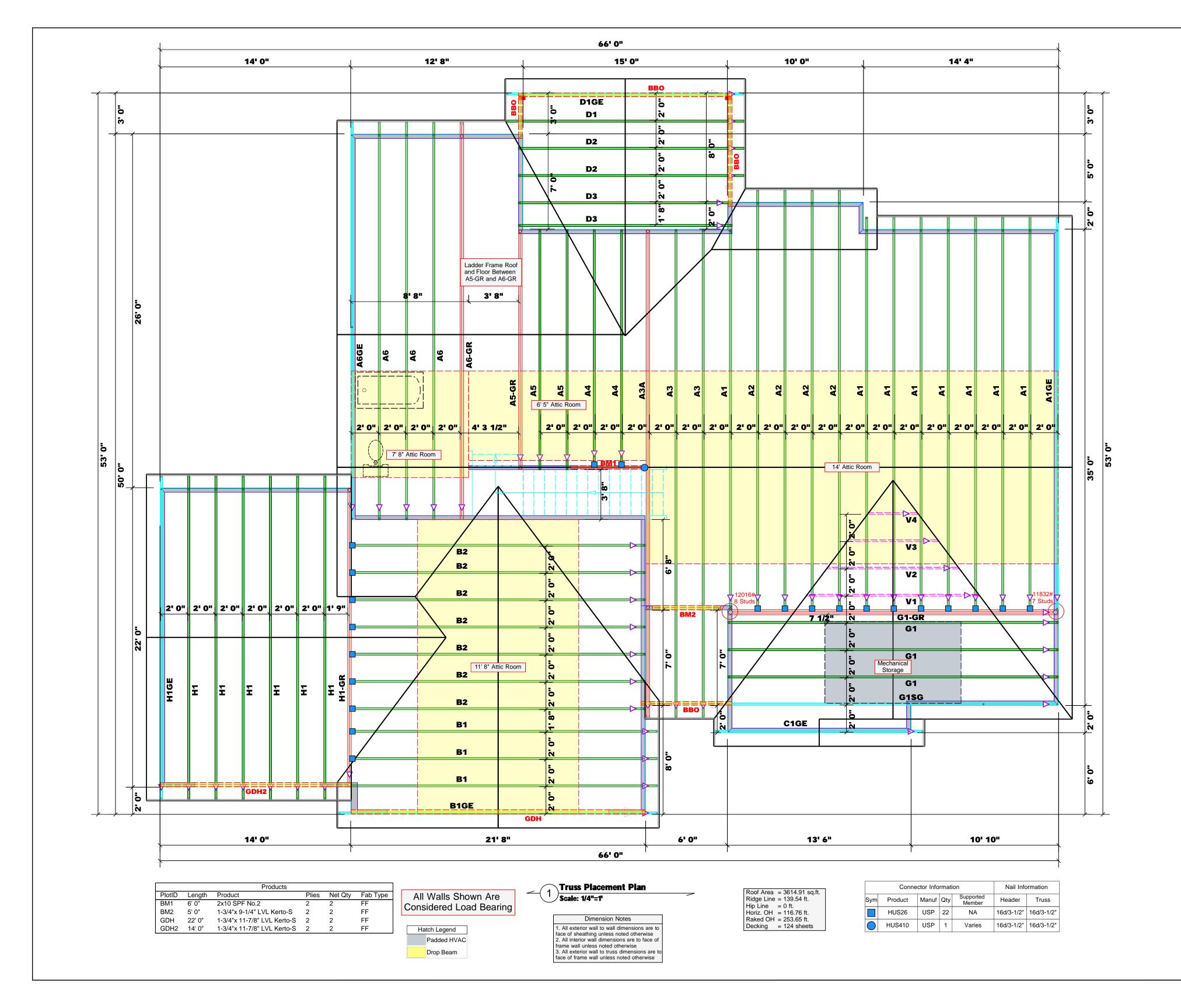


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ilding ding esigner. design ilding ilding eem and russ walls, ding	JOB #	J1121-6678	SALESMAN	SALESMAN Lenny Norris	8075H Ald (1) 804 S0F 804 S0F 4068H 1 00 400 1 2 3 4 00 00 00 4	de the tive num 3000# esign ds ined to	AS tk

support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



These the comport design a See ind identified designer for the of support and collid designer consult	BUILDER	Weaver Development Co. Inc.	COUNTY	Erwin / Harnett	those s registe design exceed	Bearing deeme require attache Code r founda require but no	ľ
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ilding ding ssigner. design lding wem and uss walls, ding acing, the	JOB #	J1121-6678	SALESMAN	SALESMAN Lenny Norris	Intend to that Image: Constraint of the second second second	tk D# are de the tive num 8000# ssign	

ISI	Design	I	Project: Address:	Halifax II	lliams Road	IIIC.	ln Jo	put by: bb Name:	11/30/2021 David Landr Lot 1 North F J1121-6678			Pag	je 1 o
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Туре:	Girder		Applicat	tion:	Floor		Brg	Live	Dead	Snow	Wind	Const	
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nalysis Res	ults						2 - SPF	3.500"	41%	919 / 919	1837 L	D+S	
•		cation /		Capacity		Case							
	2122 ft-lb 2122 ft-lb		3946 ft-lb 3654 ft-lb	0.538 (54 0.581 (58	•	L							
	1169 lb		2872 lb	0.407 (41		L							
	0.018 (L/3452)		0.126 (L/480			L							
	0.035 (L/1726)	2'9" (0.168 (L/360	0) 0.210 (21	I%) D+S	L							
esign Note	es						1						
1 Fasten all pl	lies using 2 rows of 10c	d Box nail	s (.128x3") a	at 12" o.c. M	laximum end di	stance not	1						
to exceed 6" 2 Refer to last	t page of calculations for	or fastene	ers required 1	for specified	loads.								
	designed to be support		-	je only.									
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4 Top loads m	ed at bearings.												
4 Top loads m 5 Top braced a 6 Bottom brac	4	single pr	y wiath.		Side	Dead 0.9	Live	1 Snow	1.15 Wii	nd 1.6 Const	t. 1.25 Commer	nte	
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4 Top loads m 5 Top braced a 6 Bottom brac	derness ratio based on Load Type Uniform		Location	Trib Width	Тор	334 PLF	0 PL	F 334	PLF	0 PLF	0 PLF A4	1.5	

	/	Client:	Weaver Developr	nent Co. Inc.		Date:	11/30/2021	Page 2 of 8
1	isDesign	Project: Address:	Halifax II	Deed		Input by:	David Landry Lot 1 North Pointe	
- ÷	150 cargin	Address.	Josey Williams Erwin, NC 2833	39		Project #:	J1121-6678	
BM1	S-P-F #2	2.000" X	10.000"	2-Ply	- PASSE	D L	evel: Level	
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Capacity Load		0.0 % 0.0 PLF						
Yield Limit pe		157 4 PLF 78.7 lb.						
Yield Limit pe Yield Mode		IV						
Edge Distand Min. End Dis		1 1/2" 3"						
Load Combir	nation							
Duration Fac	ctor	1.00						
						г.	Annula chunch lef-	Comtach Inc
						Ľ	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
								USA 28314 910-864-TRUS
								соттесн
1				This design	is valid until 4/24/2	2023		

BM2 Kerto-S	Address	 Josey Williams Re Erwin, NC 28339 O" X 9.250" 		JC	putby: Da bName: Lo	ivid Landry t 1 North Poi	nte		
	LVL 1.75	0" X 9.250"		Pi	oject #: J1	121-6678			
			2-Ply -	PASSI	ED Level	Level			
•	1	•	•						
. <u>.</u> .	- Fierd		A. 1						9 1/-
1 SPF End Grain		2 SPF En	d Grain						
·	4'8"								∫
	4'8"		I						
ember Information				Reaction	s UNPAT	FERNED I	b (Uplift)		
ype: Girder lies: 2		lication: Floor		Brg	Live	Dead	Snow	Wind	Const
lies: 2 loisture Condition: Dry eflection LL: 480 eflection TL: 360 nportance: Normal	Buil	ign Method: ASD ding Code: IBC/IRC d Sharing: No k: Not Che		1 2	0 0	1526 1526	1510 1510	0 0	0 0
emperature: Temp <= 1	00°F			Bearing	 i				
				Bearing 1 - SPF	Length	Cap. Rea 28% 15	act D/L lb 526 / 1510	Total Ld. Case 3036 L	e Ld. Comb. D+S
alysis Results				End Grain					
AnalysisActualMoment2881 ft-lbJnbraced2881 ft-lbShear1735 lbLL Defl inch0.015 (L/3370)FL Defl inch0.030 (L/1676)		lb 0.200 (20%) D+S	lb. Case L L L L L L	2 - SPF End Grain	3.500"	28% 15	26 / 1510	3036 L	D+S
esign Notes	of 10d Doy poils (1994	21) at 101 a.a. Mavimum	and distance not	1					
 Fasten all plies using 2 rows to exceed 6". Refer to last page of calculat Girders are designed to be si Top loads must be supported Top braced at bearings. Bottom braced at bearings. Lateral slenderness ratio bas 	ons for fasteners requi upported on the bottom equally by all plies.	red for specified loads.	end distance not						
D Load Type	Location		Dead 0.9	Live			1.6 Const.		nts
l Uniform Self Weight		Тор	647 PLF 7 PLF	0 PLI	= 647 Pl	.F 01	PLF (0 PLF A3	

isDesign	Client: Project: Address:	Weaver Development Co. Inc. Halifax II Josey Williams Road	Date: Input by: Job Nam		Page 4 of 8
BM2 Kerto-S	6 LVL 1.750'	Erwin, NC 28339 ' X 9.250'' 2-Ply	Project #	: J1121-6678 Level: Level	
				<u> </u>	
1 SPF End Grain	•	• • • • • • • • • • • • • • • • • • •	× 112"		
	4'8" 4'8"	· · · · · · · · · · · · · · · · · · ·	ŕ		1 3 1/2"
Capacity Load Yield Limit per Foot Yield Mode Edge Distance Min. End Distance Load Combination Duration Factor	0.0 % 0.0 PLF 163.7 PLF 81.9 lb. IV 1 1/2" 3" 1.00	and the second	num end distance n	Manufacturer Info	Comtech, Inc. 1001 S. Relly Road, Suite #639
Notes Calculated Structured Designs is responsibl structural adequacy of this component bi design criteria and loadings shown, responsibility of the customer and/or the application, and to verify the dimensions and Lumber 1. Dry service conditions, unless noted oth 2. UVI, not to be trasted with fire relatadin	e only of the seed on the contractor to loads trivise	ton ponding put or drilled er's product information requirements, multi-ply strength values, and code ot be used ot be used at bearing points to avoid	ovide proper drainage to prevent	Methacturer mile Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-964-TRUS
2. LVL not to be treated with fire retardant Version 20.40.075 Powered by istru	lateral displacement and		s valid until 4/24/2023		

