

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 2100795-2100795A Freedom Solar Clayton Plan

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I48131521 thru I48131538

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



September 30,2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



| | 6-1-12 6-1-12 | + <u>12-11-8</u> 6-9-12 | 8 | <u>22-11-8</u> 10-0-0 | 29-9-4 | | 35-11-0 6-1-12 | |
|--|--|--------------------------------------|---|---|--|--------------------------|----------------------------------|------------------------------------|
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI2 | 2-0-0 1.15 1.15 YES 2014 | CSI. TC 1.00 BC 0.91 WB 0.75 Matrix-MS | DEFL. ir Vert(LL) -0.41 Vert(CT) -0.68 Horz(CT) 0.10 | (loc) l/defl 15-16 >999 15-16 >632 12 n/a | L/d 240 180 n/a | PLATES MT20 Weight: 214 lb | GRIP 197/144 FT = 20% |

LUMBER-

| TOP CHORD | 2x4 SP No.2 or 2x4 SPF No.2 *Except* 1-5: 2x4 SP No.1 |
|-----------|--|
| BOT CHORD | 2x4 SP No.1 |
| WEBS | 2x4 SP No.3 |
| SLIDER | Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0 |
| | |

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 12=Mechanical Max Horz 2=271(LC 11) Max Uplift 2=-186(LC 12), 12=-160(LC 13) Max Grav 2=1510(LC 1), 12=1435(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2203/380, 4-6=-1865/378, 6-7=-1909/527, 7-8=-1911/528, 8-10=-1867/379, 10-12=-2210/381

 BOT CHORD
 2-18=-315/1974, 16-18=-315/1974, 15-16=-36/1258, 13-15=-246/1830, 12-13=-246/1830

 WEBS
 7-15=-270/1014, 8-15=-428/256, 10-15=-385/197, 7-16=-269/1011, 6-16=-428/255, 4-16=-388/194

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=160.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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| | 6-1-12 | 12-1 | 1-8 | 22-11-8 | 29-9-4 | 29-11-0 | 35-11-0 | |
|---------------|------------------|-------|-----------|----------------|--------------|---------|----------------|----------|
| | 0-1-12 | | 12 | 10-0-0 | 0-9-12 | 0-1-12 | 0-0-0 | |
| LOADING (psf) | SPACING- | 2-0-0 | CSI. | DEFL. ir | (loc) l/defl | L/d | PLATES | GRIP |
| TCLL 20.0 | Plate Grip DOL | 1.15 | TC 0.73 | Vert(LL) -0.41 | 16-17 >874 | 240 | MT20 | 197/144 |
| ICDL 10.0 | Lumber DOL | 1.15 | BC 0.78 | Vert(C1) -0.63 | 16-17 >565 | 180 | | |
| BCDL 10.0 | Code IRC2015/TPI | 2014 | Matrix-MS | Horz(CT) 0.03 | 14 n/a | n/a | Weight: 216 lb | FT = 20% |

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2 *Except*

 15-18: 2x4 SP No.1
 15-18: 2x4 SP No.3

 SLIDER
 Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

BRACING-TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 2-11-6 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 7-16

REACTIONS. (size) 2=0-3-0, 14=0-3-8 Max Horz 2=-277(LC 10) Max Uplift 2=-171(LC 12), 14=-225(LC 13) Max Grav 2=1210(LC 19), 14=1821(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1671/246, 4-6=-1338/240, 6-7=-1379/389, 7-8=-936/275, 8-10=-906/167,

10-12=-304/573

 BOT CHORD
 2-19=-281/1566, 17-19=-281/1566, 16-17=-0/801, 14-16=-394/345, 12-14=-394/345

 WEBS
 8-16=-394/251, 10-16=-204/1203, 10-14=-1647/496, 7-17=-270/1031, 6-17=-423/256, 4-17=-432/196

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This

5) One H2.5A Simpson Strong-Tile connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This connection is for uplift only and does not consider lateral forces.











