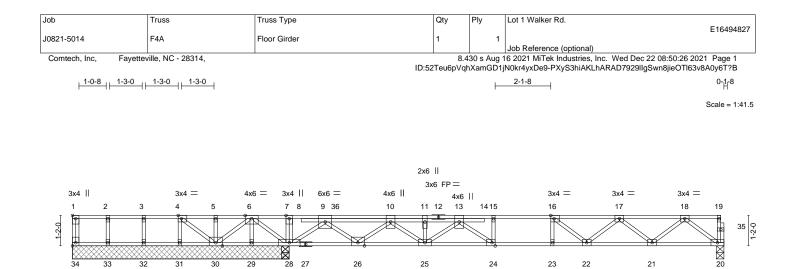


Concentrated Loads (lb) Vert: 1=-1500



December 22,2021





3x6 =

3x4 =

3x4 =

3x4 =

3x6 =

L	8-0-0 8	-3-8		24-11-0		
Plate Offsets (X,Y)	8-0-0 0 [1:Edge,0-1-8], [4:0-1-8,Edge], [16:0-1-		[34:Edge,0-1-8]	16-7-8		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.73 BC 0.85 WB 0.61 Matrix-S	DEFL. in Vert(LL) -0.16	n (loc) I/defl L/d 22-23 >999 480 22-23 >879 360 20 n/a n/a	PLATES MT20 Weight: 135 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing d except end verticals. Rigid ceiling directly applied 6-0-0 oc bracing: 29-30,28-2	or 10-0-0 oc bracing,	• •
(Ib) - Max L Max C	earings 8-3-8 except (jt=length) 20=0-3- Jplift All uplift 100 lb or less at joint(s) e 5rav All reactions 250 lb or less at joint 20=764(LC 4) . Comp./Max. Ten All forces 250 (lb) o	xcept 29=-517(LC 4), 30= (s) 34, 30, 31, 32, 33 exce	ept 28=2735(LC 1), 28=2			
11-1 BOT CHORD 29-3	0/366, 5-6=0/366, 6-7=0/2801, 7-9=0/28 3=-1711/0, 13-15=-2538/0, 15-16=-2538 0=-1208/0, 28-29=-1208/0, 26-28=-1059	8/0, 16-17=-2364/0, 17-18 9/0, 25-26=0/1024, 24-25=	=-1549/0			
WEBS 6-28 10-2	4=0/2538, 22-23=0/2538, 21-22=0/2127 =-1978/0, 6-29=0/507, 6-30=0/1075, 4-3 6=-1254/0, 10-25=0/857, 13-25=-601/0, 1=-753/0, 17-22=0/364, 16-22=-390/0	0=-459/0, 9-28=-2246/0, 9				
 All plates are 1.5x3 Plates checked for a Provide mechanical joint 30 and 226 bt Recommend 2x6 st Strongbacks to be a CAUTION, Do not e 	rongbacks, on edge, spaced at 10-0-0 of attached to walls at their outer ends or re	ts center. ng plate capable of withsta oc and fastened to each tr strained by other means.	uss with 3-10d (0.131" X	3") nails.		and the second s

chord. The design/selection of such connection device(s) is the responsibility of others.

4x6 =

3x6 FP =

3x10 =

4x6 =

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

3x4 ||

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

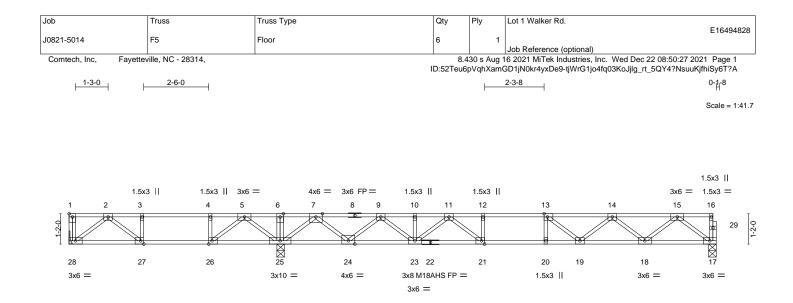
Vert: 20-34=-10, 1-19=-100

Concentrated Loads (lb) Vert: 36=-411(B)









