

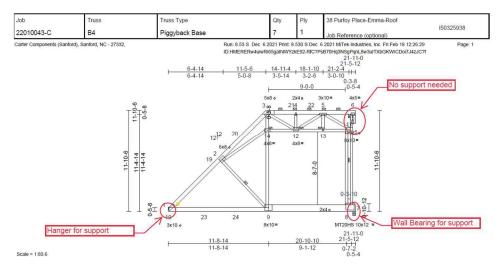
298 Harvey Faulk Rd. Sanford, NC 27332 919-775-1450

RE: Richard Beland – Lot 38 Purfoy Place

May 5, 2022

To whom it may concern:

It was brought to our attention that the that there was concern about no bearing at the top of trusses B4 & B5. The trusses were designed with only the bottom of the truss supported and no support at the top chord. See detail below:



After examination of the trusses in question it was determined that the trusses are installed as designed and no repairs or adjustments are needed.

Feel free to contact us if you have any further questions or need anything else.

Kelly Hudson

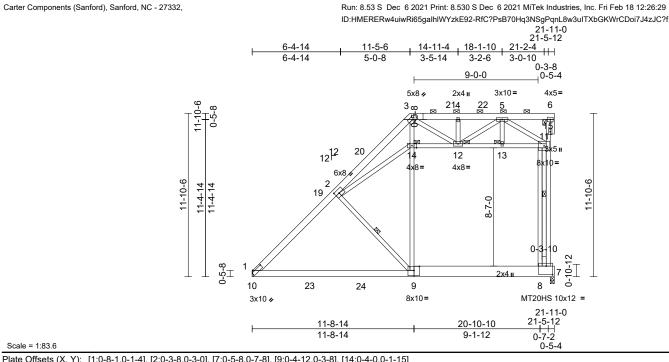
Components Specialist



A DIVISION OF CARTER LUMBER 298 Harvey Faulk Rd. Sanford, NC 27332 Office: 919-775-1450 Cell: 704-467-1857 <u>kelly.hudson@KempsvilleBuilding.com</u> <u>www.kempsvillebuilding.com</u>

Job	Truss	Truss Type	Qty Ply 38 Purfoy Place-Emma-Roof	38 Purfoy Place-Emma-Roof		
22010043-C	B4	Piggyback Base	7	1	Job Reference (optional)	150325938

Carter Components (Sanford), Sanford, NC - 27332,



Scale = 1:83.6

Plate Offsets (X, Y): [1:0-8-1,0-1-4],	, [2:0-3-8,0-3-0], [7:0-	5-8,0-7-8	3], [9:0-4-12,0-3	3-8], [14:0-4-0,0-1-	15]							
Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.98	Vert(LL)	-0.29	9-17	>899	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.80	Vert(CT)	-0.53	9-17	>482	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	YES		WB	0.95	Horz(CT)	0.01	7	n/a	n/a		
BCLL	0.0*	Code	IRC20	18/TPI2014	Matrix-MSH		Attic	-0.10	8-9	>999	360		
BCDL	10.0		-									Weight: 245 lb	FT = 8%
LUMBER			1) Wind: ASCE	7-16; Vult=145m	ph (3-se	cond gust)		14) Gra	aphical p	urlin re	presentation doe	s not depict the size
TOP CHORD	2x6 SP No.2				ph; TCDL=6.0psf;							of the purlin along	g the top and/or
BOT CHORD	2x10 SP 2400F 2.0E	E *Except* 9-10:2x6 8	SP		B; Enclosed; MWF					tom cho			
	No.2				C Exterior(2E) 0-0							IOWN IS DESIGI	NED AS
WEBS		ot* 6-7:2x4 SP 2400F			5-2, Exterior(2R) 8 -7-8 zone;C-C for					INHABI			
	2.0E, 14-11,3-9,2-14	1:2x4 SP No.2			reactions shown;				LOAD	CASE(S) Sta	ndard	
BRACING				grip DOL=1.		Lamber	502-1.00 pia						
TOP CHORD		athing directly applie			E 7-16; Pr=20.0 ps	f (roof L	.: Lum DOL=	1.15					
		cept end verticals, ar	D		1.15); Pf=20.0 psf								
BOT CHORD	2-0-0 oc purlins (4-7	applied or 10-0-0 oc		DOL=1.15);	Is=1.0; Rough Ca	ÈB; Fully	Exp.; Ce=0.9);					
	bracing.	applied of 10-0-0 oc		Cs=1.00; Ct									
NEBS	•	6-7, 2-9	3		snow loads have	been co	nsidered for th	nis					
JOINTS 1 Brace at Jt(s): 6,				design.									
	12, 13, 14		4		quate drainage to								
REACTIONS		echanical, 7=1148/0-	3-8 c		e MT20 plates unle			d.					
Max Horiz 1=571 (LC 14)					as been designed ad nonconcurrent			da					
	Max Uplift 1=-6 (LC		7										
	Max Grav 1=1213 (I	LC 40), 7=1701 (LC 3		 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 									
FORCES	(lb) - Maximum Corr	pression/Maximum		3-06-00 tall by 2-00-00 wide will fit between the bottom									
	Tension			chord and any other members, with BCDL = 10.0psf.								mun	1111
TOP CHORD	4-5=-2334/470, 5-6=-19/102,			8) Ceiling dead load (5.0 psf) on member(s). 12-14, 12-13, 11-13; Wall dead load (5.0 psf) on member(s).9-14, 8-11							IN'LY CA	ROUL	
	7-11=-2519/311, 6-1		9	 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 8-9 								O'.EESS	Idia Via
BOT CHORD	1-10=0/0, 1-8=-398/										下下	mint	unin
NEBS	2-9=-1286/553, 12-14=-606/2960,			 10) Refer to girder(s) for truss to truss connections. 11) Provide mechanical connection (by others) of truss to 									K : 5
	12-13=-258/1150, 1 9-14=-205/1622, 3-1		1		B lb uplift at jo				0	CEA.			
		3=-3/30, 3-12=-1363/2	240	1.	o ib upilit at jo	init		-		SEA			
	5-12=-233/1238, 5-1			 1. 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces. 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and 									4 : =
	2-14=-660/3131, 8-1				ing walls due	to		1					
NOTES					(s) 7. This connect						-		1. 5
				does not cor	. , ,				-,7	1. SNO	- ER: 53		
			1	3) This truss is	designed in accor	dance w	ith the 2018				14	O GIN	F.F. GUN
				Internationa	Residential Code	section	s R502.11.1 a	nd			1	TEM I	OHN
				R802.10.2 a	ind referenced stat	ndard Al	ISI/TPI 1.					WEW J	in the