| | | 6-1-12 | 7-7-8 | 12-11-8 | 1 | 22-11-8 | | | 29-9-4 | 29-1 <mark>1-0</mark> | 35-11-0 | |
|--|--|---|---|--|---|--|------------------------------|------------------------------|-------------------------------|--------------------------|----------------|------------------------|
| | | 6-1-12 | 1-5-12 | 5-4-0 | 1 | 10-0-0 | | | 6-9-12 | 0-1-12 | 6-0-0 | |
| Plate Offsets (X,Y) | [2:Edge,0-0-0] |], [5:0-1-12, | Edge], [7:0-2- | 0,0-0-0], [12: | :Edge,0-0-0] | | | | | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * | SPACII Plate G Lumber Rep Sti | NG- Jrip DOL r DOL ress Incr | 2-0-0 1.15 1.15 YES | CSI. TC BC WB | 0.70 0.40 0.62 | DEFL. Vert(LL) Vert(CT) Horz(CT) | in -0.03 -0.06 0.01 | (loc) 27-53 27-53 2 | l/defl >999 >999 n/a | L/d 240 180 n/a | PLATES MT20 | GRIP 197/144 |
| BCDL 10.0 | Code I | RC2015/1P | 12014 | Iviau | X-1VIS | | | | | | weight. 555 lb | FI = 20% |
| LUMBER-BRACING-TOP CHORD2x4 SP No.2 or 2x4 SPF No.2TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins.SOT CHORD2x4 SP No.2 or 2x4 SPF No.2BOT CHORDRigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 2-27,26-27,24-26.VEBS2x4 SP No.3WEBS1 Row at midpt7-18, 7-24SLIDERAll bearings 22-3-8 except (jt=length) 2=0-3-8, 26=0-3-8. (lb)WEBS1 Row at midpt7-18, 7-24REACTIONS. (lb)All bearings 22-3-8 except (jt=length) 2=0-3-8, 26=0-3-8. (lb)Max Horz 2=-277(LC 10) Max Uplitf All uplift 100 lb or less at joint(s) 2, 23, 15 except 18=-234(LC 13), 14=-147(LC 13), 24=-242(LC 12)) Max Grav All reactions 250 lb or less at joint(s) 21, 22, 20, 19, 16, 15, 26 except 2=514(LC 23), 18=531(LC 20), 14=976(LC 24), 24=864(LC 19)Structural wood sheathing directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 2-27,26-27,24-26. | | | | | | | | | | | | |
| FORCES. (lb) - Ma TOP CHORD 2 BOT CHORD 2-2 15 21 WEBS 7- 4-2 4-2 | ax. Comp./Max. T 4=-361/104, 8-10: 27=-155/480, 26- -22=-191/266, 20: -16=-388/331, 14 18=-279/41, 8-18: 24=-522/243 | [−] en All ford =-54/261, 1 .27=-155/48]-21=-191/2 4-15=-388/3 }=-400/252, | ces 250 (lb) o 0-12=-285/56 30, 24-26=-155 266, 19-20=-19 31, 12-14=-38 10-18=-64/29 | r less except 4 5/480, 23-24- 11/266, 18-19 18/331 5, 10-14=-74 | : when shown. =-191/266, 22 9=-191/266, 1 13/348, 6-24=- | -23=-191/266, 6-18=-388/331, -410/255, | | | | | | |
| 1) Unbalanced roof | live loads have be | een conside | ered for this de | esian | | | | | | | | |

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.







| | | | <u>35-11-0</u> 35-11-0 | | | | | |
|--|---|---|---|-----------------------------------|--|---|---|------------------------------------|
| Plate Offsets (X,Y) | [2:Edge,0-0-0] | | | | | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.11 BC 0.09 WB 0.14 Matrix-S | DEFL. Vert(LL) -0.1 Vert(CT) -0.1 Horz(CT) 0.1 | in (loc))0 1)0 1)1 22 | l/defl n/r n/r n/a | L/d 120 90 n/a | PLATES MT20 Weight: 266 lb | GRIP 197/144 FT = 20% |
| LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4 OTHERS 2x4 SLIDER Left | SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2 SP No.3 SP No.3 2x4 SP No.3 1-6-0 | | BRACING- TOP CHORD BOT CHORD WEBS | Struct excep Rigid 1 Rov | ural wood t end verti ceiling dire / at midpt | sheathing dir icals. ectly applied o 1 | rectly applied or 6-0-0 c or 10-0-0 oc bracing. 2-31, 11-32, 10-33, 13- | oc purlins, -30, 14-29 |
| REACTIONS. All (lb) - Max Max Max | bearings 35-11-0. Horz 2=281(LC 9) Uplift All uplift 100 lb or less at joint(s) 2 except 2=-109(LC 8), 39=-115(LC ' Grav All reactions 250 lb or less at joint 25, 24, 23 except 31=251(LC 13) | 2, 31, 32, 33, 34, 35, 36, 3 12), 23=-121(LC 13) (s) 22, 2, 32, 33, 34, 35, 3 | 37, 38, 30, 29, 28, 27, 6, 37, 38, 39, 30, 29, 2 | 26, 25, 24 8, 27, 26 | ι , | | | |

