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HUS26 USP 4 NA 16d/3-1/2" 16d/3-1/2" Image: transmission of transmissicon of transmission of transmission of trans	ODEL	DDRES	DDRES	DDRES	DDRES		ODEL	ATE R	RAWN	ALES
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		Thomas Farm	14-R Thomas Farm	ot 31A-R Thomas Farm	Lot 31A-R Thomas Farm	Uichary "Q" / G1 CD	Hickory "C" / GL, CP	N/A		J0122-0097

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



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		32' 6"	Dimension Notes 1. All exterior wall to wall dimensions are to face of frame wall unless noted otherwise 2. All interior wall of truss dimensions are to face of frame wall unless noted otherwise 3. All exterior wall of truss dimensions are to face of frame wall unless noted otherwise Considered Load Bearing Roof Area = 1468.1 sq.ft. Hip Line = 0 ft. Horz. Oft = 98.57 ft. Raked OH = 159.04 ft. Decking = 50 sheets	Spring Lake / Cumberland	Bill Shaw Rd.	Roof	01/12/22	David Landry	Lenny Norris
			Drop Beam Drop Beam Drop Beam Nail Information Sym Product Manuf Qty Supported Member Header Truss HUS26 USP 4 NA 16d/3-1/2" 16d/3-1/2"	CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
		5: 0"	Products PlotID Length Product Plies Net Qty Fab Type BM1 12'0" 1-3/4"x 16" LVL Kerto-S 2 2 FF BM2 15'0" 1-3/4"x 16" LVL Kerto-S 2 2 FF BM3 12'0" 2x10 SPF No.2 2 2 FF GDH 12'0" 2x12 SPF No.2 2 2 FF GDH 12'0" 2x12 SPF No.2 2 2 FF	evelopment Co. Inc.	thomas Farm	C" / GL, CP			197
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_	/			Wea	Lot	Hick	N/A		10L
	· •			BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards



RE: J0122-0097 Lot 31A-R Thomas Farm **Trenco** 818 Soundside Rd Edenton, NC 27932

> Date 11/8/2021 11/8/2021 11/8/2021 11/8/2021 11/8/2021

Site Information: Customer: Weaver Devlopment Co. Inc. Lot/Block: 31A-R Address: Bill Shaw Rd. City: Spring Lake

Project Name: J0122-0097 Model: Hickory Subdivision: Thomas Farm State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 25 individual, dated Truss Design Drawings and 0 Additional Drawings.

	0.14	T N			0	T N
INO.	Seal#	Truss Name	Date	NO.	Seal#	Truss Name
1	E16389162	A1	11/8/2021	21	E16389182	V3
2	E16389163	A1GE	11/8/2021	22	E16389183	V4
3	E16389164	A2	11/8/2021	23	E16389184	V5
4	E16389165	A3	11/8/2021	24	E16389185	V6
5	E16389166	A4	11/8/2021	25	E16389186	V7
6	E16389167	A5	11/8/2021			
7	E16389168	A5GE	11/8/2021			
8	E16389169	B1	11/8/2021			
9	E16389170	B1GE	11/8/2021			
10	E16389171	B2	11/8/2021			
11	E16389172	C1	11/8/2021			
12	E16389173	C1GE	11/8/2021			
13	E16389174	D1-GR	11/8/2021			
14	E16389175	D1GE	11/8/2021			
15	E16389176	M1	11/8/2021			
16	E16389177	M1GE	11/8/2021			
17	E16389178	M2	11/8/2021			
18	E16389179	M2-GR	11/8/2021			
19	E16389180	V1GE	11/8/2021			
20	F16389181	V2	11/8/2021			

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2022 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the design for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



November 08, 2021



11	IM	RF	R.

2x6 SP No.1 2x6 SP No.1 TOP CHORD BOT CHORD

10.0

2x4 SP No.2 WFBS REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(LC 13)

Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-S

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at joint 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTek® connectors. This does not have a seed only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **AVSUTPH Quelity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Wind(LL) 0.05 BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-11-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22,
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



November 8,2021





	10-2-8	<u> </u>	19-2-8 2 3-0-0 1	21-1-8 1-11-0	24-2-8 3-1-0	30-1-8 32-5- 5-11-0 2-3-8	0 3	
Plate Offsets (X,	') [2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0-4	-0,Edge], [12:0-2-0,0-1-4], [14	1:0-4-8,0-1-4], [15:	5:0-3-8,0-4-1	12]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 * Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.75 BC 0.67 WB 0.70 Matrix-S	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) 0. Wind(LL) 0.	in (loc) 0.21 17 0.38 17 0.09 10 0.17 2-17	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 247 lb	GRIP 244/190 FT = 20%	
LUMBER- TOP CHORD 2x6 SP No.1 BRACING- TOP CHORD BOT CHORD 2x10 SP No.1 *Except* 10-15: 2x6 SP 2400F 2.0E TOP CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins. BOT CHORD WEBS 2x4 SP No.2 Structural wood sheathing directly applied or 10-0-0 oc bracing. WEBS TOP CHORD							oc purlins.	
REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-90(LC 12), 10=-90(LC 13) Max Grav 2=1393(LC 2), 10=1353(LC 2)								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2217/403, 4-5=-1870/483, 7-9=-2258/519, 9-10=-2889/551 BOT CHORD 2-17=-193/1848, 15-17=-195/1860, 13-15=-371/2525, 10-13=-380/2525 WEBS 4-17=-29/402, 7-15=-114/967, 9-15=-1075/232, 9-13=0/616, 5-7=-1955/459								