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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February 22,2022

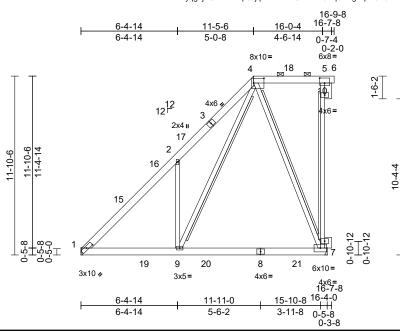
Page: 1

Job	Truss	Truss Type	Qty	Ply	38 Purfoy Place-Emma-Roof	
22010043-C	B5	Piggyback Base	2	1	Job Reference (optional)	150325939

Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.53 S Dec 6 2021 Print: 8.530 S Dec 6 2021 MiTek Industries, Inc. Fri Feb 18 12:26:29 ID:114LyqgbjZGmdz4b2p6LjqzkEVK-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:76.4

Plate Offsets (X, Y): [1:0-5-5,0-1-8], [4:0-8-4,0-3-12], [5:0-4-0,0-4-0]

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-MSH	0.22 0.50 0.53	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.14 -0.22 0.01	(loc) 7-9 7-9 7	l/defl >999 >889 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 153 lb	GRIP 244/190 ET = 8%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD WEBS	2x6 SP No.2 2x6 SP No.2 2x4 SP No.2 *Excep 11-7:2x6 SP No.2 2x4 SPF No.2(flat) * No.2 Structural wood she 6-0-0 oc purlins, ex 2-0-0 oc purlins (6-0 Rigid ceiling directly bracing.	Except* 10-5:2x6 SP athing directly applie cept end verticals, ar	d or ³⁾ nd 4)	Vasd=115mp Cat. II; Exp E zone and C-1 3-0-12 to 7-2 (2E) 13-9-8 t & MWFRS f grip DOL=1.0 TCLL: ASCE Plate DOL=1 DOL=1.15); Cs=1.00; Ct= Uhbalanced design.	7-16; Pr=20.0 ps .15); Pf=20.0 psf s=1.0; Rough Ca	BCDL=6 RS (env -12 to 3- -2-7 to 1 C for mer to for mer to the to the to	.0psf; h=35ft elope) exteric 0-12, Interior 3-9-8, Exteric mbers and foir r DOL=1.60 .: Lum DOL= .: Lum DOL= Exp.; Ce=0.9 asidered for th	(1) or rces olate 1.15 c;	LOAD	CASE(S) Sta		F I = 070
REACTIONS FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalance this design	of web with 10d (0.1 o.c.,with 3in minimu Brace must cover 9 (lb/size) 1=638/ M Mechanic Max Horiz 1=573 (LC Max Uplift 7=-338 (L Max Grav 1=817 (LC (lb) - Maximum Com Tension 1-2=-937/0, 2-4=-11 5-6=0/0, 5-7=-301/1 1-9=-353/729, 7-9=- 2-9=-603/555, 4-7=-	m end distance. 90% of web length. echanical, 7=686/ al C 14) C 40), 7=849 (LC 37) pression/Maximum 55/367, 4-5=-7/0, 20 144/247 649/389, 4-9=-519/1	ge 6) 7) 8) 9) 10 11 188 12	load of 12.0 overhangs n Provide adee This truss ha chord live loa * This truss f on the bottor 3-06-00 tall t chord and ar Refer to gird) Provide mec bearing plate joint 7. 1) This truss is International R802.10.2 at 2) Graphical pu or the orienta bottom chorc 3) Warning: Ad	psf or 1.00 times f pon-concurrent with uate drainage to is been designed ad nonconcurrent has been designed in chord in all area by 2-00-00 wide w any other members gr(s) for truss to tr hanical connection capable of withst designed in accor Residential Code and referenced star flin representation ation of the purlin is ditional permanen (not part of this c	lat roof le n other lip prevent of for a 10. with any d for a liv s where ill fit betw with BC uss conn n (by oth anding 3 dance w sections ndard AN n does ne along the t and stat	bad of 20.0 p: e loads. water ponding: 0 psf bottom other live load e load of 20.1 a rectangle ween the bottu DL = 10.0psf lections. ers) of truss t 38 lb uplift at ith the 2018 R502.11.1 a ISI/TPI 1. t depict the s e top and/or billity bracing	sf on g. ds. Opsf com f. no size		Continue.	and the second sec	SEA 4584	HA HANNE

February 22,2022

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