

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 2200084-2200084A HOFFNER JOB

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I50142123 thru I50142179

My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844



February 9,2022

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MI Iek A 818 Soundside Road Edenton, NC 27932

February 9,2022



2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 14=125, 10=143.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







ent 818 Soundside Road

Edenton, NC 27932



7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Edenton, NC 27932

044925



February 9,2022





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **ANSUTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MiTek Affilia 818 Soundside Road

Edenton, NC 27932

February 9,2022





F			45-0-0 45-0-0					
Plate Offsets (X,Y)	[6:0-6-0,0-3-4], [16:0-3-0,0-3-4], [22:0-3	-0,0-1-4], [31:0-3-10,0-2-0]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.14 BC 0.09 WB 0.12 Matrix-S	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.01	i (loc) l/defl L/ i - n/a 99 i - n/a 99 24 n/a n/	/d PL 99 M ⁻¹ 99 /a We	ATES GRIP F20 244/190 eight: 475 lb FT = 20%		
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x4 SF	P No.2 No.2 P No.3 P No.3	· · · · ·	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood shea except end verticals, Rigid ceiling directly 1 Row at midpt	athing directly app and 2-0-0 oc purl applied or 10-0-0 16-33, 15- 9-40, 8-41	lied or 6-0-0 oc purlins, ins (6-0-0 max.): 6-16. oc bracing. 34, 13-35, 12-36, 11-37, 10-39, , 7-42, 5-43, 4-44, 17-32, 18-30		
(lb) - Max H Max U Max G	arings 43-6-0. lorz 47=-292(LC 13) plift All uplift 100 lb or less at joint(s) 4 30, 29, 28, 27, 26 except 25=-104(irav All reactions 250 lb or less at joint 32, 30, 29, 28, 27, 26 except 25=43	7, 24, 33, 34, 35, 36, 37, 3(LC 9) (s) 47, 24, 33, 34, 35, 36, 3 33(LC 1)	9, 40, 41, 42, 43, 44, 45 7, 39, 40, 41, 42, 43, 4	i, 46, 32, 4, 45, 46,				
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 5-6=-233/287, 6-7=-207/262, 7-8=-207/262, 8-9=-207/262, 9-10=-207/262, 10-11=-207/262, 11-12=-207/262, 12-13=-207/262, 13-15=-207/262, 15-16=-208/262, 16-17=-237/304, 17-18=-189/276, 21-22=-255/245, 23-24=-284/183								
BOT CHORD 46-47 41-42 35-36 29-30 24-25	7=-133/290, 45-46=-133/290, 44-45=-13 2=-133/290, 40-41=-133/290, 39-40=-13 5=-133/290, 34-35=-133/290, 33-34=-13 0=-133/290, 28-29=-133/290, 27-28=-13 5=-135/292	3/290, 43-44=-133/290, 42 3/290, 37-39=-133/290, 36 3/290, 32-33=-133/290, 30 3/290, 26-27=-133/290, 25	-43=-133/290, -37=-133/290, -32=-133/290, -26=-135/292,					
WEBS 23-25	5=-307/172					1.2.1.1.1.2.2.1.		
 WEBS 23-25=-307/172 NOTES- Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. Provide adequate drainage to prevent water ponding. All plates are 2x4 MT20 unless otherwise indicated. Gable studs spaced at 2-00 oc. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 47, 24, 33, 44, 54, 64, 32, 30, 29, 28, 27, 26 except (jt=lb) 25=104. Non Standard bearing condition. Review required. Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 								
			DACE MIL 7472 5/40/202					

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 ev. 5/19/2/2/2/ BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932





Edenton, NC 27932





Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



LOADING	(psf)	SPACING- 2-0	-0 CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.1	15 TC	0.40	Vert(LL)	0.01	4-5	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL 1.1	15 BC	0.15	Vert(CT)	-0.01	4-5	>999	180		
BCLL	0.0 *	Rep Stress Incr N	IO WB	0.12	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	4 Matr	ix-MP						Weight: 140 lb	FT = 20%
	_				BRACING						

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (size) 4=Mechanical, 6=0-3-8 Max Horz 6=-283(LC 4) Max Uplift 4=-415(LC 9), 6=-529(LC 4) Max Grav 4=923(LC 29), 6=881(LC 30)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-730/310, 3-4=-638/252

BOT CHORD 5-6=-254/573

WEBS 2-6=-777/493, 2-5=-385/611, 3-5=-254/589

NOTES-

1) 2-ply truss to be connected together with 10d (0.148"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=415, 6=529.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 145 lb down and 147 lb up at 0-1-12 on top chord, and 243 lb down and 189 lb up at 2-0-12, 243 lb down and 189 lb up at 4-0-12, and 243 lb down and 189 lb up at 6-0-12, and 247 lb down and 185 lb up at 8-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 4-6=-20

Concentrated Loads (lb)

Vert: 1=-9 7=-181(B) 8=-181(B) 9=-181(B) 10=-185(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





LOWIDER		DRACING-				
TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins,			
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2		except end verticals, and	2-0-0 oc purlins (6-0-0 max.): 2-4.		
WEBS	2x4 SP No.3 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.			
:	3-13,3-12: 2x4 SP No.2 or 2x4 SPF No.2	WEBS	1 Row at midpt	2-13, 3-13, 5-12, 1-14		
REACTIONS.	(size) 14=0-3-8, 9=0-3-8					

Max Horz 14=-384(LC 8) Max Uplift 14=-71(LC 13), 9=-126(LC 13) Max Grav 14=1183(LC 2), 9=1256(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-663/276, 2-3=-511/277, 3-4=-719/324, 4-5=-1029/315, 5-7=-1302/259, 1-14=-1111/255

 BOT CHORD
 13-14=-289/328, 12-13=-96/703, 10-12=-68/972

 24-29-289/328, 12-13=-96/703, 10-12=-68/972
 24-29-268/972

WEBS 3-13=-525/182, 3-12=-96/374, 4-12=-19/285, 5-12=-445/254, 7-10=-56/920, 7-9=-1140/289, 1-13=-147/776

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 9=126.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.66 BC 0.80 WB 0.75 Matrix-MS	DEFL. ir Vert(LL) -0.26 Vert(CT) -0.41 Horz(CT) 0.03	n (loc) l/defl L/d 3 12-13 >999 240 12-13 >818 180 3 9 n/a n/a	PLATES MT20 Weight: 238 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2X4 BOT CHORD 2X4 1-14 WEBS 2X4	SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2 *Except* 2x4 SP No.3 SP No.3 *Except*		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except 2-0-0 oc purlins (6-0-0 max.): Rigid ceiling directly applied of	rectly applied or 4-1-14 : 2-4. or 10-0-0 oc bracing. E	oc purlins,
2-14 OTHERS 2x4	3-13,3-12: 2x4 SP No.2 or 2x4 SPF No.2 SP No.3		WEBS	6-0-0 oc bracing: 1-15 1 Row at midpt 2	2-14, 3-13, 5-12	·

REACTIONS. (size) 9=0-3-8, 15=0-3-8 Max Horz 15=-362(LC 13) Max Uplift 9=-114(LC 13), 15=-84(LC 13) Max Grav 9=1245(LC 20), 15=1188(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-458/219, 3-4=-716/289, 4-5=-1022/275, 5-7=-1294/221
- BOT CHORD 14-15=-79/1047, 13-14=-73/593, 12-13=-59/705, 10-12=-31/965
- WEBS 2-14=-1106/113, 2-13=-59/802, 3-13=-537/158, 3-12=-107/360, 4-12=-2/280, 5-12=-444/255, 7-10=-9/891, 7-9=-1136/274