 		-1 _F 12		24-11-0		
Plate Offsets (X,Y			, [27:0-1-8,Edge]	10-9-4		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.73 BC 0.78 WB 0.54 Matrix-S	Vert(LL) -0.21	(loc) I/defl L/d 21-23 >960 480 21-23 >717 360 17 n/a n/a	PLATES MT20 M18AHS Weight: 123 lb	GRIP 244/190 186/179 FT = 20%F, 11%E
BOT CHORD 2>	4 SP No.1(flat) 4 SP No.1(flat) 4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied o	,	oc purlins,
	(size) 28=Mechanical, 25=0-3-8, 17=0- ax Uplift 28=-16(LC 4) ax Grav 28=391(LC 3), 25=1573(LC 1), 1					
TOP CHORD	Max. Comp./Max. Ten All forces 250 (lb) 2-3=-611/299, 3-4=-611/299, 4-5=-611/299 9-10=-2489/0, 10-11=-2489/0, 11-12=-316 4-15=-1764/0	, 5-6=0/1225, 6-7=0/1225, 7	7-9=-1115/0,			
	27-28=-60/416, 26-27=-299/611, 25-26=-7 21-23=0/2903, 20-21=0/3160, 19-20=0/316					
WEBS	1-23=0/2903, 20-21=0/3100, 19-20=0/310 2-28=-521/76, 2-27=-305/250, 5-25=-821/0 14-19=0/501, 13-19=-603/0, 7-25=-1526/0 0-24=-1078/0, 9-23=0/756, 11-23=-561/0,	, 15-17=-1327/0, 15-18=0/9 , 7-24=0/1130, 5-26=0/874,	917, 14-18=-871/0,			

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x4 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Refer to girder(s) for truss to truss connections.

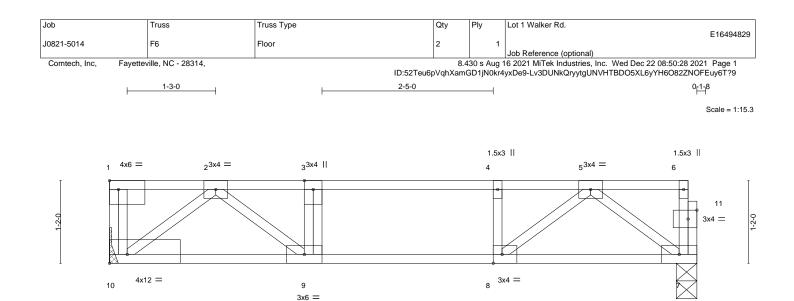
7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

8) CAUTION, Do not erect truss backwards.







			8-3-8 8-3-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [8:0-1-8,Edge], [10:Edg	e,0-1-8], [11:0-1-8,0-1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.42 BC 0.27 WB 0.22 Matrix-S	DEFL. ir Vert(LL) -0.04 Vert(CT) -0.05 Horz(CT) 0.01	9-10 >999 480 9-10 >999 360	PLATES MT20 Weight: 43 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing din except end verticals. Rigid ceiling directly applied o) oc purlins,

REACTIONS. (size) 10=Mechanical, 7=0-3-8 Max Grav 10=4092(LC 1), 7=436(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-10=-3709/0, 2-3=-810/0, 3-4=-810/0, 4-5=-810/0