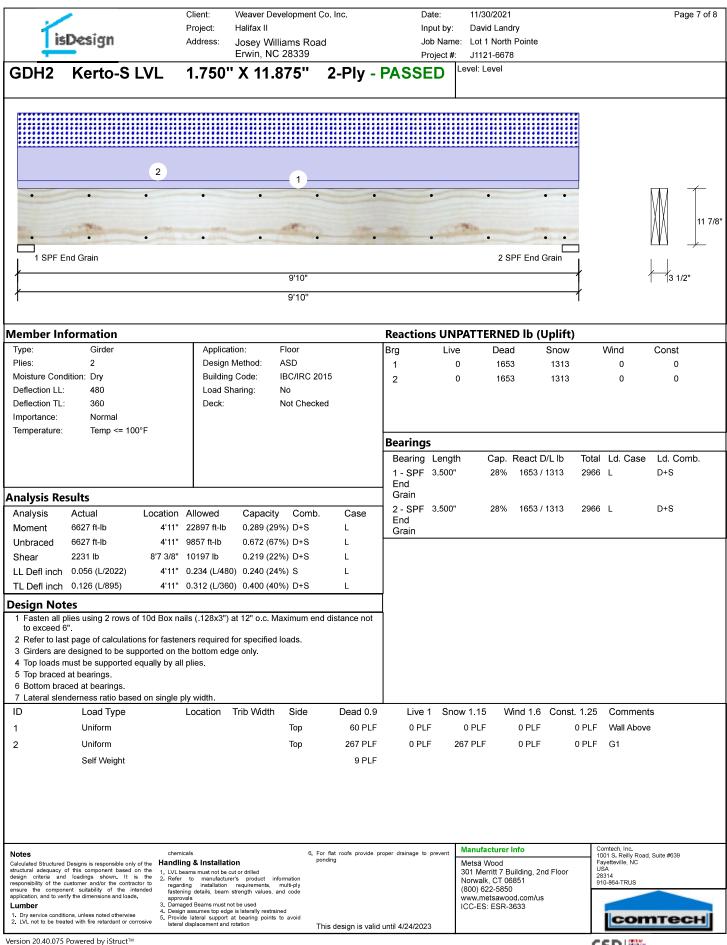
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Version 20.40.075 Powered by iStruct^T

is	Design	P	roject: Ha .ddress: Jo	aver Develop ifax II sey William	s Road	с.	•	out by:	11/30/2021 David Land Lot 1 North	-			Page 5 o
DH I	<pre>Kerto-S LV</pre>	L 1.	Er 750'' X	win, NC 283 11.875		Ply - P	ASSEC	<u> </u>	J1121-6678 /el: Level				
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		3											
	• •				1							_	≁
	- Thin				122		-	1000					11 7
1 SPF End	Grain									2 SPF	End Graii	<u>ר</u>	
					18'10"								5 1/4"
J					18'10"							I	
ember Inf	ormation							s UNPA	TTERNE	D lb (Uplift)		
īype: Plies:	Girder 3		Application: Design Met				Brg 1	Live 0	Dead 2720			Vind 0	Const 0
Noisture Cond			Building Co		/IRC 2015		2	0	2720			0	0
eflection LL:	480		Load Sharir	-	<u>.</u>								
eflection TL: mportance:	360 Normal		Deck:	Not	Checked								
emperature:	Temp <= 100°F	=											
							Bearings						
							Bearing 1 - SPF	-	Cap. 18%	React D/L lb 2720 / 188	Total 2908	Ld. Case	Ld. Comb. D+S
nalysis Re	ulto						End Grain	0.000	1070	21207 100	2000	L	5.0
Analysis Kes		_ocation A	llowed (Capacity (Comb.	Case	2 - SPF	3.500"	18%	2720 / 188	2908	L	D+S
Noment	12191 ft-lb	9'5" 2	7954 ft-lb 0	.436 (44%) [C	Uniform	End Grain						
Inbraced	13035 ft-lb	9'5" 1		.998 E 100%)	D+S	L							
Shear	2368 lb	1'2 5/8" 1		.198 (20%) E)	Uniform							
	0.037 (L/6029)	9'5 1/16" 0	.459 (L/480) (, ,		L							
L Defl inch	0.565 (L/390)	9'5 1/16" 0	.612 (L/360) (.920 (92%) [D+S	L							
esign Not	es						1						
1 Fasten all p to exceed 6	lies using 2 rows of 1 "	0d Box nails	s (.128x3") at 1	2" o.c. Maxim	um end dist	ance not							
	t page of calculations	for fastener	s required for a	pecified load	s.								
	designed to be supp nust be supported equ		-	nly.									
	e laterally braced at a												
	ed at bearings.	a a sta al a sub-											
/ Lateral sien	derness ratio based Load Type			Width S	ide l	Dead 0.9	Live 1	Snow '	1.15 \//	nd 1.6 Cons	it. 1.25	Comment	s
5	Uniform	-			op	60 PLF	0 PLF		PLF	0 PLF	0 PLF	Wall	
2	Tie-In	0-0-0 to	18-10-0 1-0-		р	20 PSF	0 PSF		PSF	0 PSF	0 PSF	Roof	
-	Uniform				р	195 PLF	0 PLF		PLF	0 PLF	0 PLF	B1GE	
	Self Weight					14 PLF							
otes		chemical			6. For flat r ponding	oofs provide pr	oper drainage to p	Jieven	nufacturer l	nfo	100	ntech, Inc. 1 S. Reilly Road	I, Suite #639
ructural adequacy o	Designs is responsible only of 1 f this component based on 1 loadings shown. It is 1	he 1. LVL bean	& Installation	rilled				30		ilding, 2nd Floor	US. 283	14	
sponsibility of the c nsure the component	ustomer and/or the contractor ant suitability of the intend	to regarding led fastening	o manufacturer's I installation requ details, beam streng	irements, multi-p	bly			(80	rwalk, CT 06 00) 622-5850		910	-864-TRUS	
oplication, and to veri umber	fy the dimensions and loads.	approval 3. Damageo	s I Beams must not be u	sed					w.metsawoo C-ES: ESR-3		5		
	ons, unless noted otherwise ed with fire retardant or corros	5. Provide	ssumes top edge is lat ateral support at be splacement and rotatio	iring points to avo		sian ie volid	until 4/24/2023				1	con	птесн
					i ilis de	Sign is valiu							

Address: Josey Williams Road Job Name: Lot 1 North Pointe Erwin, NC 28339 Project #: J1121-6678	
GDH Kerto-S LVL 1.750" X 11.875" 3-Ply - PASSED	
$\left[\begin{array}{cccccccccccccccccccccccccccccccccccc$	11 7/8"
1 SPF End Grain 2 SPF End Grain	
18'10" 18'10"	5 1/4"
Fastern all plies using 2 rows of 10d Box nails (128x3") at 12" o.c Nail from both sides. Maximum end distance not to exceed 6" Capacity 0.0 % Load 0.0 PLF Viel Limit per Foot 163.7 PLF Yiel Limit per Fastener 1.12" Min. End Distance 3" Load Combination 3" Daration Factor 1.00	10.4620
Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design on tries and loadings shown. It is the responsibility of the customer and/or the contractor to structural adequacy of this component based on the structural adequacy of this component based on the responsibility of the customer and/or the contractor to structural adequacy of this component based on the responsibility of the customer and/or the contractor to structural adequacy of this component based on the responsibility of the customer and/or the contractor to structural adequacy of this component based on the responsibility of the intended 1. I/L beams must not be cutor drilled responsibility of the customer and/or the contractor to regarding installation requirements, multi-ply calculated based on the responsibility of the customer and/or the contractor to regarding installation requirements, and code Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 Toti S, Relly Nead, SL USA 20314	JITE #639
approvals approvals Lumber 3. Damaged Beams must not be used 1. Dry service conditions, unless noted otherwise 3. Design assumes top edge is laterally restrained 2. LVL not to be treated with fire retardant or corrosive 5. Provide lateral support at beams finance of the support at bea	тесн

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isD	esign	l	Client: Project: Address:	Weaver Developm Halifax II Josey Williams Erwin, NC 2833	Road		Date: Input by: Job Name Project #:	11/30/2021 David Landry : Lot 1 North Pointe J1121-6678		Page 8 of 8
GDH2 M	Kerto-S L	VL	1.750'	' X 11.875	" 2-Ply	- PASS	SED ^L	Level: Level		
•	•	•	•	•	•	•	•	• • •	<11/2"	11 7/8"
	• •	•	•	•	•	•	•		\pm	
1 SPF End	Grain			9'10)"			2 SPF End Grain	,	3 1/2"
 				9'10				1	,	
Multi-Ply Ana										
Fasten all plies Capacity Load Yield Limit per Foot	using 2 rows	of 10d B 0.0 % 0.0 PLF 163.7 PLF	ox nails ((.128x3") at 12"	' o.c Maxim	um end dis	tance nc	ot to exceed 6"		
Yield Limit per Faste Yield Mode		81.9 lb. IV								
Edge Distance Min. End Distance		1 1/2" 3"								
Load Combination Duration Factor		1.00								
Notes Calculated Structured Desi	gns is responsible only of t		g & Installati		6. For flat roofs pro ponding	vide proper drainage	i lo preveni	Manufacturer Info Metsä Wood	Comtech, Inc. 1001 S. Reilly Road Fayetteville, NC USA	d, Suite #639
structural adequacy of thi design criteria and loa responsibility of the custon ensure the component application, and to verify the	adings shown. It is t mer and/or the contractor suitability of the intend	he 2 Refer to regardir	ng installation Ig details, beam	ut or drilled er's product information requirements, multi-ply strength values, and code				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	28314 910-864-TRUS	
Lumber 1. Dry service conditions, 2. LVL not to be treated v	unless noted otherwise	 Damage Design Provide 	ed Beams must n assumes top edg	e is laterally restrained at bearing points to avoid	This design is	valid until 4/24/2		ICC-ES: ESR-3633	con	птесн
Version 20.40.075 Pow	rered by iStruct™								CSDI	

CSD 🗱



RE: J1121-6678

Lot 1 North Pointe

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Weaver Development Co. Inc Lot/Block: 1
Address: Josey Williams Road
City: Erwin

Project Name: J1121-6678 Model: Halifax II Subdivision: North Pointe State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10

Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E16002619	A1	8/3/2021	21	E16002639	G1-GR	8/3/2021
2	E16002620	A1GE	8/3/2021	22	E16002640	G1SG	8/3/2021
3	E16002621	A2	8/3/2021	23	E16002641	H1	8/3/2021
4	E16002622	A3	8/3/2021	24	E16002642	H1-GR	8/3/2021
5	E16002623	A3A	8/3/2021	25	E16002643	H1GE	8/3/2021
6	E16002624	A4	8/3/2021	26	E16002644	V1	8/3/2021
7	E16002625	A5	8/3/2021	27	E16002645	V2	8/3/2021
8	E16002626	A5-GR	8/3/2021	28	E16002646	V3	8/3/2021
9	E16002627	A6	8/3/2021	29	E16002647	V4	8/3/2021
10	E16002628	A6-GR	8/3/2021				
11	E16002629	A6GE	8/3/2021				
12	E16002630	B1	8/3/2021				
13	E16002631	B1GE	8/3/2021				
14	E16002632	B2	8/3/2021				
15	E16002633	C1GE	8/3/2021				
16	E16002634	D1	8/3/2021				
17	E16002635	D1GE	8/3/2021				
18	E16002636	D2	8/3/2021				
19	E16002637	D3	8/3/2021				