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15 except (jt=lb) 9=114.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MiTek Affiliate 818 Soundside Road

Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	HOFFNER JOB	
2200084-2200084A	D	ATTIC	10		1	150142138
84 Components (Dunn)	, Dunn, NC - 28334,			8.530 s D	Job Reference (option lec 6 2021 MiTek Industri	al) es, Inc. Wed Feb 9 06:55:27 2022 Page 1
		8-7-1	ID:jKreZKbOsTAr	np?V0vfcf 17-3-15	f_yznEUQ-LgcvbAamup?	Bv9BXmUAHRUSzEFIAg7ccuaxZF2zmxz_
	<u>-1-6-0</u> 1-6-0	5-0-4 7-4-1 7-11-13 12-4-0 5-0-4 2-3-13 0-7-12 3-8-15	16-0-15 3-8-15	16-8 ₁ 3 19 0-7-4 2	9-7-12 24-8-0 2-3-13 5-0-4	<u>26-2-0</u> -6-0
		0-7-4	2x4	0-7-12		Scale = 1:70.7
	т	5x9 =	6 ⊠ ⊠	7	5x9 =	Ţ
		4x4 1/ 4		8	4x4 📉	
		_ 4x6 //	27	- L	<u> </u>	
	12.00 1	3	1.5X4	1	4x6 \\ 9	
	11-0		2-4			11-0
	4x6 // /		ð		u u	4x6 📎
	2	/			2-10-	10
		< │	14-4-0			
	3-3-15			16	15 1	
			17	-4	4x8	
	26 27	25 23 ²¹ 20	1.574 11	14	13	図 12
	3x7 =	3x4 = 4x6 =			3x4 =	3x7 =
	L	5-0-4 7-8-0 10-3-12	14-4-4 ₁ 17-0)-0 ₁ 19-	-7-12 24-8-0	
Plate Offsets (X,Y)	[2:0-1-8,0-2-0], [5:0-2-2,Edge]	5-0-4 2-7-12 2-7-12 [7:0-2-2,Edge], [10:0-1-8,0-2-0]	4-0-8 2-7-	12 2-	7-12 5-0-4	-1
LOADING (psf)	SPACING- 2-0	-0 CSI .	DEFL.	in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.	15 TC 0.48	Vert(LL) -0.2	4 17-19	>999 240	MT20 197/144
BCLL 0.0 *	Rep Stress Incr YE	S WB 0.83	Horz(CT) 0.0	3 12	n/a n/a	
BCDL 10.0	Code IRC2015/TPI201	4 Matrix-MS	Attic -0.1	8 15-24	971 360	Weight: 268 lb F I = 20%
LUMBER- TOP CHORD 2x6 SI	P No.2 *Except*		BRACING- TOP CHORD	Structu	ural wood sheathing dire	ectly applied or 6-0-0 oc purlins,
	11: 2x6 SP DSS 2 No 2 *Excent*			except Bigid c	t end verticals, and 2-0-	0 oc purlins (6-0-0 max.): 5-7.
15-24:	2x4 SP No.2 or 2x4 SPF No.2		BOT CHOILD	2-2-0 0	oc bracing: 18-20.	
WEBS 2x4 SI 3-25,9	-13,4-8: 2x4 SP No.2 or 2x4 SF	F No.2, 2-26,10-12: 2x6 SP No.2	JOINTS	2-8-0 0 1 Brac	ce at Jt(s): 27	
REACTIONS. (siz	e) 26=0-3-8, 12=0-3-8					
Max H Max (lorz 26=-357(LC 10) Grav 26=1780(LC 2), 12=1780(_C 2)				
TOP CHORD 2-3=	-1467/0, 3-4=-954/158, 4-5=-22	1/440, 5-6=-7/776, 6-7=-7/776, 7-8=-22	21/440,			
8-9= BOT CHORD 25-2	-954/158, 9-10=-1467/0, 2-26= 6=-335/404, 23-25=-66/1040, 2	1776/0, 10-12=-1776/0 0-23=0/2577, 18-20=0/3616, 14-18=0/2	2398,			
13-1 15-1	4=0/882, 22-24=-1596/0, 19-22 6=-1616/0	=-2798/0, 17-19=-2798/0, 16-17=-2798	/0,			
WEBS 24-2	5=-320/99, 3-24=-6/704, 13-15 1593/0, 2-25-0/1059, 10-13-	326/107, 9-15=-6/704, 4-27=-1593/0,	0-20383/0			
20-2	2=-48/1362, 17-18=-383/0, 16-	8=-55/1368, 14-15=0/1749, 23-24=0/1	749			
NOTES-						
 Unbalanced roof liv Wind: ASCE 7-10; ¹ 	e loads have been considered f /ult=130mph Vasd=103mph; Ti	or this design. CDL=6.0psf; BCDL=6.0psf; h=30ft; Cat.	II; Exp B; Enclose	d; MWFR	RS (envelope)	AND CAD
gable end zone and shown: Lumber DO	C-C Exterior(2) zone; end vert	cal left and right exposed;C-C for mem	bers and forces &	MWFRS	for reactions	N. P. B.
3) Provide adequate d	rainage to prevent water pondi	ıg.				AFRITA SCIMES
5) This truss has been	designed for a 10.0 psf bottom	chord live load nonconcurrent with any	other live loads.			
will fit between the l	bottom chord and any other me	upsi on the bottom chord in all areas w nbers.	nere a rectangle 3	-6-0 tali d	by 2-0-0 wide	SEAL
 7) Ceiling dead load (5 8) Bottom chord live load 	5.0 psf) on member(s). 3-4, 8-9, ad (40.0 psf) and additional bo	4-27, 8-27; Wall dead load (10.0psf) o tom chord dead load (5.0 psf) applied o	n member(s).3-24, only to room. 22-24	, 9-15 I, 19-22, 1	17-19, 16-17,	044925
15-16 9) Bearing at joint(s) 2	6 12 considers parallel to grain	value using ANSI/TPI 1 angle to grain	formula Building	designer	should verify	10.0
capacity of bearing	surface.				and a	GINEE
11) Attic room checke	d for L/360 deflection.	e size of the orientation of the punin ald	ng me top and/or l		ioru.	M. SEM
						February 9,2022
						•
Design valid for use of	design parameters and READ NOTES C only with MiTek® connectors. This design	N THIS AND INCLUDED MITEK REFERENCE PAG n is based only upon parameters shown, and is fo	GE MII-7473 rev. 5/19/20 r an individual building o	20 BEFORE	E USE. not	ENGINEERING BY
a truss system. Befor building design. Brad	e use, the building designer must verify ing indicated is to prevent buckling of ir	the applicability of design parameters and properly dividual truss web and/or chord members only. A	y incorporate this design dditional temporary and	n into the ov	verall bracing	i kenlu

