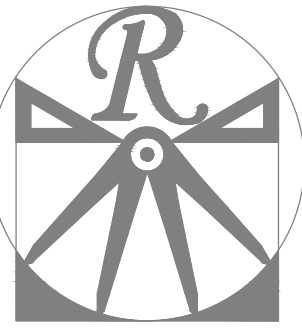


PLANS DESIGNED TO THE  
2018 NORTH CAROLINA STATE  
RESIDENTIAL BUILDING CODE.

LOT 4 MILL POND  
TBD MATTHEWS MILL POND RD  
LILLINGTON, NC 27546

GENERAL NOTES

1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
3. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED.
4. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL SITE REQUIREMENTS.



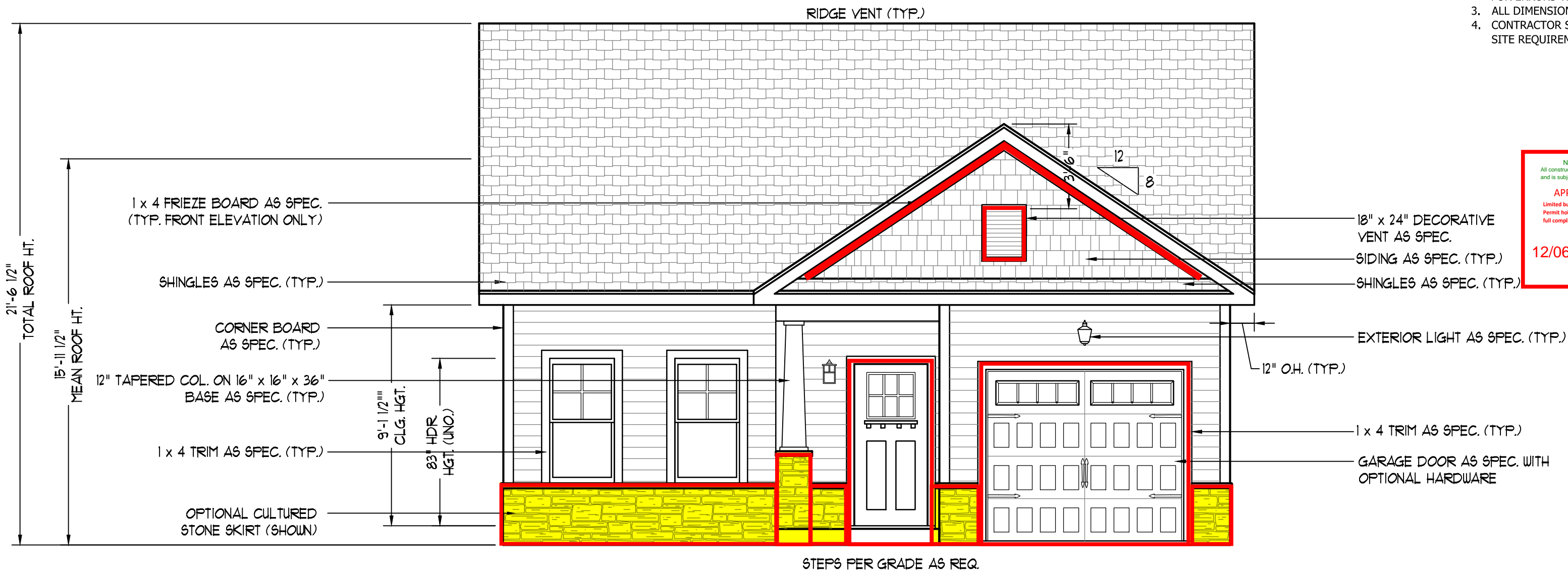
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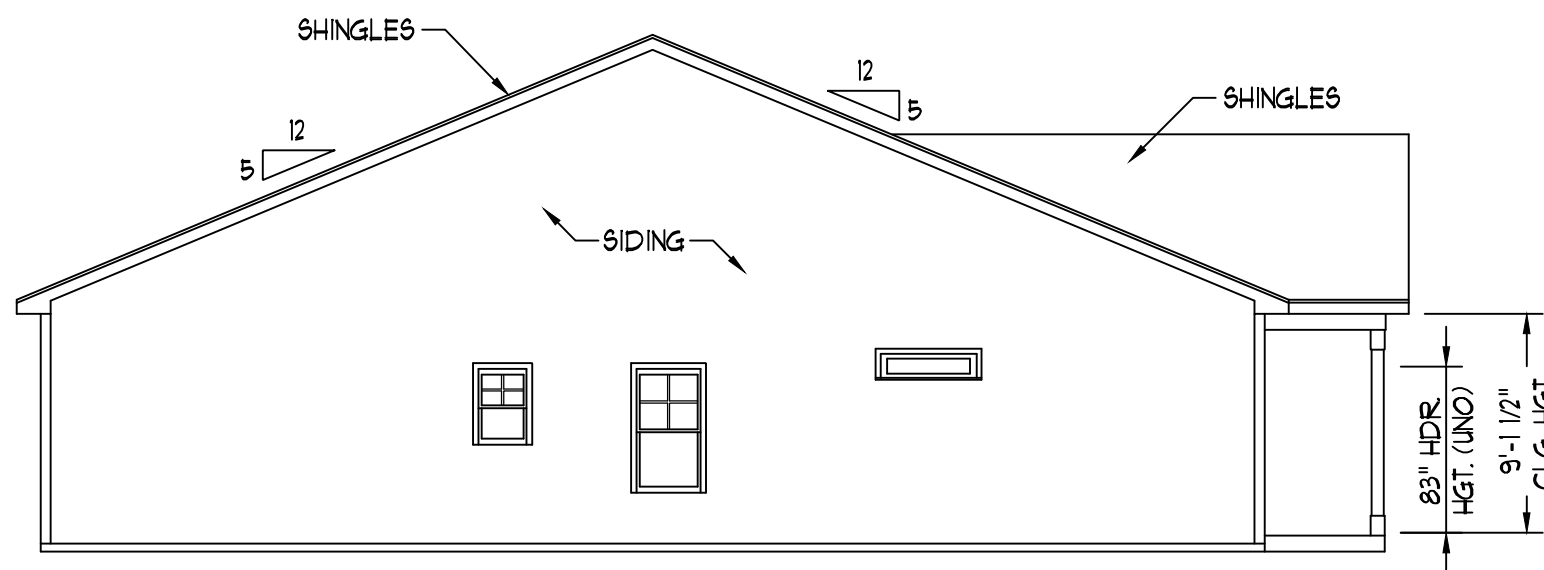
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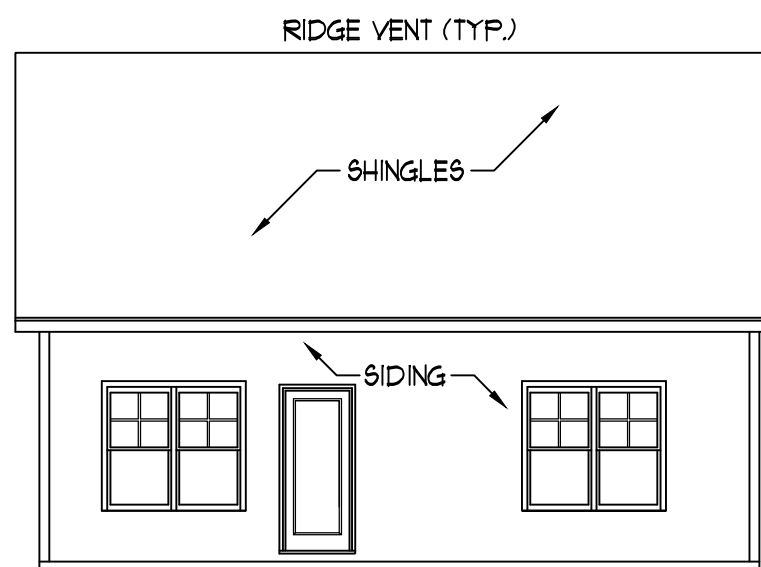


FRONT ELEVATION-A

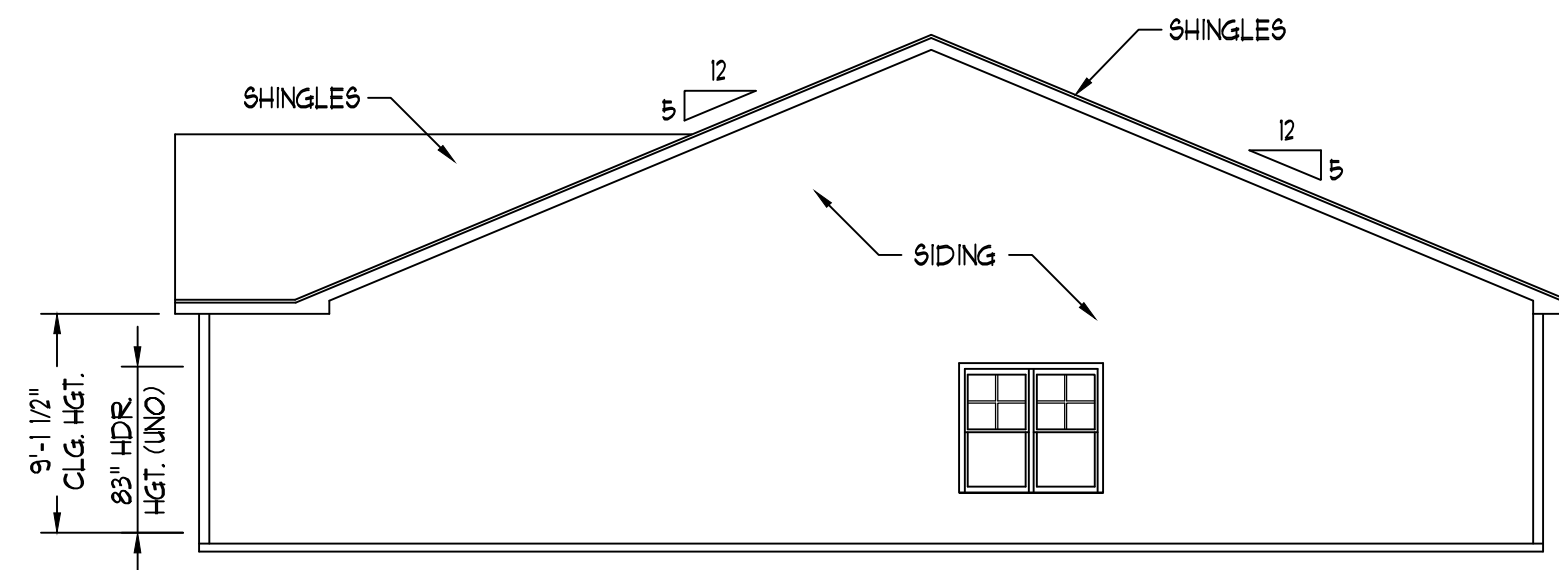
SCALE: 1/4" = 1'-0"