 BOT CHORD
 9-10=0/489, 8-9=0/810, 7-8=0/485

 WEBS
 2-10=-614/0, 2-9=0/451, 5-7=-604/0, 5-8=0/460

WEBS 2-10=-014/0, 2-9=0/451, 5-7=-004/0, 5-6=

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

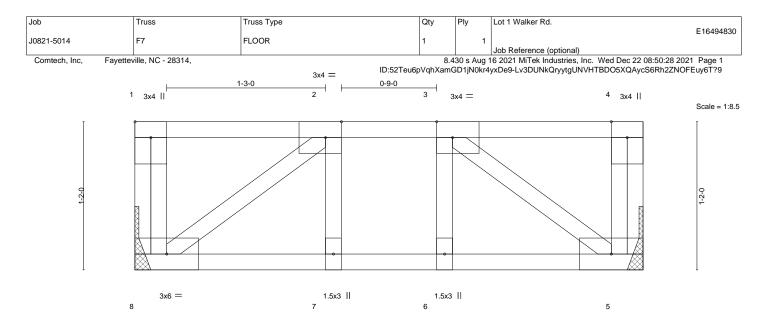
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 7-10=-10, 1-6=-100 Concentrated Loads (lb) Vert: 1=-3650

3x6 =

December 22,2021





3x6 =

Plate Offsets (X,Y)		Edaol	4-0-0 4-0-0			
Plate Offsets (X, Y)	[1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,	Edgej				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.09 BC 0.06 WB 0.06 Matrix-S	DEFL. ir Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	7 >999 480 7 >999 360	PLATES MT20 Weight: 25 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP	? No.1(flat) ? No.1(flat) ? No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing diru except end verticals. Rigid ceiling directly applied o	ectly applied or 4-0-0	

REACTIONS. (size) 8=Mechanical, 5=Mechanical Max Grav 8=206(LC 1), 5=206(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

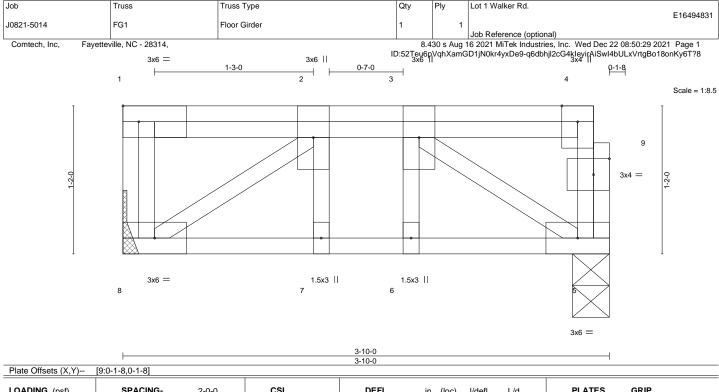
1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

a) Refer to girder(s) for truss to truss connections.
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

A designed to be December 22,2021





LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.08 WB 0.07 Matrix-S	DEFL. in Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	0 7 >999 480 0 7 >999 360	PLATES MT20 Weight: 28 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	? No.1(flat) ? No.1(flat) ? No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	<i>y</i> 11	
REACTIONS. (size Max G	e) 8=Mechanical, 5=0-3-8 Brav 8=261(LC 1), 5=234(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-257/0

BOT CHORD 7-8=0/257, 6-7=0/257, 5-6=0/257

WEBS 3-5=-307/0, 2-8=-311/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 135 lb down at 1-10-4 on top

chord. The design/selection of such connection device(s) is the responsibility of others.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

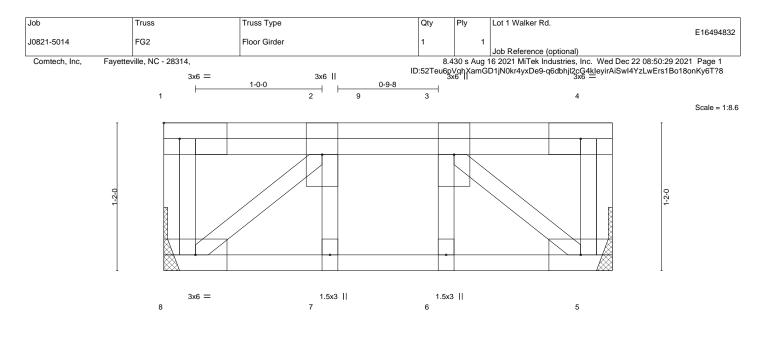
Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb)









