

RE: 2434740 - H&H/Kenzie/

Site Information:

City:

Project Customer: H AND H Project Name: 2434740 MASTER Lot/Block: Subdivision: Model: Address:

State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Wind Speed: 150 mph Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.2 Design Method: MWFRS (Envelope)/C-C hybrid Wind ASCE 7-10 Floor Load: N/A psf

Trenco

818 Soundside Rd

Edenton, NC 27932

Mean Roof Height (feet): 25

Exposure Category: C

No.	Seal#	Truss Name	Date
1 2 3 4	42553546 42553547 42553548 42553549	A01 A02 A03 A04	8/24/20 8/24/20 8/24/20 8/24/20
5 6 7 8 9	142553551 142553552 142553553 142553554	B01 B02 B03	8/24/20 8/24/20 8/24/20 8/24/20 8/24/20
10 11 12 13	142553555 142553556 142553557 142553558 142553558	B05 B06 C01 C02 CP01	8/24/20 8/24/20 8/24/20 8/24/20 8/24/20
16 17	142553560 142553560 142553561 142553562	CP02 D01	8/24/20 8/24/20 8/24/20 8/24/20
19 20 21 22	142553565 142553564 142553565 142553566 142553567	G01 G02 H01 H02 H02	8/24/20 8/24/20 8/24/20 8/24/20 8/24/20 8/24/20
25 26 27 28	142553560 142553569 142553570 142553571 142553572 142553573	H04 J01 J02 J03 V01	8/24/20 8/24/20 8/24/20 8/24/20 8/24/20 8/24/20

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters

Inscource-Sumter,SC. Iruss Design Engineer's Name: Johnson, Andrew My license renewal date for the state of North Carolina is December 31, 2020 **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building the building designer should verify applicability of the designs for any particular building to the sedeside. incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



August 24,2020



except (jt=lb) 34=115, 35=118, 37=114, 38=115, 39=114, 40=119, 42=180, 32=112, 31=119, 29=114, 28=115, 27=114, 26=119, 20=114, 24=177.

Annannin Summer Boot JOHD JULIU August 24,2020

> 818 Soundside Road Edenton, NC 27932

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		9-9-0	I	18-11-8	1	28-2-0	I	37-11-0	1	
T		9-9-0	1	9-2-8	1	9-2-8	I	9-9-0		
Plate Offs	sets (X,Y)	[3:0-5-0,0-4-8], [5:0-5-0,0	-4-8]							
LOADING	G (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) I/	defl L/d	PLATES	GRIP	-
TCLL	20.0	Plate Grip DOL	1.15	TC 0.55	Vert(LL)	-0.17 10-12 >	999 360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC 0.96	Vert(CT)	-0.42 10-12 >	999 240	MT20HS	187/143	
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.73	Horz(CT)	0.16 6	n/a n/a			

Wind(LL)

BRACING-

WEBS

TOP CHORD

BOT CHORD

0.24 12-15

>999

1 Row at midpt

Rigid ceiling directly applied

240

Structural wood sheathing directly applied.

LUMBER-

BCDL

TOP CHORD2x6 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

10.0

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=245(LC 12) Max Uplift 2=-725(LC 12), 6=-725(LC 13) Max Grav 2=1556(LC 1), 6=1556(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2015/TPI2014

TOP CHORD 2-3=-3058/2009, 3-4=-2107/1519, 4-5=-2107/1519, 5-6=-3058/2009

BOT CHORD 2-12=-1662/2737, 10-12=-1664/2734, 8-10=-1666/2734, 6-8=-1664/2737

WEBS 4-10=-601/1011, 5-10=-1020/848, 5-8=0/372, 3-10=-1020/848, 3-12=0/372

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-AS

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=725, 6=725.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



FT = 20%

Weight: 209 lb

5-10, 3-10

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7	-1-0 -1-0	13-9-9 6-8-9		24-1-7 10-3-14				30-10-0 6-8-9	37-1 7-1	1-0 -0
LOADING (psf) TCLL 20.0	SPACING- 2-0 Plate Grip DOL 1	0-0 CSI. .15 TC	0.62	DEFL. Vert(LL)	in (-0.18	(loc) 16	l/defl >999	L/d 360	PLATES MT20	GRIP 244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1 Rep Stress Incr Y	.15 BC ES WB	0.68 0.61	Vert(CT) Horz(CT)	-0.50 15 0.10	5-16 10	>902 n/a	240 n/a	MT20HS	187/143
BCDL 10.0	Code IRC2015/TPI201	14 Matr	ix-AS	Wind(LL)	0.26	17	>999	240	Weight: 241 lb	FT = 20%

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied.

6-14, 6-17

Rigid ceiling directly applied.