LEFT ELEVATION  
SCALE: 1/8" = 1'-0"



REAR ELEVATION  
SCALE: 1/8" = 1'-0"



RIGHT ELEVATION  
SCALE: 1/8" = 1'-0"

PLUMBING: DOUBLE J  
ELECTRICAL: PIONEER  
HVAC: TBD

WEAVER HOMES  
CAROLINA COLLECTION  
LEYLAND

DATE: JUNE 15, 2020  
REV.:  
SCALE: AS NOTED  
DRAWN BY: WG  
ENGINEERED BY:  
REVIEWED BY:

A - ELEVATIONS

A-1

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.  
11x17 PRINTS ARE NOT TO SCALE

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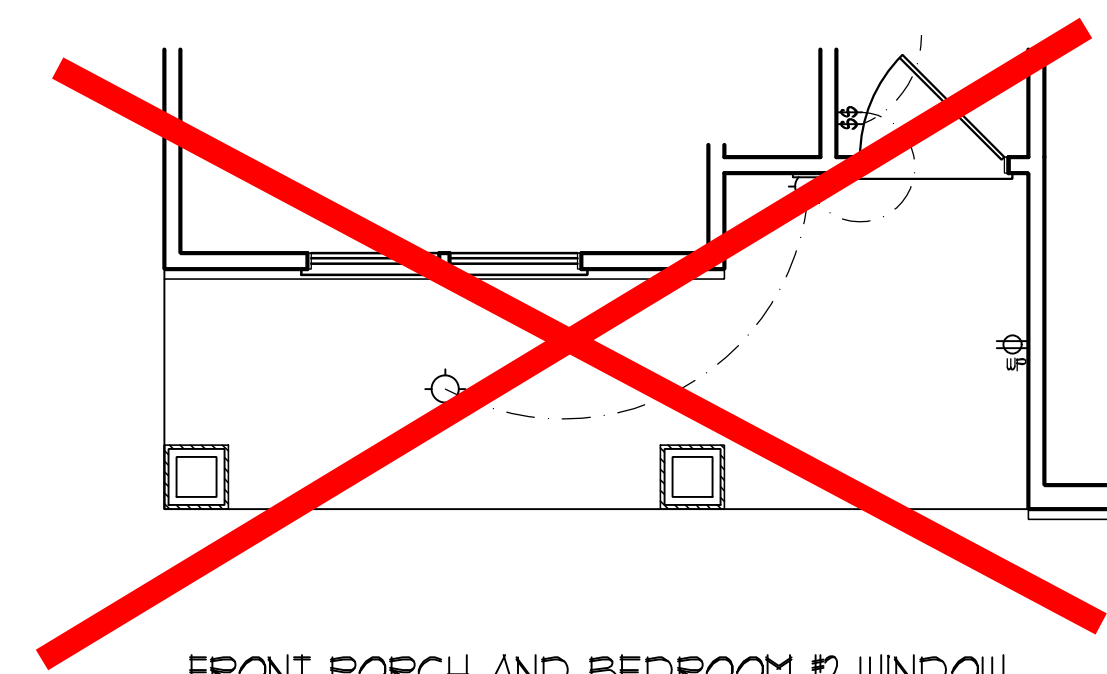
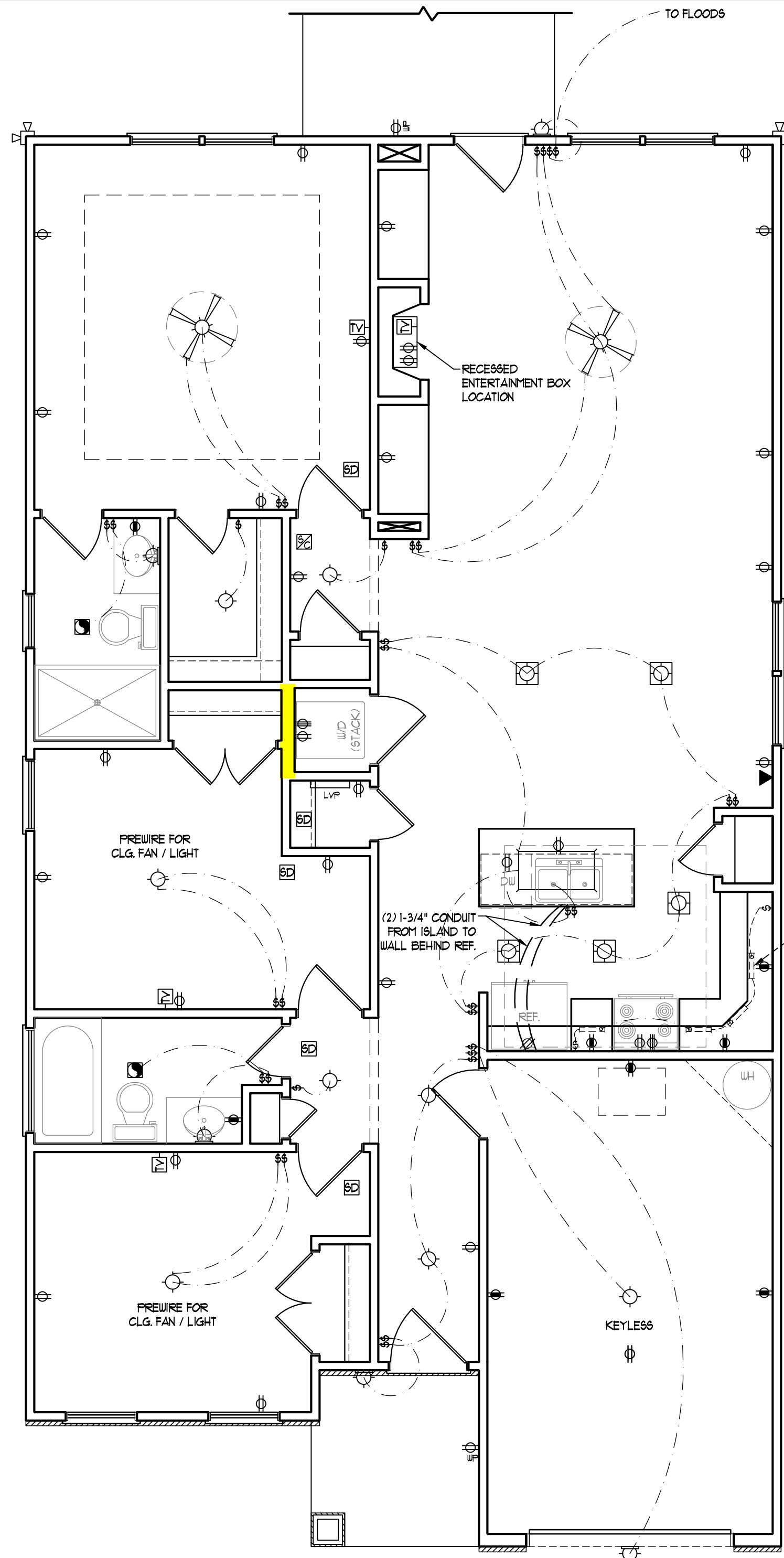


**ELECTRICAL LAYOUT NOTES:**

- 1.) BLOCK AND WIRE FOR ALL CEILING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET @ 30" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN) FROM ROUGH OPENINGS.

