

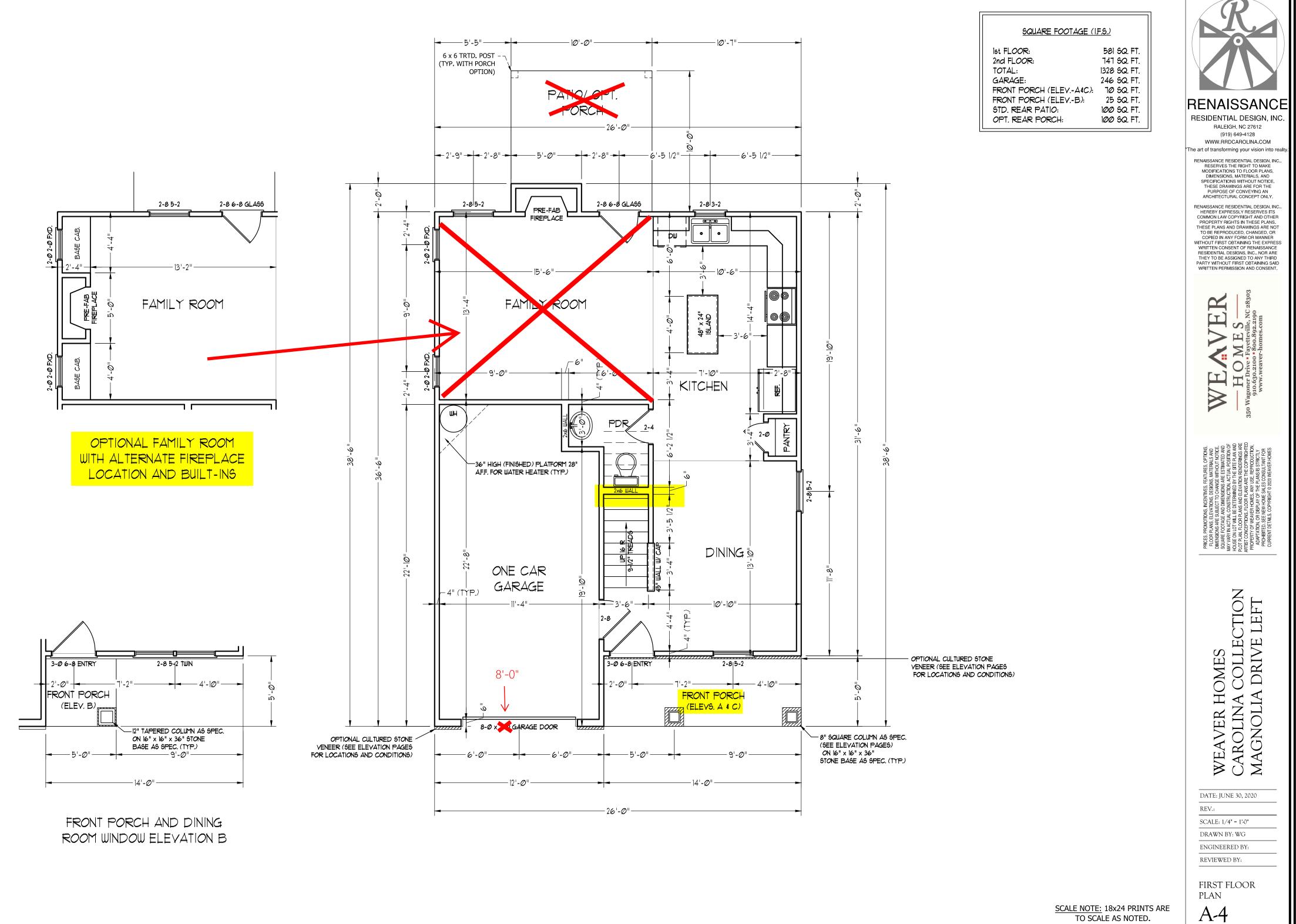
2018 NORTH CAROLINA STATE **RESIDENTIAL BUILDING CODE.**

RENAISSANCE RESIDENTIAL DESIGN, INC. RALEIGH, NC 27612 (919) 649-4128 WWW RRDCAROLINA COM e art of transforming your vision into RENAISSANCE RESIDENTIAL DESIGN, INC.. RESERVES THE RIGHT TO MAKE MODIFICATIONS TO FLOOR PLANS, DIMENSIONS, MATERIALS, AND SPECIFICATIONS WITHOUT NOTICE. THESE DRAWINGS ARE FOR THE PURPOSE OF CONVEYING AN ARCHITECTURAL CONCEPT ONLY RENAISSANCE RESIDENTIAL DESIGN, INC. HERAISSANCE HESIDEN TIAL DESIGN, INC., HEREBY EXPRESSLY RESERVES ITS COMMON LAW COPYRIGHT AND OTHER PROPERTY RIGHTS IN THESE PLANS. THESE PLANS AND DRAWINGS ARE NOT TO BE REPRODUCED, CHANGED, OR COPIED IN ANY FORM OR MANNER WITHOUT FIRST OBTAINING THE EXPRESS WRITTEN CONSENSE OF DEMANSEANCE WRITTEN CONSENT OF RENAISSANCE RESIDENTIAL DESIGNS, INC...NOR ARE THEY TO BE ASSIGNED TO ANY THIRD PARTY WITHOUT FIRST OBTAINING SAID WRITTEN PERMISSION AND CONSEN E) S WE NOIT Ľ Ш Ц Ĕ > \mathcal{O} WEAVER HOME CAROLINA COL DRI AROLINA (AGNOLIA DATE: JUNE 30, 2020 REV.: SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY:

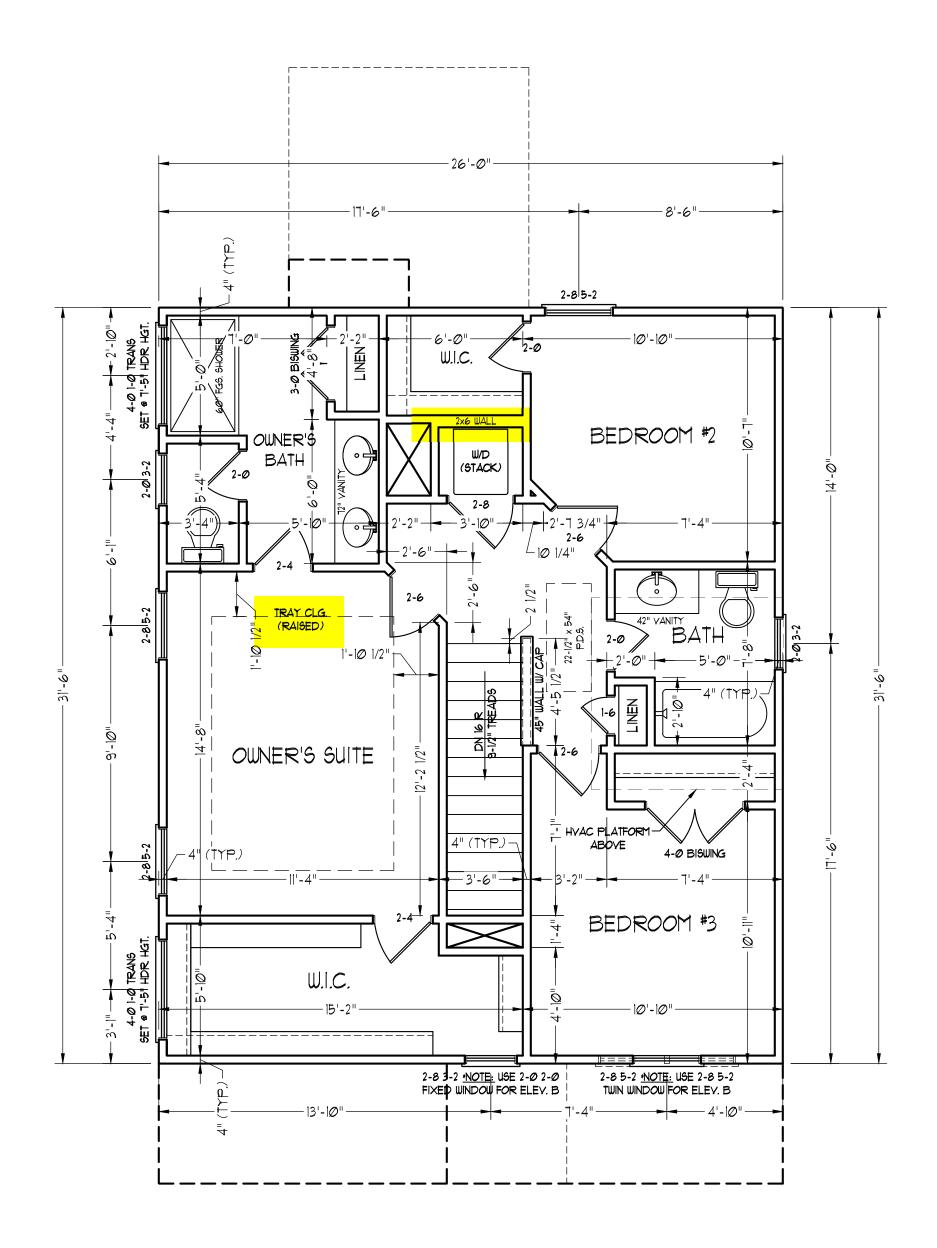
REVIEWED BY:

A-3

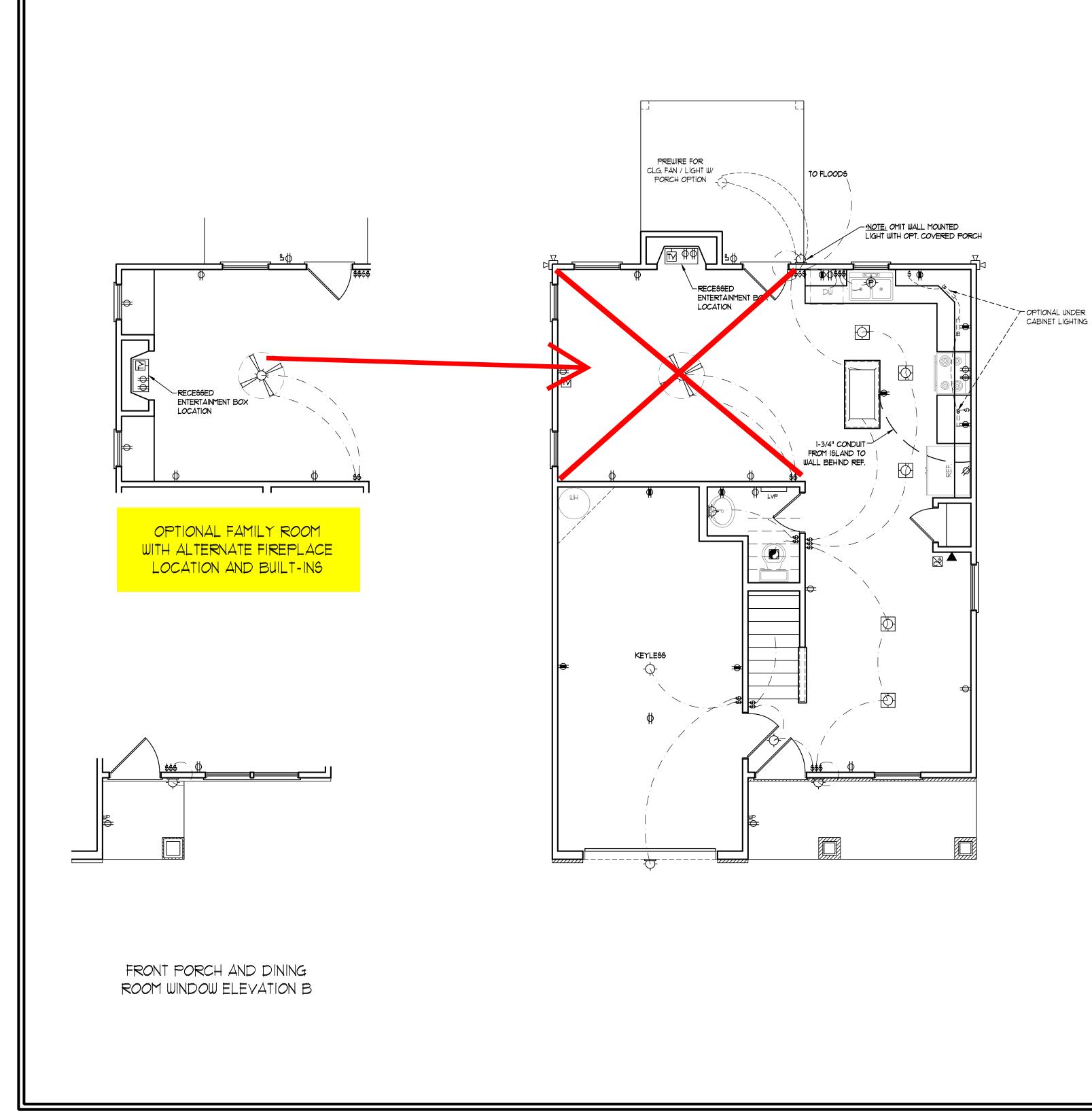
C - ELEVATIONS

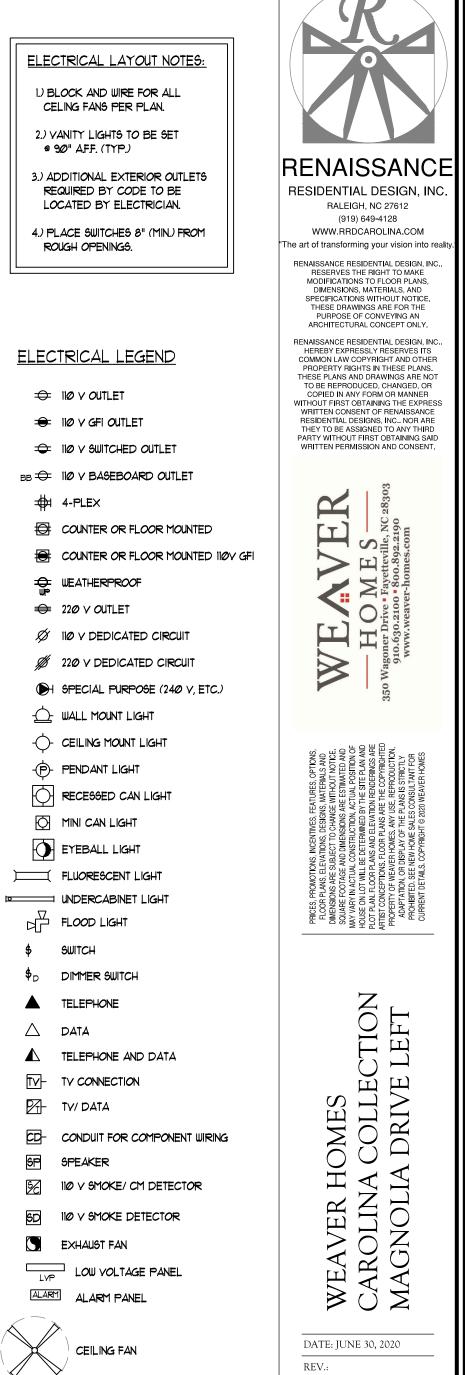


11x17 PRINTS ARE NOT TO SCALE









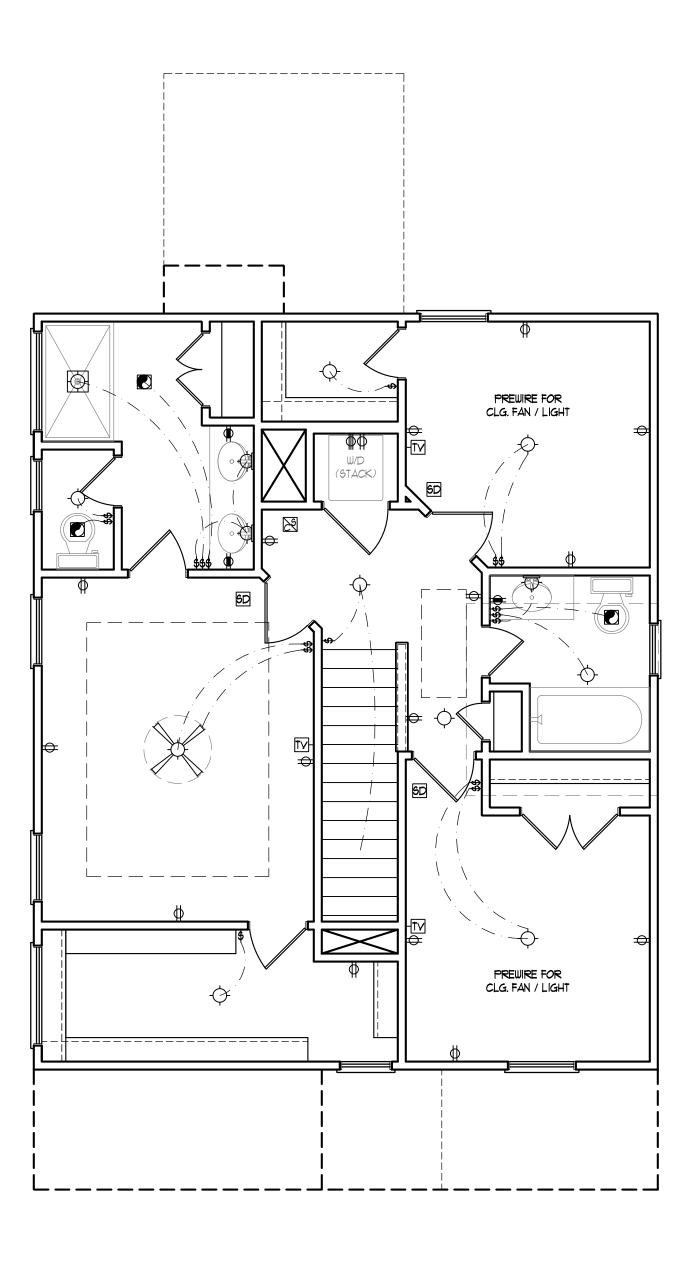
SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE**