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







	9-11-8	18-11-8	20-10-8	23-11-8	29-10-8	32-2-0	-1	
Plate Offsets (X,Y)	9-11-8 [1:0-9-6.0-1-2], [5:0-3-0.Edge], [9:0-4-0.	9-0-0 Edge], [11:0-2-0.0-1-4], [13	1-11-0 0-4-4.0-1-4], [14:0-3-8	3-1-0	5-11-0	2-3-8	•	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.76 BC 0.66 WB 0.70 Matrix-S	DEFL. in Vert(LL) -0.21 Vert(CT) -0.36 Horz(CT) 0.09 Wind(LL) 0.16	i (loc) I/defl 16 >999 16 >999 9 n/a 1-16 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 243 lb	GRIP 244/190 FT = 20%	
BRACING- TOP CHORD 2x6 SP No.1 BOT CHORD 2x10 SP No.1 *Except* 9-14: 2x6 SP 2400F 2.0E BOT CHORD WEBS 2x4 SP No.2								
REACTIONS. (siz Max H Max L Max C	REACTIONS. (size) 1=Mechanical, 9=0-3-8 Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 9=-90(LC 13) Max Grav 1=1345(LC 2), 9=1347(LC 2)							
FORCES. (lb) - Max. TOP CHORD 1-3= BOT CHORD 1-16 WEBS 6-14	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-3=-2189/401, 3-4=-1853/491, 6-8=-2237/517, 8-9=-2874/549 BOT CHORD 1-16=-198/1827, 14-16=-200/1839, 12-14=-375/2511, 9-12=-384/2511 WEBS 6-14=-117/966, 3-16=-53/392, 4-6=-1931/474, 8-14=-1081/228, 8-12=0/620							
NOTES-								

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.







Plate Offsets (X,Y)	[1:0-1-14,0-1-8]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.34 9-12 >999 360 MT20 244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.64	Vert(CT) -0.47 9-12 >822 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.27	Horz(CT) 0.05 7 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05 12 >999 240 Weight: 204 lb FT = 20%	

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No 2 WFBS

REACTIONS. (size) 1=Mechanical, 7=0-3-8 Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13) Max Grav 1=1278(LC 1), 7=1331(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





	<u> </u>	22-2-8		+	32-5-0 10-2-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 * Rep Stress Incr YES Code IRC2015/TPI2014	CSI. DEFL TC 0.29 Vert(L BC 0.65 Vert(C WB 0.27 Horz(C Matrix-S Wind(in (loc) -) -0.34 9-12 T) -0.47 9-12 CT) 0.05 7 L) 0.05 12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	PLATES GF MT20 24 Weight: 206 lb F	21 P 4/190 T = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 8,202





Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22, 21, 19

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 polate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0 to 4-4-13, Interior(1) 4-4-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.







REACTIONS. All bearings 11-11-0.

(lb) - Max Horz 1=-167(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 12 except 15=-181(LC 12), 11=-169(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 12 except (it=lb) 15=181, 11=169.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.



November 8,2021

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 818 Soundside Road Edenton, NC 27932



Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 31A-R Thomas Farm
					E16389171
J0122-0097	B2	ROOF SPECIAL	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:41 2021 Page 2

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:41 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-DVHKf?NYrcltLtm1zJkUuSAMIgPyMTNu4cZDVoyLH1y

	ID:1yUksKymplk2404uf
LOAD CASE(S) Standard	
2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (plf)	
Vert: 1-2=-250, 3-4=-50, 4-7=-50, 6-10=-20	
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25	
Uniform Loads (pir)	
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60. Plate Increase=1.60	
Uniform Loads (olf)	
Vert: 1-2=-156, 3-4=27, 4-12=35, 6-12=27, 6-7=20, 6-10=-12	
Horz: 3-4=-39, 4-12=47, 6-12=39, 6-7=32	
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-1/0, 3-4=35, 4-11=27, 6-11=35, 6-7=58, 6-10=-12	
1012.5-4=-47, 4-11=59, 0-11=47, 0-7=70	
Uniform Loads (plf)	
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=-51, 6-10=-20	
Horz: 3-4=38, 4-6=-38, 6-7=-31	
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=11, 6-10=-20	
1012.5 - 4 = 30, 4 - 0 = -30, 0 - 7 = 31 8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase - 1.60. Plate Increase - 1.60	
Uniform Loads (plf)	
Vert: 1-2=-179, 3-4=-13, 4-6=11, 6-7=4, 6-10=-12	
Horz: 3-4=1, 4-6=23, 6-7=16	
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-191, 3-4=11, 4-b=-13, 6-7=2, 6-10=-12 Horz: 3-4=-23, 4-6=-1, 6-7=14	
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60. Plate Increase=1.60.	
Uniform Loads (plf)	
Vert: 1-2=-201, 3-4=-35, 4-6=-11, 6-7=-4, 6-10=-20	
Horz: 3-4=15, 4-6=9, 6-7=16	
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60)
Uniform Loads (plf)	
Veil. 1-2=-213, 3-4=-11, 4-0=-33, 0-7=-20, 0-10=-20 Horz: 2-40, 4-615, 6-78	
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60. Plate Increase	e=1.60
Uniform Loads (plf)	
Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12	
Horz: 3-4=-33, 4-6=21, 6-7=14	
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase	e=1.60
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12	
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60. Plate Increase	e=1 60
Uniform Loads (plf)	5-1100
Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12	
Horz: 3-4=-33, 4-6=21, 6-7=14	
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase	e=1.60
Uniform Loads (plf)	
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12 Horz: 2-4=-21, 4-6=23, 6-7=26	
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60. Plate Increase	e=1 60
Uniform Loads (plf)	5-1.00
Vert: 1-2=-201, 3-4=-1, 4-6=-13, 6-7=-6, 6-10=-20	
Horz: 3-4=-19, 4-6=7, 6-7=14	
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase	se=1.60
ven: 1-2=-213, 3-4=-13, 4-b=-1, b-7=b, b-1U=-2U Horz: 3-47, 4-6-19, 6-7-26	
18) Dead: Lumber Increase=0.90. Plate Increase=0.90 Plt_metal=0.90	
Uniform Loads (plf)	
Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-20	
19) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.6	0, Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=-236, 3-4=-61, 4-6=-43, 6-7=-38, 6-10=-20	
$\Pi U [2, 3-4=1], 4-6=7, 6-7=12$	

20) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-245, 3-4=-43, 4-6=-61, 6-7=-56, 6-10=-20

Horz: 3-4=-7, 4-6=-11, 6-7=-6 21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60



Job	Truss	Truss Type	Qty	Ply	Lot 31A-R Thomas Farm
					E16389171
J0122-0097	B2	ROOF SPECIAL	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fa	yetteville, NC - 28314,		8	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:41 2021 Page 3

ID:1yUksKymplk2404ufZYCrxyoKUD-DVHKf?NYrcItLtm1zJkUuSAMIgPyMTNu4cZDVoyLH1y

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-36, 4-6=-45, 6-7=-40, 6-10=-20

Horz: 3-4=-14, 4-6=5, 6-7=10

22) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-245, 3-4=-45, 4-6=-36, 6-7=-31, 6-10=-20 Horz: 3-4=-5, 4-6=14, 6-7=19

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-260, 3-4=-60, 4-7=-20, 6-10=-20

- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-220, 3-4=-20, 4-7=-60, 6-10=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-250, 3-4=-50, 4-7=-20, 6-10=-20 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-50, 6-10=-20





			8-3-8				
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.84 BC 0.24 WB 0.00 Matrix-P	DEFL. in Vert(LL) -0.05 Vert(CT) -0.10 Horz(CT) 0.00 Wind(LL) 0.10	l (loc) l/defl 2-5 >999 2-5 >969 5 n/a 2-5 >886	L/d 360 240 n/a 240	PLATES MT20 Weight: 37 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x6 SF	P No.1 P No.1 P No.2 P No.1		BRACING- TOP CHORD BOT CHORD	Structural wood except end vert Rigid ceiling dire	sheathing dir cals. ectly applied c	ectly applied or 5-3-4 c or 10-0-0 oc bracing.	oc purlins,

8-3-8

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed; C-C for members and forces & MWFRS for
- reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.







			8-3-8		
	I		8-3-8		
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [12:0-2-0,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	DEFL. ir Vert(LL) 0.09 Vert(CT) -0.08 Horz(CT) -0.00	n (loc) I/defl L/d 9 10-11 >999 240 3 10-11 >999 240 0 8 n/a n/a	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 41 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x4 SF 8-12: 2	2 No.1 2 No.1 2 No.2 2 No.2 *Except* tx6 SP No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 oc purlins, r 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8) Max Uplift 2=-216(LC 8), 8=-188(LC 8)

Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207 BOT CHORD

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) All plates are MT20 plates unless otherwise indicated.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216.8=188.



November 8,2021





7

4

3x10 ||

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

8

l/defl

>999

>999

>999

n/a

7-11-0

3-11-8

3-4

3-4

3-4

3

in (loc)

-0.02

-0.04

0.01

0.01

3x10 ||

L/d

360

240

n/a

240

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2

 WEDGE
 Left: 2x4 SP No.2, Right: 2x4 SP No.2

20.0

10.0

0.0

10.0

REACTIONS. (size) 1=0-3-8, 3=0-3

 (size) 1=0-3-8, 3=0-3-8 Max Horz 1=91(LC 24) Max Uplift 1=-191(LC 8), 3=-180(LC 9) Max Grav 1=2919(LC 1), 3=2779(LC 2)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-2418/177, 2-3=-2418/177

 BOT CHORD
 1-4=-100/1678, 3-4=-100/1678

1-0-3

3x10 ||

Н

2-0-0

1.15

1.15

NO

5

6

3-11-8

3-11-8

0.38

0.57

0.39

CSI.

тс

BC

WB

Matrix-P

WEBS 2-4=-154/3142

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-3-0 oc.

- Webs connected as follows: 2x4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 1-3=-20



1-0-3

Structural wood sheathing directly applied or 6-0-0 oc purlins.

PLATES

Weight: 100 lb

MT20

GRIP

244/190

FT = 20%



Job	Truss	Truss Type	Qty	Ply	Lot 31A-R Thomas Farm	
.10122-0097	D1-GR	Common Girder	1		E16389	9174
00122 0001				2	Job Reference (optional)	
Comtech, Inc, Fayetter	rille, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:45 2021 Page 2	2
		ID:1yUks	Kymplk24	04ufZYCr	yoKUD-6HWqVNQ2vrpJqU3oC9pQ2IL_QHhilDCU?DXReayLH1u	J

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)





OTHERS 2x4 SP No.1

WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. All bearings 7-11-0.

 (Ib) - Max Horz 2=-118(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable End Details as applicable, or consult qualified building designer
 Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152.8=148.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932



			5-0-0		1
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.28	DEFL. in (loc) Vert(LL) -0.01 2-4) l/defl L/d 4 >999 360	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.08 WB 0.00 Matrix-P	Vert(CT) -0.01 2-4 Horz(CT) 0.00 Wind(LL) 0.01 2-4	4 >999 240 n/a n/a 4 >999 240	Weight: 24 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

BRACING-TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=63(LC 8) Max Uplift 2=-102(LC 8), 4=-79(LC 8)

Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.







			5-0-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Pop Strass Incr. VES	CSI. TC 0.09 BC 0.09 WB 0.02	DEFL. in (loc) l/defl L/d Vert(LL) 0.01 8 >999 240 Vert(CT) -0.01 8 >999 240 Horz(CT) -0.01 6 p/a p/a	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 27 lb FT = 20%

LUMBER-

2x4 SP No.1 2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WFBS OTHERS 2x4 SP No 2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8)

Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147.6=115.





⁴⁾ This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.