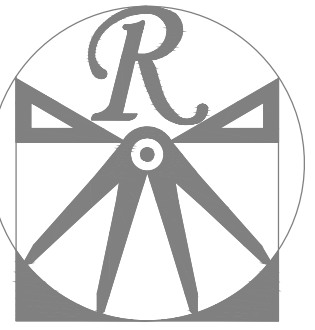
**ELECTRICAL LEGEND**

- 110 V OUTLET
- 110 V GFI OUTLET
- 110 V SWITCHED OUTLET
- 110 V BASEBOARD OUTLET
- 4-FLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GFI
- WEATHERPROOF
- 220 V OUTLET
- 110 V DEDICATED CIRCUIT
- 220 V DEDICATED CIRCUIT
- SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- PENDANT LIGHT
- RECESSED CAN LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT
- UNDERCABINET LIGHT
- FLOOD LIGHT
- SWITCH
- DIMMER SWITCH
- TELEPHONE
- DATA
- TELEPHONE AND DATA
- TV CONNECTION
- TV/ DATA
- CONDUIT FOR COMPONENT WIRING
- SPEAKER
- 110 V SMOKE/ CH DETECTOR
- 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM PANEL
- CEILING FAN
- CEILING FAN W/ LIGHT



FRONT PORCH AND BEDROOM #2 WINDOW ELEVATIONS-B&C

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.  
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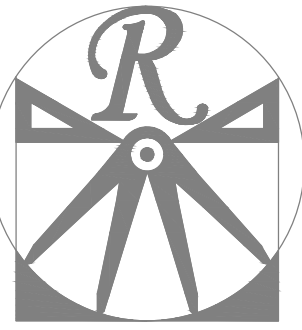
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**CAROLINA COLLECTION**  
**LEYLAND**

DATE: JUNE 15, 2020  
REV.:  
SCALE: 1/4" = 1'-0"  
DRAWN BY: WG  
ENGINEERED BY:  
REVIEWED BY:

FIRST FLOOR ELECTRICAL PLAN

E-1





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SCALE: 1/4" = 1'-0"

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ENGINEERED BY:

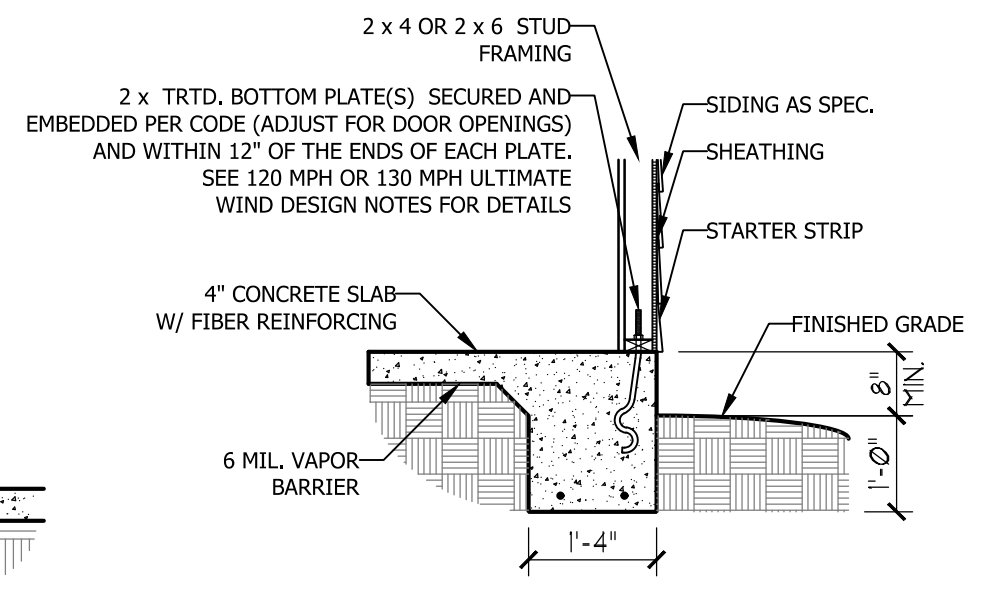
REVIEWED BY:

MONO SLAB  
FOUNDATION  
PLAN

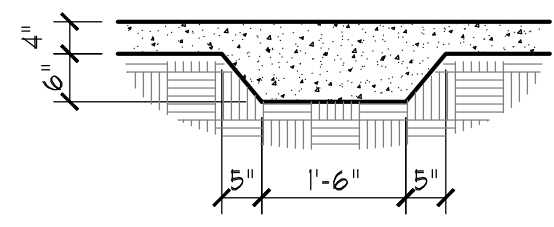
S-1

**ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:**

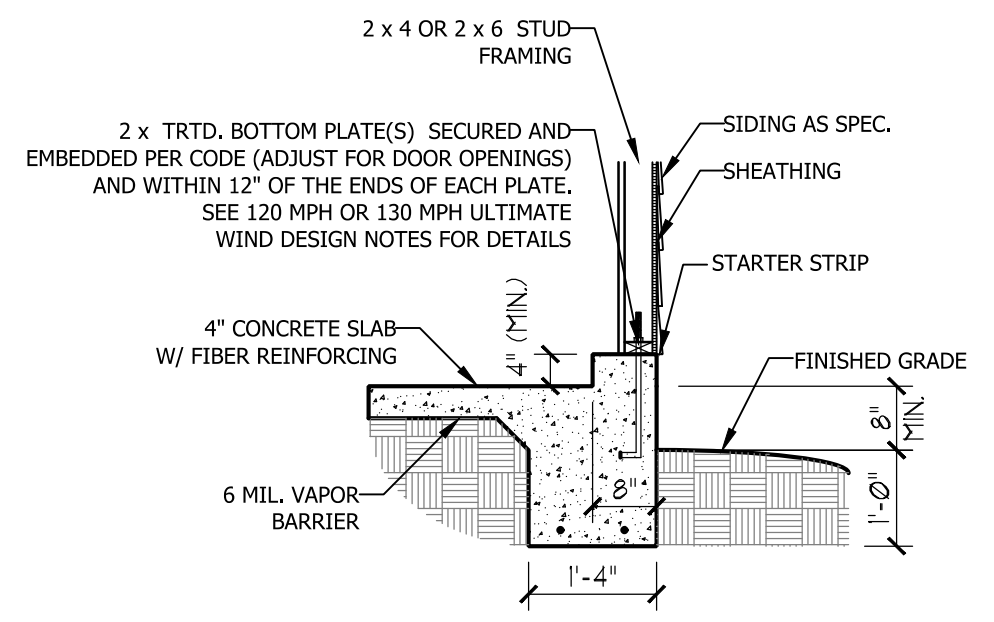
1. STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
2. FOR 120 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 6'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
3. FOR 130 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 4'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
4. MEAN ROOF HEIGHT IS LESS THAN 30 FEET.
5. EXTERIOR WALLS DESIGNED FOR 120 OR 130 MPH WINDS.
6. INSTALL 7/16" OSB SHEATHING ON ALL EXTERIOR WALLS OF ALL STORIES IN ACCORDANCE WITH SECTION R602.10.3 OF THE NRC, 2018 EDITION.
7. ENERGY EFFICIENCY COMPLIANCE AND INSULATION VALUES OF THE BUILDING TO BE IN ACCORDANCE WITH CHAPTER 11 OF THE NRC, 2018 EDITION.