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 rev. 5/19/2/02/ BEFORE USE. Design valid for use only with MITeR connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply HOFFNER JOB	150142139			
2200084-2200084A	DE	GABLE	1	1	nal)			
84 Components (Dunn),	Dunn, NC - 28334,		8 ID:iKreZKbOsT	.530 s Dec 6 2021 MiTek Indus	tries, Inc. Wed Feb 9 06:55:30 2022 Page 1			
	, -1-6- 0 ,	8-7-1 5-0-4 7-4-1 7-11-13 12-4-0	16-0-15 16	17-3-15 -8-3 19-7-12 24-8-0	126-2-01			
	1-6-0	5-0-4 2-3-13 0-7-12 3-8-15	3-8-15 0-	7-4 2-3-13 5-0-4 0-7-12	1-6-0 Scale - 1:72 2			
		5x9 =	2x4 6 7	5x6 /\	State = 1.72.2			
	I	4x4 1/			I			
		4	27	4x4				
	12.00 12	_ 4x6 // 2x4		4x6 \\				
		3		9 2x4				
	م 2x4		4		x4 II 오			
		R	9-2.		4x6 ×			
	440 7				10			
	1		14-4-0		11			
	-			6 N 15 H				
			17	6x6 = 4x8				
	8 26	2x4 25 23 ²¹ 20	18	14 13 2x4				
	3x7 —	3x4 = 4x6 = 6x6 = 6x6 = 6x6 = 6x6	6x6 =	6x6 = 3x4 =	3x7 —			
		5-0-4 7-8-0 6x6 ±0-3-12	14-4-4 . 17-0-() 19-7-12 24-8-0				
Plate Offsets (X V) [2:0-1-8 0-2-0] [5:0-2-2 Edge] [5-0-4 2-7-12 2-7-12 7:0-2-2 Edge] [10:0-1-8 0-2-0]	4-0-8 2-7-12	2 2-7-12 5-0-4	—————			
	SPACING 2.0 (DEEL in					
TCLL 20.0	Plate Grip DOL 1.1	5 TC 0.48	Vert(LL) -0.24	17-19 >999 240	MT20 197/144			
BCLL 10.0 *	Rep Stress Incr YES	S BC 0.97 S WB 0.83	Horz(CT) -0.40	17-19 >718 180 12 n/a n/a				
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS	Attic -0.18	15-24 971 360	Weight: 300 lb FT = 20%			
LUMBER- TOP CHORD 2x6 SP	No.2 *Except*		BRACING- TOP CHORD	Structural wood sheathing d	irectly applied or 6-0-0 oc purlins,			
1-5,7-11 BOT CHORD 2x6 SP	: 2x6 SP DSS			except end verticals, and 2-0 Rigid ceiling directly applied	0-0 oc purlins (6-0-0 max.): 5-7.			
15-24: 2 WERS	2x4 SP No.2 or 2x4 SPF No.2		DOT ONORD	2-2-0 oc bracing: 18-20.				
3-25,9-1	13,4-8: 2x4 SP No.2 or 2x4 SPF	No.2, 2-26,10-12: 2x6 SP No.2	JOINTS	1 Brace at Jt(s): 27				
OTHERS 2x4 SP	N0.3							
REACTIONS. (size) Max Ho) 26=0-3-8, 12=0-3-8 prz 26=-357(LC 10)							
Max Gr	av 26=1780(LC 2), 12=1780(L	C 2)						
FORCES. (lb) - Max. (TOP CHORD 2-3=-1	Comp./Max. Ten All forces 25 467/0, 3-4=-954/158, 4-5=-221	0 (lb) or less except when shown. /440, 5-6=-7/776, 6-7=-7/776, 7-8=-22	21/440.					
8-9=-9 BOT CHORD 25-26-	054/158, 9-10=-1467/0, 2-26=-1	776/0, 10-12=-1776/0	2308					
13-14-	=0/882, 22-24=-1596/0, 19-22=	-2798/0, 17-19=-2798/0, 16-17=-2798	/0,					
WEBS 24-25:	=-1616/0 =-320/99, 3-24=-6/704, 13-15=-	326/107, 9-15=-6/704, 4-27=-1593/0,						
8-27≕ 20-22=	-1593/0, 2-25=0/1059, 10-13=0 =-48/1362, 17-18=-383/0, 16-18	/1061, 22-23=-722/0, 14-16=-722/0, 1 3=-55/1368, 14-15=0/1749, 23-24=0/1	9-20=-383/0, 749					
NOTES-					SAMULTITE.			
 Unbalanced roof live Wind: ASCE 7-10: Vt 	loads have been considered fo ult=130mph Vasd=103mph; TC	r this design. DL=6.0psf: BCDL=6.0psf: h=30ft: Cat.	II: Exp B: Enclosed	: MWFRS (envelope)	TH CARO			
gable end zone and (C-C Exterior(2) zone; end vertic	al left and right exposed;C-C for mem	bers and forces & M	IWFRS for reactions	O FESSION N			
3) Truss designed for w	ind loads in the plane of the true	ss only. For studs exposed to wind (n	ormal to the face), s	ee Standard Industry	South Servier			
4) Provide adequate dra	ainage to prevent water ponding				E SEAL			
6) Gable studs spaced a	at 2-0-0 oc.	1.			044925			
7) This truss has been c8) * This truss has been	designed for a 10.0 psf bottom of designed for a live load of 20.0	chord live load nonconcurrent with any opsf on the bottom chord in all areas w	v other live loads. vhere a rectangle 3-	6-0 tall by 2-0-0 wide	医风口 二人间			
will fit between the bo 9) Ceiling dead load (5.0	will fit between the bottom chord and any other members. 9) Ceiling dead load (5.0 pst) on member(s) 3-4 8-9 4-27. Wall dead load (10 0pst) on member(s) 3-24 9-15							
10) Bottom chord live lo	 a) Centing dead load (3.0 ps) of member(s). 3-4, o-3, 4-27, o-27, wail dead load (10.0ps) of member(s).3-24, 9-15 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 22-24, 19-22, 17-19, 16-17, 15 16 							
11) Bearing at joint(s) 26, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify								
capacity or bearing surface. February 9,2022 Clahtificed brade bageling representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.								
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.								
a truss system. Before building design. Bracin	use, the building designer must verify the g indicated is to prevent buckling of indi	e applicability of design parameters shown, and is to e applicability of design parameters and properl vidual truss web and/or chord members onlv. A	y incorporate this design dditional temporary and r	into the overall ermanent bracing	I RENCO			
is always required for s fabrication, storage, del	tability and to prevent collapse with pos livery, erection and bracing of trusses and valiable from Truss Plate Institute, 2020	sible personal injury and property damage. For ind truss systems, see ANSI/TPI1 Qual Crain Hindway, Suite 203 Waldorf, MD 20001	general guidance regardi ity Criteria, DSB-89 and	ng the BCSI Building Component	A Mi lek Affiliate 818 Soundside Road			
Salety mornation at	valiable from fruss frate fristitute, 2070	crain nighway, Suite 205 Waldon, MD 20001			Edenton, NC 27932			

Job	Truss	Truss Type	Qty	Ply	HOFFNER JOB	
						150142139
2200084-2200084A	DE	GABLE	1	1		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8	.530 s Dec	6 2021 MiTek Industries, Inc. Wed Feb 9 06:55:30 2022	Page 2

ID:jKreZKbOsTAmp?V0vfcf_yznEUQ-IFI1DBcfBkNmmdw6Sdj_264UTSnttUM2bY9DrMzmxyx

NOTES-

13) Attic room checked for L/360 deflection.