8/3/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

G1

Truss Design Engineer's Name: Gilbert, Eric

E16002638

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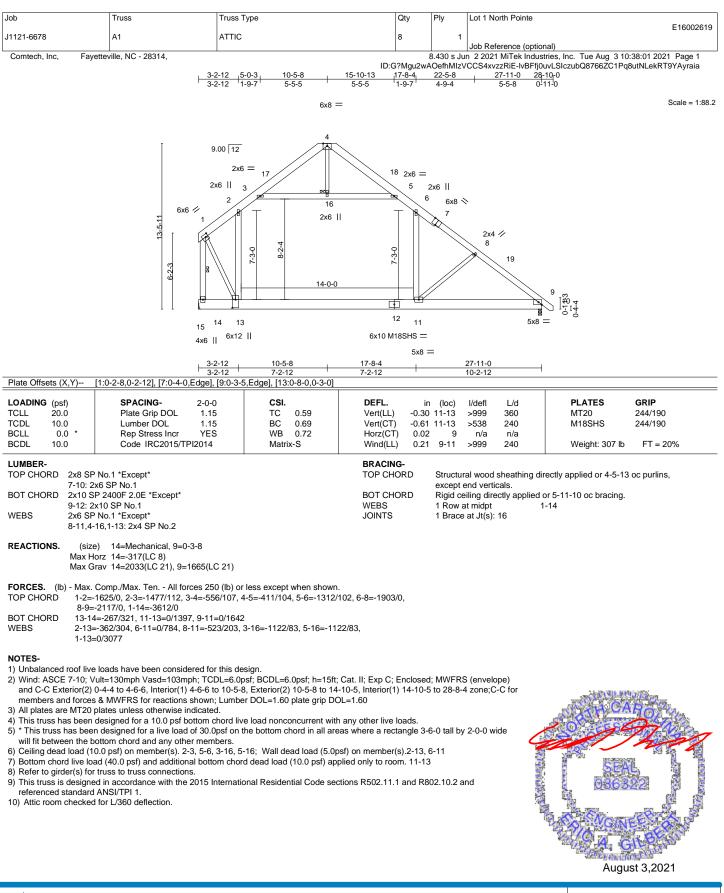
My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

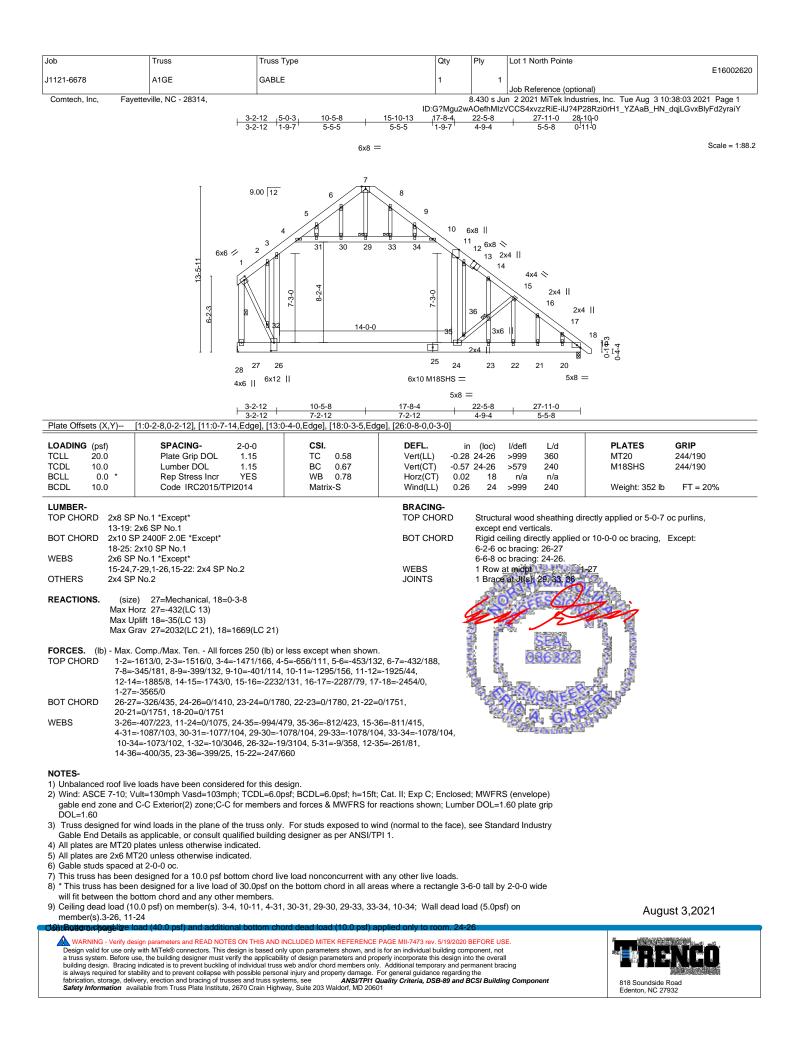
August 03, 2021



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932



Job		Truss	Truss Type	Qty	Ply	Lot 1 North Pointe
J1121-6678		A1GE	GABLE	1	1	E16002620
						Job Reference (optional)
Comtech, Inc,	Fayettev	ille, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 10:38:03 2021 Page 2
			ID:C	G?Mgu2w/	AOefhMIz\	/CCS4xvzzRiE-ilJ?4P28Rzi0rH1_YZAaB_HN_dqjLGvxBlyFd2yraiY

NOTES-

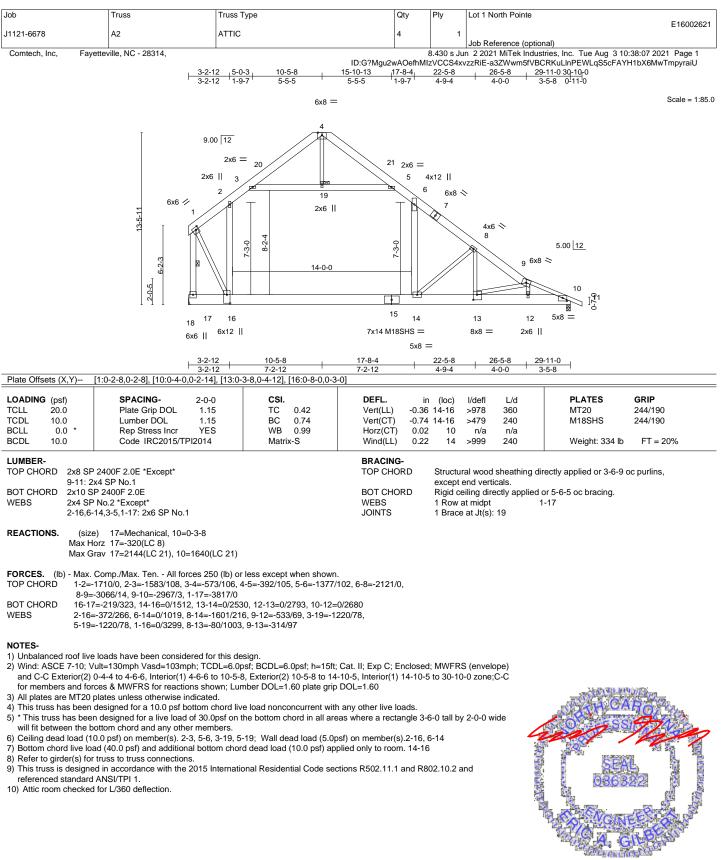
11) Refer to girder(s) for truss to truss connections.

- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18.
- 13) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

14) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design properly incorporet his design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

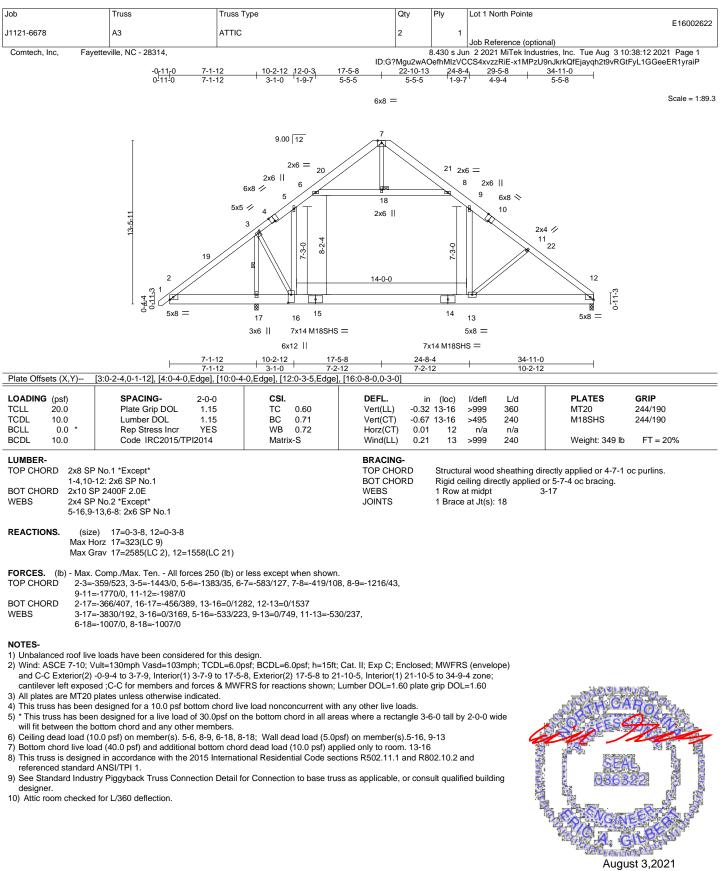




August 3,2021



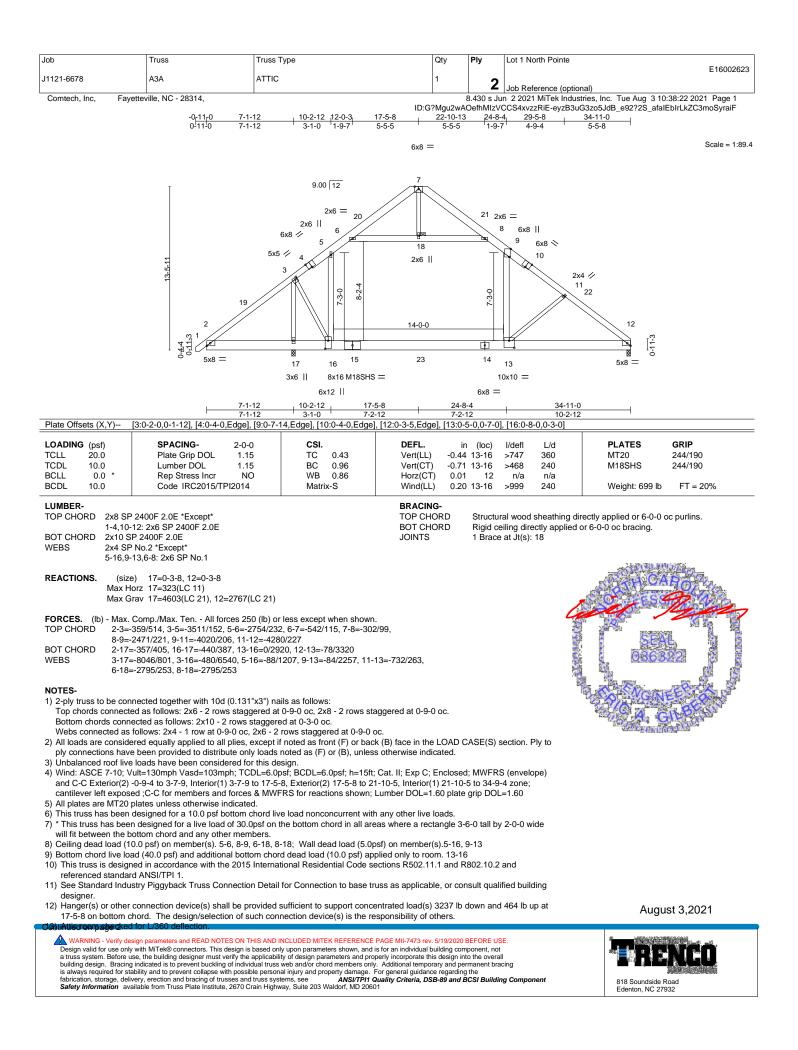
TRENCO 818 Soundside Road Edenton, NC 27932