Design valid for use only with MiTek® connectors. This AND INCLUDED MITER KEPERENCE PAGE MIT-473 fev. 5192/200 BEPORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 TRENCO

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 31A-R Thomas Farm
					E16389178
J0122-0097	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:48 2021 Page 2

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:48 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-WsCz7OSxCmBuhyoNtHM7gwzXrVoXVgzwhBm5EuyLH1r

LOAD CASE(S) Standard
Concentrated Loads (Ib)
Vert: 9=-350
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (pit)
Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40
Concentrated Loads (ib)
Vert: 9=-300
Holorom Loads (off)
Vent 1-2=70 2-3=58 3-4=153 5-6=12 2-8=52 8-10=115 7-10=52
Horz: 1-282, 2-370, 3-4165, 3-555
Concentrated Loads (lb)
Vert: 9=548
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
Concentrated Loads (ID)
Vell. 9=000
Inform Lads (nf)
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=-19. 2-3=25. 3-4=-37. 3-5=51
Concentrated Loads (lb)
Vert: 9=-420
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
HOIZ: 1-2=19, 2-3=25, 3-4=19, 3-5=51
Vort e - 420
8) Dead + 0.6 MWFRS Wind (Pos Internal) Left: Lumber Increase=1.60 Plate Increase=1.60
Uniform Loads (olf)
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10
Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
Concentrated Loads (Ib)
Vert: 9=154
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Veil. 172-0, 25-12, 54-22, 50-1, 27-12 Hore: 1-2-18, 2-3-24, 3-4-40, 3-5-27
Concentrated Loads (Ib)
Vert: 9=43
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
11) Dead + 0.6 MWERS Wind (Neg. Internal) Right: Lumber Increase=1.60. Plate Increase=1.60.
Uniform Loads (plf)
Vert: 1-2-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20
Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
Concentrated Loads (lb)
Vert: 9=-234
12) Dead + 0.6 MWFKS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Unitorm Loads (pii)
Vol. 1-2-14, 2-0-21, 0-4-14, 0-0-11, 2-1-12 Hore: 1-0-26 2-2-33 2-4-26 2-5-230
Concentrated Loads (/b)
Vert: 9=43
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
veri 5=45 14) Dead + 0.6 MWERS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (off)
Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12
Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
Concentrated Loads (lb)
Vert: 9=43
וס) Deau + ט.ס ויויערגס יעוחם (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60



Job	Truss	Truss Type	Qty	Ply	Lot 31A-R Thomas Farm
					E16389178
J0122-0097	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayetter	/ille, NC - 28314,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:48 2021 Page 3

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:48 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-WsCz7OSxCmBuhyoNtHM7gwzXrVoXVgzwhBm5EuyLH1r

LOAD CASE(S)	Standard
Uniform Loads	(pr)
vert:	1-2=2, 2-3=9, 3-4=2, 5-0=1, 2-7=-12
Concentrated	1-2=-14, 2-3=-21, 3-4=-14, 3-3=-27
Vort	
16) Dead + 0.6 M	7-4-0 MERS Wind (Neg. Internal) 1st Parallel: Lumber Increase-1.60. Plate Increase-1.60
Liniform Loads	in the wind (Neg. internal) for Faranci. Earlier inclease 1.00, Frate inclease 1.00
Vert	1/2=6 2-3=-1 3-4=6 5-6=-33 2-7=-20
Horz.	1-226 2-319 3-426 3-512
Concentrated	Loads (b)
Vert:	9=-234
17) Dead + 0.6 M\	WFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads	s (plf)
Vert:	1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20
Horz:	1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
Concentrated	Loads (lb)
Vert: 9	9=-234
18) Dead: Lumber	Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
Uniform Loads	s (plt)
Vert:	1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20
Concentrated	
10) Deed + 0.75 B	J=-ZUU José Live (bol.) + 0.75 Attic Floor + 0.75 (0.6 MM/FDS Wind (Nor. Int) Left) Lumber Jacksons 1.60. Plote Jacksons 1.60.
19) Deau + 0.75 R	Job Live (bal.) + 0.75 Allic Floor + 0.75(0.6 MWYRKS Wind (Neg. int) Leit). Lumber increase=1.60, Flate increase=1.60
Vort:	י (עדיי) 1.231 2.336 3.431 5.905 6.9125 2.83 8.10-13 7.103
Horz:	12=-19, 23=-14, 3-4=-19, 3-5=26
Concentrated	Loads (b)
Vert:	9=-454
20) Dead + 0.75 R	voof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads	s (plf)
Vert:	1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20
Horz:	1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
Concentrated	Loads (Ib)
Vert:	9=-375
21) Dead + 0.75 R	.oof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads	; (plt)
Vert:	1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20
HOIZ:	1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
Vort	
22) Dead + 0.75 R	5-513 Anof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWERS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads	
Vert:	1-2=-40. 2-3=-45. 3-4=-40. 5-9=-86. 6-9=-116. 2-7=-20
Horz:	1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
Concentrated	Loads (lb)
Vert:	9=-375
23) 1st Dead + Ro	of Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads	s (plf)
Vert:	1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20
Concentrated	Loads (lb)
Vert:	9=-400
24) 2nd Dead + R	oor Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads	(pi) () () () () () () () () () (
Concentrated	1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20
Vort	
25) 3rd Dead ± 0	5-400 75 Poof Live (upbalanced): Lumber Increase=1 15 Plate Increase=1 15
Uniform Loads	s (nf)
Vert	1-3-50, 3-4=-50, 5-6=-100, 2-7=-20
Concentrated	Loads (lb)
Vert:	9=-350 `
26) 4th Dead + 0.7	75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads	s (plf)
Vert:	1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20
Concentrated	Loads (Ib)
Vert: 9	9=-350







Job	Truss	Truss Type	Qty	Ply	Lot 31A-R Thomas Farm
					E16389179
J0122-0097	M2-GR	HALF HIP	1	2	
				-	Job Reference (optional)
Comtech, Inc, Fayetter	/ille, NC - 28314,		. 8.	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:48 2021 Page 2

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:48 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-WsCz7OSxCmBuhyoNtHM7gwzZIVp2VgzwhBm5EuyLH1r