1 Row at midpt

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP No.2
WEBS	2x4 SP No.3 *Except
	20-21: 2x4 SP No.2

- REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=248(LC 12) Max Uplift 2=-635(LC 12), 10=-635(LC 13) Max Grav 2=1669(LC 1), 10=1669(LC 1)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-3402/1863, 3-5=-2850/1501, 5-6=-2837/1710, 6-7=-2837/1710, 7-9=-2850/1501,
- 9-10=-3402/1863
- BOT CHORD 2-19=-1555/3071, 17-19=-1555/3071, 16-17=-663/1951, 15-16=-663/1951,
- 14-15=-663/1951, 12-14=-1562/3071, 10-12=-1562/3071
- WEBS 6-21=-602/1113, 14-21=-631/1098, 7-14=-376/480, 9-14=-588/606, 17-20=-631/1098,
- 6-20=-602/1113, 5-17=-376/480, 3-17=-588/607

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) 200.0lb AC unit load placed on the bottom chord, 18-11-8 from left end, supported at two points, 5-0-0 apart.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 635 lb uplift at joint 2 and 635 lb uplift at ioint 10.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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1		7-1-0	13-9-9		24-1-7		30-10)-0 3	37-11-0
		7-1-0	6-8-9	I	10-3-14	1	6-8-	.9	7-1-0
Plate Offs	ets (X,Y)	[2:0-3-0,Edge], [10:0)-3-0,Edgej					L.	
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DC Lumber DOL Rep Stress Ir Code IRC20	2-5-8 DL 1.15 1.15 https://www.action. 15/TPI2014	CSI. TC 0.56 BC 0.97 WB 0.83 Matrix-MS	DEFL. Vert(LL) - Vert(CT) - Horz(CT) Wind(LL)	in (loc) -0.17 16 -0.50 15-16 0.13 10 0.25 17	I/defI L/ >999 36 >908 24 n/a n/ >999 24	d PLATES 0 MT20 0 a 0 Weight: 275	GRIP 244/190 Ib FT = 20%
LUMBER TOP CHC BOT CHC WEBS	- DRD 2x6 SF DRD 2x6 SF 2x4 SF 20-21:	P No.2 P No.2 P No.3 *Except* 2x4 SP No.2			BRACING- TOP CHORD BOT CHORD WEBS) Structo Rigid o 1 Row	ural wood shea ceiling directly at midpt	thing directly applied or 3-2 applied or 5-0-4 oc bracing. 6-14, 6-17	-9 oc purlins.
REACTIC	MS. (size Max H Max U Max G	e) 2=0-3-8, 10=0-3 lorz 2=301(LC 12) plift 2=-792(LC 12), irav 2=2013(LC 1), 1	-8 10=-792(LC 13) 10=2013(LC 1)						
FORCES. TOP CHC	(lb) - Max. RD 2-3=- 9-10=	Comp./Max. Ten / 4235/2417, 3-5=-349 =-4235/2417	All forces 250 (lb) or 90/1932, 5-6=-3472/	less except when showr 2188, 6-7=-3472/2188, 7	n. 7-9=-3490/1932,				
BOT CHC	RD 2-19=	=-2042/3858, 17-19= 5=-874/2378, 12-14=	-2042/3858, 16-17= -2047/3858, 10-12=	-874/2378, 15-16=-874/2 -2047/3858	2378,				
WEBS	6-21= 17-20	=-783/1356, 14-21=-8)=-817/1342, 6-20=-7	817/1342, 7-14=-455 783/1356, 5-17=-455	5/592, 9-14=-813/786, 9- 5/592, 3-17=-813/786, 3-	-12=0/257, -19=0/257				
NOTES- 1) Unbala 2) Wind: / MWFR MWFR 2) 200 Off	nced roof live ASCE 7-10; V S (envelope) S for reaction	e loads have been cc /ult=150mph (3-seco gable end zone and as shown; Lumber D(nsidered for this dea nd gust) Vasd=119n C-C Exterior(2) zon DL=1.60 plate grip D m chord 18 11 9 fra	sign. nph; TCDL=6.0psf; BCD e; end vertical left and ri OL=1.60 m loft end, supported of	L=6.0psf; h=25ft; Ca ght exposed;C-C for	at. II; Exp C; I members ar	Enclosed; Id forces &		

200.0lb AC unit load placed on the bottom chord, 18-11-8 from left end, supported at two points, 5-0-0 apart.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 792 lb uplift at joint 2 and 792 lb uplift at joint 10.



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Edenton, NC 27932



	<u>8-9-0</u> 8-9-0		<u>17-11-8</u> 9-2-8		27-2-0 9-2-8		<u>35-11-0</u> 8-9-0	
Plate Offsets (X,Y)	[3:0-5-0,0-4-8], [5:0-5-0,0-	4-8]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC 0.49 BC 0.98 WB 0.56 Matrix-AS	DEFL. i Vert(LL) -0.1 Vert(CT) -0.4 Horz(CT) 0.1 Wind(LL) 0.2	n (loc) l/def 7 10-12 >999 2 10-12 >999 4 6 n/a 1 10-12 >999	fl L/d 9 360 9 240 a n/a 9 240	PLATES MT20 MT20HS Weight: 199 lb	GRIP 244/190 187/143 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x4 SP	2 No.2 2 No.2			BRACING- TOP CHORD BOT CHORD	Structural wo Rigid ceiling	ood sheathing dir directly applied.	ectly applied.	