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818 Soundside Road Edenton, NC 27932



Job		Truss	Truss Type	Qty	Ply	Lot 1 North Pointe
		101	17710			E16002623
J1121-6678		A3A	ATTIC	1	2	Job Reference (optional)
Comtech, Inc,	Fayettev	rille, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 10:38:22 2021 Page 2

8.430 s Jun 2 2021 MiTek Industries, Inc. Tue Aug 3 10:38:22 2021 Page 2 ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-eyzB3uG3zo5JdB_e92?2S_afalEbIrLkZC3moSyraiF

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-60, 5-6=-80, 6-7=-60, 7-8=-60, 8-9=-80, 9-12=-60, 2-16=-20, 13-16=-40, 12-13=-20, 6-8=-20

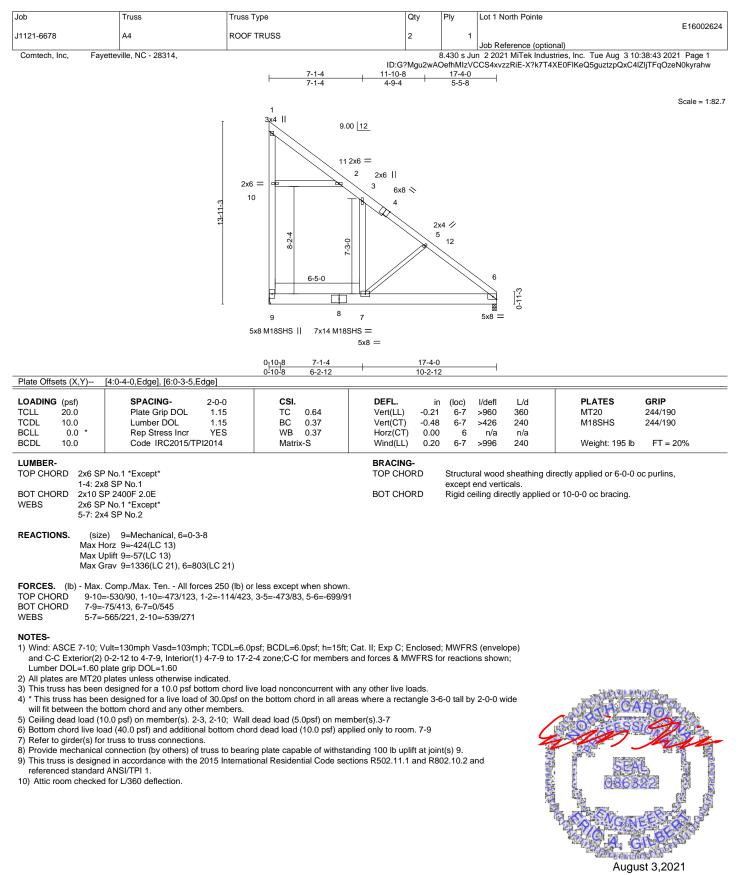
Drag: 5-16=-10, 9-13=-10

Concentrated Loads (lb)

Vert: 23=-1837(F)

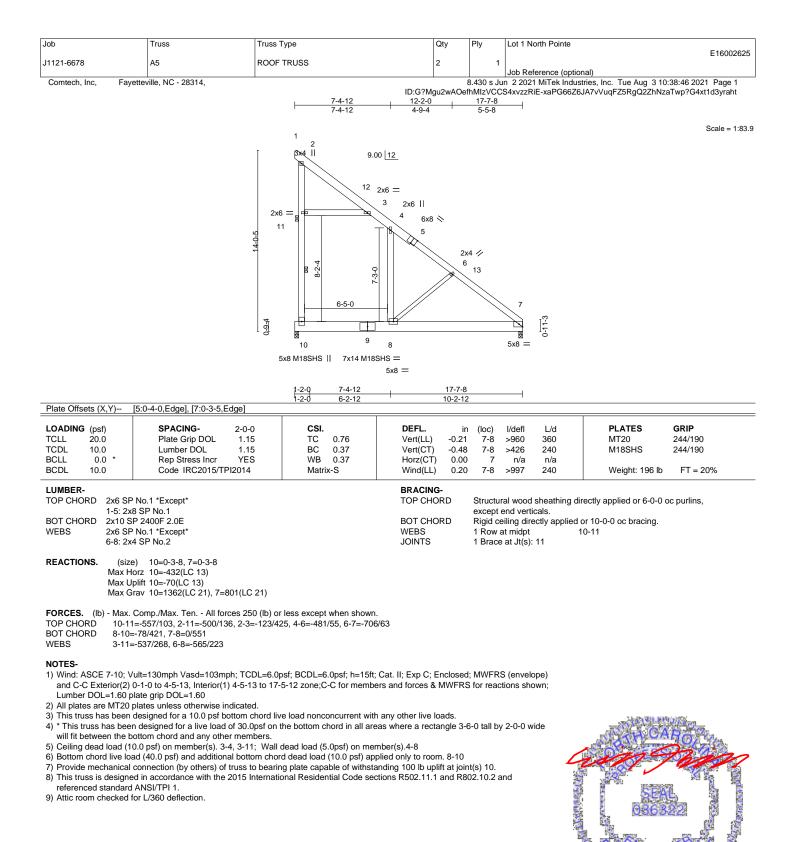
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design properly incorporet his design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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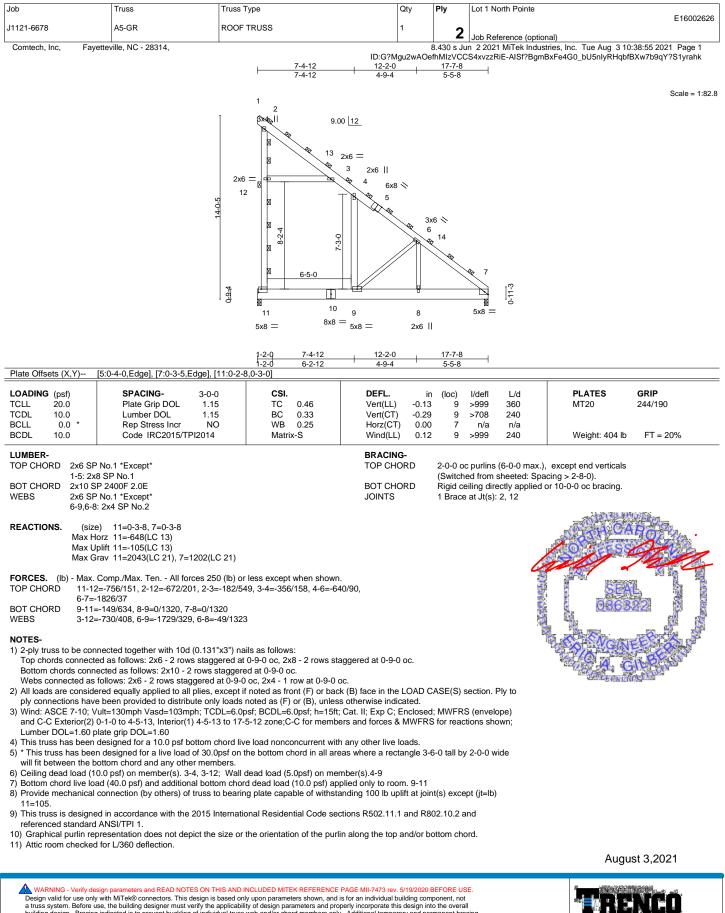




August 3,2021

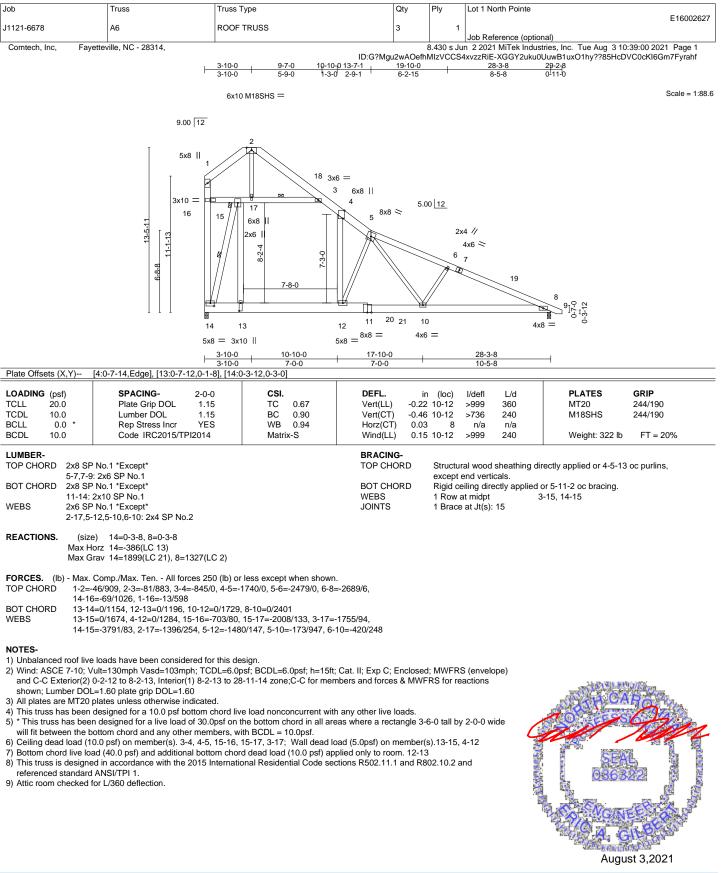
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLUDED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