	Concentrated Loads (lb)
2)	Vert: 9=-400 Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15. Plate Increase=1.15
-,	Uniform Loads (plf)
	Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20
	Vert: 9=-350
3)	Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
	Uniform Loads (plf)
	Concentrated Loads (Ib)
~	Vert: 9=-300
4)	Uniform Loads (plf)
	Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52
	Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55 Concentrated Loads (lb)
	Vert: 9=548
5)	Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52
	Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
	Concentrated Loads (lb)
6)	Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vent. 1-2=-1, 2-3=-45, 3-4=17, 3-6=-178, 2-6=-9, 6-10=2, 7-10=-9 Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
	Concentrated Loads (lb)
7)	Vert: 9=-420 Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60 Plate Increase=1.60
• ,	Uniform Loads (plf)
	Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
	Concentrated Loads (lb)
0)	Vert: 9=-420
0)	Uniform Loads (plf)
	Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-131, 2-8=10, 8-10=33, 7-10=10
	Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7 Concentrated Loads (lb)
	Vert: 9=154
9)	Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12
	Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
	Vert: 9=43
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
	Concentrated Loads (Ib)
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
	Concentrated Loads (Ib)
12	vert: 9=-234) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60. Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
	Concentrated Loads (lb)
10	Vert: 9=43
13	Uniform Loads (plf)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
	понд. 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (Ib)
	Vert: 9=43
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	Lot 31A-R Thomas Farm
					E16389179
J0122-0097	M2-GR	HALF HIP	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayet	eville, NC - 28314,		8	430 s Aug	16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:48 2021 Page 3

8.430 s Aug 16 2021 MiTek Industries, Inc. Mon Nov 8 10:10:48 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-WsCz7OSxCmBuhyoNtHM7gwzZIVp2VgzwhBm5EuyLH1r

LOAD CASE(S) Standard
Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
Concentrated Loads (lb) Vert: 9=43
 Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
Concentrated Loads (lb) Vert: 9=43
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
Vert: 9=-234
 Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20 Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
Vert: 9=-234
 Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20 Concentrated Loads (lb)
Vert: 9=-200 19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3
Concentrated Loads (lb)
20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20 Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
Concentrated Loads (lb) Vert: 9=-375
21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
Concentrated Loads (lb) Vert: 9=-375
22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20 Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
Concentrated Loads (lb) Vert: 9=-375
23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20 Concentrated Loads (lb)
Vert: 9=-400 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20
Concentrated Loads (lb) Vert: 9=-400
25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20 Concentrated Loads (lb)
verr: 9=-350 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20
Concentrated Loads (lb) Vert: 9=-350





				8-6-5			4	-4-4				
Plate Offsets (X,Y)	[7:0-3-11,Edge], [9:0-1-6	,0-1-0], [10:0-2	2-0,0-0-10]									
LOADING (psf) FCLL 20.0 FCDL 10.0 3CLL 0.0 * 3CDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matrii	0.06 0.03 0.08 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 7	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 75 lb	GRIP 244/190 FT = 20%	

11

10 9 3x4 =

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No 2 OTHERS

BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

8

12-10-9

10.00 12

REACTIONS. All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12), 8=-126(LC 13)

13

12

8-6-5

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

3x4 🥢

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-295/189

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.



November 8,2021







BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 14-10-0. (lb) - Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)

WEBS 2-8=-338/247, 4-6=-338/247

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb)
 8=135, 6=135.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. All bearings 12-5-4

(lb) - Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-8=-312/241, 4-6=-312/241

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0ps; BCDL=6.0ps; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123. 6=123.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. MARNING - Verify design pa Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932



REACTIONS. (size) 1=10-0-7, 3=10-0-7, 4=10-0-7

Max Horz 1=-92(LC 8) Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

- and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

2x4 SP No.1 2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10

Max Horz 1=68(LC 9) Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

- and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.









REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13

Max Horz 1=-44(LC 8) Max Uplift 1=-15(LC 13), 3=-19(LC 13)

Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

- and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.









3x4 🥢

3x4 📎

			2-10-0						
Plate Offsets (X,Y) [2:0-2-0,Edge]									
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLAT	ES GRIP					
TCLL 20.0	Plate Grip DOL 1.15	TC 0.01	Vert(LL) n/a - n/a 999 MT20	244/190					
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) n/a - n/a 999						
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a						
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Weigl	nt: 8 lb FT = 20%					
LUMBER-			BRACING-						

2-10-0

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=2-10-0, 3=2-10-0 Max Horz 1=-20(LC 8)

Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 8,2021







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)122-0098		/A	ickory "C" / GL, CP	t 31A-R Thomas Farm	'eaver Development Co. Inc.	tion russ /3-1/2" Qty Fab Type FF FF	IT exact. IT exact. Id 24"oc. Nail Informat Header T 16d/3-1/2" 16d Plies Net (2 2 2 2 2 2 2 2	Drop Notes Drop Notes Dros shown are NC LL plumbing drop ing Floor Trusses eded not to excee D Supported Member NA Products "LVL Kerto-S "LVL Kerto-S	Plumbing abing drop location ractor to verify A tions prior to sett st spacing as new for Information Manuf Qty USP 14 USP 14 Product 1-3/4"x 16 1-3/4"x 16	1. Plu 2. Con loc 3. Adj Conned Product HUS410 tID Lengt 1 12' 0" 2 15' 0"	
		SEAL DATE N/	PLAN Hi	JOB NAME Lo	BUILDER	FF FF FF	2 2 2 2 2 2	No.2 No.2 No.2 ment Plan	1-3/4"x 16 2x10 SPF 2x12 SPF Fruss Place Scale: 1/4"=1	$\begin{array}{c} 2 & 15' 0'' \\ 3 & 12' 0'' \\ H & 12' 0'' \\ \hline & \hline $	BN G[
	IAGR# ndividu nto the build each ring. T	VENT DI Net as in orated in n of the sets for or tempor	PLACEN e design e incorp cificatio sign she placeme pasible fu	A TRUSS USSES ari- ents to b at the spe- vidual de d on the is response	THIS IS These to compore design See ind identific design			ment Plan	Fruss Place Scale: 1/4"=1"	-1	

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



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Spring Lake / Cumberland	Bill Shaw Rd.	Floor	01/12/22	David Landry	Lenny Norris			
CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.			
Weaver Development Co. Inc.	Lot 31A-R Thomas Farm	Hickory "C" / GL, CP	N/A		J0122-0098			
BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #			

Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.

Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Info	ormation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

Products									
PlotID	Length	Product	Plies	Net Qty	Fab Type				
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF				
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF				
BM3	12' 0"	2x10 SPF No.2	2	2	FF				
GDH	12' 0"	2x12 SPF No.2	2	2	FF				

Truss Placement Plan



design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

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is	Design	A	ddress: E	Bill Shaw F	Road		J	lob Name	e: Lot 31A-R	Thomas Farm		
			5	Spring Lak	e, NC 28390	1	F	Project #:	J0122-009	98		
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				CE111"						I		
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							Bearing		h Can	React D/L lb		ase Id Comb
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							2 SDE	3 500"	95%	2869 / 2079	4948 1	D+S
Analysis Re	sults						2 011			2000 / 2010		
Analysis	Actual	Location A	llowed	Capacity	Comb.	Case	1					
Moment	13679 ft-lb	5'11 1/2" 3	9750 ft-lb	0.344 (34%	%) D+S	L						
Unbraced	13679 ft-lb	5'11 1/2" 1	3695 ft-lb	0.999	D+S	L						
	0050 #		0700 1	(100%)	0.5.0							
Shear	3659 10	1'6 5/8" 1	3739 ID	0.266 (279	6) D+S	L						
LL Defi Inch	0.069 (L/2000)	5111/2 0	207 (L/400)	0.240 (24%	o) o () D (C	L .						
I L Defi Inch	0.164 (L/840)	5111/2 0	.383 (L/360)	0.430 (43%	o) D+S	L	-					
Design Not	es											
1 Fasten all p	olies using 3 rows of	f 10d Box nails	s (.128x3") at	12" o.c. Ma	ximum end dis	stance not						
2 Refer to las	, st page of calculation	ns for fastener	s required for	r specified le	oads.							
3 Girders are	designed to be sup	ported on the	bottom edge	only.								
4 Top loads n	nust be supported e	equally by all p	lies.									
6 Lateral sler	e raterally praced at iderness ratio base	d on single plv	width.									
ID	Load Type	L	ocation Tr	rib Width	Side	Dead 0.9	Live	1 Sno	w 1.15 V	Vind 1.6 Const	1.25 Com	ments
1	Uniform				Тор	120 PLF	0 PI	F	0 PLF	0 PLF	0 PLF Wall	
2	Uniform				Top	349 PI F	0 PI	F 3	349 PLF	0 PLF	0 PLF A2	
	Solf Woight				.~ P		011	0		<u>.</u>		
	Sell Weight											
Notes		chemical	s		6. For flat	roofs provide p	proper drainage to	o prevent	Manufacturer	Info	Comtech, In 1001 S. Reil	c. Ily Road, Suite #639
Calculated Structured structural adequacy of	Designs is responsible only of this component based o	of the Handling n the 1. LVL bean	s installation	ı or drilled	ponality				Metsä Wood 301 Merritt 7 E	Building, 2nd Floor	Fayetteville, USA 28214	NG
design criteria and responsibility of the o	I loadings shown. It is customer and/or the contrac	the 2 Refer t tor to regarding	to manufacturer's installation re	product infor equirements, m	mation ulti-ply				Norwalk, CT 0	6851	28314 910-864-TR	US
application, and to ver	ify the dimensions and loads.	approvals	i details, beam stre s d Beams must not h	ength values, an	a code				www.metsawc	od.com/us		
1. Dry service conditi	ons, unless noted otherwise	4 Design a 5 Provide	ssumes top edge is lateral support at 1	laterally restraine bearing points fr	d avoid				ICC-ES: ESR-	3033		omtecul
2. LVL not to be trea	ted with fire retardant or con	rosive lateral dis	splacement and rota	Ition	This d	lesign is valid	l until 4/24/202	23			5	ыпесн
Vanian 20 40 075	D											

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isDe	sign	Client: Project: Address:	Weaver Developme Hickory Bill Shaw Road Spring Lake, NC	ent Co. Inc. 28390		Date: Input by: Job Name Project #:	1/12/2022 David Landry : Lot 31A-R Thomas Farm J0122-0098	Page 2 of 8
BM1 Ke	rto-S LVL	1.750" 2	X 16.000"	2-Ply	- PASSI	ED L	evel: Level	
						I		
	· · ·	• •	· · ·	•	· ·	•	· · · · · · · · · · · · · · · · · · ·	1'4"
1 SPF			11'11"					↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ 21/2"
1			11'11"				1	
Fasten all plies Capacity Load Yield Limit per Foot Yield Mode Edge Distance Load Combination Duration Factor	ysis <u>using 3 rows of</u> 0,0 0,0 245 ner 81. IV 1 1. 3" 1.0	10d Box nails (% PLF 5.6 PLF 9 lb. /2" 0	(.128x3") at 12"	o.c Maxim	num end dis	stance no	t to exceed 6"	
Notes Calculated Structured Desig structural adequacy of this design criteria and loac responsibility of the custom ensure the component s application, and to verify the Lumber 1. Dry service conditions, u	is is responsible only of the component based on the ings shown. It is the er and/or the contractor to uitability of the intended dimensions and loads.	chemicals Handling & Installati 1. LVL beams must not be c 2. Refer to manufactur regarding installation fastening details, beam approvals 3. Damaged Beams must not 4. Design assumes top edgr 5. Provide lateral support	ON ut or drilled er's product information requirements, multi-ply strength values, and code ot be used oi laterally restrained at bearing points to avoid	6. For flat roofs pr ponding	ovide proper drainag	e to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Comtech, Inc. 1001 S. Relly Road, Suite #639 Fayetterille, NC USA 28314 910-864-TRUS
Version 20.40.075 Powe	red by iStruct™	lateral displacement and	rotation	This design is	s valid until 4/24/	2023		CEDI

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Version 20.40.075 Powered by iStruct™