CEILING FAN W/ LIGHT

S N E S PRICES, Fr FLOOR P DIMENSIC SQUARE MAY VARY HOUSE OI HOUSE OI ATTIST C '''OPEI TION Ìц Ц Ц Ĕ > \mathcal{O}

S

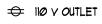
$\mathbb{A} \cap \mathbb{A}$
DATE: JUNE 30, 2020
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:
FIRST FLOOR ELECTRICAL PLAN E-1





- 1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET @ 90" A.F.F. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND



- 👄 110 V GFI OUTLET

- + + 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GFI

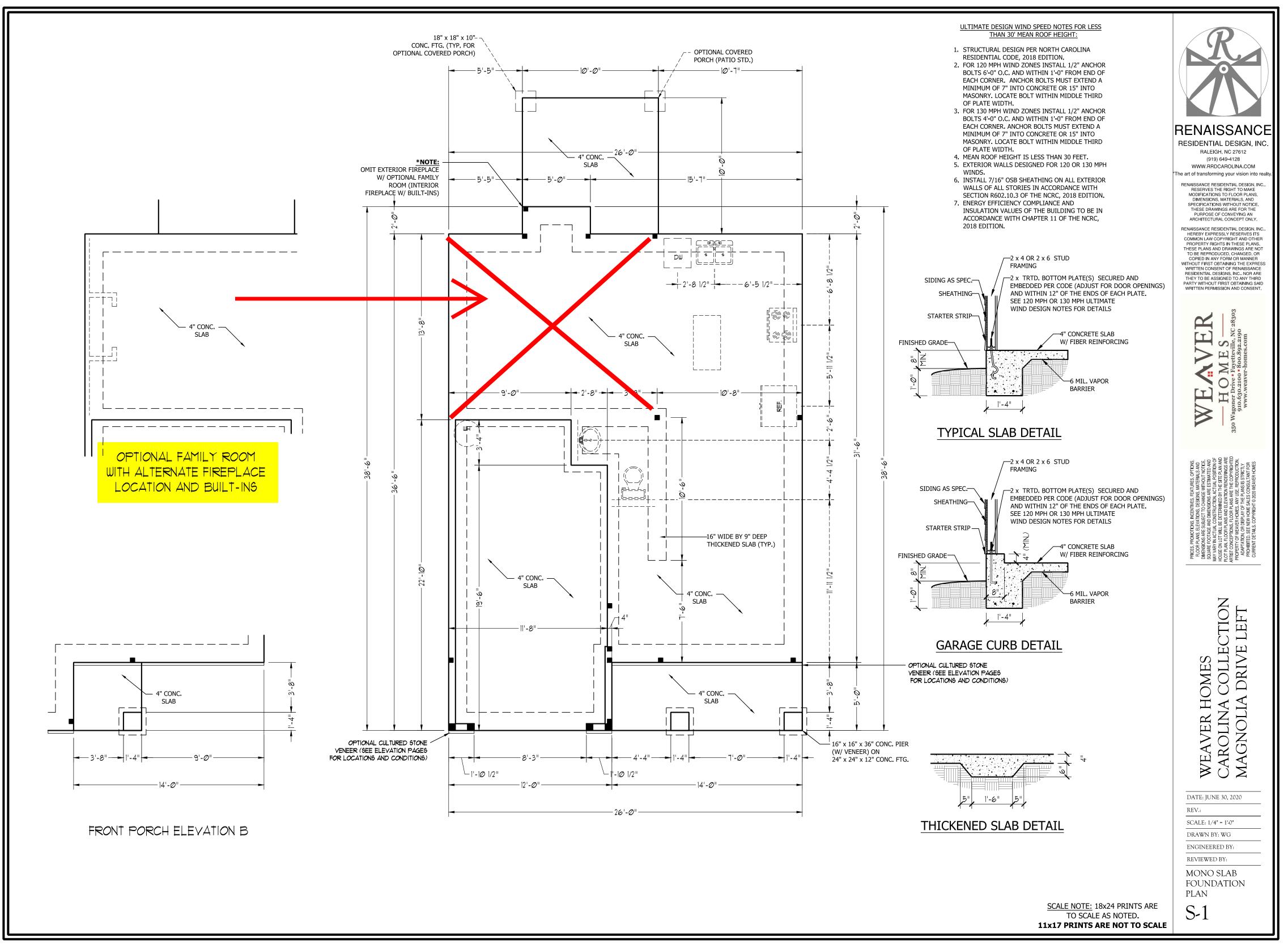
- Ø 10 V DEDICATED CIRCUIT
- 120 V DEDICATED CIRCUIT
- ●H SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- (P- PENDANT LIGHT
- MINI CAN LIGHT
- FLUORESCENT LIGHT
- \$ SWITCH
- \$_D DIMMER SWITCH
- riangle data
- TELEPHONE AND DATA
- TV- TV CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- 110 V SMOKE/ CO DETECTOR
- 5D 110 Y SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL

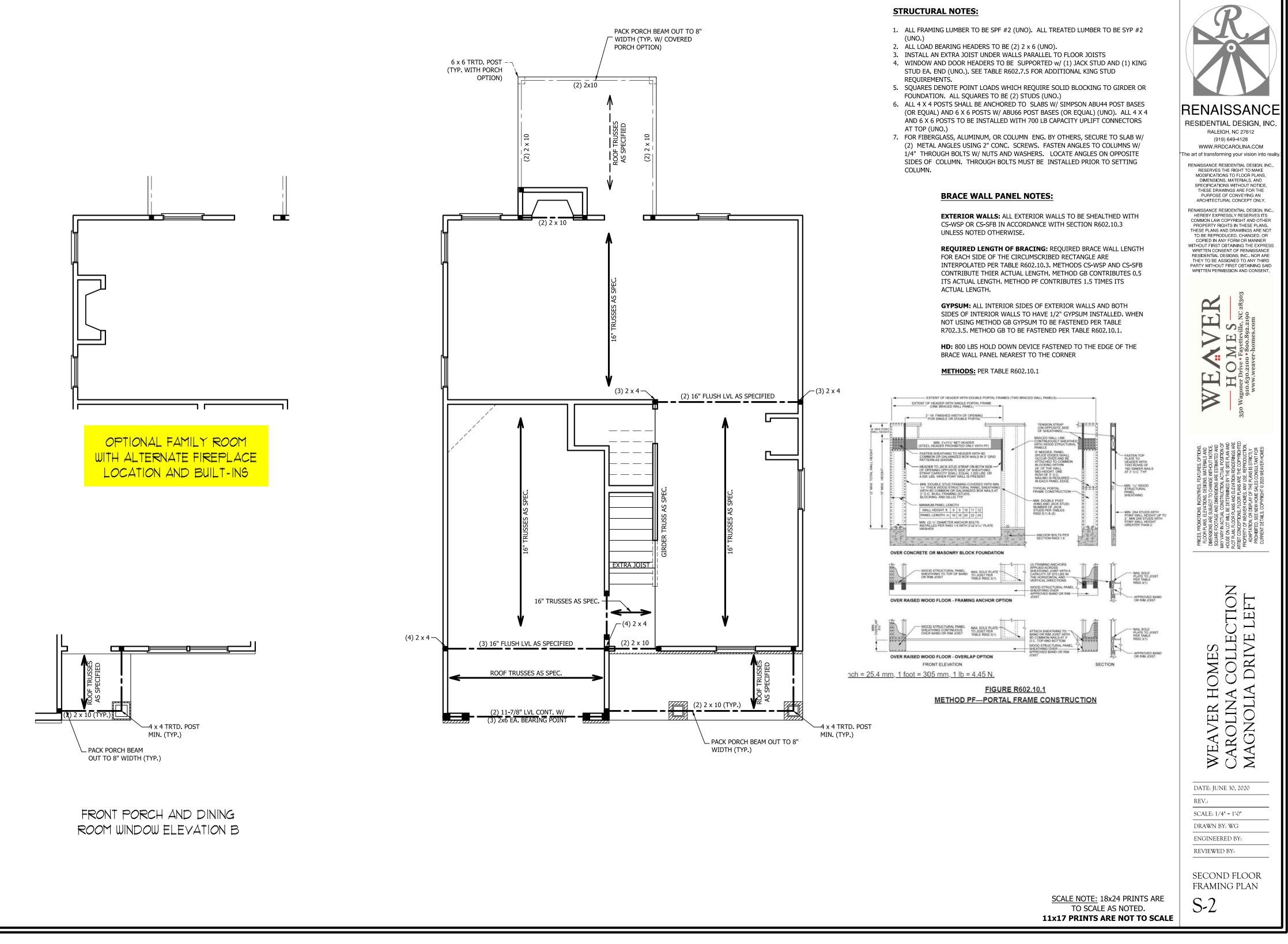
~

CEILING FAN W/ LIGHT



DATE: JUNE 30, 2020 REV.: SCALE: 1/4" = 1'0" DRAWN BY: WG ENGINEERED BY: REVIEWED BY: SECOND FLOOR ELCTRICAL PLAN E-2	$\geq \bigcirc \geq$
SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY: REVIEWED BY: SECOND FLOOR ELCTRICAL PLAN	DATE: JUNE 30, 2020
DRAWN BY: WG ENGINEERED BY: REVIEWED BY: SECOND FLOOR ELCTRICAL PLAN	REV.:
ENGINEERED BY: REVIEWED BY: SECOND FLOOR ELCTRICAL PLAN	SCALE: 1/4" = 1'-0"
REVIEWED BY: SECOND FLOOR ELCTRICAL PLAN	DRAWN BY: WG
SECOND FLOOR ELCTRICAL PLAN	ENGINEERED BY:
ELCTRICAL PLAN	REVIEWED BY:
	ELCTRICAL PLAN





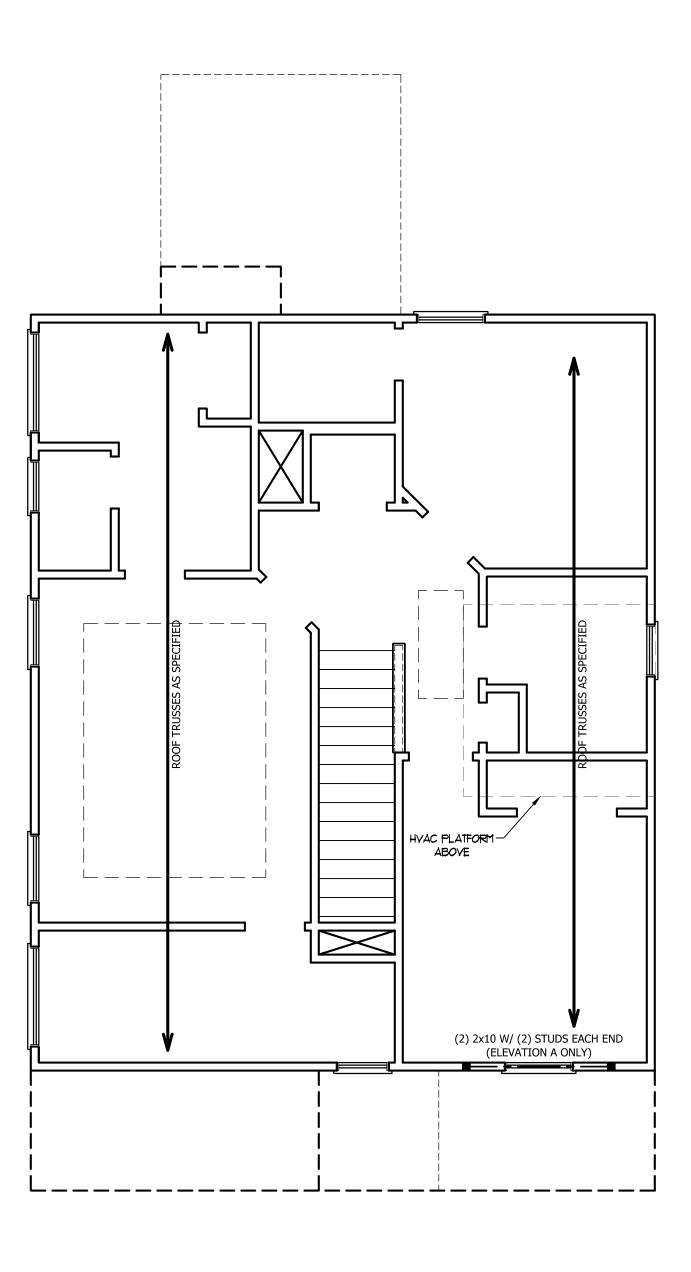


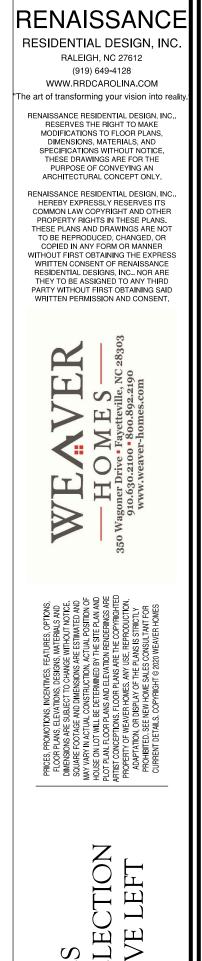
TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHES) (PER TABLE R602.3(5)									
(• == •)	16	24								
UP TO 3'	1	1								
4'	2	1								
8'	3	2								
12'	5	3								
16'	6	4								

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET





DATE: JUNE 30, 2020

REV.:	
SCALE: 1/4" = 1'-0"	
DRAWN BY: WG	
ENGINEERED BY:	
REVIEWED BY:	

ATTIC FLOOR FRAMING PLAN

S-3

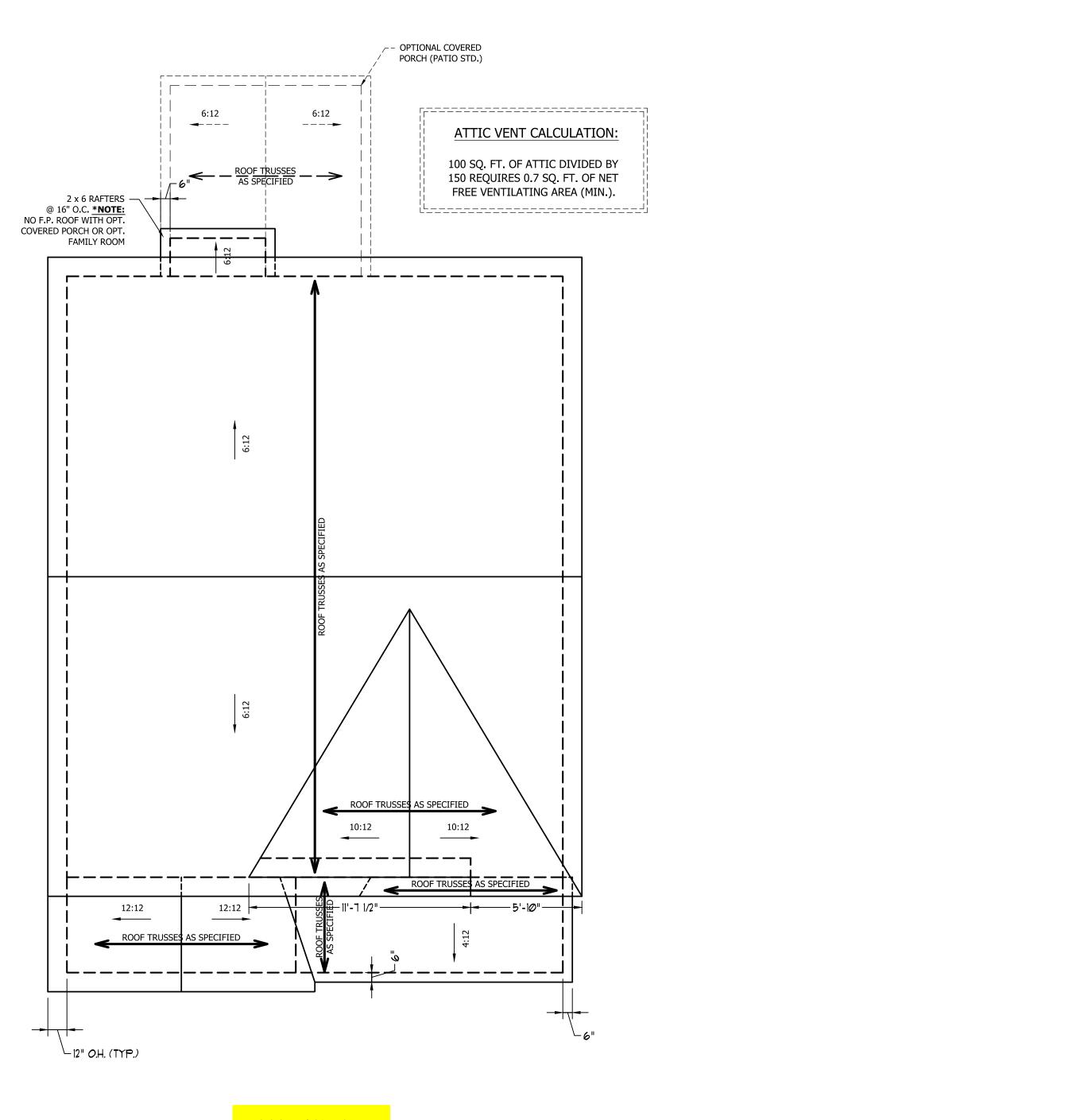
ATTIC VENT CALCULATION:

1040 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 6.9 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

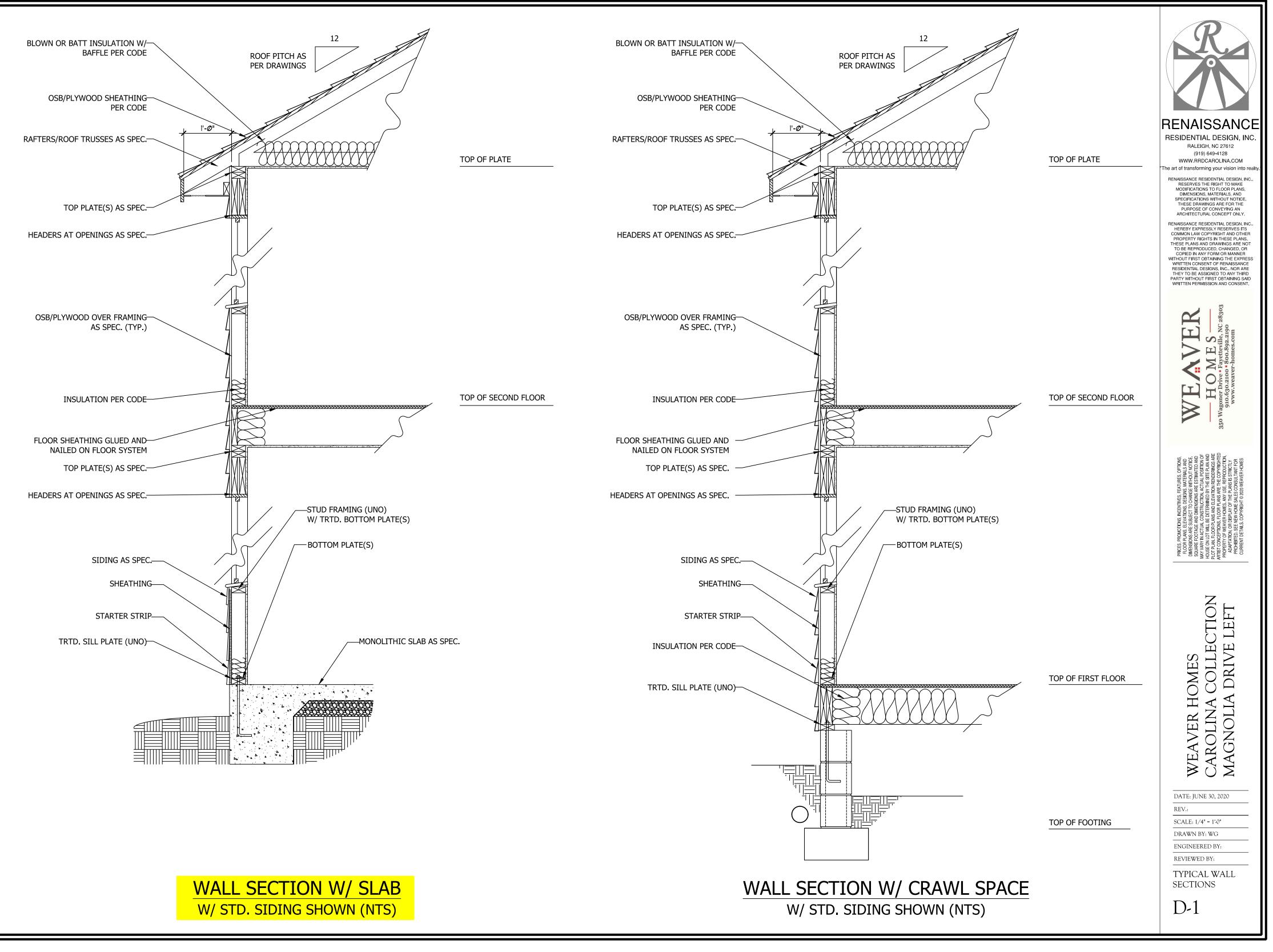
- ALL FRAMING LUMBER TO BE #2 SPF (UNO).
 HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- 5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.

C:\Users\Wade\Documents\Projects\Westan-Weaver\Magnolia\Magnolia_GL_6-30-20.dwg, 7/22/2020 6:48:02 AM

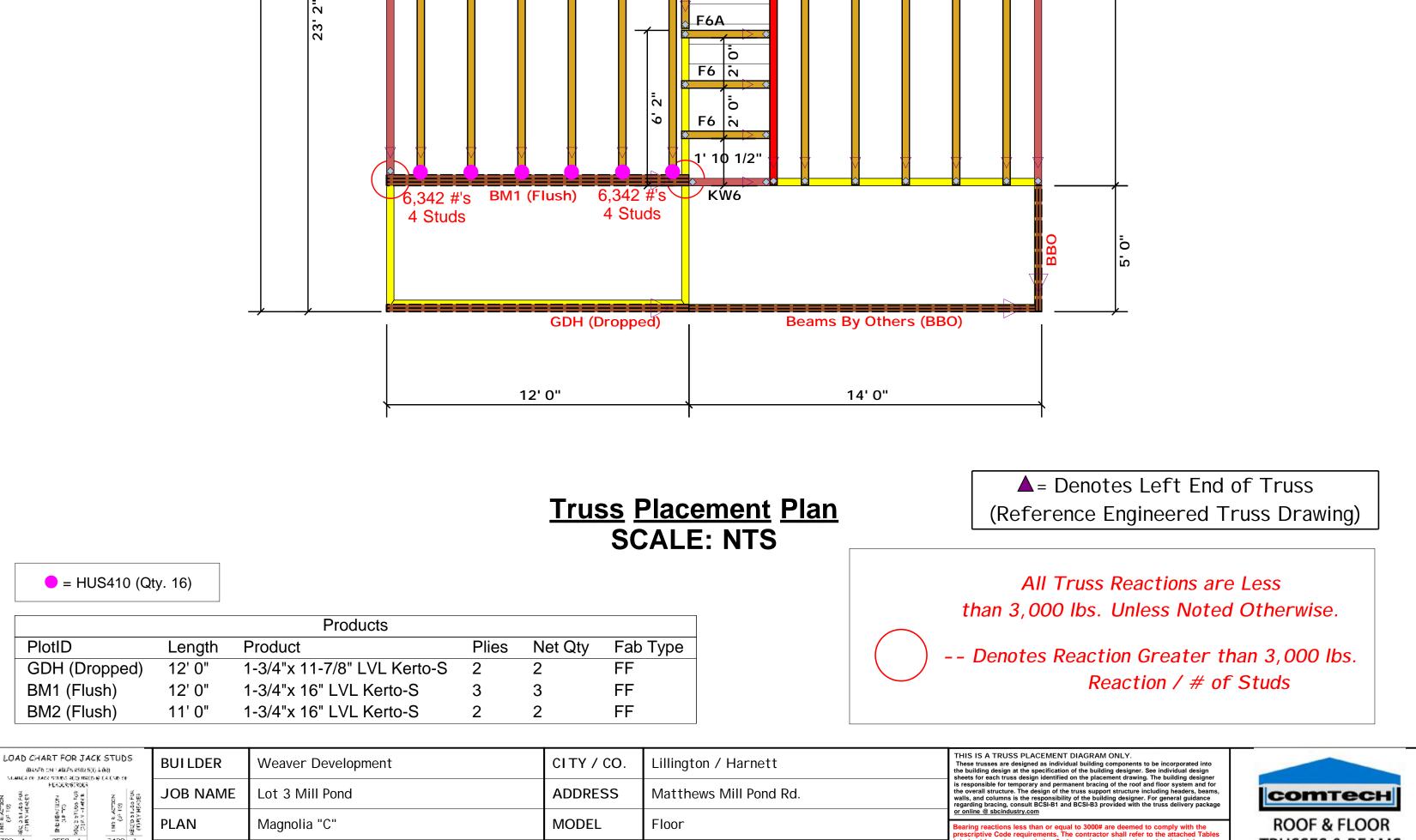


ELEVATION C





		5' 5'				10' 0"					10)' 7"			4
					BBO	===									
10'0"			BBO						BBO						
	<u>ک</u>	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"	1.3"	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"	1.3"	⊃ ≁
13' 4"								F3	F3	F4	F4	F4	F4	F4	KW4
	_										, ,	×			
	F1	F1	F1	F	F1	F1	• 4 3 4 3	,589 #'s 3 Studs			BM2	Flush)			4,589 #'s 3 Studs
								3' 6"	_		10' 9	0 1/2"			
=									F2A	F2	F2	F2	F2	F2	KW2



MODEL

DATE REV.