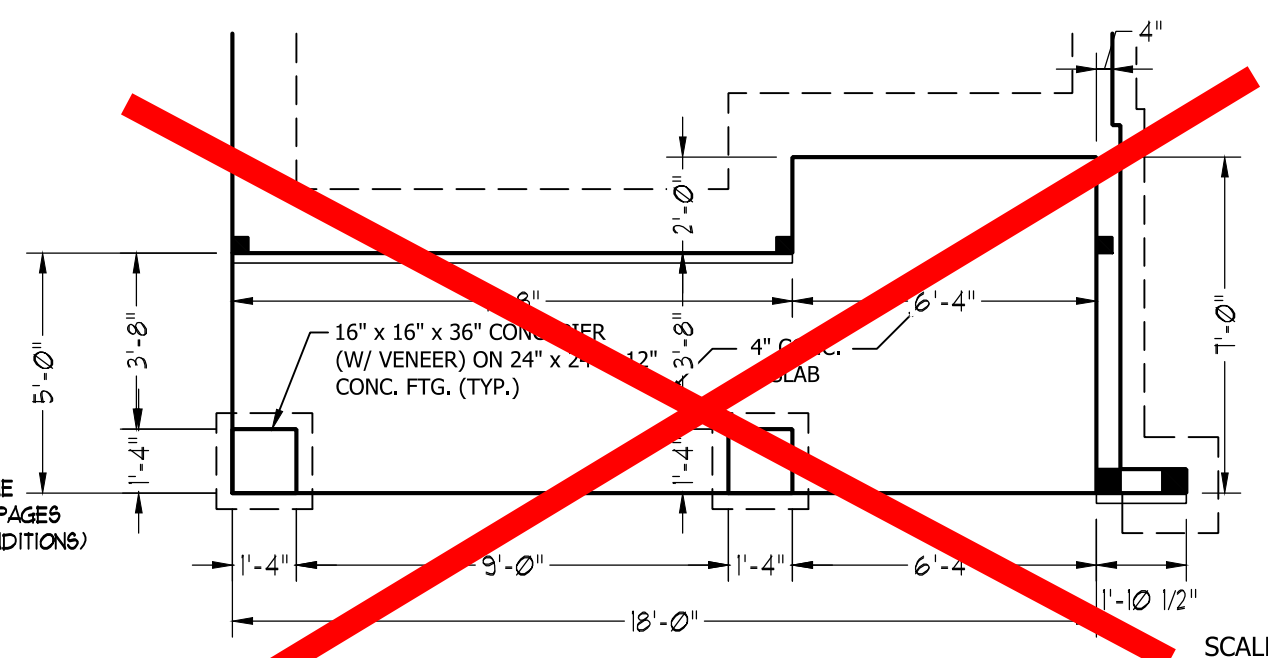
**TYPICAL SLAB DETAIL**



**THICKENED SLAB DETAIL**

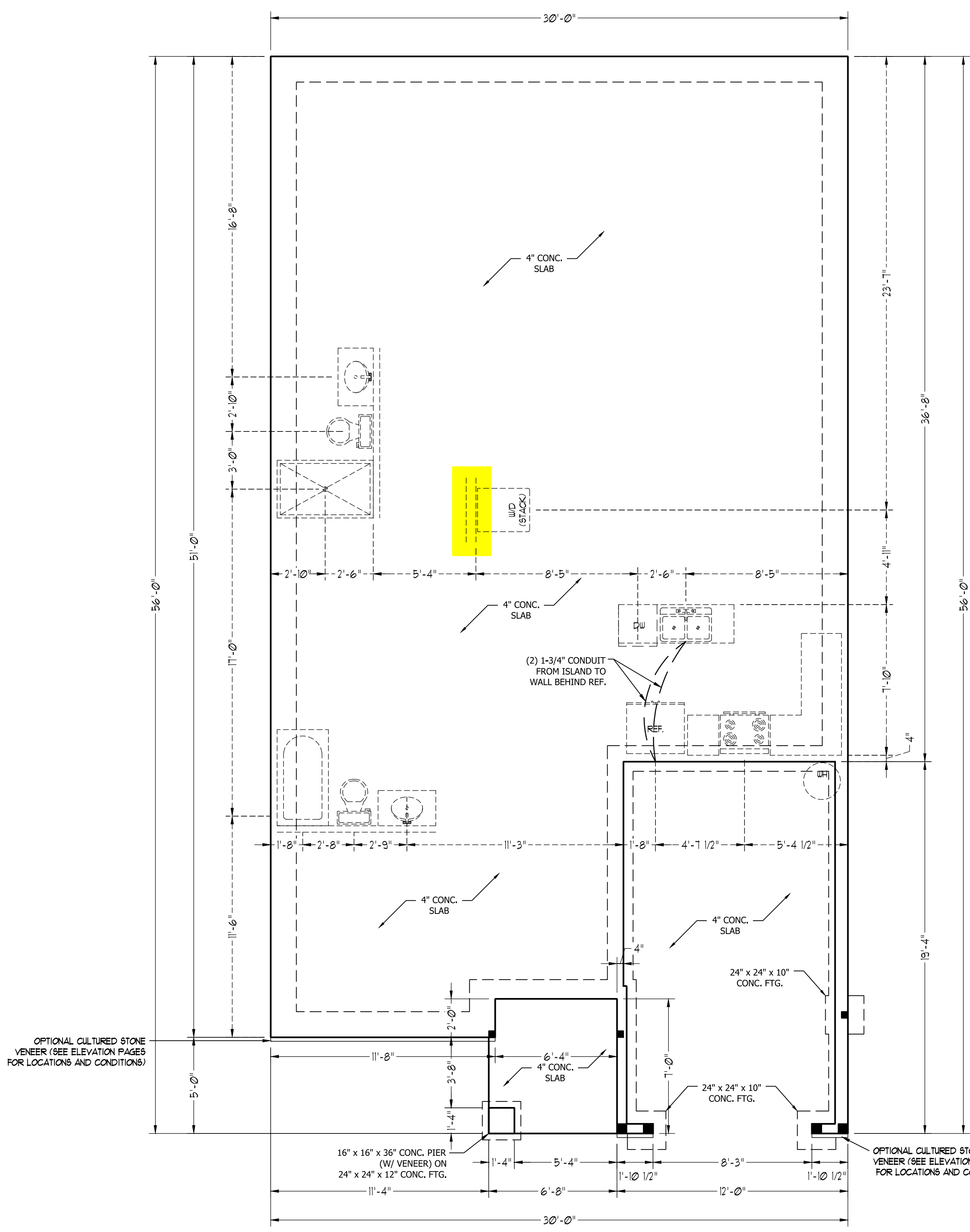


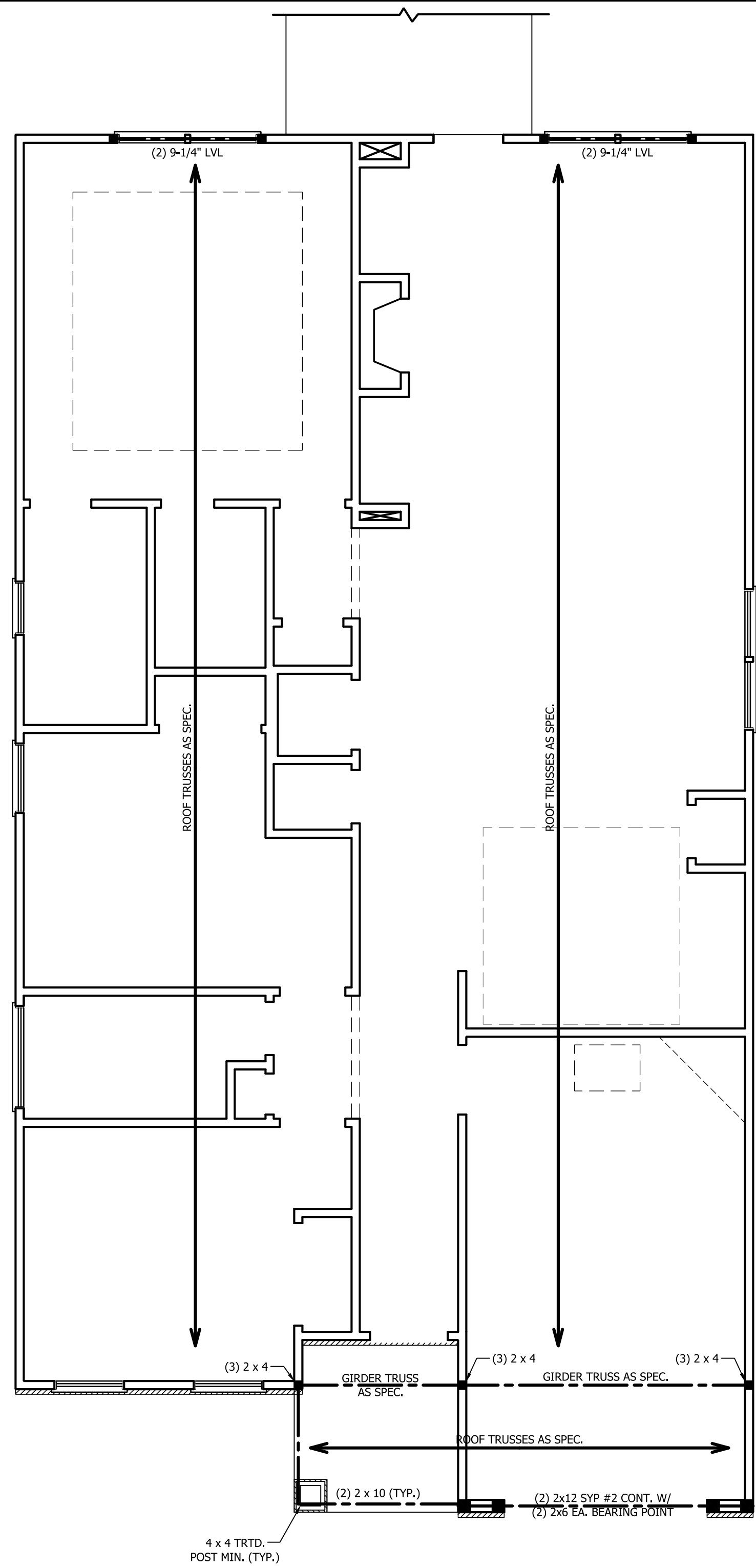
**GARAGE CURB DETAIL**



**FRONT PORCH ELEVATIONS-B&C**

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.  
11x17 PRINTS ARE NOT TO SCALE





**STRUCTURAL NOTES:**

1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 4 (UNO).
3. INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
4. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
7. FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2\"/>

**BRACE WALL PANEL NOTES:**

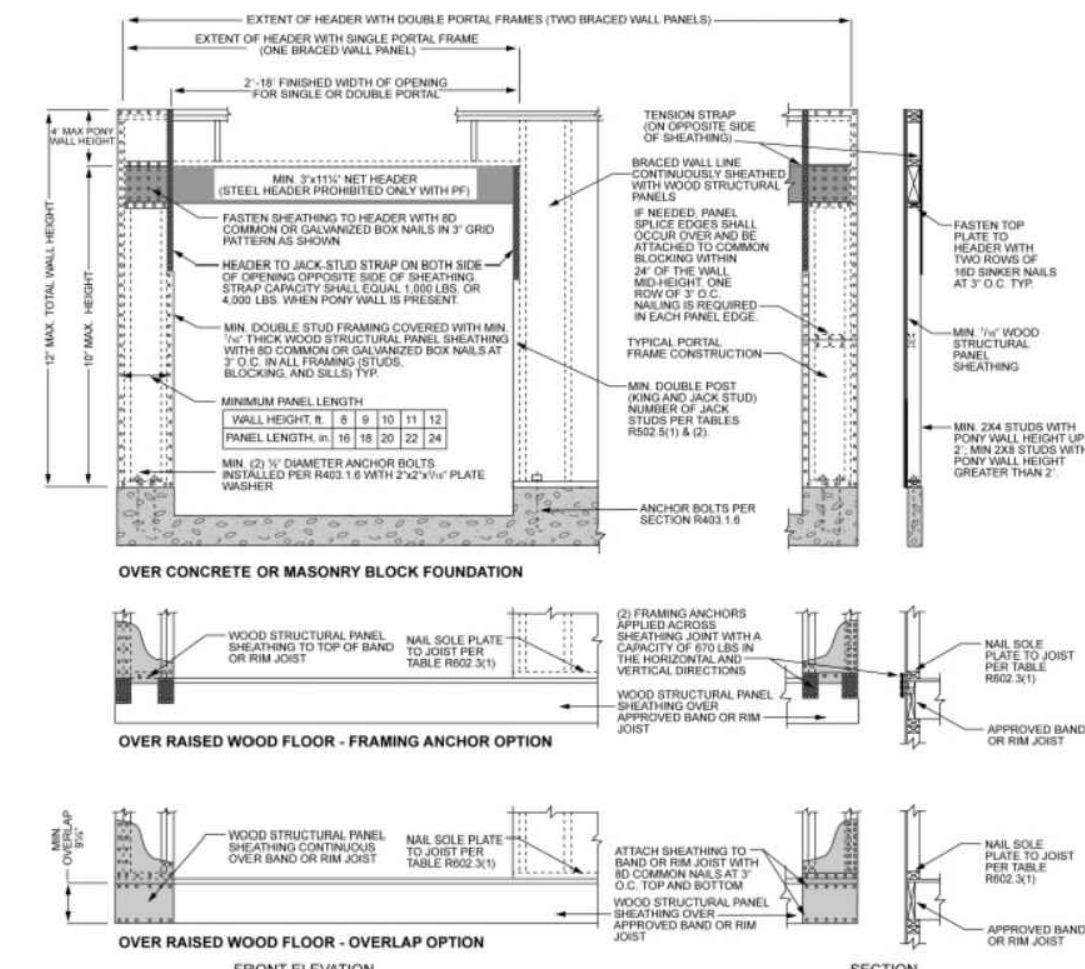
**EXTERIOR WALLS:** ALL EXTERIOR WALLS TO BE SHEATHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