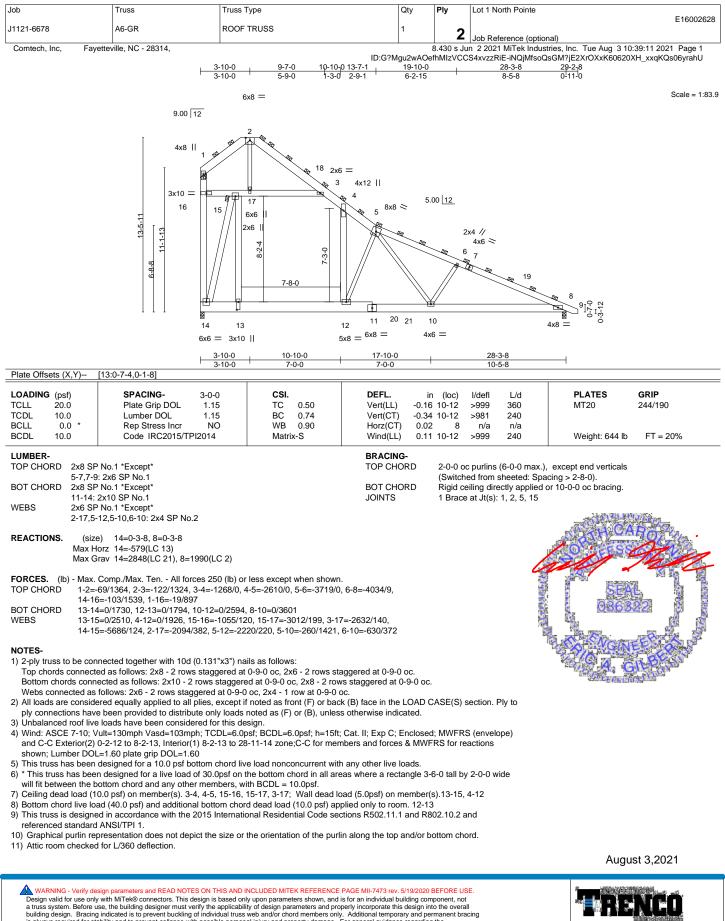




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818 Soundside Road Edenton, NC 27932



Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLUDED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 1 North Pointe				
J1121-6678	A6GE	GABLE	1	1 1	Lot I North Pointe		E16002629		
	etteville, NC - 28314,				Job Reference (optional) In 2 2021 MiTek Industries,		:39:06 2021 Page 1		
Connech, inc, ray	Stevine, NO - 20014,	3-10-0 9-7-0 10-10	ID:G?Mgu2wAOe		S4xvzzRiE-MQdqlxofbKe4 28-3-8 29-2	/yM5kloMEGO9U1H			
3-10-0 5-9-0 1-3-0 2-9-1 6-2-15 8-5-8 0 ¹ 11-0									
6x10 M18SHS = Scale = 1:88.6									
9.00 12									
2									
Plate Offsets (X,Y)	5x8 II 1 3x10 = = 29 3x10 = = 29 4 5x8 II 1 29 4 29 6 29 8 10 10 10 10 10 10 10 10 10 10	3 3 4 28 30 35 36 6x6 11 7 7 7 26 8 3x10 11 3-10-0 10-10-0 3-10-0 7-0-0	25 24 23 22 2 6x8 =		$\begin{array}{c} 4 \ // \\ 4x6 \approx \\ 12 \ 2x4 \ \\ 13 \ 2x4 \ \\ 14 \ 2x4 \ \\ 15 \ 15 \\ 20 \ 19 \ 18 \ 4x8 = \\ \underline{28\cdot3\cdot8} \\ 10\cdot5\cdot8 \\ \end{array}$	6 子子 5.5-5-6			
LOADING (psf) TCLL 20.0	SPACING- 2-0- Plate Grip DOL 1.1	5 TC 0.56	DEFL. i Vert(LL) -0.17	n (loc) 7 25	l/defl L/d >999 360	PLATES MT20	GRIP 244/190		
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.1 Rep Stress Incr YE		Vert(CT) -0.37 Horz(CT) 0.03	723-25 316	>898 240 n/a n/a	M18SHS	244/190		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		3 23-25	>999 240	Weight: 372 lb	FT = 20%		
LUMBER- TOP CHORD 2x8 SP No.1 "Except" TOP CHORD 2x8 SP No.1 "Except" Structural wood sheathing directly applied or 4-11-1 oc purlins, except end verticals. BOT CHORD 2x8 SP No.1 "Except" BOT CHORD 2x8 SP No.1 "Except" BOT CHORD 2x8 SP No.1 "Except" BOT CHORD 2x8 SP No.1 "Except" C-11-2 oc bracing: 26-27 6-11-2 oc bracing: 25-26. 230,8-25,8-21,11-21: 2x4 SP No.2 WEBS 1 Row at micpl 27-28.8-25 1 Brace at Jtici 26 35 39 REACTIONS. (size) 27-0-3-8, 16=0-3-8 Max Horz 27=-563(LC 13) Max Horz 27=-563(LC 13) Max Grav 27=1798(LC 21), 16=-134(LC 13) Max Grav 27=1798(LC 21), 16=-134(LC 13) Max Grav 27=1798(LC 21), 16=-134(LC 13) Max Grav 27=1798(LC 21), 16=-1286(B, 22-29)(608, 2-23-20)(468, 1-29=-80/527) SOF CHORD 26-27-0(1083, 25-26-0)(1123, 23-25-0)(1668, 22-23-0)(1668, 21-22-0)(1688, 1-29=-80/527) SOF CHORD 26-27-0)(1083, 25-26-0)(123, 23-25-0)(1668, 21-22-0)(1688, 1-29=-80/527) SOF CHORD 26-27-0)(1083, 25-26-0)(123, 23-25-0)(1668, 21-22-0)(1688, 1-29=-80/527) SOF CHORD 26-27-0)(1023, 23-25-0)(1668, 22-23-0)(1668, 21-22-0)(1688, 1-29=-80/527) SOF CHORD 26-27-0)(1023, 23-25-0)(1668, 22-23-0)(1668, 21-22-0)(1688, 1-29=-80/527) SOF CHORD 26-27-0)(1023, 23-25-0)(1668, 21-22-0)(1688, 1-29=-80/527) SOF CHORD 26-27-0)(1023, 23-25-0)(1668, 21-23-0)(1688, 1-29=-80/527) SOF CHORD 26-27-0)(1023, 23-25-0)(1668, 21-23-0)(168, 21-23-0)(1688, 1-29=-80/527) SOF CHORD 26-27-0)(1023, 23-25-0)(1668, 21-23-0)(1688, 1-29=-80/528) SOF CHORD 26-27-0)(1023, 23-25-0)(1688, 21-23-0)(1688, 1-29=-80/528) SOF CHORD 26-26-65/1010, 0-25-26/9999, 28-29=-									
NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) All plates are MT20 plates unless otherwise indicated. 5) All plates are 2x6 MT20 unless otherwise indicated. 6) Gable studs spaced at 2-0-0 oc. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 9) Ceiling dead load (10.0 psf) on member(s). 5-6, 28-29, 28-30, 30-35, 35-36, 5-36; Wall dead load (5.0psf) on member(s).26-28, 6-25 6-25 August 3,2021									
10) Bottom chord live lo Continued on page 2	oad (40.0 psf) and additional bo	ttom chord dead load (10.0 psf) a	applied only to room. 25-	26		, agu			
Design valid for use only a truss system. Before u building design. Bracing is always required for st fabrication, storage, deli	Marking - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Image: Comparison of the comparison								

[Job	Truss	Truss Type	Qty	Ply	Lot 1 North Pointe	
	J1121-6678	A6GE	GABLE	1	1	E16002629	
						Job Reference (optional)	
	Comtech, Inc, Fayetteville, NC - 28314,		8.430 s Jun 2 2021 MiTek Industries, Inc. Tue Aug 3 10:39:06 2021 Page 2				
		ID:G?M	gu2wAOe	hMIzVCCS	S4xvzzRiE-MQdqlxofbKe4vyM5kIoMEGO9U1Hvcn5Cg2j5KvyrahZ		

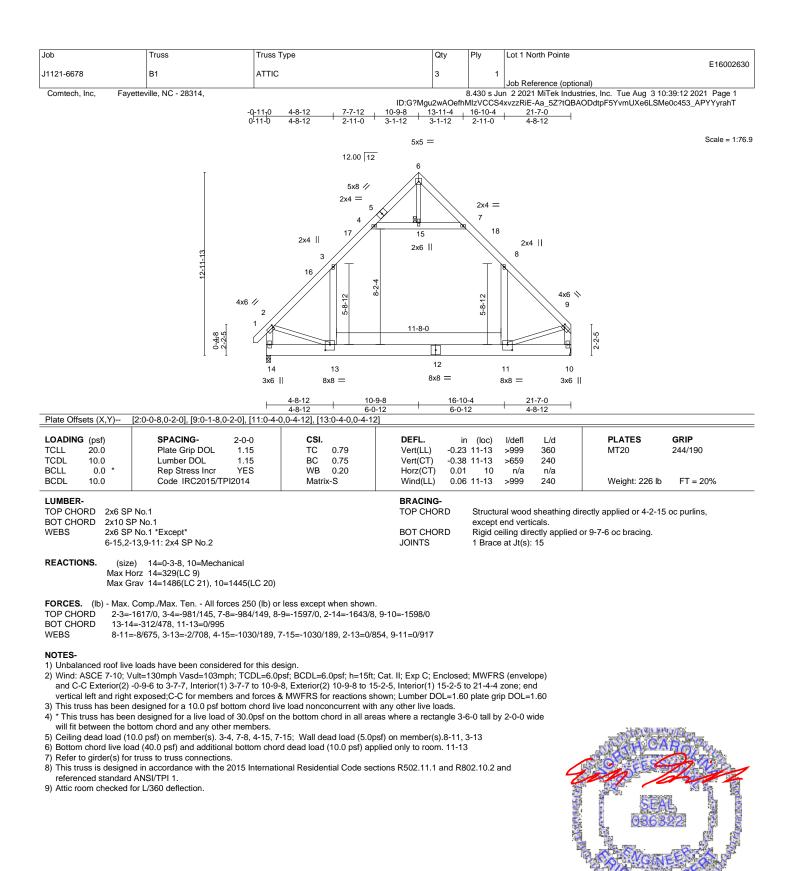
NOTES-

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27 except (jt=lb) 16=134.
 This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Attic room checked for L/360 deflection.