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is	Design		Client: Project: Address:	Weaver Developm Hickory Bill Shaw Road Spring Lake, NO	ent Co. Inc.		Date: Input by: Job Name Project #:	1/12/2022 David Landry : Lot 31A-R Thomas Farm J0122-0098	Page 4 of 8
BM2 I	Kerto-S	LVL	1.750"	X 16.000"	2-Ply	- PASSE	ED L	_evel: Level	
	• •	•	•	· · ·	••••	•	• • •	· · ·	
1 SPF En	nd Grain				A'7 1/2"			2 SPF End	d Grain
<i> </i>				1	4'7 1/2"				5 1/2
Fasten all pl Capacity Load Yield Limit per F Yield Mode Edge Distance Min. End Distan Load Combinati Duration Factor	ies using 3 ro	Dws of 10d 79.8 % 196.0 Pl 245.6 Pl 81.9 lb. IV 1 1/2" 3" D+L 1.00	Box nails .F .F	(.128x3") at 12"	o.c Maxim	um end dis	tance no	nt to exceed 6"	Contech. Inc.
Notes Calculated Structured structural adequacy design criteria and responsibility of the ensure the compor application, and to ver Lumber	Designs is responsible of this component base of loadings shown. I customer and/or the col- nent suitability of the rify the dimensions and lo	che chy of the Hand ad on the 1 LVL t is the 2 Ref intractor to reg intended fast bads app 3 Dar	micais ling & Installa beams must not be er to manufactu arding installatior ening details, bear rovals naged Beams must	tion cut or drilled urer's product information requirements, multi-ply n strength values, and code not be used	o. ⊢or flat roofs pro ponding	ονιαe proper drainage	∍ to prevent	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR_3633	1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
1. Dry service condit 2. LVL not to be treat	ions, unless noted otherw ated with fire retardant o	vise 5 Pro r corrosive late	ign assumes top ec vide lateral suppor ral displacement an	lge is laterally restrained t at bearing points to avoid d rotation	This design is	s valid until 4/24/2	2023	100-LO. LON-0000	соттесн
Version 20.40.075	Powered by iStruc	t™							CSDI

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1	isDesign	Client: Project: Address:	Weaver Development Co. Inc. Hickory Bill Shaw Road Spring Lake, NC 28390	Date: Input by: Job Nam Project #	1/12/2022 David Landry e: Lot 31A-R Thomas Farm	Page 6 of 8
BM3	S-P-F #2	2.000" X	10.000" 2-Ply	- PASSED	Level: Level	
					<u> </u>	Ξ
	• •	•	• • •	• •	• • •	• 9 1/4"
	• • • • F End Grain	•	• • •	• •	• • • • • • • • • • • • • • • • • • •	
			12'			
1			12'			1
Multi-Ply	y Analysis					
Capacity Load Yield Limit pr Yield Limit pr Yield Mode Edge Distan Min. End Dis Load Combin Duration Fac	er Foot er Fastener ce stance nation ctor	0.0 % 0.0 PLF 157.4 PLF 78.7 Ib. IV 1 1/2" 3" 1.00			Manufacturer Info	Comtech, Inc.
						U001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314
			This desig	n is valid until 4/24/2023		910-864-TRUS



		Client:	Weaver Developme	nt Co. Inc.		Date:	1/12/2022	Page 8 of 8
isDesi	gn	Address:	Bill Shaw Road	28390		Job Name	: Lot 31A-R Thomas Farm	
GDH S-P-	F #2	2.000" X	12.000"	2-Ply -	PASS	ED	Level: Level	
•••		• •	•	•	•	•	•••	T T
							1 1/2	11 1/4"
		• •	•	•	•	•	••++	
1 SPF End Grain							2 SPF End Grain	
			8'10"					1 13"
			010				I	
Multi-Ply Analysis	5							
Fasten all plies usin	ng 2 rows o	f 10d Box nails ((.128x3") at 12" -	o.c Maxim	num end di	stance no	ot to exceed 6"	
Load Yield Limit per Foot	0. 1!	0 PLF 57 4 PLF						
Yield Limit per Fastener	78	3.7 lb.						
Edge Distance	1	1/2"						
Min. End Distance	3"							
Duration Factor	1.	00						
						F	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
								USA 28314 910-864-TRUS
				This design is	s valid until 4/24/	2023		соттесн



RE: J0122-0098 Lot 31A-R Thomas Farm **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Weaver Development Co. Inc. Lot/Block: 31A-R Address: Bill Shaw Rd. City: Spring Lake

Project Name: J0122-0098 Model: Hickory Subdivision: Thomas Farm State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.3 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 8 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E16389591	ET1	11/8/2021
2	E16389592	ET2	11/8/2021
3	E16389593	ET3	11/8/2021
4	E16389594	F1	11/8/2021
5	E16389595	F2	11/8/2021
6	E16389596	F3	11/8/2021
7	E16389597	F4	11/8/2021
8	E16389598	F5	11/8/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric My license renewal date for the state of North Carolina is December 31, 2022

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 8-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb) Vert: 4=-71 7=-77 19=-71 20=-71

Plate Offsets (X,Y)	[4:0-1-8,Edge], [15:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. ir Vert(LL) n/z Vert(CT) n/z Horz(CT) 0.00	n (loc) l/defl L/d - n/a 999 - n/a 999 11 n/a n/a	PLATES MT20 Weight: 54 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI	⊃ No.1(flat) ⊃ No.1(flat)		BRACING- TOP CHORD	Structural wood sheathing d except end verticals.	irectly applied or 6-0-0	oc purlins,

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

2x4 SP No.1(flat) 2x4 SP No.3(flat) BOT CHORD WFBS 2x4 SP No.3(flat) OTHERS

REACTIONS.