**REQUIRED LENGTH OF BRACING:** REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THEIR ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

**GYPNUM:** ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2\"/>

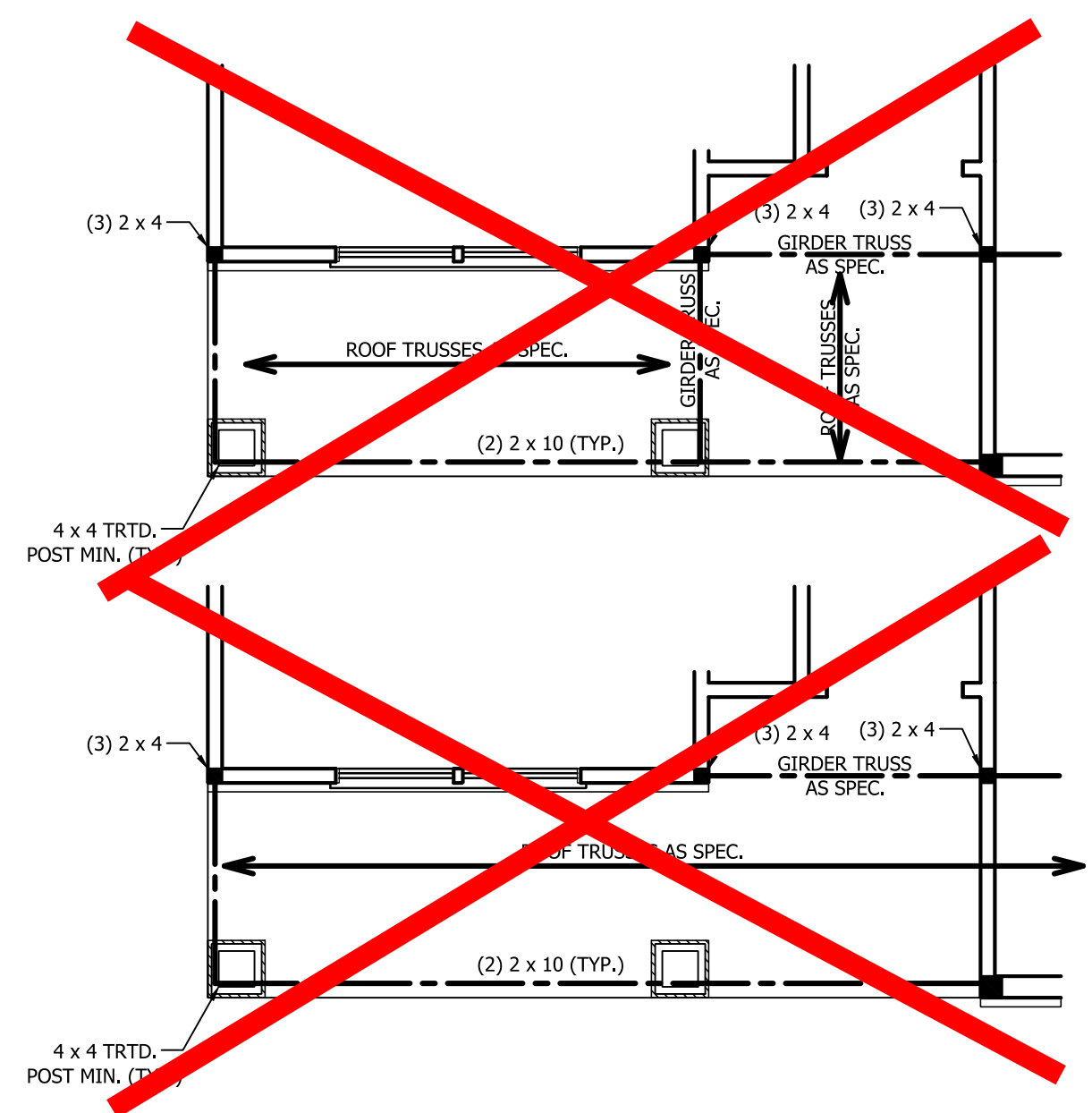
**HD:** 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

**METHODS:** PER TABLE R602.10.1



nch = 25.4 mm, 1 foot = 305 mm, 1 lb = 4.45 N.

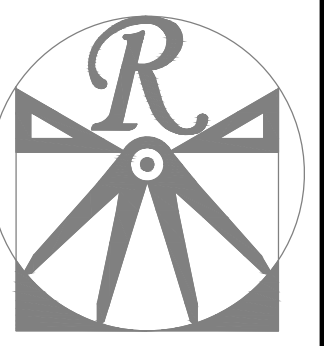
**FIGURE R602.10.1**  
**METHOD PF—PORTAL FRAME CONSTRUCTION**



**FRONT PORCH AND BEDROOM #2 WINDOW ELEVATION B**

**FRONT PORCH AND BEDROOM #2 WINDOW ELEVATION C**

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.  
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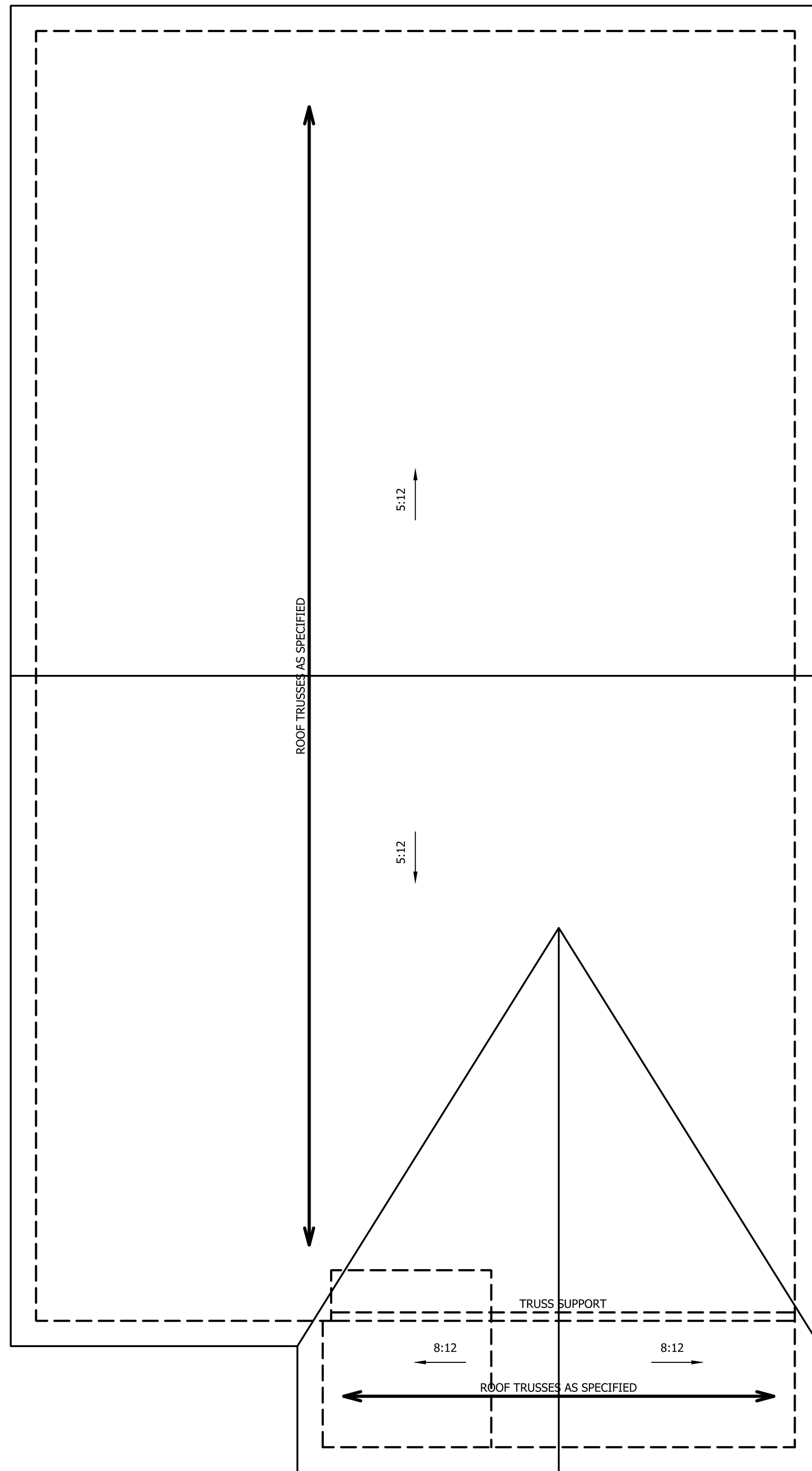
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ENGINEERED BY:  
REVIEWED BY:

SECOND FLOOR FRAMING PLAN





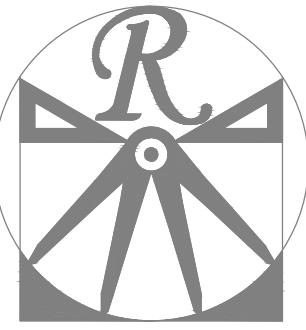
ELEVATION A

**ATTIC VENT CALCULATION:**

1756 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 11.7 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