DRAWN BY

SALES REP.

Floor

11

Christine Shivy

Lenny Norris

END REAC

Magnolia "C"

Seal Date

Quote #

J1021-6294

PLAN

SEAL DATE

QUOTE #

JOB #

3400 1 6600 2

10200 3

13600 4 17000 5

2550 1 5100 2

7650 3

10200 4 12750 5

15300 6

	R	OOF	&	F	LO	0	R	
т	RI	ISSE	S	8	RF	A	M	S

or online @ sbcindustry.com

Signatur

on size and number of wood st

n 3000# but not greater than 15000#. A re

ion the support system for all react

Code requirements) to dete

Christine Shivy

Christine Shivy

ids required to sup

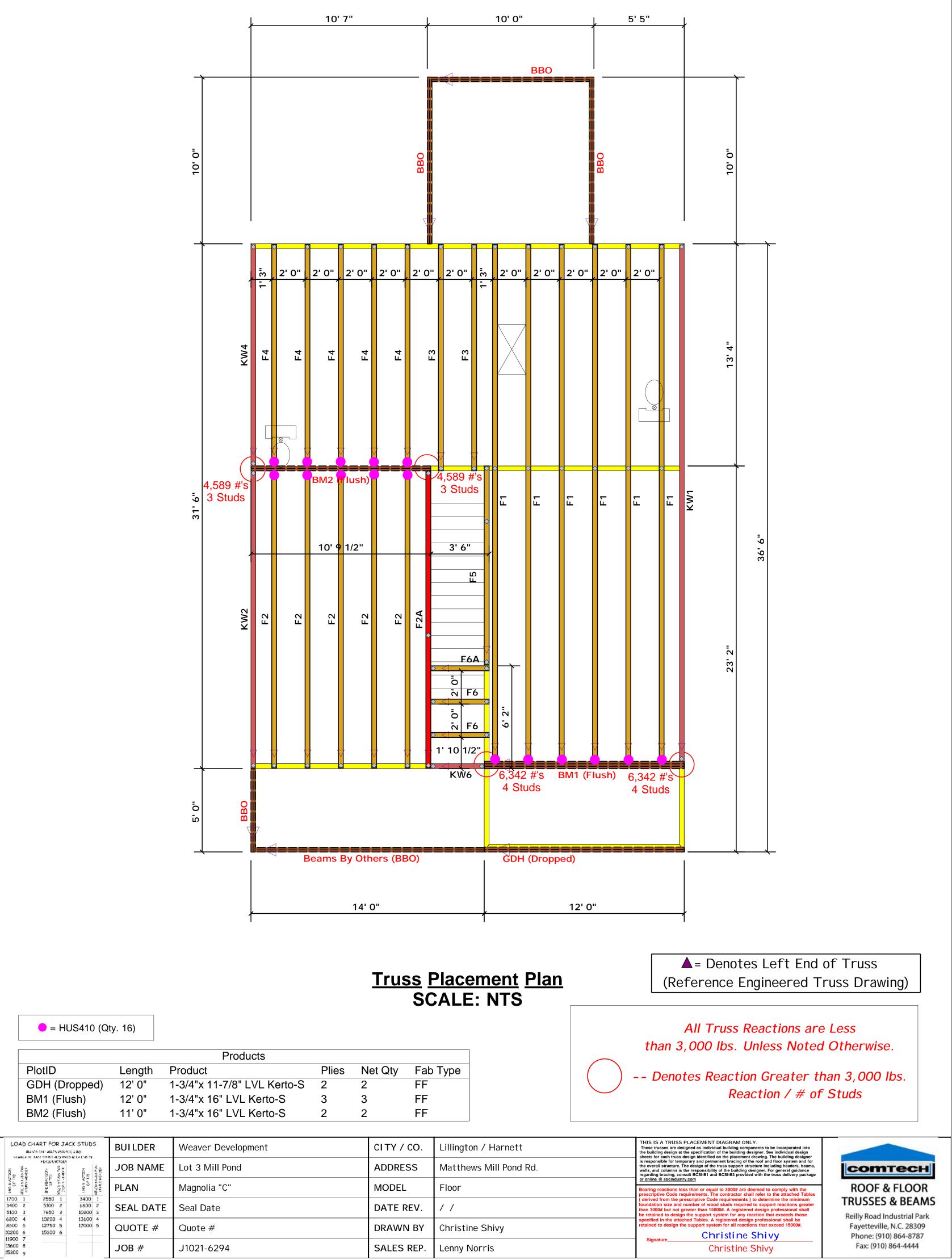
ns that exceed

ns grea

nal shall

nall be

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444



10200 4 12750 5

15300 6

13600 4

17000 5

QUOTE #

JOB #

Quote #

J1021-6294

DRAWN BY

SALES REP.