WEBS

1 Row at midpt

5-10, 3-10

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=233(LC 16)

2x4 SP No.3

Max Uplift 2=-688(LC 12), 6=-688(LC 13) Max Grav 2=1476(LC 1), 6=1476(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2924/1923, 3-4=-2007/1434, 4-5=-2007/1434, 5-6=-2924/1923

BOT CHORD 2-12=-1601/2621, 10-12=-1602/2618, 8-10=-1605/2618, 6-8=-1603/2621

WFBS 4-10=-514/914, 5-10=-977/812, 5-8=0/349, 3-10=-977/812, 3-12=0/349

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=688, 6=688.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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818 Soundside Road

Edenton, NC 27932

- ----August 24,2020



H	8-9-0	+ 17-10 9-1	0-14			27-2-0 9-3-2			35-11-0	
Plate Offsets (X,Y)	[3:0-5-0,0-4-8], [5:0-5-0,0-4-	8]				002				
LOADING (psf)	SPACING- 2	2-0-0 CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15 TC	0.50	Vert(LL)	-0.18	7-9	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15 BC	0.98	Vert(CT)	-0.43	7-9	>999	240	MT20HS	187/143
BCLL 0.0 *	Rep Stress Incr	YES WB	0.56	Horz(CT)	0.14	6	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI20	014 Mat	rix-AS	Wind(LL)	0.22	9-11	>999	240	Weight: 197 lb	FT = 20%
LUMBER-		ŀ	·	BRACING-						

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied.

5-9, 3-9

Rigid ceiling directly applied

1 Row at midpt

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

- REACTIONS. (size) 2=0-3-8, 6=Mechanical Max Horz 2=239(LC 16) Max Uplift 2=-688(LC 12), 6=-658(LC 13)
 - Max Grav 2=1477(LC 1), 6=1436(LC 1)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-2927/1928, 3-4=-2012/1438, 4-5=-2014/1439, 5-6=-2938/1944
- BOT CHORD 2-11=-1623/2624, 9-11=-1624/2623, 7-9=-1646/2652, 6-7=-1643/2657
- 4-9=-519/922, 5-9=-1004/835, 5-7=0/353, 3-9=-977/810, 3-11=0/348 WFBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7) 2=688, 6=658
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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 Satisfies
 Ansi/TPH Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





6	6-10-0 13-7-2		22-3-14	29-1-0	35-11-0
Diata Offacta (X X)	6-9-2 [2:0 2 0 0 2 0] [7:0 2 0 0 2 0]		8-8-11	6-9-2	6-10-0
Plate Offsets (A, f)	[3.0-3-0,0-3-0], [7.0-3-0,0-3-0]	1			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.61 BC 0.59 WB 0.93 Matrix-AS	DEFL. in Vert(LL) -0.16 Vert(CT) -0.40 Horz(CT) 0.09 Wind(LL) 0.23	(loc) l/defl L/d 13 >999 360 12-13 >999 240 8 n/a n/a 14 >999 240	PLATES GRIP MT20 244/190 Weight: 226 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP 17-18:	No.2 No.2 No.3 *Except* 2x4 SP No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir Rigid ceiling directly applied.	ectly applied.
REACTIONS. (size Max H Max U Max G	e) 2=0-3-8, 8=Mechanical orz 2=246(LC 12) plift 2=-599(LC 12), 8=-557(LC 13) rav 2=1590(LC 1), 8=1536(LC 1)				
FORCES. (lb) - Max. TOP CHORD 2-3=- 7-8=- 7-8=- BOT CHORD 2-16= 11-12 12 WEBS 5-18= 5-17=	Comp./Max. Ten All forces 250 (lb) or 3223/1737, 3-4=-2655/1390, 4-5=-2635 3229/1744 =-1474/2907, 14-16=-1476/2905, 13-14= 2=-624/1847, 9-11=-1482/2912, 8-9=-14 568/1025, 11-18=-602/1013, 6-11=-35 e-567/1024, 4-14=-353/454, 3-14=-603/	less except when shown /1584, 5-6=-2636/1585, 6 624/1847, 12-13=-624/1 80/2914 3/453, 7-11=-610/599, 14 593	-7=-2656/1390, 847, -17=-601/1011,		
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) reactions shown; Lur 3) 200.0lb AC unit load 4) All plates are 2x4 MT 5) This truss has been (* This truss has been will fit between the b 7) Refer to girder(s) for 8) Provide mechanical 2=599, 8=557.	e loads have been considered for this de ult=150mph (3-second gust) Vasd=119 gable end zone and C-C Exterior(2) zor mber DOL=1.60 plate grip DOL=1.60 placed on the bottom chord, 17-11-8 fr 720 unless otherwise indicated. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on 1 ottom chord and any other members. truss to truss connections. connection (by others) of truss to bearin	sign. mph; TCDL=6.0psf; BCDI te; end vertical left expose orn left end, supported at e load nonconcurrent with he bottom chord in all are g plate capable of withsta	L=6.0psf; h=25ft; Cat. II; E ed;C-C for members and f two points, 5-0-0 apart. n any other live loads. eas where a rectangle 3-6- anding 100 lb uplift at joint	xp C; Enclosed; orces & MWFRS for -0 tall by 2-0-0 wide (s) except (jt=lb)	SEAL