A MHek 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	HOFFNER JOB	
						150142140
2200084-2200084A	DG1	ATTIC GIRDER	2	2		
				5	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8	.530 s Dec	6 2021 MiTek Industries, Inc. Wed Feb 9 06:55:32 2022	Page 2
		ID:jKre	ZKbOsTA	mp?V0vfcf	yznEUQ-idPoetevjMdU0w4UZ1mS8X9h8GTJLSxL2seKwF	zmxyv

NOTES-

10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 22-24, 19-22, 17-19, 16-17, 15-16

Bearing at joint(s) 26, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 26=391, 12=391.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

14) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-4=-70, 4-5=-60, 5-7=-60, 7-8=-60, 8-9=-70, 9-10=-60, 10-11=-60, 12-26=-295(F=-275), 15-24=-30, 4-8=-10 Drag: 3-24=-20, 9-15=-20



Job	Truss	Truss Type		Qty	Ply	HOFFNER JOB		150142141
2200084-2200084A	DG2	ATTIC GIRDER		1	2	Job Reference (optiona	al)	
84 Components (Dunn),	Dunn, NC - 28334,		ID:iKreZ	KbOsTA	8.530 s Deo mp?V0vfcf	c 6 2021 MiTek Industrie vznEUQ-aPfJUFhQma7	es, Inc. Wed Feb 9 06:	55:36 2022 Page 1 IFHxzUdY30zmxvr
	L	8-7-1 5-0-4 _ 7-4-1 7-11-13 _ 1	2-4-0 16-	-0-15 16	17-3-15 -8 ₁ 3 19-7-	12 24-8-0	······································	
		5-0-4 2-3-13 0-7-12 3	-8-15 3-	8-15 0	-7-4 2-3-1 0-7-12	13 5-0-4	1	Scale = 1:75.5
		4x0 1/ 4x0 1/	2,74	_ 6	4x0 \\ 6 4x	8 🔨		
	Ţ	3x4 ℕ 3		× (7	3x4 1/	Ī	
	12.0	0 12 3x6 //	25					
		2	1.5x4)¢	3x6 \\ 8		
					-			
	-11-0 -		2-4				110	
	4x8 //		ல்		9	4x8	× 1	
					5-10		Т	
	12		14-4-0				- <u>2</u>	
			⊲ 7↓	5		3	9-14- 9-14- 10-14-	
			5x4 1 18 26	.5x4	_6x6 ⁻¹ =4x8			
	24 3x7 =	3x4 = 4x6 = 6	x6 =	6x6 =	6x6 =	3x4 = 3x7	=	
		6x6 = 6x6 =						
		5-0-4 7-8-0 10-3-12	, 12-4-0 , 14-4-4	4 17-0-	0 , 19-7-1	12 24-8-0		
Plate Offsets (X,Y) [4:0	0-2-2.Edge]. [6:0-2-2.Edge]	5-0-4 2-7-12 2-7-12	2-0-4 2-0-4	2-7-1	2 2-7-1	2 5-0-4	1	
	SBACING- 2-0-0	691	DEEL	i	n (loc)	l/defl L/d		GPIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.41	Vert(LL)	-0.1	9 15-17	>999 240	MT20	197/144
BCLL 0.0 *	Rep Stress Incr NC	WB 0.68	Horz(CT)) -0.30	2 10	>960 180 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS	Attic	-0.1	4 13-22	1226 360	Weight: 519 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP No	0.2 *Except*		BRACIN TOP CH	G- ORD	Structur	al wood sheathing dire	ctly applied or 6-0-0 o	c purlins,
1-4,6-9: 2x	x6 SP DSS SS *Except*		BOT CH	ORD	except e	end verticals, and 2-0-0) oc purlins (10-0-0 ma	ax.): 4-6.
13-22: 2x4 SP No	4 SP No.2 or 2x4 SPF No.2			one	5-0-0 oc	c bracing: 13-22		
2-23,8-11,	,3-7: 2x4 SP No.2 or 2x4 SPF	No.2, 1-24,9-10: 2x6 SP No.2	JUINTS		I DIACE	al JI(S). 25		
REACTIONS. (size)	24=0-3-8, 10=0-3-8							
Max Horz Max Grav	: 24=-318(LC 4) / 24=2133(LC 2), 10=2209(L0	C 16)						
FORCES. (Ib) - Max. Co	mp./Max. Ten All forces 250) (lb) or less except when shown						
TOP CHORD 1-2=-191	18/0, 2-3=-1173/66, 3-4=-27/7	20, 4-5=0/1209, 5-6=0/1209, 6-7	=-17/735,					
BOT CHORD 23-24=-3	307/352, 21-23=-91/1281, 18-	21=-27/3591, 16-18=0/6199, 12-	16=0/3839,					
13-14=-2	2760/122	J=-4893/0, 15-17=-4893/0, 14-15	=-4893/0,					
WEBS 22-23=-3 7-25=-24	301/130, 2-22=0/991, 11-13=- 420/26, 1-23=0/1406, 9-11=0/	384/149, 8-13=0/1050, 3-25=-24 1412, 20-21=-1312/47, 17-18=-4	20/26, 16/0,					
12-14=-1 18-20=-2	1224/36, 15-16=-315/0, 12-13 270/3196	=0/2921, 14-16=-221/2756, 21-2	2=0/2475,					
NOTES-								110.
1) 2-ply truss to be connected	cted together with 10d (0.148	x3") nails as follows:					TH CA	RO
Bottom chords connected	ed as follows: 2x6 - 2 rows stagg	aggered at 0-9-0 oc, 2x4 - 1 row a	at 0-9-0 oc.				(OF HESS	ip: No
2) All loads are considered	d equally applied to all plies, e	except if noted as front (F) or back	k (B) face in the	LOAD	CASE(S) s	section. Ply to	Start?	Sinne
ply connections have be3) Unbalanced roof live lost	een provided to distribute only ads have been considered for	Ioads noted as (F) or (B), unless this design.	s otherwise indi	cated.		1	SEA	
 Wind: ASCE 7-10; Vult= gable end zone; end ve 	=130mph Vasd=103mph; TCI ertical left and right exposed; L	DL=6.0psf; BCDL=6.0psf; h=30ft; .umber DOL=1.60 plate grip DOL	Cat. II; Exp B; .=1.60	Enclose	d; MWFRS	S (envelope)	0449	25
5) Provide adequate drain6) This truss has been des	age to prevent water ponding signed for a 10.0 psf bottom c	hord live load nonconcurrent with	any other live	loads.		-		1 E
 7) * This truss has been do will fit between the better 	esigned for a live load of 20.0	psf on the bottom chord in all are	as where a rec	tangle 3	-6-0 tall by	2-0-0 wide		EER
 8) Ceiling dead load (5.0 p 	osf) on member(s). 2-3, 7-8, 3	-25, 7-25; Wall dead load (10.0p	sf) on member	(s).2-22,	8-13		TTM	SEVILIN
13-14	(40.0 psi) and additional bolic			n. 20-22	, 17-20, 10	5-17, 14-15,	in min	mm
10) Bearing at joint(s) 24, Continueacity pageazing su	10 considers parallel to grain rface.	value using ANSI/TPI 1 angle to	grain formula.	Building	designer	should verify	Februa	ary 9,2022
WARNING - Verify desig	gn parameters and READ NOTES ON	THIS AND INCLUDED MITEK REFERENC	E PAGE MII-7473 re	ev. 5/19/20	20 BEFORE L	JSE.	ENGINEE	RING BY
Design valid for use only v a truss system. Before use	with MiTek® connectors. This design i e, the building designer must verify the	s based only upon parameters shown, and e applicability of design parameters and pu	d is for an individua	l building o this design	component, no into the over	ot rall	TRE	:NCO
building design. Bracing in is always required for stab	naicated is to prevent buckling of indi- ility and to prevent collapse with poss	ible personal injury and property damage.	ny. Additional temp For general guida	oorary and nce regard	permanent b ling the	racing		A MiTek Affiliate

fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	HOFFNER JOB	
						150142141
2200084-2200084A	DG2	ATTIC GIRDER	1	2		
				2	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8	.530 s Dec	6 2021 MiTek Industries, Inc. Wed Feb 9 06:55:36 2022	Page 2
		ID:jKre2	ZKbOsTAn	np?V0vfcf	vznEUQ-aPfJUFhQma7vUYNGotgOINKW tgrHFHxzUdY30	0zmxyr