Christine Shivy

Lenny Norris

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

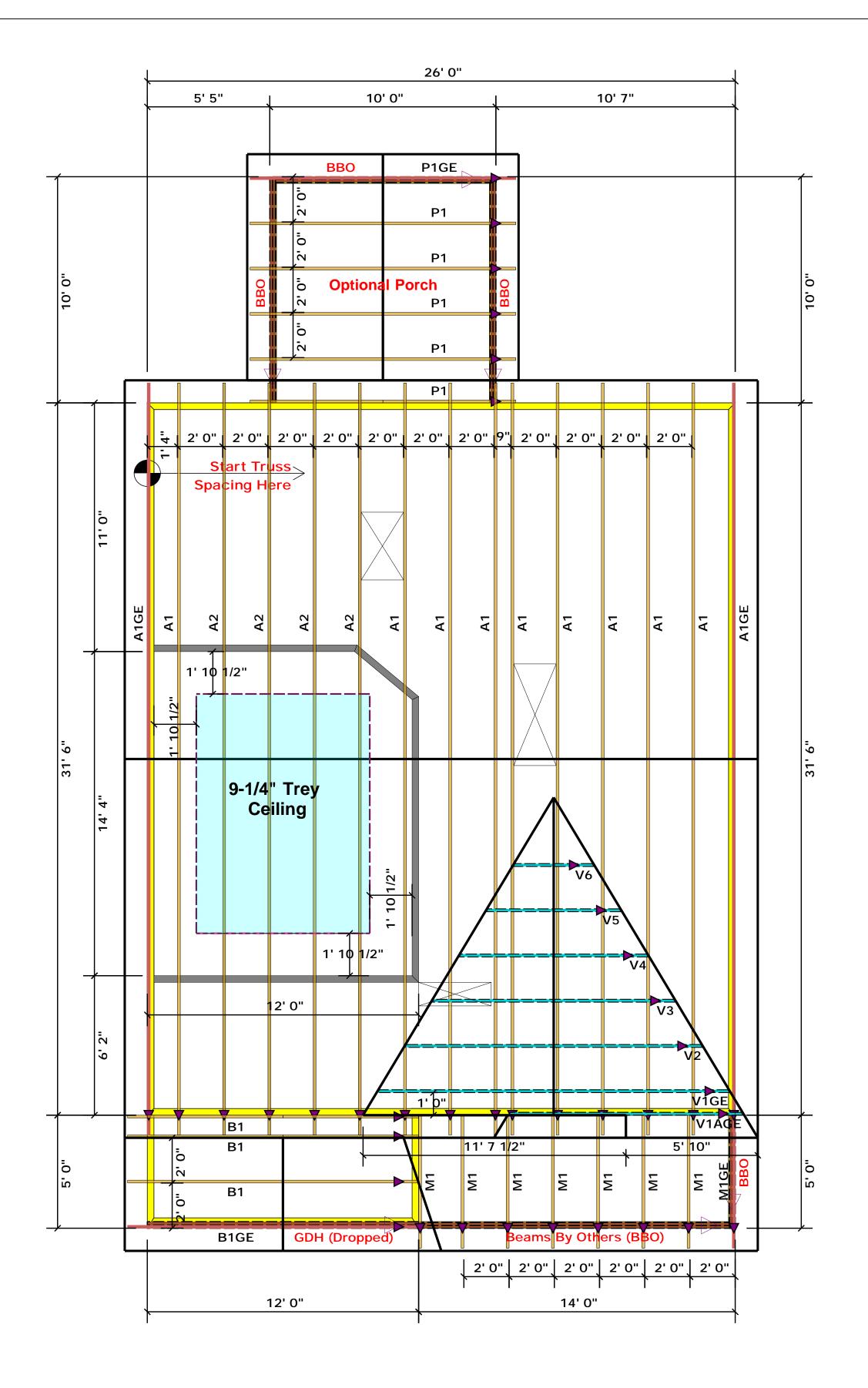
all be

an the support system

Signatur

Christine Shivy

Christine Shivy



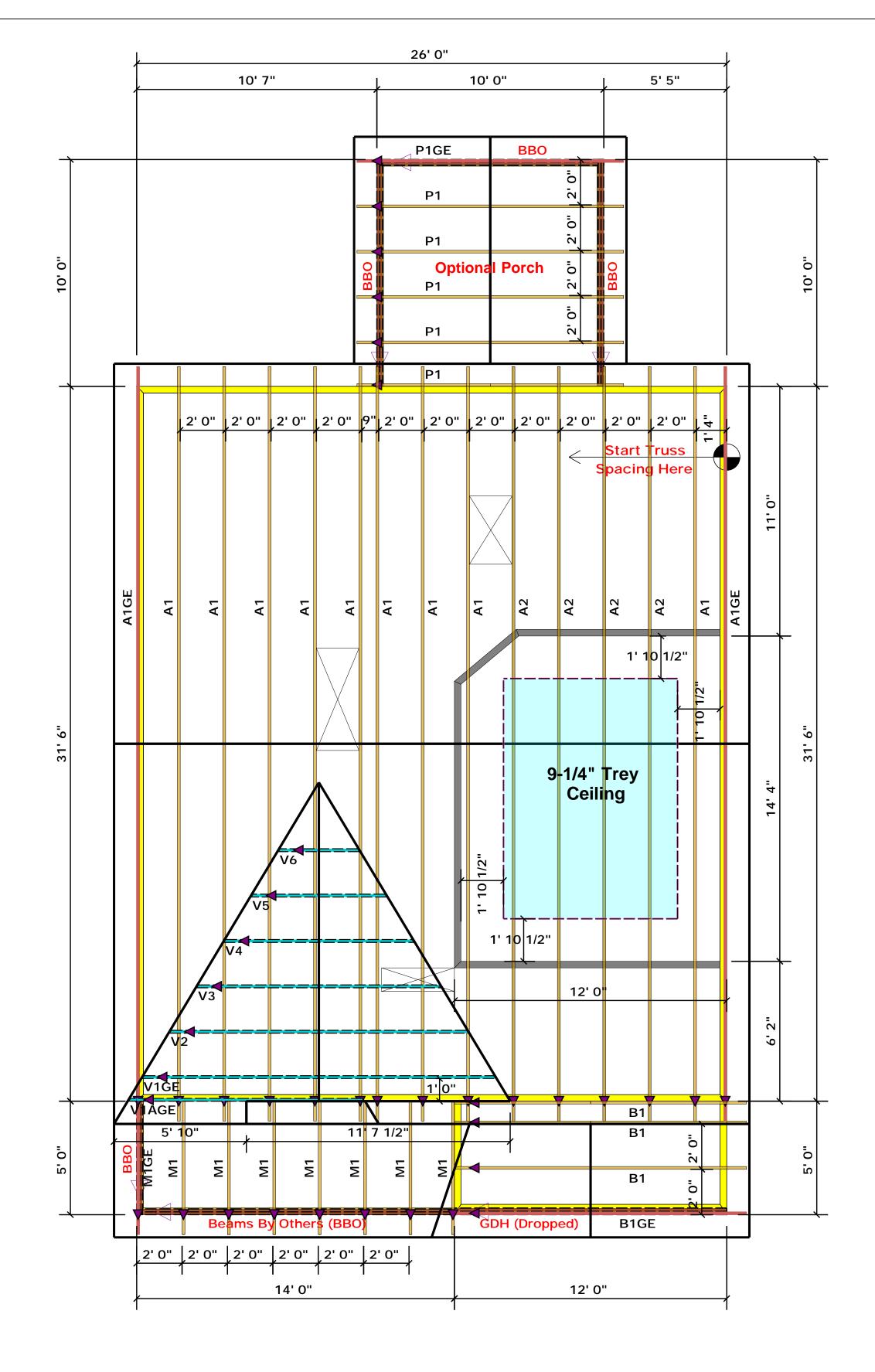
All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)

<u>Truss Placement Plan</u> SCALE: NTS

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

0 4	WART FOR JA AND ON 1 ABLES (25025) HIJAGA STUDIA (COURT)	5(1) & (6))	BUILDER	Weaver Development	CITY/CO.	Lillington / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
z Žű	FEADER/FERDER	z Ég	JOB NAME	Lot 3 Mill Pond	ADDRESS	Matthews Mill Pond Rd.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
IND REACTIO (01 TO) MEC 0 STUDIO	15 4 0 0 0 16 4 0 0 0 16 4 0 0 0 16 4 0 0 16 10 0 10 0	NN NU NN NH NN NH NN	PLAN	Magnolia "C"	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	Seal Date	DATE REV.	/ /	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Christine Shivy	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Christine Shivy	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9			JOB #	J1021-6293	SALES REP.	Lenny Norris	Christine Shivy	Fax: (910) 864-4444



All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)

<u>Truss</u> <u>Placement</u> <u>Plan</u> SCALE: NTS

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

(045F	ART FOR JA 55 ON 1 ABLES (\$502.5) AGK STURG (\$COURD)	(0, 4, 66)	BUILDER	Weaver Development	CITY/CO.	Lillington / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
IN BLACTION (0^1 TO) (0^2 TO) (0^2 TO) (0^3 HEADER		z Žŵ	JOB NAME	Lot 3 Mill Pond	ADDRESS	Matthews Mill Pond Rd.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
<u> </u>	ano ano ano ano ano ano ano ano ano ano ano ano ano	INN KU UN UN UN	PLAN	Magnolia "C"	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	Seal Date	DATE REV.	11	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 300% but not greater than 15000%. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Christine Shivy	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Christine Shivy	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9			JOB #	J1021-6293	SALES REP.	Lenny Norris	SignatureChristine Shivy	Fax: (910) 864-4444

			aver Homes Inolia Elev. C		Dat		/1/2021 ristine Shivy		Page 1 c
is	Design	Address: Ma	gnolia Elev. C				agnolia Elev. C		
BM1 I	Kerto-S LVL	1.750" X 1	6 000" 3-P	lv - P		ject #: Level:	Level		
		11750 X 1	0.000 01	'y ' <i>r</i>		, 			
					3		44444444		
	2								
•	• • •			•		-	• •		
• •	• •	• •	• •	•	•	•	•		
1	C. Min	The state	all the second	10	-	- the			- WW '
1 SPF							2 SPF		
ļ			12'						5 1/4"
<u> </u>			12'						1 1
lember In	formation				Reactions	S UNPAT	FERNED Ib (Uplif	t)	
Туре:	Girder	Application:	Floor		Brg	Live	Dead Snov		
Plies: Moisture Cond	3 dition: Drv	Design Meth Building Cod			1 2	1908 1908	3448 1950 3448 1950		
Deflection LL:	480	Load Sharin			2	1300	0440 100	, ,	0
Deflection TL: Importance:	360 Normal	Deck:	Not Checked						
Temperature:	Temp <= 100°F								
				-	Bearings		0 0 0 0	-	<u> </u>
					Bearing 1 - SPF	•	Cap. React D/L lb 81% 3448 / 2894	Total Ld. 6342 L	Case Ld. Comb. D+0.75(L+S)
					2 - SPF		81% 3448 / 2894	6342 L	D+0.75(L+S)
Analysis Re Analysis		ation Allowed C	apacity Comb.	Case					
Moment	17663 ft-lb		285 (28%) D+0.75(L+S)						
Unbraced	17663 ft-lb		998 D+0.75(L+S) 00%)) L					
Shear	4536 lb 1'6		253 (25%) D+L	L					
	0.065 (L/2129)		230 (23%) 0.75(L+S)	L					
	0.143 (L/972)	6 0.385 (L/360) 0.	370 (37%) D+0.75(L+S)						
	blies using 4 rows of 10d	Box nails (.128x3") at 12	o.c. Maximum end dist	ance not					
to exceed 6 2 Refer to las	5". It page of calculations for	fasteners required for sp	ecified loads.						
	designed to be supporte	-	ly.						
5 Top must b	e laterally braced at a ma								
	ced at bearings. Iderness ratio based on s	single ply width.							
ID	Load Type	Location Trib		Dead 0.9	Live 1				nments
1	Uniform		Тор	125 PLF	0 PLF	0 PL 225 PI			erior Wall
2 3	Uniform Uniform		Top Far Face	325 PLF 106 PLF	0 PLF 318 PLF	325 PL 0 PL		0 PLF A2 0 PLF F1	
~	Self Weight			19 PLF	5.01 6	0.1	··		
	~								
							factoria 1. f	Control	loc
Notes Calculated Structured	Designs is responsible only of the	chemicals Handling & Installation	6. For flat r ponding	roofs provide pro	per drainage to p	Metsa	facturer Info Wood	Fayetteville	eilly Road, Suite #639
structural adequacy design criteria and responsibility of the o	of this component based on the loadings shown. It is the customer and/or the contractor to	1. LVL beams must not be cut or dri 2. Refer to manufacturer's p regarding installation requi	oduct information			Norw	lerritt 7 Building, 2nd Floor alk, CT 06851	. USA 28314 910-864-T	RUS
application, and to ver	ent suitability of the intended ify the dimensions and loads.	fastening details, beam strengt approvals 3. Damaged Beams must not be us	values, and code			www.	622-5850 metsawood.com/us		
Lumber 1. Dry service conditi 2. LVL not to be trea	and contains a start attain and a	 Damaged Beams must not be us Design assumes top edge is late Provide lateral support at beam lateral displacement and rotation 	rally restrained ing points to avoid			ICC-E	S: ESR-3633	lc	оттесн
	Powered by iStruct [™]	ateral displacement and rotation	This de	esign is valid u	Intil 1/8/2023			CSD	