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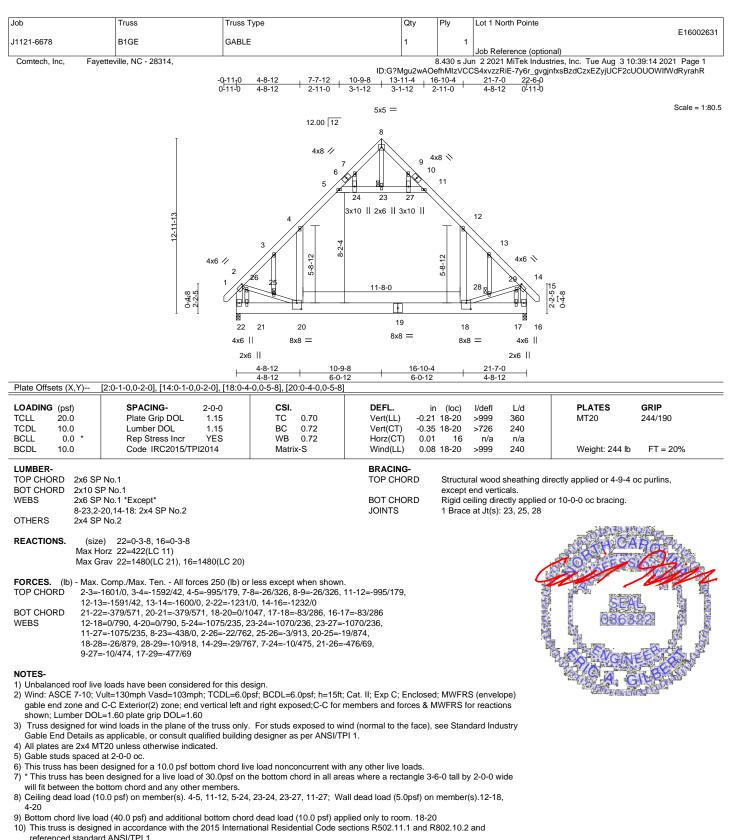




August 3,2021

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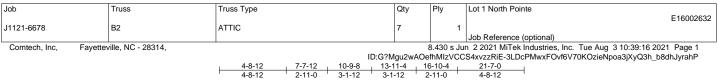


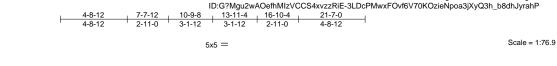
11) Attic room checked for L/360 deflection.

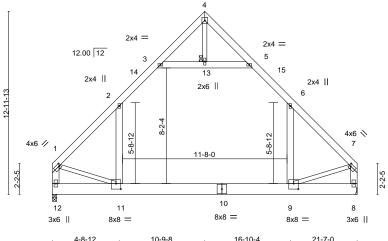
August 3,2021

🛕 WARNING - Verify design p eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601









		4-8-12 10-9-8 4-8-12 6-0-12		4-8-12				
Plate Offsets (X,Y)	[1:0-1-4,0-2-0], [7:0-1-4,0-2-0], [9:0-4-0		0-0-12	4-0-12				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.78 BC 0.75 WB 0.20 Matrix-S	Horz(CT) 0.01	9-11 >999 360 9-11 >653 240	PLATES GRIP MT20 244/190 Weight: 223 lb FT = 20%			
BOT CHORD 2x10 S WEBS 2x6 SF	TOP CHORD 2x6 SP No.1 TOP CHORD Structural wood sheathing directly applied or 4-2-11 oc purlins, except end verticals. BOT CHORD 2x10 SP No.1 Except end verticals.							
REACTIONS. (size) 12=0-3-8, 8=Mechanical Max Horz 12=313(LC 11) Max Grav 12=1446(LC 21), 8=1446(LC 20)								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-1600/0, 2-3=-984/147, 5-6=-984/147, 6-7=-1600/0, 1-12=-1600/0, 7-8=-1601/0 BOT CHORD 11-12=-303/406, 9-11=0/997 WEBS 6-9=-6/678, 2-11=-7/678, 3-13=-1036/187, 5-13=-1036/187, 1-11=0/915, 7-9=0/919								
 Wind: ASCE 7-10; V and C-C Exterior(2) vertical left and right This truss has been 	e loads have been considered for this de /ult=130mph Vasd=103mph; TCDL=6.0 0-2-12 to 4-8-12, Interior(1) 4-8-12 to 1 t exposed;C-C for members and forces designed for a 10.0 psf bottom chord lin n designed for a live load of 30.0psf on	osf; BCDL=6.0psf; h=15ft;)-9-8, Exterior(2) 10-9-8 to & MWFRS for reactions sh ve load nonconcurrent with	o 15-2-5, Interior(1) 15-2 nown; Lumber DOL=1.60 n any other live loads.	-5 to 21-4-4 zone; end) plate grip DOL=1.60				

will fit between the bottom chord and any other members.

- 5) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-13, 5-13; Wall dead load (5.0psf) on member(s).6-9, 2-11
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 9-11

7) Refer to girder(s) for truss to truss connections.8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

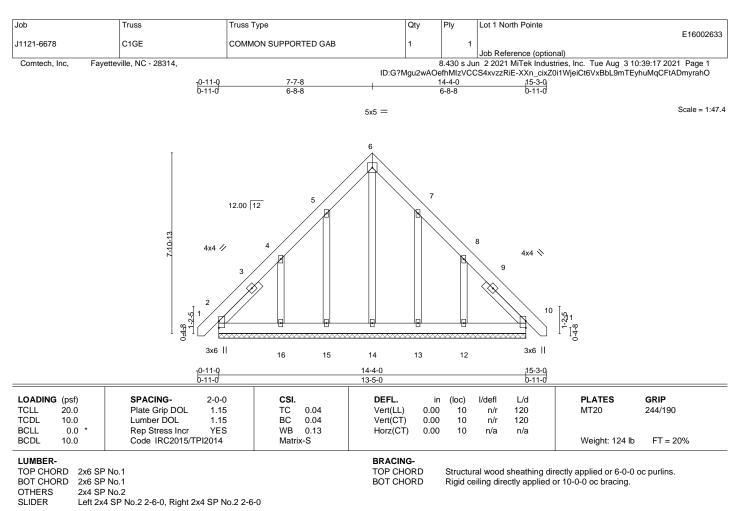
9) Attic room checked for L/360 deflection.



August 3,2021

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REACTIONS. All bearings 13-5-0

(lb) - Max Horz 2=-224(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 13 except 16=-256(LC 12), 12=-251(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 13 except 16=270(LC 19), 12=265(LC 20)

WEBS 4-16=-280/263. 8-12=-280/260

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 13 except (it=lb) 16=256, 12=251,
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