NOTES-

- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the

responsibility of others. 13) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-70, 3-4=-60, 4-6=-60, 6-7=-60, 7-8=-70, 8-9=-60, 10-24=-20, 13-22=-30, 3-7=-10

Drag: 2-22=-20, 8-13=-20

Concentrated Loads (lb) Vert: 26=-571(F)





F	<u>8-3-13</u> 8-3-13		<u>16-4-3</u> 8-0-5			<u>24-8-0</u> 8-3-13	
Plate Offsets (X,Y) [2:0-2-0,0-1-12], [6:0-2-0,0-1-12]					0010	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.46 BC 0.73 WB 0.43 Matrix-MS	DEFL. ii Vert(LL) -0.12 Vert(CT) -0.25 Horz(CT) 0.06	n (loc) 2 8-9 5 8-9 6 8	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 129 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 22 BOT CHORD 22 WEBS 22	4 SP No.2 or 2x4 SPF No.2 4 SP No.2 or 2x4 SPF No.2 4 SP No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structur except e Rigid ce 1 Row a	al wood sheathing dira end verticals. eiling directly applied o at midpt 3-	ectly applied or 3-11-0 or 9-8-7 oc bracing. -12, 5-8	oc purlins,
REACTIONS. N N N	(size) 12=0-3-8, 8=0-3-8 ax Horz 12=-52(LC 17) ax Uplift 12=-202(LC 8), 8=-202(LC 9) ax Grav 12=1074(LC 1), 8=1074(LC 1)					,	
FORCES. (lb) - TOP CHORD	Max. Comp./Max. Ten All forces 250 (lb) o 2-3=-331/129, 3-4=-1633/429, 4-5=-1633/42 5-8=-371/206	r less except when shown. 9, 5-6=-331/129, 2-12=-371	1/206,				

BOT CHORD 11-12=-354/1610, 9-11=-199/1220, 8-9=-344/1610

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=202, 8=202.





WEBS 4-9=-64/467, 4-11=-64/467, 3-12=-1489/379, 5-8=-1489/379







1	8-3-13	1	16-4-3	1	24-8-0	1			
	8-3-13		8-0-5		8-3-13				
Plate Offsets (X,Y)	[2:0-2-0,0-1-12], [3:0-0-14,0-2-0], [4:0-2	-0,0-0-4], [5:0-0-14,0-2-0]	, [6:0-2-0,0-1-12], [9:0-2-	0,0-0-7], [11:0-2-0,0-1	1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.46 BC 0.73 WB 0.43 Matrix-MS	DEFL. in Vert(LL) -0.12 Vert(CT) -0.25 Horz(CT) 0.06	(loc) l/defl L 8-9 >999 24 8-9 >999 18 8 n/a n	/d PLATES 40 MT20 80 //a Weight: 167 lb	GRIP 197/144 FT = 20%			
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF OTHERS 2x4 SF	P No.2 or 2x4 SPF No.2 No.2 or 2x4 SPF No.2 No.3 P No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood she except end verticals Rigid ceiling directly 1 Row at midpt	athing directly applied or 3-11-0 applied or 9-8-7 oc bracing. 3-12, 5-8) oc purlins,			
REACTIONS. (size) 12=0-3-8, 8=0-3-8 Max Horz 12=-52(LC 17) Max Uplift 12=-202(LC 8), 8=-202(LC 9) Max Grav 12=1074(LC 1), 8=1074(LC 1)									
FORCES. (lb) - Max. TOP CHORD 2-3=- 6-8=- BOT CHORD 11-12 WEBS 4-9=-	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-331/129, 3-4=-1633/429, 4-5=-1633/429, 5-6=-331/129, 2-12=-371/206, 6-8=-371/206 BOT CHORD 11-12=-354/1610, 9-11=-199/1220, 8-9=-344/1610 WEBS 4-9=-64/467, 4-11=-64/467, 3-12=-1489/379, 5-8=-1489/379								
 BOT CHORD 11-12=-354/1610, 9-11=-199/1220, 8-9=-344/1610 WEBS 4-9=-64/467, 4-11=-64/467, 3-12=-1489/379, 5-8=-1489/379 NOTES- Uhbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. All plates are 1.5x4 MT20 unless otherwise indicated. Gable studs spaced at 2-0-0 oc. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=202, 8=202. 									







	-	3-4-9	+	6-7-7				10-0-0	
		3-4-9		3-2-13				3-4-9	,
LOADING ((psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 4	40.0	Plate Grip DOL 1.00	TC 0.25	Vert(LL) -0.05	6-7	>999	480	MT20	197/144
TCDL 1	10.0	Lumber DOL 1.00	BC 0.42	Vert(CT) -0.07	6-7	>999	360		
BCLL	0.0	Rep Stress Incr YES	WB 0.53	Horz(CT) 0.01	5	n/a	n/a		
BCDL	5.0	Code IRC2015/TPI2014	Matrix-MS					Weight: 48 lb	FT = 20%
LUMBER-		· · · · · ·	·	BRACING-				•	

BOT CHORD

UMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.3 WEBS

REACTIONS. 8=Mechanical, 5=Mechanical (size) Max Grav 8=534(LC 1), 5=534(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-8=-502/0, 1-2=-1143/0, 2-3=-1143/0 TOP CHORD

BOT CHORD 6-7=0/1145, 5-6=0/1145

WEBS 1-7=0/1118, 2-7=-326/0, 3-5=-1122/0

NOTES-

1) Refer to girder(s) for truss to truss connections.

2) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

- The Andrews and the Andrews 044925 S mmm February 9,2022

2-0-0 oc purlins (5-2-0 max.): 1-4, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.





	3-10-15 3-10-15	7-8-1 3-9-3				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 DOD 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. DEFL. TC 0.37 Vert(LL) BC 0.56 Vert(CT) WB 0.71 Horz(CT)	in (loc) -0.09 6-7 -0.13 6-7 0.02 5	l/defl L/d >999 480 >999 360 n/a n/a	PLATES GF MT20 19	RIP 7/144
LUMBER-	Code IRC2015/1PI2014	BRACING-			weight: 55 b	FT = 20%

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.3 WEBS

REACTIONS. 8=Mechanical, 5=0-3-8 (size) Max Grav 8=621(LC 1), 5=621(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-8=-584/0, 1-2=-1547/0, 2-3=-1547/0 TOP CHORD

BOT CHORD 6-7=0/1549, 5-6=0/1549

WEBS 1-7=0/1495, 2-7=-379/0, 3-5=-1498/0

NOTES-

1) Refer to girder(s) for truss to truss connections.

2) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



2-0-0 oc purlins (4-3-1 max.): 1-4, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.





		3-10-15 3-10-15	+	7-8-1 3-9-3					11-7-0 3-10-15	
LOADI TCLL TCDL BCLL BCDL	NG (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.33 BC 0.05 WB 0.09 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 55 lb	GRIP 197/144 FT = 20%
	D_	· · ·	L. L	BRACING						

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

TOP CHORD BOT CHORD

2-0-0 oc purlins (6-0-0 max.): 1-4, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5 except 7=454(LC 1), 6=438(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-7=-398/0, 3-6=-399/0

NOTES-

1) Gable requires continuous bottom chord bearing.

2) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.







BOT CHORD

2-0-0 oc purlins: 1-2, except end verticals.

Rigid ceiling directly applied or 2-6-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.3 WEBS

2x4 SP No.2 or 2x4 SPF No.2

REACTIONS. 4=Mechanical, 3=0-3-8 (size)

Max Grav 4=572(LC 1), 3=1089(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) 2-ply truss to be connected together with 10d (0.148"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-5-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Refer to girder(s) for truss to truss connections.

4) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 611 lb down at 1-9-12, and 615 Ib down at 4-1-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 1-2=-100, 3-4=-10

Concentrated Loads (lb) Vert: 3=-615(B) 5=-611(B)







	<u>3-4-9</u> 3-4-9	6-7-7 3-2-13			<u>10-0-0</u> 3-4-9	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. DEFL. TC 0.25 Vert(LL) BC 0.03 Vert(CT) WB 0.08 Horz(CT) Matrix-S Horz(CT)	in (loc) n/a - n/a - 0.00 5) l/defl L/d - n/a 999 - n/a 999 5 n/a n/a	PLATES MT20 Weight: 48 lb	GRIP 197/144 FT = 20%
LUMBER.	· · · · ·	BRACING-				

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

TOP CHORD BOT CHORD

2-0-0 oc purlins (6-0-0 max.): 1-4, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-0-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5 except 7=394(LC 1), 6=372(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-7=-343/0, 3-6=-339/0

NOTES-

1) Gable requires continuous bottom chord bearing.

2) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.







- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 6 and 47 lb uplift at joint 8.







Plate Offsets (X,Y) [6:0-4-0),0-4-8]						
LOADING (psf) S TCLL 20.0 F TCDL 10.0 L BCLL 0.0 * F BCDL 10.0 C	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.74 BC 0.57 WB 0.46 Matrix-MS	DEFL. ii Vert(LL) -0.08 Vert(CT) -0.16 Horz(CT) 0.00	n (loc) l/defl 3 6-7 >999 5 6-7 >867 0 5 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 180 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 o BOT CHORD 2x6 SP DSS WEBS 2x4 SP No.3 *I 1-7: 2x4 SP No	r 2x4 SPF No.2 Except* 5.2 or 2x4 SPF No.2		BRACING- TOP CHORD BOT CHORD	Structural wood except end ver Rigid ceiling di	d sheathing directicals. rectly applied or	ctly applied or 6-0-0 o	oc purlins,
REACTIONS. (size) 5=0 Max Horz 7=- Max Uplift 5=- Max Grav 5=1	0-3-8, 7=0-3-8 201(LC 4) 324(LC 9), 7=-465(LC 9) 1713(LC 1), 7=3134(LC 1)						
FORCES. (lb) - Max. Comp.// TOP CHORD 1-2=-1904/42 BOT CHORD 6-7=-235/561 WEBS 2-6=-458/220	Max. Ten All forces 250 (lb) or 43, 2-3=-1911/442, 3-4=-44/376, 1 07, 3-6=-321/1461, 1-6=-268/872	less except when shown. 1-7=-1647/363 , 3-5=-2192/500					
 NOTES- 1) 2-ply truss to be connected as for Bottom chords connected as for Bottom chords connected as Webs connected as follows: 2) All loads are considered equiply connections have been ply connect	together with 10d (0.148"x3") na llows: 2x4 - 1 row at 0-9-0 oc. s follows: 2x6 - 2 rows staggered : 2x4 - 1 row at 0-9-0 oc. : ally applied to all plies, except if provided to distribute only loads in have been considered for this de imph Vasd=103mph; TCDL=6.0p I left and right exposed; Lumber ed for a 10.0 psf bottom chord livu- hed for a live load of 20.0psf on the hord and any other members. I'rs parallel to grain value using A tion (by others) of truss to bearin on device(s) shall be provided su and 142 lb up at 3-11-12, and 83 innection device(s) is the respons	Is as follows: at 0-7-0 oc. noted as front (F) or back hoted as (F) or (B), unless sign. sf; BCDL=6.0psf; h=30ft; i DOL=1.60 plate grip DOL= e load nonconcurrent with he bottom chord in all area NSI/TPI 1 angle to grain for g plate capable of withstar fficient to support concent 38 lb down and 435 lb up a ibility of others.	(B) face in the LOAD (otherwise indicated. Cat. II; Exp B; Enclosed =1.60 any other live loads. as where a rectangle 3- prmula. Building design nding 324 lb uplift at joi trated load(s) 1531 lb d at 5-11-0 on bottom ch	CASE(S) section. d; MWFRS (enve 6-0 tall by 2-0-0 v ner should verify nt 5 and 465 lb u own and 142 lb u ord. The	Ply to lope) wide blift at p at	SE 0449 Febru	AL 925 VEEP. HEAL ary 9,2022
LOAD CASE(S) Standard WARNING - Verify design par Design valid for use only with M a truss system. Before use, the building design. Bracing indicat is always required for stability an fabrication, storage, delivery, en Safety Information available fi	ameters and READ NOTES ON THIS AND ITek® connectors. This design is based of building designer must verify the applicab ed is to prevent buckling of individual trus nd to prevent collapse with possible persc ection and bracing of trusses and truss sy rom Truss Plate Institute, 2670 Crain Higt	INCLUDED MITEK REFERENCE nly upon parameters shown, and ility of design parameters and pro sweb and/or chord members onl nal injury and property damage. stems, see ANS/ITPI1 way, Suite 203 Waldorf, MD 206	PAGE MII-7473 rev. 5/19/202 is for an individual building o operly incorporate this design y. Additional temporary and For general guidance regard Quality Criteria, DSB-89 and 01	20 BEFORE USE. omponent, not into the overall permanent bracing ing the I BCSI Building Corr	ponent	818 Soundside	ERING BY A MITEK Affiliate P Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	HOFFNER JOB	
						150142150
2200084-2200084A	G1	Common Girder	1	2		
				_	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8	.530 s Dec	6 2021 MiTek Industries, Inc. Wed Feb 9 06:55:44 2022	Page 2

ID:jKreZKbOsTAmp?V0vfcf_yznEUQ-Lx8K9_nRu18nSm?oGZzGd3fpu5gO9vi6pjZzLYzmxyj

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-4=-60, 4-7=-20 Concentrated Loads (lb)

Vert: 6=-742(B) 10=-1531(B) 11=-1531(B)









1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 8.