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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L (6-10-0 1	3-7-2	22-3-14	I	29-1-0	J 35-1	11-0
	6-10-0	ò-9-2	8-8-11		6-9-2	6-1	0-0
Plate Offsets (X,Y)	[2:0-1-0,Edge], [3:0-5-0,0-4-8], [7	0-5-0,0-4-8], [8:0-1-0,Edge]					
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-5-8 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.54 BC 0.86	DEFL. in Vert(LL) -0.15 Vert(CT) -0.39	(loc) l/defl 13 >999 12-13 >999	L/d 360 240	PLATES MT20	GRIP 244/190
BCLL 0.0 *	Rep Stress Incr NO	WB 0.84	Horz(CT) 0.12	8 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS	Wind(LL) 0.22	14 >999	240	Weight: 259 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP 17-18: 17-18:	P No.2 P No.2 P No.3 *Except* 2x4 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood Rigid ceiling din 1 Row at midpt	d sheathing di ectly applied	rectly applied or 3-4-3 o or 5-2-9 oc bracing. 5-11, 5-14	oc purlins.
REACTIONS. (size Max H Max U Max G	e) 2=0-3-8, 8=Mechanical orz 2=296(LC 12) plift 2=-747(LC 12), 8=-708(LC 13 rav 2=1915(LC 1), 8=1865(LC 1)	3)					
FORCES. (lb) - Max. TOP CHORD 2-3=- 7-8=-	Comp./Max. Ten All forces 250 4008/2256, 3-4=-3246/1789, 4-5= 4012/2261	(lb) or less except when shown. -3219/2026, 5-6=-3219/2027, 6-	7=-3247/1790,				
BOT CHORD 2-16= 11-12	=-1925/3649, 14-16=-1928/3643, ⁻ 2=-820/2251, 9-11=-1932/3648, 8-	3-14=-820/2251, 12-13=-820/22 9=-1930/3654	251,				
WEBS 5-18= 14-17	731/1242, 11-18=-771/1232, 6-1 7=-770/1231, 5-17=-730/1241, 4-1	1=-422/551, 7-11=-829/773, 7-9 4=-422/552, 3-14=-824/769, 3-1	=0/282, 6=0/281				
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) reactions shown; Lu 3) 200.0lb AC unit load 4) All plates are 2x4 M	e loads have been considered for f /ult=150mph (3-second gust) Vaso gable end zone and C-C Exterior mber DOL=1.60 plate grip DOL=1 / placed on the bottom chord, 17-1 T20 unless otherwise indicated.	his design. I=119mph; TCDL=6.0psf; BCDL= 2) zone; end vertical left expose .60 1-8 from left end, supported at tv	=6.0psf; h=25ft; Cat. II; l d;C-C for members and wo points, 5-0-0 apart.	Exp C; Enclosed forces & MWFR	; S for	INTH C	ARO

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=747, 8=708.



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- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10 except (jt=lb) 16=159, 17=153, 18=193, 14=157, 13=153, 12=186.

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H/Kenzie/	
						142553558
2434740	C02	Common Girder	3	2		
				2	Job Reference (optional)	
Builders FirstSource, S	umter, SC - 29153,		8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon Aug 24 13:45:36 2020	Page 2
		ID:2zFdWT	uXpJBB?	X9xOWC	pYycTNe-EFJLc7c4dThrwZvWt3MyWYpY3SdhQ M4Ssobt	oxykd j

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 9-12=-20

Concentrated Loads (lb)

Vert: 6=-1536(B) 15=-1649(B) 16=-1649(B) 17=-1649(B) 18=-1988(B) 19=-1988(B) 20=-1536(B) 21=-1536(B)

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			<u> </u>			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.12 BC 0.04 WB 0.06 Matrix-S	DEFL. ir Vert(LL) 0.00 Vert(CT) 0.00 Horz(CT) -0.00	n (loc) l/defl L/d 0 1 n/r 120 0 1 n/r 120 0 8 n/a n/a	PLATES GRIF MT20 244/ Weight: 42 lb F	190 Γ = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP	No.2 No.2 No.3	1	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 oc pu or 10-0-0 oc bracing.	rlins,

OTHERS 2x4 SP No.3

REACTIONS. All bearings 10-0-0.

(lb) -Max Horz 2=195(LC 9)

2-3=-280/149

Max Uplift All uplift 100 lb or less at joint(s) 8, 9, 2, 10 except 11=-100(LC 12), 12=-102(LC 8), 13=-111(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 8, 9, 2, 10, 11, 12, 13

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

TOP CHORD

1) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 9, 2, 10 except (jt=lb) 11=100, 12=102, 13=111.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTReK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component
 Satisfies
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2=434, 5=403.8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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August 24,2020