**STRUCTURAL NOTES:**

1. ALL FRAMING LUMBER TO BE #2 SPF (UNO).
2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



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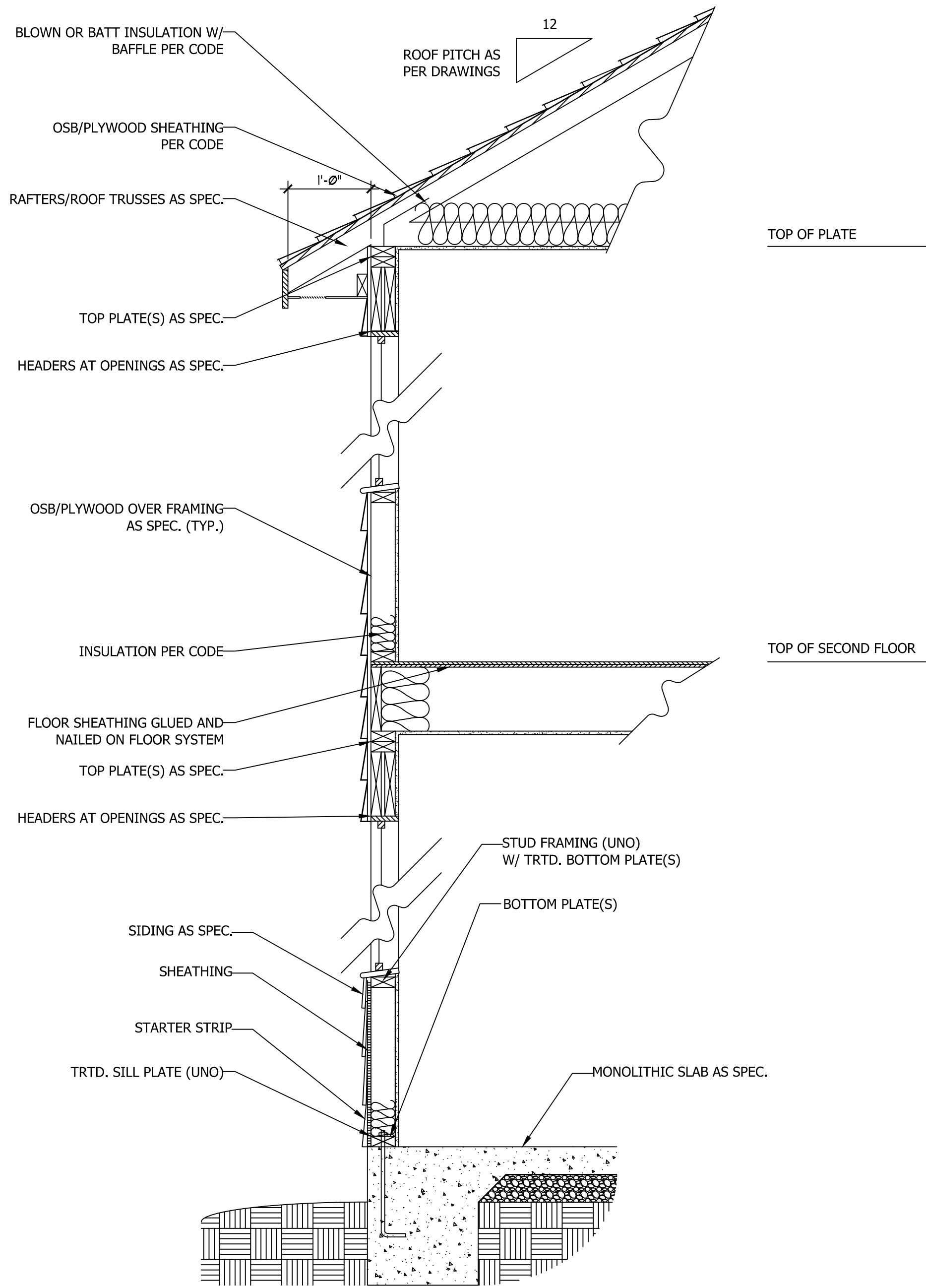
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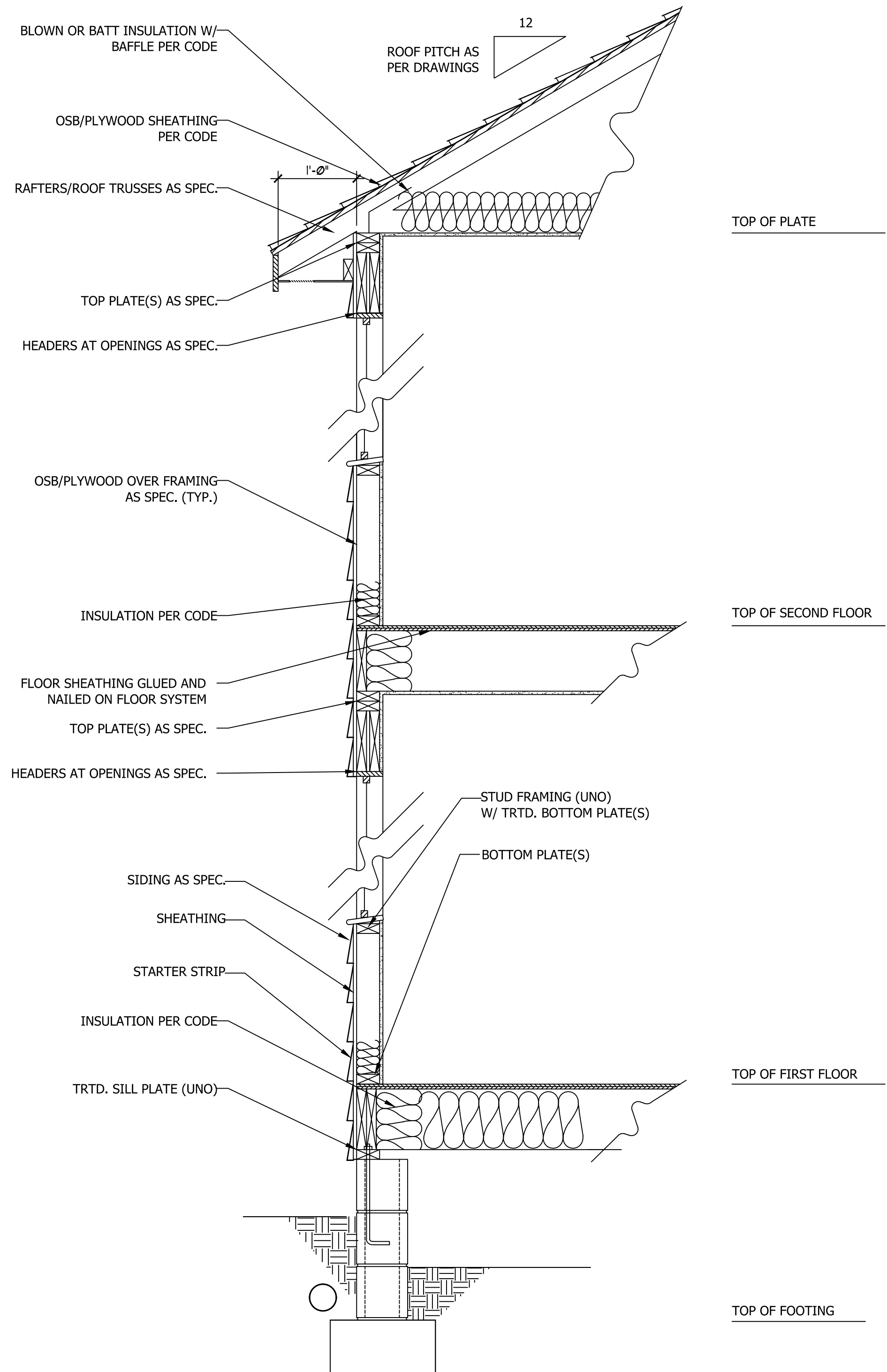
ROOF PLAN  
ELEVATION-A

S-3

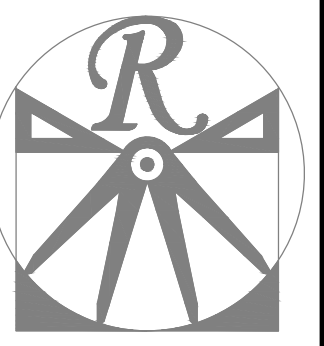
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**WALL SECTION W/ SLAB  
W/ STD. SIDING SHOWN (NTS)**