Plate Offsets (X,Y) [4:Edge,0-3-8], [5:0-4-0,0-4-4]			
LOADING (psf) SPACING- 2-0-0 TCLL 20.0 Plate Grip DOL 1.15 TCDL 10.0 Lumber DOL 1.15 BCLL 0.0 * Rep Stress Incr NO BCDL 10.0 Code IRC2015/TPI2014 100	CSI. DEFL. in TC 0.54 Vert(LL) -0.03 BC 0.62 Vert(CT) -0.07 WB 0.40 Horz(CT) 0.00 Matrix-MS Horz(CT) 0.00	(loc) I/defl L/d PLATES 5-6 >999 240 MT20 5-6 >999 180 4 n/a n/a Weight: 168 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 REACTIONS. (size) 6=0-3-8, 4=0-3-8 Max Horz 6=198(LC 5) Max Uplift 6=-107(LC 9), 4=-111(LC 8) Max Grav 6=2080(LC 1), 4=2186(LC 1)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 6-0-0 oc except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.	purlins,
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less e TOP CHORD 1-2=-1709/157, 2-3=-1709/157, 1-6=-1534/115, 3-4 BOT CHORD 5-6=-222/353 WEBS 2-5=-35/1914, 1-5=-94/933, 3-5=-96/927	except when shown. I≕-1532/115		
 NOTES- 1) 2-ply truss to be connected together with 10d (0.148"x3") nails as a Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-5 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted ply connections have been provided to distribute only loads noted 3) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BC gable end zone; end vertical left and right exposed; Lumber DOL= 5) This truss has been designed for a 10.0 psf bottom chord live load 6) * This truss has been designed for a live load of 20.0psf on the bot will fit between the bottom chord any other members. 7) Provide mechanical connection (by others) of truss to bearing platt 6=107, 4=111. 8) Hanger(s) or other connection device(s) shall be provided sufficier 2-1-12, 669 lb down and 33 lb up at 4-1-12, 669 lb down and 33 lb up at 10-1-12 on bottom chord. The design/sel LOAD CASE(S) Standard 	follows: 9-0 oc. d as front (F) or back (B) face in the LOAD C/ as (F) or (B), unless otherwise indicated. CDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; 1.60 plate grip DOL=1.60 nonconcurrent with any other live loads. ttom chord in all areas where a rectangle 3-6 e capable of withstanding 100 lb uplift at join at to support concentrated load(s) 669 lb dow b up at 6-1-12, and 669 lb down and 33 lb up ection of such connection device(s) is the res	ASE(S) section. Ply to ; MWFRS (envelope) S-0 tall by 2-0-0 wide tt(s) except (jt=lb) vn and 33 lb up at p at 8-1-12, and 669 sponsibility of others.	ROL LEEP. HEILING

LOAD CASE(S) Standard

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



February 9,2022

Job	Truss	Truss Type	Qty	Ply	HOFFNER JOB	
						I50142154
2200084-2200084A	HG	Common Girder	1	2		
				_	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8	.530 s Dec	6 2021 MiTek Industries, Inc. Wed Feb 9 06:55:48 2022	2 Page 2

ID:jKreZKbOsTAmp?V0vfcf_yznEUQ-EiNr?LqxyGeCwOIZVP2CnvpXyi0T5kdikLXAUKzmxyf

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 4-6=-20

Concentrated Loads (lb)

Vert: 5=-669(B) 7=-669(B) 8=-669(B) 9=-669(B) 10=-669(B)



Job	Truss	Truss Type	Qty	Ply	HOFFNER JOB		150140455
2200084-2200084A	M1	Monopitch Supported Gable	1	1	1		150 142 155
84 Components (Dunn).	Dunn. NC - 28334.			3.530 s De	Job Reference (option ac 6 2021 MiTek Industr	al) ies. Inc. Wed Feb 9.0	6:55:48 2022 Page 1
er componente (2 ann),	20001,		ID:jKreZKbOsT	Amp?V0v	/fcf_yznEUQ-EiNr?Lqxy(GeCwOIZVP2CnvpWxi	035pEikLXAUKzmxyf
			7-3-8 7-3-8				
							Capita 4:45.4
			3x4	5			Scale = 1:45.4
		12.00 12 1.5x4 1.5x4 1.5x4 2 1.5x4 2 2x4 // 9 8	1.5x4 4 8 7	5			
		1.5x4 1.5x	4 1.5x4 7-3-8	3x4	1		
			7-3-8				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.11 Lumber DOL 1.11 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. 5 TC 0.60 5 BC 0.58 5 WB 0.10 Matrix-S	DEFL. i Vert(LL) n/ Vert(CT) n/ Horz(CT) -0.00	n (loc) a - a -) 6	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 51 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SP N BOT CHORD 2x4 SP N WEBS 2x4 SP N OTHERS 2x4 SP N	0.2 or 2x4 SPF No.2 0.2 or 2x4 SPF No.2 0.3 0.3		BRACING- TOP CHORD BOT CHORD	Structu except Rigid c	ural wood sheathing dir end verticals. ceiling directly applied o	ectly applied or 6-0-0 r 6-0-0 oc bracing.	oc purlins,
REACTIONS. All bear (Ib) - Max Hor: Max Upli Max Grav	ings 5-9-8. z 9=269(LC 9) ft All uplift 100 lb or less at jo v All reactions 250 lb or less	bint(s) 7 except 6=-106(LC 11), 8=-3 at joint(s) 6, 7, 8 except 9=456(LC	344(LC 12), 9=-106(L 20)	C 8)			
FORCES. (lb) - Max. Cd TOP CHORD 1-2=-39 BOT CHORD 1-9=-32 WEBS 3-8=-28	omp./Max. Ten All forces 25 4/365, 2-3=-505/443, 3-4=-27 3/372 9/309	0 (Ib) or less except when shown. 2/266					
NOTES- 1) Wind: ASCE 7-10; Vult gable end zone and C- shown; Lumber DOL= ² 2) Truss designed for win Gable End Details as a 3) Gable studs spaced at 4) This trunch here been defined	=130mph Vasd=103mph; TC C Exterior(2) zone; end vertic .60 plate grip DOL=1.60 d loads in the plane of the true pplicable, or consult qualified 2-0-0 oc.	DL=6.0psf; BCDL=6.0psf; h=30ft; C al left and right exposed;C-C for me ss only. For studs exposed to wind building designer as per ANSI/TPI	at. II; Exp B; Enclose embers and forces & I (normal to the face), 1.	d; MWFR MWFRS f see Stan	S (envelope) for reactions dard Industry		

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 6=106, 8=344, 9=106.

7) Non Standard bearing condition. Review required.





Job	Truss	Truss Type		Qty	/	Ply	HOFF	NER JOB		1504 404 50
2200084-2200084A	M2	Monopitch		4		1				150142156
							Job Re	eference (optional)		
84 Components (Dunn),	Dunn, NC - 28334,				8.	.530 s De	ec 6 2021	1 MiTek Industries,	Inc. Wed Feb 9 (06:55:49 2022 Page 1
			1-7-12	ID:JKreZKbC 7-3-8	JSIAn	np?vuvrc	f_yznEU0	Q-IVXDDnrZjam3Y/	Ktm36ZRK/MCN6F	RFqDkry?Gk0mzmxye
			1-7-12	5-7-12						
										0
					1.5x4	11				Scale = 1:45.3
		-				3				
					/					
					//					
			10.00 4	- /						
			12.00 12	/	/					
		٢.								
		-7-7								
			240 4							
			3X0 //							
			2							
				<u> </u>						
			. //		_					
		15								
		ļÄ								
		0				4				
			3x4 // 15x4 II			3×4 —				
			1.074 11			0,4 -				
				7.0.0						
			1-6-0 0-1-12	5-7-12						
Plate Offsets (X,Y) [1	:0-2-0,0-1-3]									
	SPACING. 2	0-0	C 51	DEEL	in	(loc)	l/defl	L/d		CPIP
TCLL 20.0	Plate Grip DOL 1	.15	TC 0.94	Vert(LL)	-0.03	4-5	>999	240	MT20	197/144
TCDL 10.0	Lumber DOL 1	.15	BC 0.27	Vert(CT)	-0.06	4-5	>999	180		
BCLL 0.0 *	Rep Stress Incr Y	ES	WB 0.28	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI20	14	Matrix-MP						Weight: 47 lb	FT = 20%
LUMBER-				BRACING-				i		
TOP CHORD 2x4 SP N	lo.2 or 2x4 SPF No.2			TOP CHORE	C	Structu	ral wood	I sheathing direct	y applied or 6-0-0) oc purlins,
BOT CHORD 2x4 SP N	lo.2 or 2x4 SPF No.2					except	end vert	icals.		•
WEBS 2x4 SP N	lo.3			BOT CHORE	D	Rigid c	eiling dir	ectly applied or 8	-2-1 oc bracing.	
	5-0-3-8 4-Machanical									
Max Hor	5=0-5-6, $4=1000000000000000000000000000000000000$									
Max Upl	ft 4=-169(LC 9)									
Max Gra	v 5=416(LC 20), 4=282(LC	19)								
	44 T 4 0 <i>1</i>	050 (11)								
TOP CHORD 2 2 2	omp./Max. Len All forces	250 (Ib) or less e	except when shown							
BOT CHORD 4-5=-51	3/523									