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Job	Truss	Truss Type	Qty	Ply H&H	/Kenzie/	
2434740	E01	Common Structural Gable	3	1		142553562
Builders FirstSource.	Sumter, SC - 29153.			Job F 3.240 s Mar 9 202	Reference (optional) 20 MiTek Industries, Inc.	Mon Aug 24 13:45:47 2020 Page 1
,	-0-10-8 3-1	1-4 7-8-12 10	ID:2zFdWTuXpJBB'	21X9xOWCpYycT 21-11-0	Ne-QMTVwulz1s3HkFFd 22-9-	00t2XTsmQGuR0V4gi_3zhUoykd_Y
	0-10-8 3-1	1-4 3-9-8 3	-2-12 U-6-12	10-4-12	0-10-	8 Scale = 1:62.5
			4x0 —			00410 - 1.02.0
	7x10 =	4x6 = 10.00 12 $4x6 \neq$ 3 3 4 4 4 4 4 5 4 4 4 5 5		12 B	4 15 3x6 16	
	N 4 ²			$1^{3}_{235x8} =$	10 17	
			L L L L L L L L L L L L L L L L L L L			1-0-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1
	凶 27	26 4x8 =	25 4x6 =	24 22	4x6 = 3x8 21 20 19	
		7.9.12	15 5 9		21.11.0	
Plate Offsets (X Y)	[2:0-1-12 0-1-7] [2:Edge 0-2-12	7-8-12 7-8-12	7-8-12		6-5-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0- Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.34 BC 0.22 WB 0.23 Matrix-AS	DEFL. i Vert(LL) -0.02 Vert(CT) -0.04 Horz(CT) 0.07 Wind(LL) 0.07	n (loc) l/defl 2 26-27 >999 4 26-27 >999 1 19 n/a 1 26 >999	L/d P 360 M 240 n/a 240 V	PLATES GRIP 1T20 244/190 Veight: 195 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP OTHERS 2x4 SP	No.2 No.2 No.3 No.3		BRACING- TOP CHORD BOT CHORD JOINTS	Structural woo Rigid ceiling d 1 Brace at Jt(s	d sheathing directly ap irectly applied. s): 8, 11	plied, except end verticals.
REACTIONS. All be (lb) - Max H Max U Max G	earings 6-5-4 except (jt=length) 2 orz 27=-481(LC 10) plift All uplift 100 lb or less at jo 12), 27=-255(LC 12) rav All reactions 250 lb or less 24=607(LC 3)	7=0-3-8, 24=0-3-8. int(s) 19, 20, 24 except 22=-139 at joint(s) 19, 22, 23 except 20=;	(LC 13), 21=-404(LC 13 260(LC 22), 21=258(LC), 23=-432(LC 11), 27=703(LC	1),	
FORCES. (Ib) - Max. TOP CHORD 2-3=- 4-6=- 13-23	Comp./Max. Ten All forces 25 412/225, 3-4=-625/279, 14-15=- 600/361, 6-8=-657/417, 8-9=-64 8=-676/517	0 (lb) or less except when shown 364/174, 15-16=-503/278, 2-27= 5/394, 9-11=-402/279, 11-13=-48	n. 445/292, 80/348,			
BOT CHORD 26-27 WEBS 12-13 16-21	7=-348/763, 24-26=-180/620, 23 3=-269/242, 16-20=-298/121, 4-2 1=-258/421_9-23=-253/155	·24=-180/620, 22-23=-358/523, 2 :6=-154/417, 3-26=-315/319, 3-2	21-22=-358/523 27=-429/94,			
 NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) MWFRS for reaction 3) Truss designed for v Gable End Details a 4) All plates are 2x4 M 5) Gable studs spaced 6) This truss has been 7) * This truss has been 7) * This truss has been 8) Provide mechanical (jt=lb) 22=139, 21=4 9) This truss design register sheetrock be applied 10) Graphical purlin register 	e loads have been considered fo fult=150mph (3-second gust) Va gable end zone and C-C Exterio is shown; Lumber DOL=1.60 pla vind loads in the plane of the true s applicable, or consult qualified T20 unless otherwise indicated. at 2-0-0 oc. designed for a 10.0 psf bottom of n designed for a live load of 20.0 ottom chord and any other mem connection (by others) of truss t 04, 23=432, 27=255. quires that a minimum of 7/16" s d directly to the bottom chord. presentation does not depict the	this design. sd=119mph; TCDL=6.0psf; BCD r(2) zone; end vertical left and rig te grip DOL=1.60 ss only. For studs exposed to wi building designer as per ANSI/T hord live load nonconcurrent with psf on the bottom chord in all are bers. bearing plate capable of withsta ructural wood sheathing be appl size or the orientation of the purl	L=6.0psf; h=25ft; Cat. II; ght exposed;C-C for me ind (normal to the face), 'PI 1. th any other live loads. eas where a rectangle 3 anding 100 lb uplift at joi lied directly to the top ch lin along the top and/or t	Exp C; Enclose mbers and force: see Standard Ind -6-0 tall by 2-0-0 nt(s) 19, 20, 24 (ord and 1/2" gyp pottom chord.	d; s & dustry wide except sum	SEAL 45844 NGINEER SO
WARNING - Verify Design valid for use o a truss system. Before building design. Braci	design parameters and READ NOTES ON nly with MITek® connectors. This design use, the building designer must verify th ng indicated is to prevent buckling of ind sphiltik and to prevent buckling of ind	THIS AND INCLUDED MITEK REFERENC is based only upon parameters shown, ar e applicability of design parameters and p vidual truss web and/or chord members o bile personal billiour and upon the demonstry data truss the personal billiour and the personal demonstry of the personal billiour and the personal demonstry	CE PAGE MII-7473 rev. 5/19/20 nd is for an individual building o properly incorporate this design only. Additional temporary and a. For general guidance reserve	20 BEFORE USE. component, not i into the overall permanent bracing ling the		TRENCO AMITEK ATRIIIATE