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-283/239, 10-11=-223/262, 11-12=-260/292, 12-13=-260/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) N/A

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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ACTIONS. (size) 14=0-3-8, 10=0-3-8 Max Horz 14=271(LC 11) Max Uplift 14=-121(LC 12), 10=-121(LC 13) Max Grav 14=1026(LC 1), 10=1026(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-4=-452/214, 4-5=-1061/330, 5-6=-1061/330, 6-8=-452/214, 2-14=-447/220, 8-10=-446/220

 BOT CHORD
 13-14=-135/988, 11-13=0/675, 10-11=-43/856

 WEBS
 5-11=-161/546, 6-11=-309/268, 5-13=-161/546, 4-13=-309/268, 4-14=-882/47, 6-10=-882/47

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 10. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 ENGINEERING BY ERENCO A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

| Job | Truss | Truss Type | Qty | Ply | Freedom Solar Clayton Plan | |
|-----------------------|-------------------|---------------|---------|------------|---|-----------|
| | | | | | | 148131529 |
| 2100795-2100795A | BGR | COMMON GIRDER | 1 | 2 | | |
| | | | | J | Job Reference (optional) | |
| 84 Components (Dunn), | Dunn, NC - 28334, | | 8.5 | 20 s Aug 2 | 27 2021 MiTek Industries, Inc. Wed Sep 29 07:47:50 2021 | Page 2 |
| | | ID:ad6 | wRvDk0A | 62AolHxoo | c6?XyYrFB-dnZL5JTwJvRqlA7sJtqYZH6QhWG t8e8k??6da | dyYqs7 |

NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1415 lb down and 180 lb up at 2-0-12, 1415 lb down and 180 lb up at 4-0-12, 1415 lb down and 180 lb up at 6-0-12, 1415 lb down and 180 lb up at 7-6-4, 1415 lb down and 180 lb up at 9-6-4, 1415 lb down and 180 lb up at 11-6-4, 1415 lb down and 180 lb up at 13-6-4, 1415 lb down and 180 lb up at 13-6-4, 1415 lb down and 180 lb up at 13-6-4, 1415 lb down and 180 lb up at 13-6-4, and 1415 lb down and 180 lb up at 13-6-4, and 1415 lb down and 180 lb up at 13-6-4, and 1415 lb down and 180 lb up at 13-6-4, and 1421 lb down and 174 lb up at 23-6-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 10-13=-20 Concentrated Loads (lb)

Vert: 6=-1415(B) 9=-1415(B) 15=-1421(B) 16=-1415(B) 17=-1415(B) 18=-1415(B) 19=-1415(B) 20=-1415(B) 21=-1415(B) 22=-1415(B) 23=-1415(B) 24=-1415(B) 24





| | | | 11-11-0 | | | | |
|---|---|---------------------------------------|--|-------------------------------------|------------------------------------|--------------------|------------------------|
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES | CSI. TC 0.13 BC 0.05 WB 0.07 | DEFL. in Vert(LL) -0.01 Vert(CT) -0.01 Horz(CT) 0.00 | (loc) l/de 9 n, 9 n, 10 n/ | fl L/d /r 120 /r 90 a n/a | PLATES (MT20 1 | GRIP 197/144 |
| | | Matrix-R | BRACING. | | | Weight: 69 lb | FT = 20% |

LUMBER-

| TOP CHORD | 2x4 SP No.2 or 2x4 SPF No.2 |
|-----------|-----------------------------|
| BOT CHORD | 2x4 SP No.2 or 2x4 SPF No.2 |
| WEBS | 2x4 SP No.3 |
| OTHERS | 2x4 SP No.3 |

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 11-11-0.

(lb) - Max Horz 16=-160(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11

Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







| | 5-1-12 | 2 | 7-11-0 2-9-4 | <u>12-11-0</u> 5-0-0 | |
|--|--|--|---|---|---|
| Plate Offsets (X,Y) | - [2:0-3-11,0-0-4], [6:0-3-11,0-0-4] | | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014 | CSI. TC 0.72 BC 0.81 WB 0.33 Matrix-MP | DEFL. ir Vert(LL) 0.06 Vert(CT) -0.05 Horz(CT) 0.03 | n (loc) I/defl L/d 5 9-12 >999 240 9-12 >999 180 3 2 n/a n/a | PLATES GRIP MT20 197/144 Weight: 66 lb FT = 20% |
| LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 | SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2 | | BRACING- TOP CHORD BOT CHORD | Structural wood sheathing dir Rigid ceiling directly applied c | ectly applied or 6-0-0 oc purlins. ır 5-9-13 oc bracing. |

WEBS 2x4 SP No.3 SLIDER Left 2x6 SP No.2 1-6-0, Right 2x6 SP No.2 1-6-0

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-111(LC 10) Max Uplift 2=-63(LC 12), 8=-136(LC 13) Max Grav 2=302(LC 23), 8=973(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-349/347, 4-6=-350/554

BOT CHORD 2-9=-190/299, 8-9=-212/287, 6-8=-422/409

WEBS 4-8=-812/397

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.