**WALL SECTION W/ CRAWL SPACE  
W/ STD. SIDING SHOWN (NTS)**



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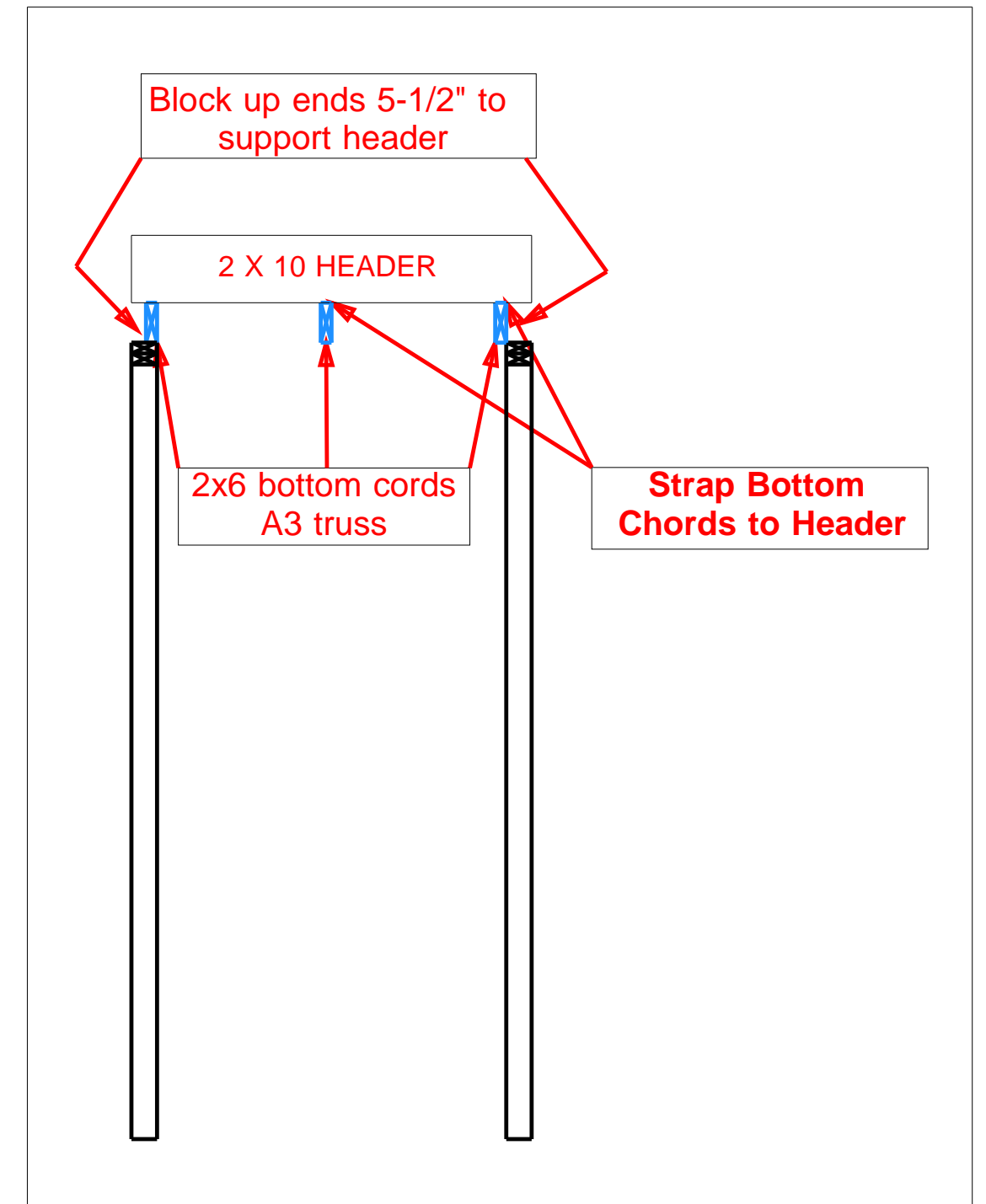
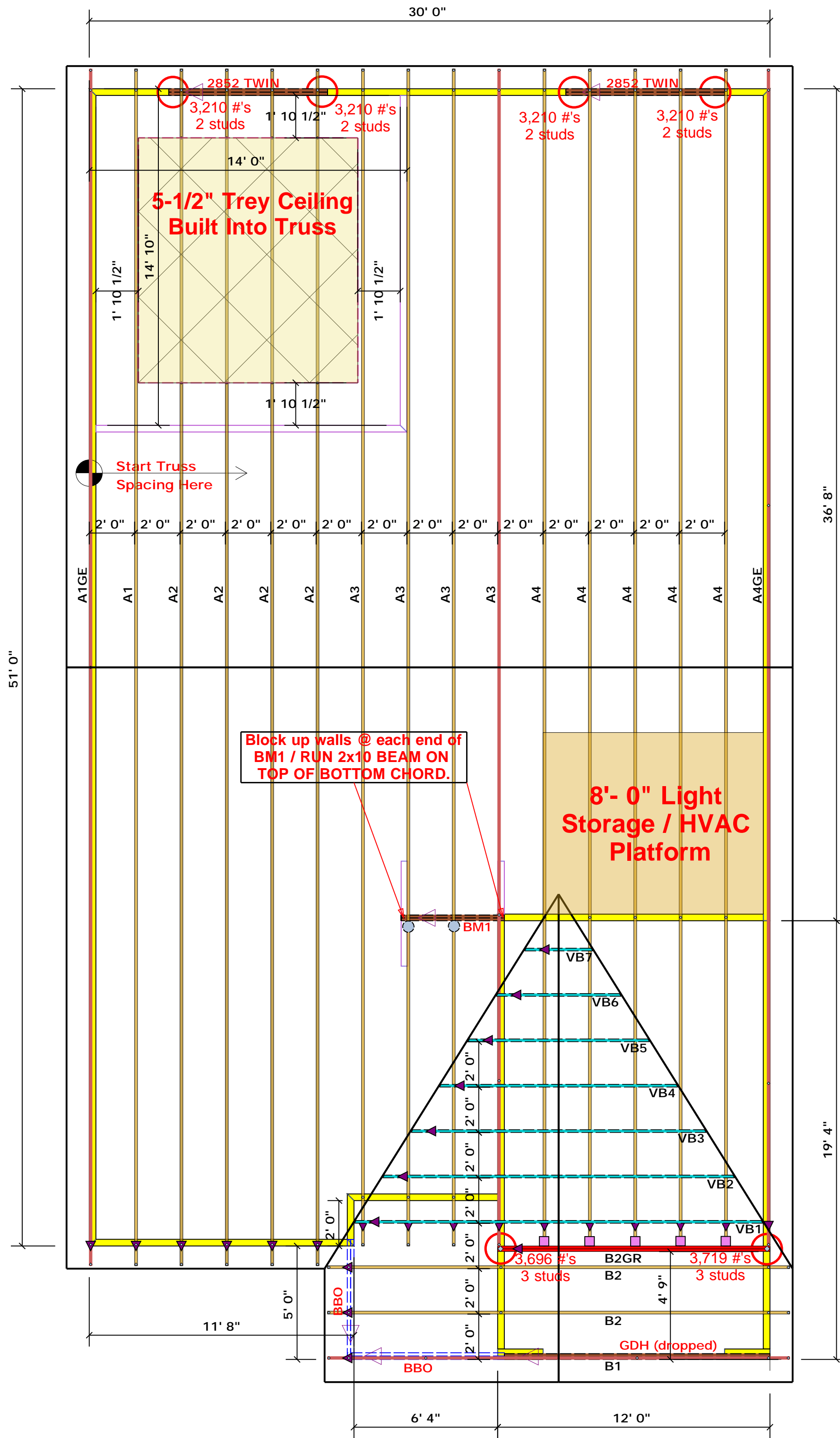
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ENGINEERED BY:  
REVIEWED BY:

TYPICAL WALL SECTIONS  
**D-1**



Block up walls @ each end of BM1 / RUN 2x10 BEAM ON TOP OF BOTTOM CHORD.

Start Truss Spacing Here

■	HUS28	USP	5		16d/3-1/2"	16d/3-1/2"
●	MSH422	USP	2	Varies	10d/3"	10d/3"

### Truss Placement Plan SCALE: 1/4" = 1'-0"

▲ = Denotes Left End of Truss  
(Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

Estimation			
Name	Selection	Formula	Calculation
Roof Area	1st Floor	Roof Area	1981.44
Roof Decking	1st Floor	Roof Decking	68 sheets

BEAM LEGEND					
PlotID	Length	Product	Plies	Net Qty	Fab T
2852 TWIN	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4	FF
GDH (dropped)	12' 0"	2x12 SPF No.2	2	2	FF

LOAD CHART FOR JACK STUDS			
MEMBER	SPACING	LOAD	REMARKS
1700	1	2550	3400
3400	2	5100	6800
5100	3	7650	10200
6800	4	10200	13600
8500	5	12750	17000
10200	6	15300	
11900	7		
13600	8		
15300	9		

BUILDER	Weaver Development	CITY / CO.	Lillington / Harnett
JOB NAME	Lot 4 Mill Pond	ADDRESS	Matthews Mill Pond Rd.
PLAN	Leyland "A"	MODEL	ROOF
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Lenny Norris
JOB #	J1021-6299	SALES REP.	Lenny Norris