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Job	Truss	Truss Type	Qty	Ply	H&H/Kenzie/	
						142553563
2434740	E02	Common Girder	3	2		
				-	Job Reference (optional)	
Builders FirstSource, S	umter, SC - 29153,		8	.240 s Mar	9 2020 MiTek Industries, Inc. Mon Aug 24 13:45:52 2020	Page 2

8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Aug 24 13:45:52 2020 Page 2 ID:2zFdWTuXpJBB?1X9xOWCpYycTNe-mKHOzco6sOiZr07bpQejAwTGsu4GAChR8LgRA?ykd_T

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 10-13=-20

Concentrated Loads (lb)

Vert: 8=-1416(B) 6=-1416(B) 16=-1416(B) 17=-1416(B) 18=-1416(B) 19=-1741(B) 20=-1741(B) 21=-1416(B) 22=-1416(B) 23=-1416(B) 24=-1416(B) 24=-140(B) 24=

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Edenton, NC 27932

August 24,2020



- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=394, 4=394.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCDL 10.0	Lumber DOL 1.15 Rep Stress Incr. YES	BC 0.05	Vert(CT) -0.00 9 n/r 120 W120 244/190 Horz(CT) 0.00 9 n/r 120
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 63 lb FT = 20%
LUMBER-	No 2		BRACING-
	No.2		event and verticely applied of 0 0 0 0 0 pulling,

 BOT CHORD
 2x4 SP No.2
 except end verticals.

 WEBS
 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0 oc bracing

 OTHERS
 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0 oc bracing

REACTIONS. All bearings 10-10-0.

(lb) - Max Horz 15=-232(LC 10)

Max Uplift All uplift 100 b or less at joint(s) 15, 10, 13, 12 except 14=-239(LC 12), 11=-238(LC 13) Max Grav All reactions 250 lb or less at joint(s) 15, 10, 13, 14, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-14=-253/210, 7-11=-252/211

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 Cable stude spaced at 2.0.0 cc.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 10, 13, 12 except (jt=lb) 14=239, 11=238.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=189, 2=219.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=173, 2=219.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Edenton, NC 27932



exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=208, 6=156.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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818 Soundside Road Edenton, NC 27932

Max Horz 2=86(LC 9) Max Uplift 6=-52(LC 8), 2=-103(LC 8), 7=-106(LC 12) Max Grav 6=71(LC 1), 2=117(LC 1), 7=165(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 2=103, 7=106.

818 Soundside Road Edenton, NC 27932

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Plate Offsets	(X,Y)	[4:Edge,0-1-14]											
LOADING (p TCLL 20 TCDL 10 BCLL 0	osf) 0.0 0.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.13 0.12 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.01 -0.00	(loc) 4-7 4-7 2	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20	GRIP 244/190	
BCDL 10	0.0	Code IRC2015/TP	12014	Matrix	K-MP						Weight: 18 lb	FT = 20%	
				•		BRACING							

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.2WEBS2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=84(LC 11)

Max Uplift 2=-160(LC 8), 4=-87(LC 12) Max Grav 2=211(LC 1), 4=146(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

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NOTES-

- Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=160.

Structural wood sheathing directly applied or 3-11-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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		2-11-8 2-11-8						3-11-8 1-0-0			
Plate Offsets (X,Y) [2:0-3-6,0-0-1]		1								
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	-0.00	9	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	-0.00	9	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB	0.06	Horz(CT)	0.00	2	n/a	n/a		
BCDL 10.0	Code IRC2015/1	[PI2014	Matri	K-MP	Wind(LL)	0.00	9	>999	240	Weight: 19 lb	FT = 20%
LUMBER-					BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x6 SP No.2 BOT CHORD WEBS 2x4 SP No.3

REACTIONS. (size) 4=0-1-8, 2=0-3-8 Max Horz 2=61(LC 11) Max Uplift 2=-132(LC 8)

Max Grav 4=282(LC 2), 2=247(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-3=-258/64 TOP CHORD

WEBS 3-5=-321/75

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=150mph (3-second gust) Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) C-C wind load user defined.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=132
- 10) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S)

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-80, 5-7=-20 Concentrated Loads (lb) Vert: 3=-150