DNS. All bearings 11-1-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 19, 11, 18, 17, 16, 15, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

BOT CHORD Rigic

except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

14-7-12				26-5-0					
Plate Offsets ()	(,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge], [13:0-1-	8,Edge], [19:0-1-8,Edge]				11.0 -	T	
LOADING (psf TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0)))))	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.47 BC 0.70 WB 0.44 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.10 26-27 -0.13 26-27 0.03 16	l/defl >999 >999 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 136 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- BRACING- TOP CHORD 2x4 SP No.1(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD 2x4 SP No.1(flat) BOT CHORD BOT CHORD WEBS 2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.				oc purlins,					
REACTIONS. (size) 29=0-3-8, 21=0-3-8, 16=0-3-8 Max Grav 29=727(LC 10), 21=1671(LC 1), 16=557(LC 4)									
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1257/0, 3-4=-1883/0, 4-5=-2002/0, 5-6=-1658/0, 6-8=-756/224, 8-9=0/1400, 9-10=0/1400, 10-11=-468/367, 11-12=-1158/0, 12-13=-1158/0, 13-14=-884/0									
BOT CHORD	BOT CHORD 28-29=0/771, 27-28=0/1718, 26-27=0/2002, 25-26=0/2002, 24-25=0/2002, 22-24=-33/1341, 21-22=-443/147, 20-21=-637/20, 19-20=-175/900, 18-19=0/1158, 17-18=0/1158, 16-17=0/585								
WEBS	2-29= 10-20 5-24=	=-1023/0, 2-28=0/677, 3-28=-640/0, 8-2 D=0/687, 11-20=-690/0, 11-19=0/582, 12 =-633/0, 14-16=-776/0, 14-17=0/416, 13	1=-1289/0, 8-22=0/923, 10 2-19=-278/0, 6-22=-882/0, 0 3-17=-373/90	-21=-1064/0, 6-24=0/527,					

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means. 5) CAUTION, Do not erect truss backwards.

10-10-0							
Plate Offse	ets (X,Y)	[3:0-1-8,Edge], [4:0-1-8,Edge]		10-10-0			
LOADING TCLL TCDL BCLL BCDL	(psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.36 BC 0.45 WB 0.21 Matrix-S	DEFL. in Vert(LL) -0.07 Vert(CT) -0.08 Horz(CT) 0.01	(loc) l/defl L/d 10 >999 480 10 >999 360 7 n/a n/a	PLATES MT20 Weight: 56 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD BRACING- 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) BOT CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.					oc purlins,		
REACTION	IS (size) 12=0-3-8 7=0-3-8					

Max Grav 12=576(LC 1), 7=576(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-925/0, 3-4=-1240/0, 4-5=-925/0

11-12=0/603, 10-11=0/1240, 9-10=0/1240, 8-9=0/1240, 7-8=0/603 BOT CHORD

WEBS 2-12=-800/0, 2-11=0/449, 3-11=-473/0, 5-7=-800/0, 5-8=0/449, 4-8=-473/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

Plate checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

			11-11-0			
I			11-11-0			1
Plate Offsets (X,Y)	[5:0-1-8,Edge], [11:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.52 BC 0.63 WB 0.27 Matrix-S	DEFL. in Vert(LL) -0.13 Vert(CT) -0.16 Horz(CT) 0.02	n (loc) I/defl L/d 11-12 >999 480 11-12 >894 360 8 n/a n/a	PLATES MT20 Weight: 61 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.			
REACTIONS (siz	e) 13-0-3-8 8-0-3-8					

Max Grav 13=635(LC 1), 8=635(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1060/0, 3-4=-1495/0, 4-5=-1495/0, 5-6=-1059/0

12-13=0/676, 11-12=0/1395, 10-11=0/1495, 9-10=0/1495, 8-9=0/658 BOT CHORD 2-13=-898/0, 2-12=0/534, 3-12=-466/0, 3-11=-19/356, 6-8=-873/0, 6-9=0/557, WEBS 5-9=-617/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

November 8,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidual truss evel and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

11 11 0

H			14-6-0			
Plate Offsets (X,Y)	[5:0-1-8,Edge], [14:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.62 BC 0.78 WB 0.35 Matrix-S	DEFL. in Vert(LL) -0.17 Vert(CT) -0.22 Horz(CT) 0.03	i (loc) I/defl L/d 12-13 >999 480 12-13 >790 360 10 n/a n/a	PLATES MT20 Weight: 76 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,

REACTIONS. (size) 16=0-3-8, 10=Mechanical

Max Grav 16=778(LC 1), 10=784(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1348/0, 3-4=-2244/0, 4-5=-2244/0, 5-6=-2152/0, 6-7=-2152/0, 7-8=-1359/0

 BOT CHORD
 15-16=0/834, 14-15=0/1856, 13-14=0/2244, 12-13=0/2244, 11-12=0/1857, 10-11=0/835

 WEBS
 2-16=-1109/0, 2-15=0/714, 3-15=-707/0, 3-14=0/697, 4-14=-339/0, 8-10=-1111/0, 8-11=0/729, 7-11=-693/0, 7-12=0/401, 5-12=-438/123

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

November 8,2021

			<u>11-7-8</u> 11-7-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [5:0-1-8,Edge], [11:0-1-8	3,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.56 WB 0.25 Matrix-S	DEFL. in Vert(LL) -0.10 Vert(CT) -0.13 Horz(CT) 0.02	(loc) l/defl L/d 11-12 >999 480 11-12 >999 360 8 n/a n/a	PLATES MT20 Weight: 61 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI WEBS 2x4 SI	P No.1(flat) P No.1(flat) P No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,	
REACTIONS (siz	e) 13-Mechanical 8-0-3-8					

Max Grav 13=626(LC 1), 8=619(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1025/0, 3-4=-1427/0, 4-5=-1427/0, 5-6=-1022/0

12-13=0/658, 11-12=0/1345, 10-11=0/1427, 9-10=0/1427, 8-9=0/644 BOT CHORD WEBS 2-13=-876/0, 2-12=0/510, 3-12=-446/0, 3-11=-41/323, 6-8=-854/0, 6-9=0/526, 5-9=-570/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

818 Soundside Road Edenton, NC 27932