3x6	=
-----	---

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

						3-6-8 3-6-8						
LOADING TCLL TCDL	(psf) 40.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.00 1.00	CSI. TC BC	0.22 0.16	DEFL. Vert(LL) Vert(CT)	in -0.00 -0.01	(loc) 7 7	l/defl >999 >999	L/d 480 360	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 5.0	Rep Stress Incr Code IRC2015/TF	NO	WB Matri	0.18	Horz(CT)	0.00	5	>9999 n/a	n/a	Weight: 27 lb	FT = 20%F, 11%E
LUMBER-		P No.1(flat)		1		BRACING- TOP CHOR		Structu	ral wood	sheathing di	irectly applied or 3-6-8	oc purlins,

BOT CHORD

TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat) 2x4 SP No.3(flat) WFBS

REACTIONS. (size) 8=Mechanical, 5=Mechanical Max Grav 8=546(LC 1), 5=511(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-590/0

BOT CHORD 7-8=0/590, 6-7=0/590, 5-6=0/590

WEBS 2-8=-765/0, 3-5=-765/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 720 lb down at 1-8-4 on top

chord. The design/selection of such connection device(s) is the responsibility of others.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

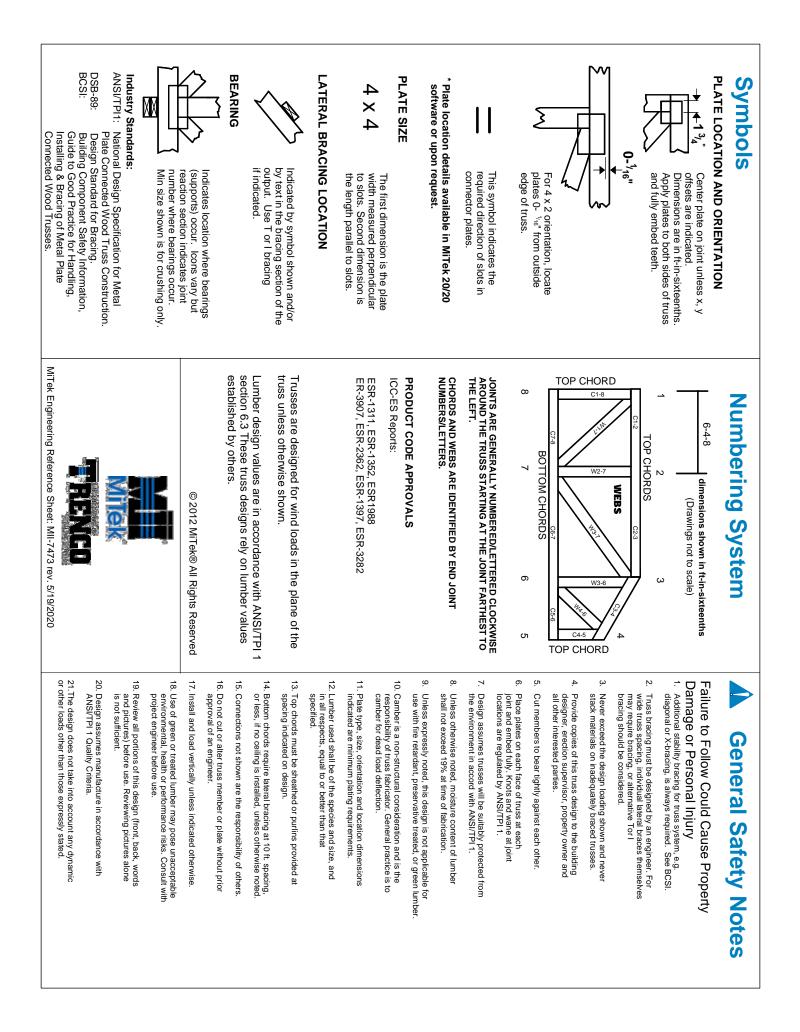
Uniform Loads (plf)