Continued on page 2

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 Satisfies
 Ansi/TPH Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Structural wood sheathing directly applied or 3-11-8 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job	Truss	Truss Type	Qty	Ply	H&H/Kenzie/	142553572					
2434740	J03	Half Hip	42	1	Job Reference (optional)						
Builders FirstSource,	Sumter, SC - 29153,		8.1 0.27EdW/TuXp IPP2	240 s Mar	9 2020 MiTek Industries, Inc. Mon Aug 24 1	3:46:05 2020 Page 2					
		IL	лагийн ихрэвв?	1X9XOWC	prycrne-uqzjnzzGoOKjuuu44inmCrwzAo	GGIGHWI7SKU7Iyku_G					
2) Dead + 0.75 Roof Live	(balanced) + 0.75 Attic Floor:	Lumber Increase=1.15, Plate Increase=	=1.15								
Uniform Loads (plf)	, 2 4- 00 5 7- 20										
Concentrated Loads (Ib)))										
Vert: 4=-15 3= 3) Dead + Uninhabitable A	-130 Attic Without Storage: Lumber	Increase=1.25, Plate Increase=1.25									
Uniform Loads (plf) Vert: 1-3=-20	3-4=-40 5-7=-40										
Concentrated Loads (lb) Vert: 3=-130											
 Vert: 3=-130 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 											
Uniform Loads (plf) Vert: 1-2=110. 2-3=92. 3-4=86. 5-7=-12											
verc: 1-2=110, 2-3=92, 3-4=86, 5-7=-12 Horz: 1-2=-122, 2-3=-104, 3-4=110, 4-5=71											
Vert: 3=-130))										
5) Dead + 0.6 C-C Wind (Uniform Loads (plf)	Pos. Internal) Case 2: Lumbe	r Increase=1.60, Plate Increase=1.60									
Vert: 1-2=82, 2	2-3=92, 3-4=86, 5-7=-12										
Concentrated Loads (lb)										
Vert: 3=-130 6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumbe	r Increase=1.60, Plate Increase=1.60									
Uniform Loads (plf) Vert: 1-2=8 2-	3=-58 3-4=-59 5-7=-20										
Horz: 1-2=-28,	2-3=38, 3-4=-27, 4-5=-66										
Vert: 3=-130))										
 Dead + 0.6 C-C Wind (Uniform Loads (plf) 	Neg. Internal) Case 2: Lumbe	r Increase=1.60, Plate Increase=1.60									
Vert: 1-2=-48,	2-3=-58, 3-4=-59, 5-7=-20 2-3=38, 3-4=-27, 4-5=51										
Concentrated Loads (lb	rated Loads (lb)										
Vert: 3=-130 8) Dead + 0.6 MWFRS W	ind (Pos. Internal) Left: Lumb	er Increase=1.60, Plate Increase=1.60									
Uniform Loads (plf) Vert: 1-2=80_2	2-3=57 3-4=45 5-7=-12										
Horz: 1-2=-92,	2-3=-69, 3-4=69, 4-5=37										
Vert: 3=-130))										
9) Dead + 0.6 MWFRS W Uniform Loads (plf)	ind (Pos. Internal) Right: Lum	ber Increase=1.60, Plate Increase=1.60)								
Vert: 1-2=23, 2	2-3=33, 3-4=45, 5-7=-12 2-345, 3-4=69, 4-5=-28										
Concentrated Loads (lb)										
Vert: 3=-130 10) Dead + 0.6 MWFRS V	Vind (Neg. Internal) Left: Lum	ber Increase=1.60, Plate Increase=1.60)								
Uniform Loads (plf) Vert: 1-2=39.	2-3=29, 3-4=17, 5-7=-20										
Horz: 1-2=-59	9, 2-3=-49, 3-4=49, 4-5=17										
Vert: 3=-130	(di										
11) Dead + 0.6 MWFRS V Uniform Loads (plf)	Vind (Neg. Internal) Right: Lu	mber Increase=1.60, Plate Increase=1.6	50								
Vert: 1-2=15, Horz: 1-234	2-3=5, 3-4=17, 5-7=-20 5, 2-3=-25, 3-4=49, 4-5=-48										
Concentrated Loads (lb)										
Vert: 3=-130 12) Dead + 0.6 MWFRS V	Vind (Pos. Internal) 1st Parall	el: Lumber Increase=1.60, Plate Increas	se=1.60								
Uniform Loads (plf) Vert: 1-2=47.	2-3=57. 3-4=15. 5-7=-12										
Horz: 1-2=-59	9, 2-3=-69, 3-4=39, 4-5=34										
Vert: 3=-130	(U)										
 Dead + 0.6 MWFRS V Uniform Loads (plf) 	Vind (Pos. Internal) 2nd Paral	lel: Lumber Increase=1.60, Plate Increa	se=1.60								
Vert: 1-2=17,	2-3=27, 3-4=15, 5-7=-12										
Concentrated Loads (HORZ: 1-2=-29, 2-3=-39, 3-4=39, 4-5=-24 Concentrated Loads (lb)										
Vert: 3=-130 14) Dead + 0.6 MWFRS V	Vind (Pos. Internal) 3rd Parall	el: Lumber Increase=1.60, Plate Increas	se=1.60								
Uniform Loads (plf)	2-3=57 3-4=15 5-712										
Horz: 1-2=47,	2, 2-3=-69, 3-4=39, 4-5=34										
Concentrated Loads (u)										