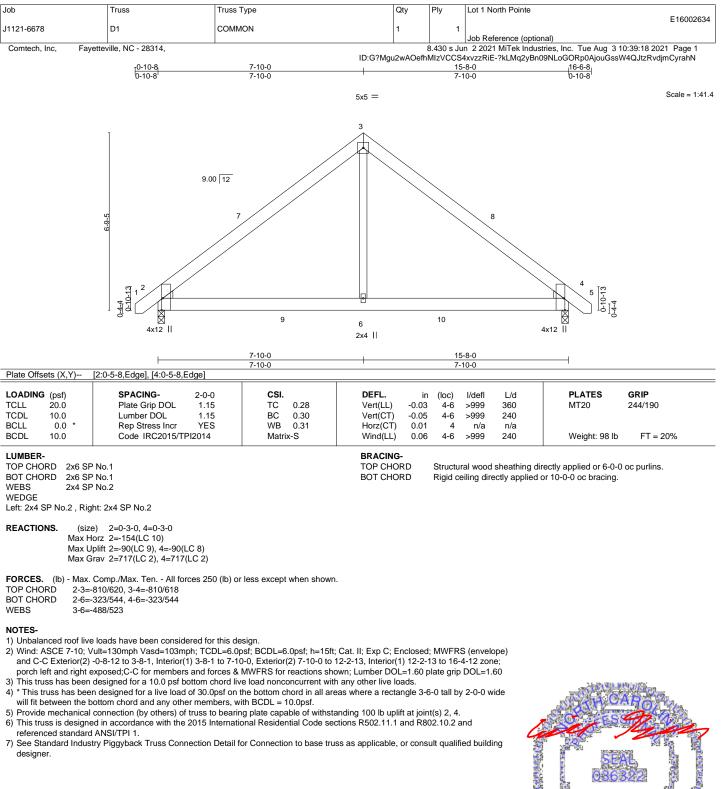


August 3,2021

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



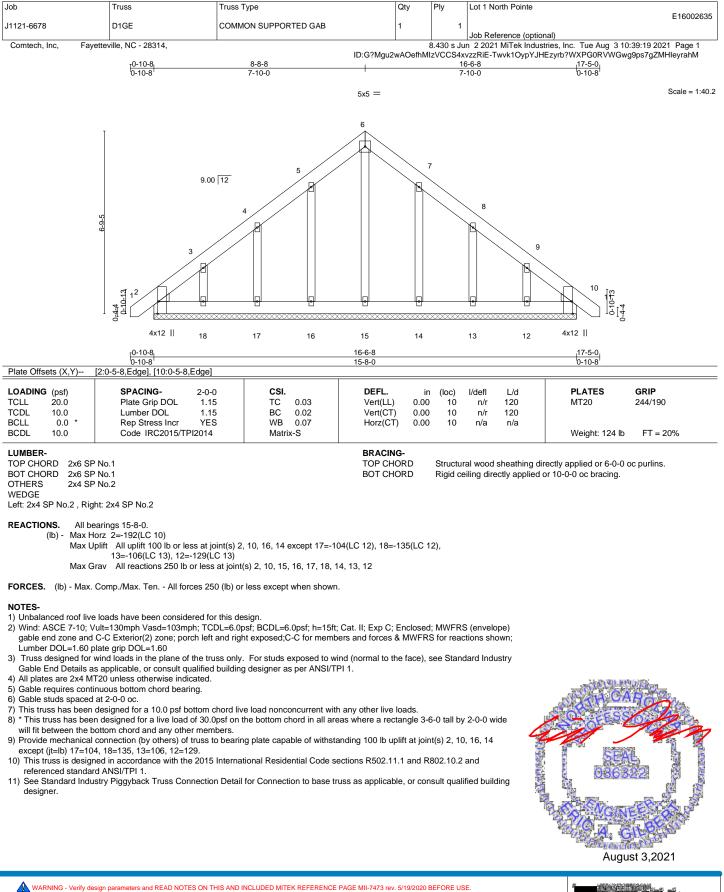


August 3,2021

🗥 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



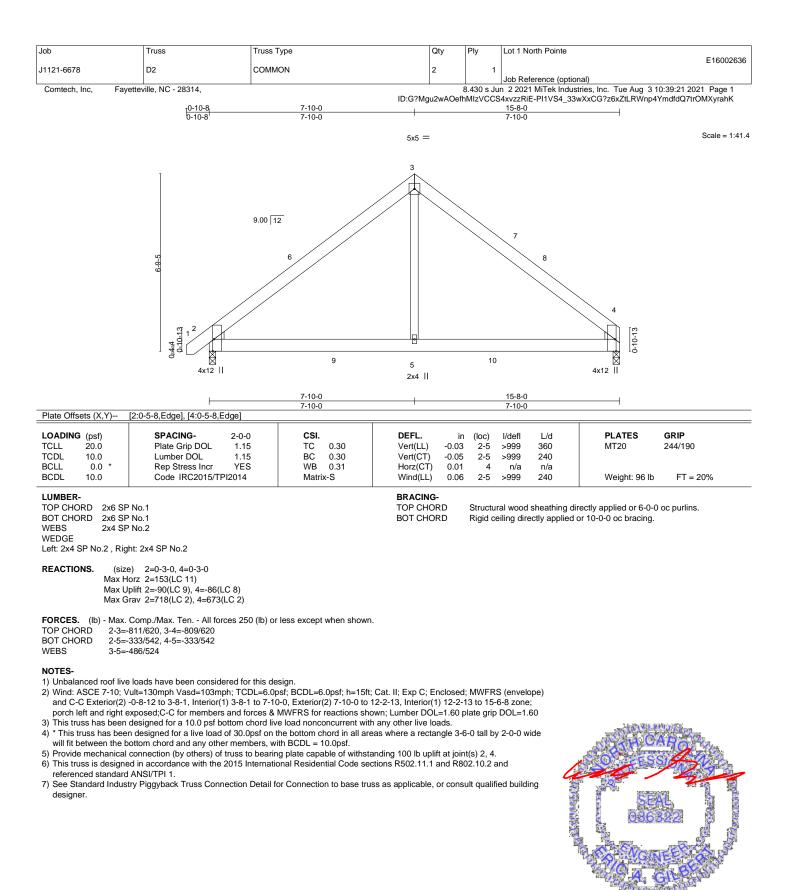
Edenton, NC 27932



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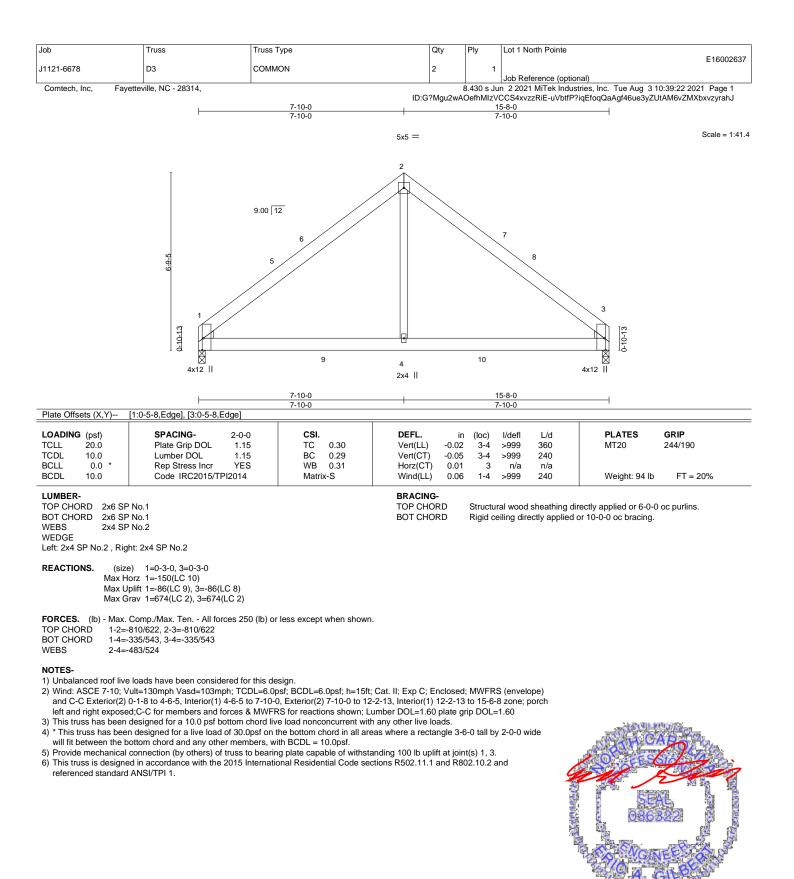
Edenton, NC 27932



August 3,2021

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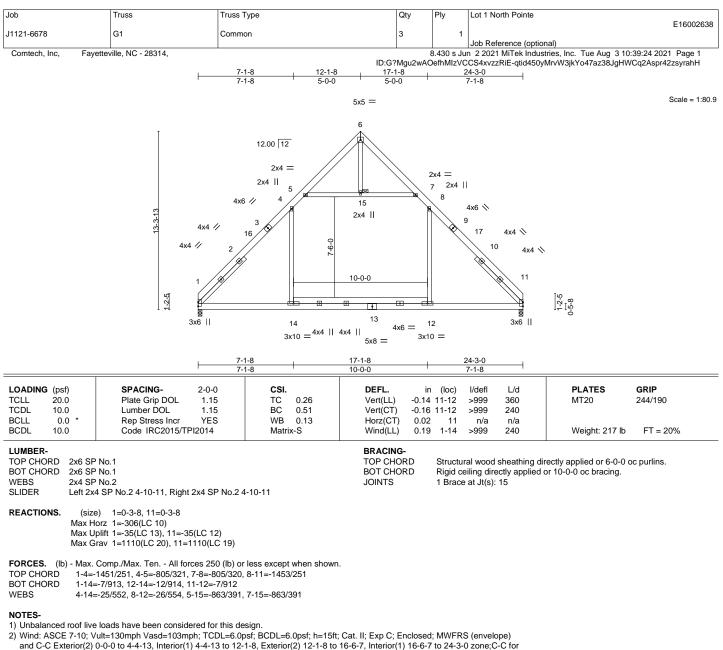




August 3,2021

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members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

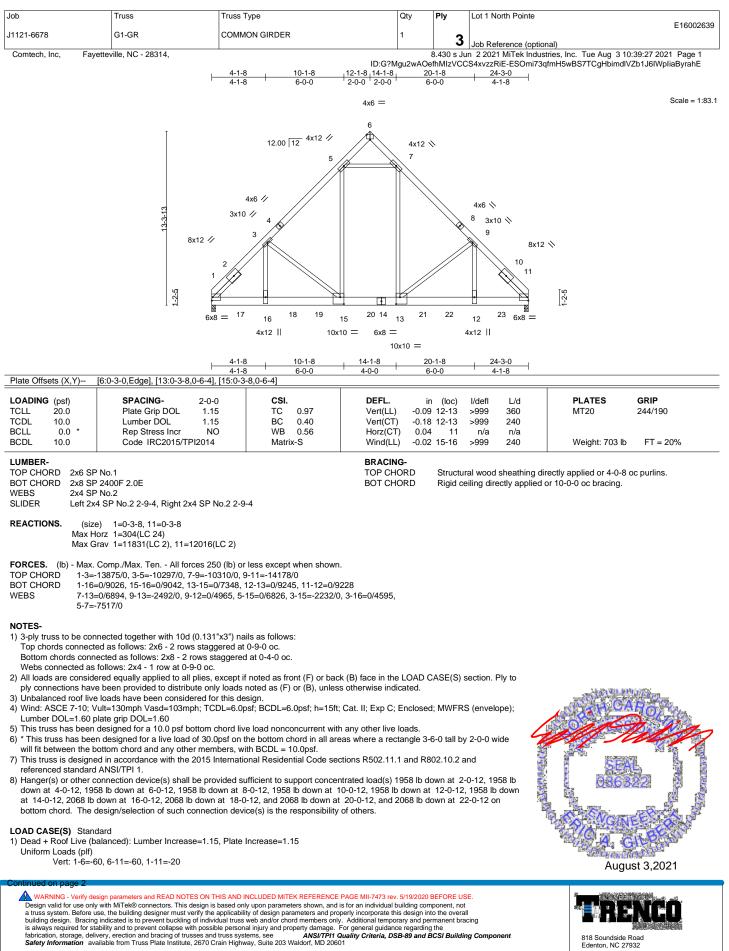
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.
 6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 August 3,2021





818 Soundside Road Edenton, NC 27932

Job		Truss	Truss Type	Qty	Ply	Lot 1 North Pointe
						E16002639
J1121-667	8	G1-GR	COMMON GIRDER	1	2	
						Job Reference (optional)
Comtech	, Inc, Fayettev	rille, NC - 28314,			8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 10:39:27 2021 Page 2

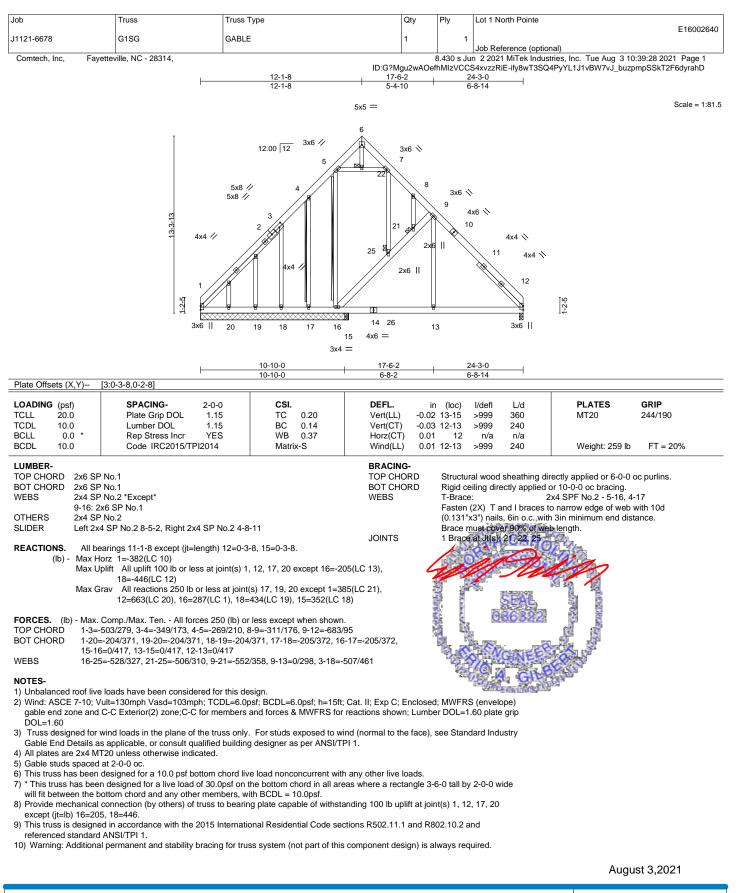
ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-ESOmi73qfmH5wBS7TCgHbimdIVZb1J6IWpliaByrahE

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 13=-1547(B) 12=-1645(B) 15=-1547(B) 16=-1547(B) 17=-1547(B) 18=-1547(B) 19=-1547(B) 20=-1547(B) 21=-1645(B) 22=-1645(B) 23=-1645(B) 23=-1645(B)

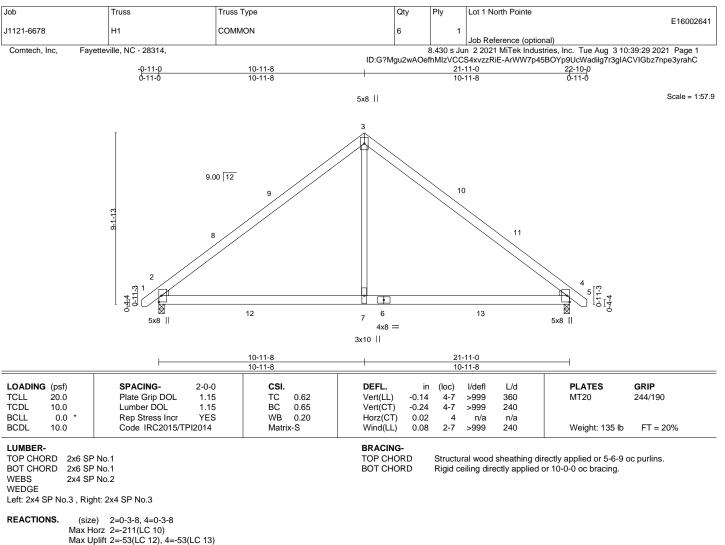
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Max Grav 2=1125(LC 19), 4=1125(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1278/229, 3-4=-1278/229 BOT CHORD 2-7=0/940, 4-7=0/940

WEBS 3-7=0/893

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-4 to 3-7-9, Interior(1) 3-7-9 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-8-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