SEAL 044925 September 30,2021

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Edenton, NC 27932



| | 5-1- | 12 | 7-11-0 | 12-11-0 | |
|--|---|---|---|---|---|
| Plate Offsets (X,Y) | [2:0-3-11,0-0-4], [4:0-1-8,0-1-8], [6:0-3- | 11,0-0-4] | 2-9-4 | 5-0-0 | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.72 BC 0.81 WB 0.33 Matrix-MP | DEFL. in Vert(LL) 0.06 Vert(CT) -0.09 Horz(CT) 0.03 | (loc) l/defl L/d 9-21 >999 240 9-21 >999 180 2 n/a n/a | PLATES GRIP MT20 197/144 Weight: 84 lb FT = 20% |
| LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF OTHERS 2x4 SF SLIDER Left 2x | P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3 P No.3 6 SP No.2 1-6-0, Right 2x6 SP No.2 1-6 | ;0 | BRACING- TOP CHORD BOT CHORD | Structural wood sheathing di Rigid ceiling directly applied | rectly applied or 6-0-0 oc purlins. or 5-9-13 oc bracing. |
| REACTIONS. (siz Max H Max U Max C | e) 2=0-3-8, 8=0-3-8 lorz 2=-111(LC 10) lplift 2=-63(LC 12), 8=-136(LC 13) grav 2=302(LC 23), 8=973(LC 1) | | | | |
| FORCES. (lb) - Max. TOP CHORD 2-4=: BOT CHORD 2-9=: WEBS 4-8=: | Comp./Max. Ten All forces 250 (lb) o -349/347, 4-6=-350/554 -190/299, 8-9=-212/287, 6-8=-422/409 -812/397 | · less except when shown. | | | |
| NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; \ gable end zone and forces & MWFRS fo 3) Truss designed for Gable End Details a 4) All plates are 2x4 M | e loads have been considered for this de /ult=130mph Vasd=103mph; TCDL=6.0 C-C Exterior(2) zone; cantilever left and r reactions shown; Lumber DOL=1.60 p wind loads in the plane of the truss only is applicable, or consult qualified buildin T20 unless otherwise indicated. | rsign. ssf; BCDL=6.0psf; h=30ft; 0 J right exposed ; end vertica late grip DOL=1.60 For studs exposed to wind g designer as per ANSI/TP ¹ | Cat. II; Exp B; Enclosed; al left and right exposed d (normal to the face), so I 1. | ; MWFRS (envelope) l;C-C for members and ee Standard Industry | |

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.





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REACTIONS. (size) 1=9-0-7, 3=9-0-7, 4=9-0-7 Max Horz 1=-76(LC 10) Max Uplift 1=-39(LC 12), 3=-49(LC 13) Max Grav 1=181(LC 1), 3=181(LC 1), 4=294(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







| | | 0-0-5 | | 5-11-7 | | 1 |
|--------|---------|----------------------|-------------|-----------------|--------------------|------------------------|
| LOADIN | G (psf) | SPACING- 2-0-0 | CSI. | DEFL. in | n (loc) l/defl L/d | PLATES GRIP |
| | 20.0 | Plate Grip DOL 1.15 | TC 0.20 | Vert(LL) n/a | - n/a 999 | MT20 244/190 |
| TCDL | 10.0 | Lumber DOL 1.15 | BC 0.11 | Vert(CT) n/a | n - n/a 999 | |
| BCLL | 0.0 * | Rep Stress Incr YES | WB 0.02 | Horz(CT) 0.00 | 3 n/a n/a | |
| BCDL | 10.0 | Code IRC2015/TPI2014 | Matrix-P | | | Weight: 21 lb FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

OTHERS 2x4 SP No.3

REACTIONS. 1=5-11-2, 3=5-11-2, 4=5-11-2 (size) Max Horz 1=-47(LC 10) Max Uplift 1=-24(LC 12), 3=-31(LC 13) Max Grav 1=113(LC 1), 3=113(LC 1), 4=183(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

\sim Contraction of the WITTER PARTY SEAL 044925 mm September 30,2021

Structural wood sheathing directly applied or 5-11-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