Vert: 3=-130

Continued on page 3

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Job	Truss	Truss Type	Qty	Ply	H&H/Kenzie/	
2434740	J03	Half Hip	42	1	1425	53572
Builders FirstSource,	Sumter, SC - 29153,		8.2	240 s Mar	Job Reference (optional) 9 2020 MiTek Industries, Inc. Mon Aug 24 13:46:05 2020 Page	e 3
		ID	:2zFdWTuXpJBB?1	X9xOWC	pYycTNe-uqZJh2zGoOKju0d44fNmCfWZX8cGjGhM7sKd7lykd_	G
LOAD CASE(S)	Wind (Dag. Internal) 4th Dara	lel: Lumber Increase 1.60. Diete Increase	-1.60			
Uniform Loads (plf)	wind (Pos. Internal) 4th Paral	iei: Lumber increase=1.60, Plate increas	ie=1.60			
Vert: 1-2=17	, 2-3=27, 3-4=15, 5-7=-12					
Horz: 1-2=-2 Concentrated Loads	9, 2-3=-39, 3-4=39, 4-5=-24 (lb)					
Vert: 3=-130			4.00			
Uniform Loads (plf)	wind (Neg. Internal) 1st Para	liei: Lumber increase=1.60, Plate increas	e=1.60			
Vert: 1-2=39,	, 2-3=29, 3-4=-13, 5-7=-20					
Horz: 1-2=-5 Concentrated Loads (9, 2-3=-49, 3-4=19, 4-5=14 (lb)					
Vert: 3=-130	(15)					
17) Dead + 0.6 MWFRS	Wind (Neg. Internal) 2nd Para	allel: Lumber Increase=1.60, Plate Increa	se=1.60			
Vert: 1-2=9, 2	2-3=-1, 3-4=-13, 5-7=-20					
Horz: 1-2=-2	9, 2-3=-19, 3-4=19, 4-5=-44					
Vert: 3=-130	(di)					
18) Dead: Lumber Increa	se=0.90, Plate Increase=0.90	Plt. metal=0.90				
Uniform Loads (pit) Vert: 1-3=-20), 3-4=-40, 5-7=-20					
Concentrated Loads	(lb)					
Vert: 3=-130 19) Dead + 0.75 Roof Liv	e (bal.) + 0.75 Attic Floor + 0.	75(0.6 MWERS Wind (Neg. Int) Left): Lu	mber Increase=1 (30 Plate	Increase=1.60	
Uniform Loads (plf)				50, 1 iato		
Vert: 1-2=-5,	2-3=-13, 3-4=-55, 5-7=-20 5 2-3=-37 3-4=37 4-5=12					
Concentrated Loads ((lb)					
Vert: 4=-15 3	3=-130 (hal.) + 0.75 Attic Eloor + 0.	75(0.6 MW/ERS Wind (Neg. Int) Right): I	umber Increase-1	60 Plate	e Increase-1 60	
Uniform Loads (plf)	e(bal.) + 0.75 Allic Floor + 0.	75(0.0 MWPRS Wild (Neg. int) Right). L		.00, Flate		
Vert: 1-2=-24	4, 2-3=-31, 3-4=-55, 5-7=-20					
Concentrated Loads	(lb)					
Vert: 4=-15 3	3=-130		U-N-1			
Uniform Loads (plf)	e (dal.) + 0.75 Attic Floor + 0.	75(0.6 MWFRS Wind (Neg. Int) 1st Para	liei): Lumber Incre	ase=1.60	J, Plate Increase=1.60	
Vert: 1-2=-5,	2-3=-13, 3-4=-78, 5-7=-20					
Horz: 1-2=-4 Concentrated Loads (5, 2-3=-37, 3-4=15, 4-5=10 (lb)					
Vert: 4=-15 3	3=-130					
22) Dead + 0.75 Roof Liv	e (bal.) + 0.75 Attic Floor + 0.	75(0.6 MWFRS Wind (Neg. Int) 2nd Para	allel): Lumber Incre	ease=1.6	0, Plate Increase=1.60	
Vert: 1-2=-28	3, 2-3=-35, 3-4=-78, 5-7=-20					
Horz: 1-2=-2	2, 2-3=-15, 3-4=15, 4-5=-33					
Vert: 4=-15 3	3=-130					
23) 1st Dead + Roof Live	(unbalanced): Lumber Increa	se=1.15, Plate Increase=1.15				
Vert: 1-3=-60), 3-4=-80, 5-7=-20					
Concentrated Loads	(lb)					
24) 2nd Dead + Roof Live	e (unbalanced): Lumber Incre	ase=1.15, Plate Increase=1.15				
Uniform Loads (plf)						
Vert: 1-3=-20 Concentrated Loads (), 3-4=-80, 5-7=-20 (lb)					
Vert: 3=-150						
25) 3rd Dead + 0.75 Root Uniform Loads (plf)	f Live (unbalanced) + 0.75 Att	tic Floor: Lumber Increase=1.15, Plate In	crease=1.15			
Vert: 1-3=-50), 3-4=-99, 5-7=-20					
Concentrated Loads (Vert: 4=-15.3	(מו) 3=-130					
26) 4th Dead + 0.75 Roof	f Live (unbalanced) + 0.75 Att	ic Floor: Lumber Increase=1.15, Plate In	crease=1.15			
Uniform Loads (plf) Vert: 1-3=-20), 3-4=-99, 5-7=-20					
Concentrated Loads ((lb)					
Vert: 4=-15 3	3=-130					

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 14, 12 except (jt=lb) 1=128, 16=184, 17=139, 11=151, 10=145.

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