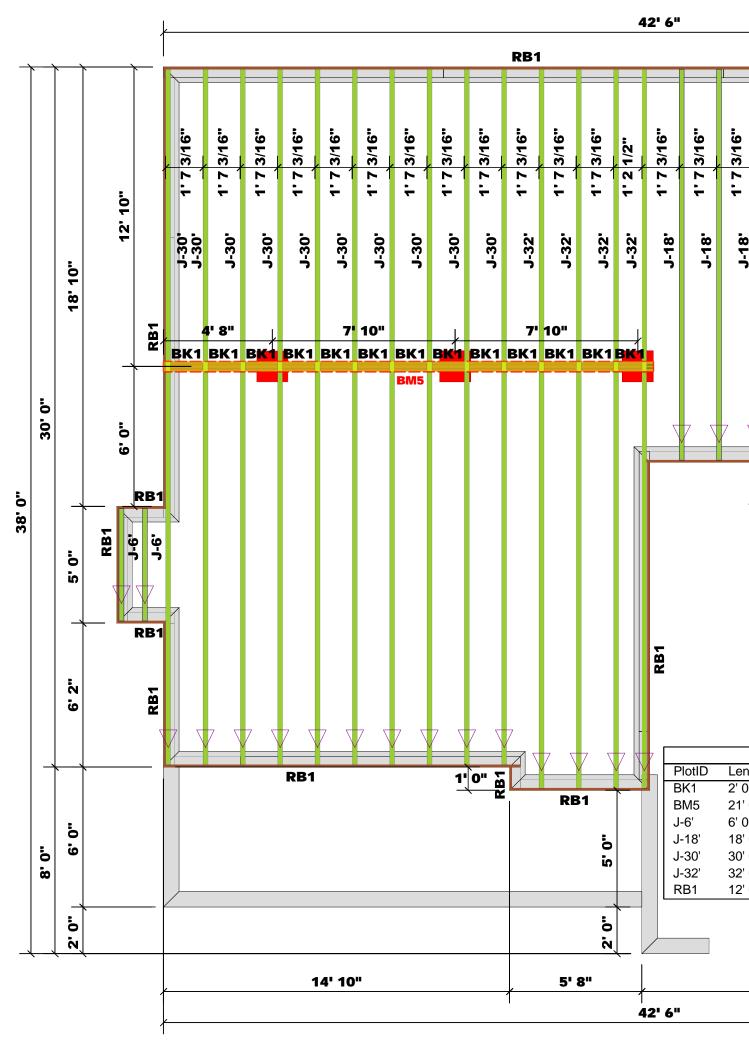
Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb)

Vert: 9=-695(B)









×	1. 7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1.7 3/16"	1. 7 3/16"	1.7 3/16"	1' 7 3/16"	,		-	
	J-18'	J-18'	J-18'	J-18'	J-18'	J-18'	J-18'	J-18'	J-18') J-18'	J-18'	RB1	16. 11 1/4"	
	7 7	7 \	7	7 \	7 7	7 \	7 5	7		7 5		7		
			RB1 ss Pla	ceme	nt Pla									-
	1. F 2. (All N OnSi Plumbing Contractions	ss Pla 2 1/4"= Walls dere Plum g drop lo or to ver s prior to	s Sh d Lc bing Dro cations ify ALL	OWN pad E pp Notes shown a plumbing Floor Tr	Are Beari	exact.						0 3/4"	38' 0"
t ''''	C 1. F 2. (3. / 1. T 1.	All N OnSi Plumbing Contractions	SS Pla 2 1/4"= Wall: dere Plum g drop lc or to ver s prior to pacing a NI-40x 9-1/4" NI-40x NI-40x NI-40x NI-40x NI-40x NI-40x	s Sh d LC bing Dro cations ify ALL setting s neede Produ	OWN pad E shown a plumbing Floor Tr d not to acts erto-S	Are Beari	exact. 24"oc.	L)	Fab FF FF MFI MFI MFI FF	Type			21' 0 3/4"	38' 0"

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identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com