				er Homes blia Elev. C				1/2021 ristine Shivy			Page 1 of
	sDesign	Ad	dress: Magr	nolia Elev. C				gnolia Elev. C			
BM2	Kerto-S LVI	17	50" X 16	000" 2-	Ply - P		oject #: Level:	Level			
		_ 1.7		1000 Z-1	i iy - i /	AUULL					
	2										
	· ·	•			-	<u> </u>					$n \uparrow$
											M
1	S.C. S. Marken		-	at the	17	-	C. State				1'4"
	•	•			•	•	2 SPF				
/			10'9 1/2	<u>, </u>							3 1/2"
<u> </u>			10'9 1/2								1 10
							·				
lember li	nformation					Reaction	s UNPAT1	ERNED Ib (Up	lift)		
Туре:	Girder		Application:	Floor		Brg	Live			Wind	Const
Plies: Moisture Co	2 ndition: Dry		Design Method Building Code:			1	3389 3389	1200 1200	0 0	0 0	0 0
Deflection L	-		Load Sharing:	No		2	0000	1200	0	0	Ū
Deflection T			Deck:	Not Checked							
Importance: Temperature	Normal :: Temp <= 100°F	-									
remperature	. Temp <= 100 P					Bearings					
						Bearing		Cap. React D/L	lb Total	Ld. Case	Ld. Comb.
						1 - SPF	3.500"	88% 1200 / 33	39 4589	L	D+L
nalysis R	esults	I				2 - SPF	3.500"	88% 1200 / 33	39 4589	L	D+L
Analysis		ocation All	owed Car	acity Comb.	Case]					
Moment	11397 ft-lb	5'4 3/4" 34	565 ft-lb 0.33	0 (33%) D+L	L						
Unbraced	11397 ft-lb			0 (97%) D+L	L						
Shear	4386 lb	1'6 5/8" 119		7 (37%) D+L	L						
	0.085 (L/1457)		259 (L/480) 0.33		L						
I L Deti inci	n 0.115 (L/1076)	5'4 3/4" 0.3	345 (L/360) 0.33	0 (33%) D+L	L	ł					
	2010				istance not	1					
Design No 1 Fasten al		0d Box nails (′.128x3") at 12" c								
1 Fasten al to exceed	plies using 3 rows of 1 6".		. ,								
 Fasten al to exceed Refer to la Girders al 	plies using 3 rows of 1 6". ast page of calculations re designed to be suppo	for fasteners	required for spec	cified loads.							
 Fasten al to exceed Refer to la Girders al Top brace 	plies using 3 rows of 1 6". ast page of calculations re designed to be suppo ed at bearings.	for fasteners	required for spec	cified loads.							
 Fasten al to exceed Refer to la Girders a Top brace Bottom bi 	plies using 3 rows of 1 6". ast page of calculations re designed to be suppo	for fasteners orted on the b	required for spec ottom edge only.	cified loads.							
 Fasten al to exceed Refer to la Girders a Top brace Bottom bi Lateral slip 	plies using 3 rows of 1 6". ast page of calculations re designed to be suppo ed at bearings. acced at bearings. enderness ratio based of Load Type	for fasteners orted on the b on single ply v	required for spec ottom edge only.	rified loads.	Dead 0.9	Live 1				Comment	s
 Fasten al to exceed Refer to la Girders a Top brace Bottom bi Lateral sle ID 1 	plies using 3 rows of 1 6". ast page of calculations re designed to be suppo ed at bearings. aced at bearings. enderness ratio based of Load Type Uniform	for fasteners orted on the b on single ply v	required for spec ottom edge only. vidth.	cified loads. Iidth Side Far Face	Dead 0.9 89 PLF	267 PLF	O PL	F 0 PLF	0 PLF	F4	s
 Fasten al to exceed Refer to la Girders a Girders a Top brace Bottom bit Lateral slit 	plies using 3 rows of 1 6". ast page of calculations re designed to be suppo ed at bearings. acced at bearings. enderness ratio based of Load Type	for fasteners orted on the b on single ply v	required for spec ottom edge only. vidth.	rified loads.	Dead 0.9		O PL	F 0 PLF		F4	s



	sDesign	Project: Address:	Magnolia Elev. C Magnolia Elev. C		Job Pro	Name: Ma oject #:	ristine Shivy gnolia Elev. C		
GDH	Kerto-S LVL	1.750"	X 11.875"	2-Ply - F	ASSED	Level:	Levei		
	2	•	1		•		• •		
-	C. The		and the second s		The second	C. Mar			
	End Grain					2 SPF End	Grain		
1			8'10"						3 1/2"
ł			8'10"				1		
	<u> </u>				D			<u></u>	
Type:	nformation _{Girder}	Applica	ation: Floor		Brg		ERNED Ib (Uplif		Const
Plies:	2	Design	Method: ASD		1	0	1101 17		0
	ndition: Dry		g Code: IBC/IRC	2015	2	0	1101 17	77 0	0
Deflection L Deflection T		Load S Deck:	haring: No Not Che	cked					
nportance:		Dook							
emperature	e: Temp <= 100°F								
					Bearings		0		
					Bearing 1 - SPF	-	Cap. React D/L lb 12% 1101 / 177		E Ld. Comb. D+S
nalysis R	esults				End Grain				
Analysis	Actual Lo	cation Allowed	Capacity Com	b. Case	2 - SPF End	3.500"	12% 1101 / 177	1277 L	D+S
Moment	2185 ft-lb	4'5" 17919 ft-lb	0.122 (12%) D	Uniform	Grain				
Jnbraced	2536 ft-lb 797 lb 7	4'5" 10756 ft-lb "7 3/8" 7980 lb	0.236 (24%) D+S 0.100 (10%) D	L Uniform					
Shear _L Defl inc		7 3/8 7 980 lb 5 1/16" 0.209 (L/48	. ,	L					
	(L/18257)	· ·	, , ,	-					
L Defl inc	h 0.040 (L/2525) 4'5	5 1/16" 0.279 (L/36	0) 0.140 (14%) D+S	L	4				
esign No					4				
to exceed	l plies using 2 rows of 10d 16".	Box halls (.128x3")	at 12° o.c. Maximum e	end distance not					
	ast page of calculations fo re designed to be support								
4 Top loads	s must be supported equal		goonyi						
	ed at bearings. raced at bearings.								
7 Lateral sl	enderness ratio based on								
D	Load Type	Location	Trib Width Side	Dead 0.9	Live 1				
	Uniform		Тор	200 PLF	0 PLF			0 PLF Exterior	
2	Uniform		Тор	40 PLF	0 PLF	40 PL	F 0 PLF	0 PLF 2'-0" Gal	ble End
	Self Weight			9 PLF					
tructural adequac	ed Designs is responsible only of the y of this component based on the and loadings shown. It is the customer and/or the contractor to ornent suitability of the intended wrify the dimensions and loads.	1. LVL beams must not be 2. Refer to manufactur regarding installation fastening details, beam	ion	For flat roofs provide p ponding	roper drainage to p	Metsä 301 M Norwa (800)	facturer Info Wood erritt 7 Building, 2nd Floo Ik, CT 06851 622-5850	Comtech, Inc. 1001 S. Reilly Rc Fayetteville, NC USA 28314 910-864-TRUS	oad, Suite #639
umber	ditions, unless noted otherwise	approvals 3. Damaged Beams must r 4. Design assumes top edge	e is laterally restrained			ICC-E	netsawood.com/us S: ESR-3633		
		 uesign assumes top edg 	e is laterally restrained at bearing points to avoid rotation					llee	тесн