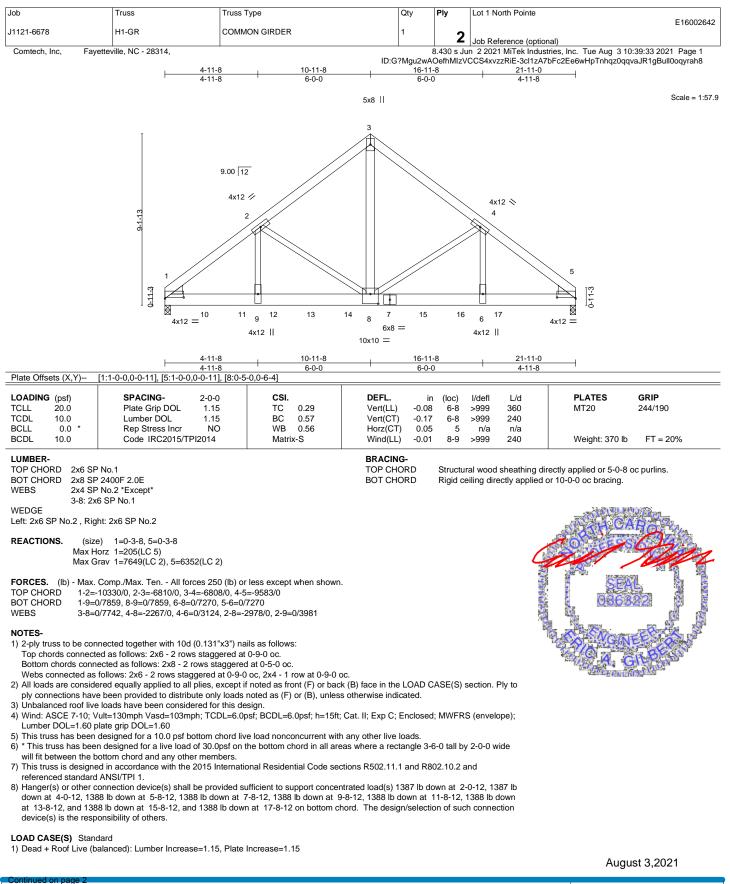
7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



August 3,2021

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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[Job	Truss	Truss Type	Qty	Ply	Lot 1 North Pointe
						E16002642
	J1121-6678	H1-GR	COMMON GIRDER	1	2	Job Reference (optional)
l						
	Comtech, Inc, Fayettev	ille, NC - 28314,		;	8.430 s Ju	n 2 2021 MiTek Industries, Inc. Tue Aug 3 10:39:34 2021 Page 2

ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-XpJPAW8D?wA5GGVTNAIwNAZ?aJwYAUwK7PVaKHyrah7

LOAD CASE(S) Standard

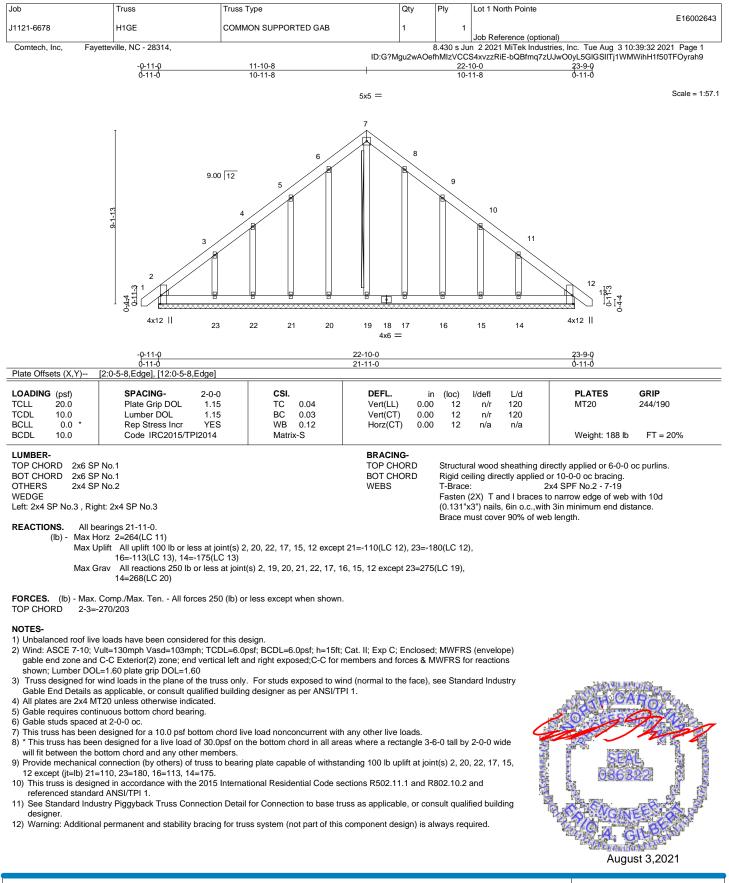
Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-1130(B) 10=-1128(B) 11=-1128(B) 12=-1130(B) 13=-1130(B) 14=-1130(B) 15=-1130(B) 16=-1130(B) 17=-1130(B)

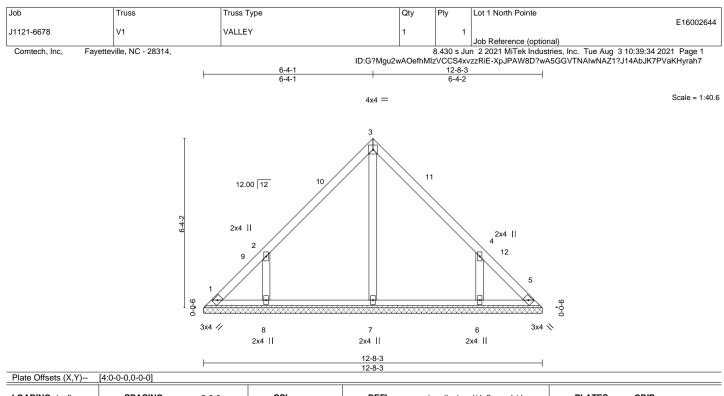
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidal truss event and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.14 BC 0.09 WB 0.09 Matrix-S	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES GRIP MT20 244/190 Weight: 58 lb FT = 20%
LUMBER-			BRACING-				

TOP CHORD

BOT CHORD

UMBER

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. All bearings 12-8-3

(lb) - Max Horz 1=144(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-161(LC 12), 6=-161(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=342(LC 19), 6=342(LC 20)

WEBS 2-8=-356/291, 4-6=-355/291

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-4-1, Exterior(2) 6-4-1 to 10-8-14, Interior(1) 10-8-14 to 12-3-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161.6=161.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

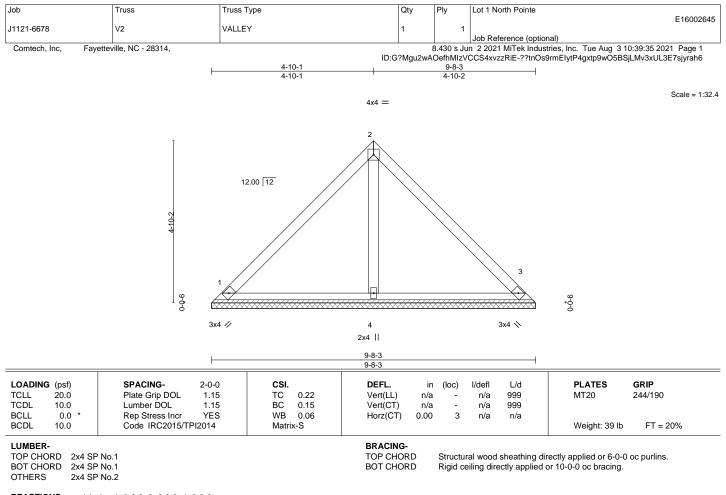
Rigid ceiling directly applied or 10-0-0 oc bracing.

August 3,2021

ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. 🛕 WARNING - Verify design pa Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



REACTIONS. (size) 1=9-8-3, 3=9-8-3, 4=9-8-3 Max Horz 1=-108(LC 8)

Max Uplift 1=-27(LC 13), 3=-27(LC 13)

Max Grav 1=204(LC 1), 3=204(LC 1), 4=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

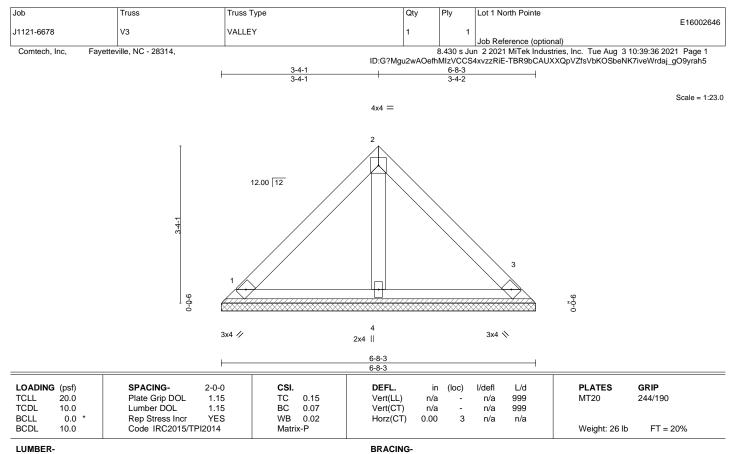
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





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TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=6-8-3, 3=6-8-3, 4=6-8-3

Max Horz 1=-72(LC 8)

Max Uplift 1=-26(LC 13), 3=-26(LC 13)

Max Grav 1=146(LC 1), 3=146(LC 1), 4=187(LC 1)

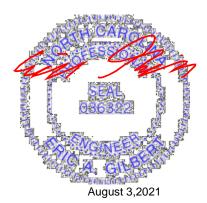
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

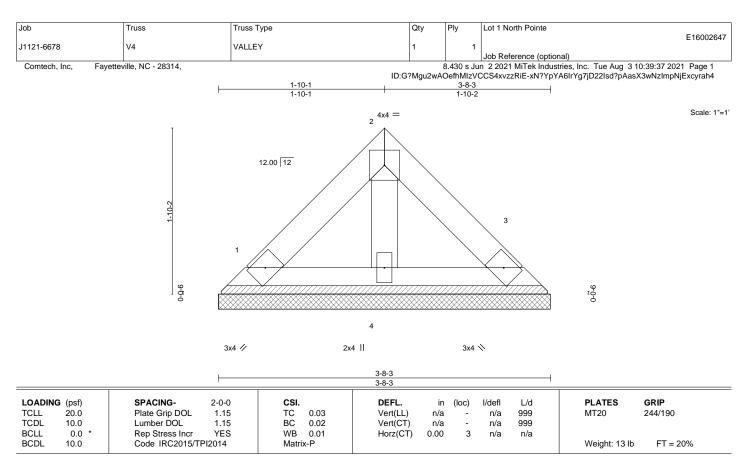


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-8-3, 3=3-8-3, 4=3-8-3

Max Horz 1=-36(LC 8) Max Uplift 1=-13(LC 13), 3=-13(LC 13)

Max Grav 1=72(LC 1), 3=73(LC 1), 4=93(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 3-8-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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