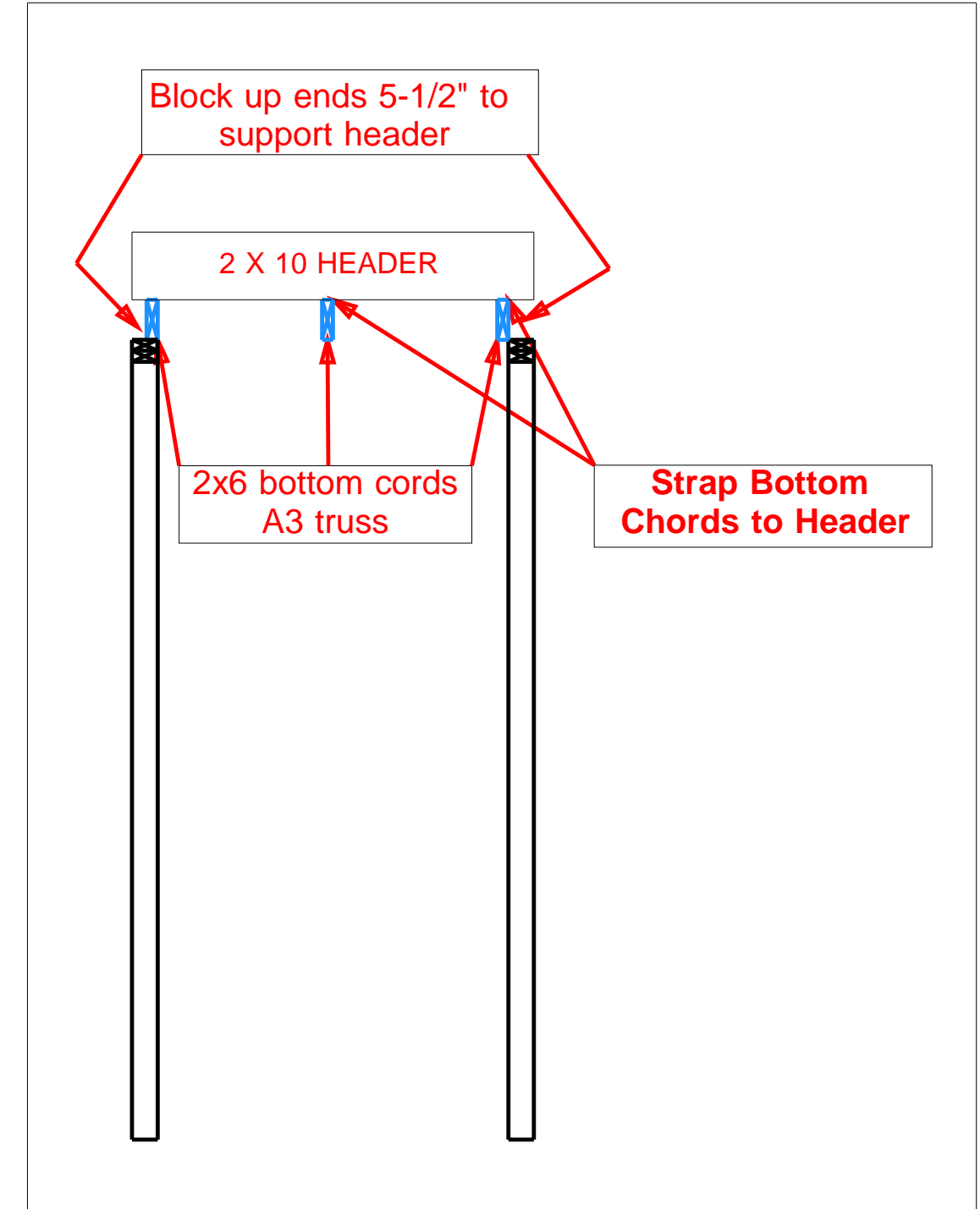
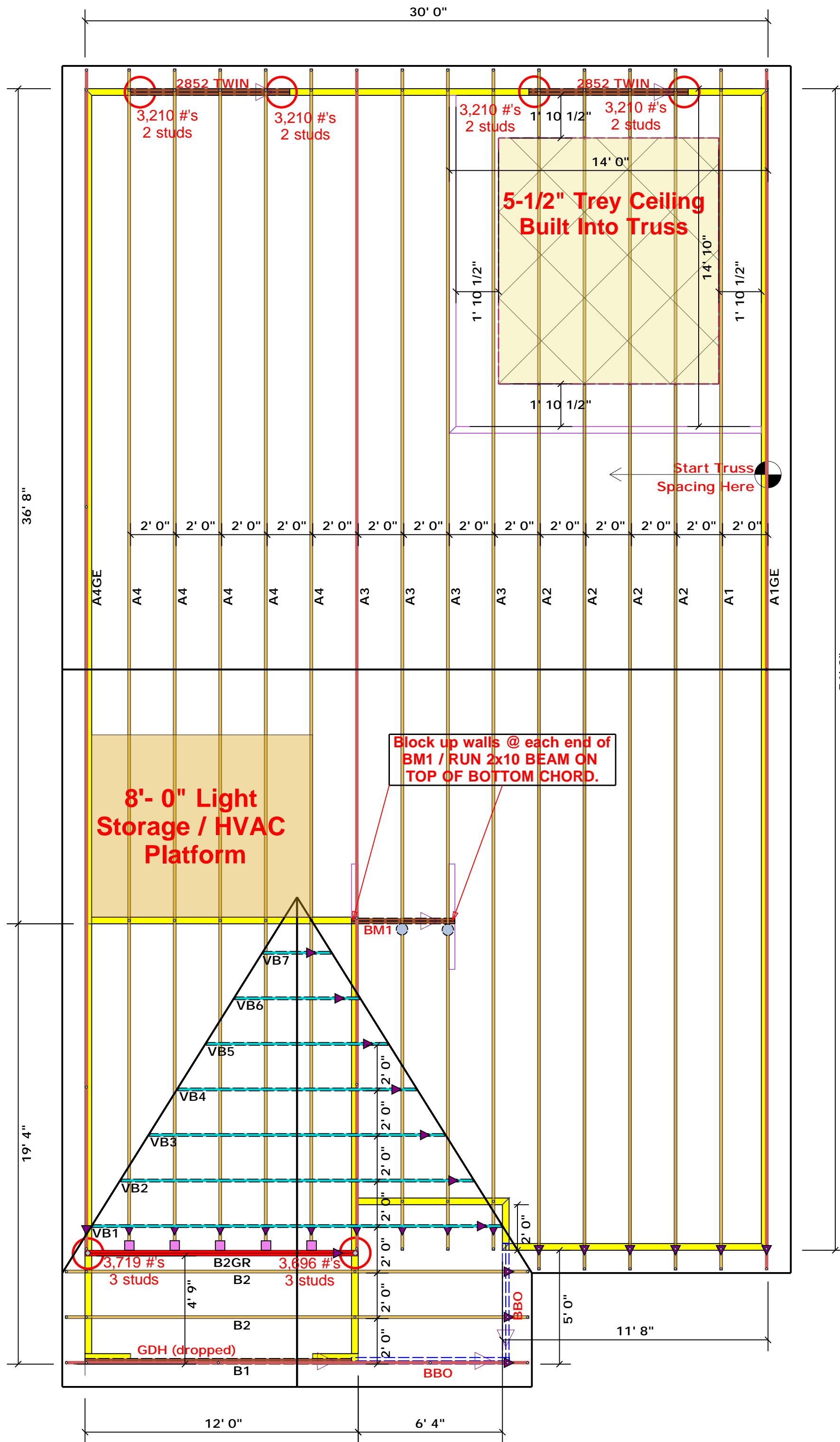
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSH-B1 and BCSH-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: Lenny Norris  
Lenny Norris

**ROOF & FLOOR TRUSSES & BEAMS**  
Reilly Road Industrial Park  
Fayetteville, N.C. 28309  
Phone: (910) 864-8787  
Fax: (910) 864-4444





**Truss Placement Plan**  
**SCALE: 1/4" = 1'-0"**

▲ = Denotes Left End of Truss  
 (Reference Engineered Truss Drawing)

*All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.*

○ -- Denotes Reaction Greater than 3,000 lbs.  
 Reaction / # of Studs

■	HUS28	USP	5		16d/3-1/2"	16d/3-1/2"
●	MSH422	USP	2	Varies	10d/3"	10d/3"

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BEAM LEGEND					
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2852 TWIN	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4	FF
GDH (dropped)	12' 0"	2x12 SPF No.2	2	2	FF

LOAD CHART FOR JACK STUDS			
IRG REACTION (UP TO 1000 LBS)	IRG REACTION (UP TO 2000 LBS)	IRG REACTION (UP TO 3000 LBS)	IRG REACTION (UP TO 4000 LBS)
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		

BUILDER	Weaver Development	CITY / CO.	Lillington / Harnett
JOB NAME	Lot 4 Mill Pond	ADDRESS	Matthews Mill Pond Rd.
PLAN	Leyland "A"	MODEL	ROOF
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Lenny Norris
JOB #	J1021-6299	SALES REP.	Lenny Norris

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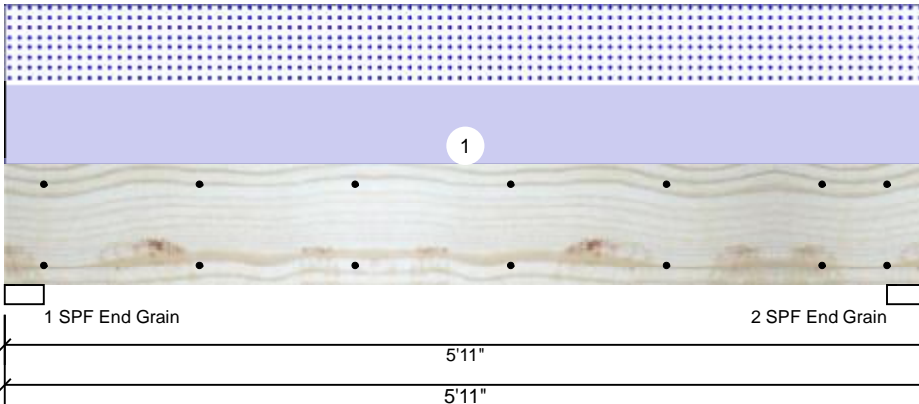
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 Phone: (910) 864-8787  
 Fax: (910) 864-4444

**2852 TWIN Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED**

Level: Level



**Member Information**

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

**Reactions UNPATTERNED Ib (Uplift)**

Brg	Live	Dead	Snow	Wind	Const
1	0	1616	1595	0	0
2	0	1616	1595	0	0

**Bearings**

Bearing	Length	Cap.	React D/L	Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	35%	1616 / 1595	3210	L	D+S	
2 - SPF End Grain	3.000"	35%	1616 / 1595	3210	L	D+S	

**Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4166 ft-lb	2'11 1/2"	14423 ft-lb	0.289 (29%)	D+S	L
Unbraced	4166 ft-lb	2'11 1/2"	11027 ft-lb	0.378 (38%)	D+S	L
Shear	2170 lb	4'11 1/2"	7943 lb	0.273 (27%)	D+S	L
LL Defl inch	0.032 (L/2069)	2'11 1/2"	0.139 (L/480)	0.230 (23%)	S	L
TL Defl inch	0.065 (L/1028)	2'11 1/2"	0.185 (L/360)	0.350 (35%)	D+S	L

**Design Notes**

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	539 PLF	0 PLF	539 PLF	0 PLF	0 PLF	TRUSSES A2,A4
	Self Weight				7 PLF					

**Notes**

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

**Manufacturer Info**

Metsä Wood  
301 Merritt 7 Building, 2nd Floor  
Norwalk, CT 06851  
(800) 622-5850  
[www.metsawood.com/us](http://www.metsawood.com/us)  
ICC-ES: ESR-3633

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28314  
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