WEBS 2-5=-307/200, 2-4=-445/446

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=169.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MILIEK Affilia 818 Soundside Road Edenton, NC 27932



	1-6-0 0-1-12	3-9-4	
LOADING (psf) SPACING- 2-0-0 TCLL 20.0 Plate Grip DOL 1.15 TCDL 10.0 Lumber DOL 1.15 BCLL 0.0 * Rep Stress Incr YES BCDL 10.0 Code IRC2015/TPI2014 YES	CSI. TC 0.27 BC 0.10 WB 0.07 Matrix-MP	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 4-5 >999 240 Vert(CT) -0.01 4-5 >999 180 Horz(CT) 0.00 5 n/a n/a	PLATES GRIP MT20 197/144 Weight: 30 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

REACTIONS. (size) 4=0-1-8, 5=0-3-8 Max Horz 4=149(LC 9) Max Uplift 4=-78(LC 9), 5=-13(LC 12)

Max Uplift 4=-78(LC 9), 5=-13(LC 12) Max Grav 4=153(LC 19), 5=307(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5.



Structural wood sheathing directly applied or 5-5-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.





			100 0112		
LOADING	(psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.27	Vert(LL) 0.02 4-5 >999 240 MT20 197/144	
TCDL	10.0	Lumber DOL 1.15	BC 0.30	Vert(CT) -0.02 4-5 >999 180	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT) 0.00 4 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-MP	Weight: 25 lb FT = 209	6

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

REACTIONS. (size) 5=0-3-8, 4=0-1-8 Max Horz 5=149(LC 9) Max Uplift 5=-13(LC 12), 4=-78(LC 9) Max Grav 5=307(LC 1), 4=153(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-5=-254/174

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.



Structural wood sheathing directly applied or 5-5-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





WEBS OTHERS 2x4 SP No.3

REACTIONS. All bearings 6-4-8.

Max Horz 1=-84(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1 except 10=-105(LC 12), 8=-102(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 7, 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 1, 7, 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 10=105.8=102.

8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.





¹⁾ Unbalanced roof live loads have been considered for this design.



REACTIONS. All bearings 6-4-8.

(lb) - Max Horz 1=-84(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) except 1=-170(LC 19), 5=-134(LC 20), 2=-222(LC 12), 4=-200(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 6 except 2=346(LC 19), 4=322(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 1, 5, 2, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 1, 134 lb uplift at joint 5, 222 lb uplift at joint 2 and 200 lb uplift at joint 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.





¹⁾ Unbalanced roof live loads have been considered for this design.



Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	HOFFNER JOB	
						150142161
2200084-2200084A	PB3	Piggyback	1	2		
				2	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8	.530 s Dec	6 2021 MiTek Industries, Inc. Wed Feb 9 06:55:58 2022	Page 2

ID:jKreZKbOsTAmp?V0vfcf_yznEUQ-xd_d6myDbLvo7w3U5VEYB0EN7kZ9RLzA1vyiqlzmxyV

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-83, 2-3=-60, 3-4=-60, 4-5=-83, 2-4=-20 Concentrated Loads (Ib)

Vert: 3=-2880(F)





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 1.5x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 14, 10, 13, 11.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

¹⁾ Unbalanced roof live loads have been considered for this design.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932









February 9,2022

ENGINEERING BY **TREENCO** A MITek Affiliate 818 Soundside Road Edenton, NC 27932





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MITek Affiliat 818 Soundside Road

Edenton, NC 27932



REACTIONS. (size) 2=7-3-5, 4=7-3-5, 6=7-3-5 Max Horz 2=-75(LC 10) Max Uplift 2=-45(LC 12), 4=-55(LC 13)

Max Uplift 2=-45(LC 12), 4=-55(LC 13) Max Grav 2=189(LC 1), 4=189(LC 1), 6=254(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







			8-7-11						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.05 BC 0.03 WB 0.03 Matrix-P	DEFL. ir Vert(LL) 0.00 Vert(CT) 0.00 Horz(CT) 0.00	n (loc)) 6) 6) 6	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 34 lb	GRIP 197/144 FT = 20%	
LUMBER-		induix i	BRACING-				Wolght. 0 His	11 - 20/0	

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. All bearings 7-3-5. (lb) -

Max Horz 2=-75(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 8 Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 8.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



¹⁾ Unbalanced roof live loads have been considered for this design.



3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=207.6=207.







REACTIONS. (size) 1=8-11-0, 3=8-11-0, 4=8-11-0 Max Horz 1=-102(LC 8) Max Uplift 1=-49(LC 13), 3=-49(LC 13) Max Grav 1=197(LC 1), 3=197(LC 1), 4=266(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







REACTIONS. (size) 1=5-11-0, 3=5-11-0, 4=5-11-0 Max Horz 1=-65(LC 8) Max Uplift 1=-31(LC 13), 3=-31(LC 13) Max Grav 1=126(LC 1), 3=126(LC 1), 4=169(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







REACTIONS. (size) 1=2-11-0, 3=2-11-0 Max Horz 1=28(LC 9) Max Uplift 1=-7(LC 12), 3=-7(LC 13) Max Grav 1=90(LC 1), 3=90(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932









REACTIONS. (size) 1=6-9-12, 3=6-9-12, 4=6-9-12 Max Horz 1=-76(LC 8) Max Uplift 1=-36(LC 13), 3=-36(LC 13) Max Grav 1=147(LC 1), 3=147(LC 1), 4=198(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







				5-10-0				0-0-4		
LOADING	i (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.04	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.01	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 14 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 2x4 SP No.3

OTHERS

REACTIONS. 1=3-9-12, 3=3-9-12, 4=3-9-12 (size) Max Horz 1=-39(LC 8) Max Uplift 1=-19(LC 13), 3=-19(LC 13) Max Grav 1=75(LC 1), 3=75(LC 1), 4=101(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 3-10-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.48 BC 0.20 WB 0.09 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 29 lb FT = 20%

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.3

 BOT CHORD
 2x4 SP No.3

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

REACTIONS. (size) 1=5-7-12, 4=5-7-12, 5=5-7-12

Max Horz 1=195(LC 9) Max Uplift 1=-125(LC 10), 4=-72(LC 9), 5=-215(LC 12)

Max Grav 1=173(LC 12), 4=159(LC 19), 5=367(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-422/391 WEBS 2-5=-349/293

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 1=125, 5=215.



Structural wood sheathing directly applied or 5-8-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



LOADING (ps TCLL 20. TCDL 10. BCLL 0. BCDL 10.	sf)).0).0).0 *).0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES VI2014	CSI. TC BC WB Matrix	0.51 0.27 0.00 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 19 lb	GRIP 244/190 FT = 20%	
LUMBER-						BRACING-							

BOT CHORD

TOP CHORD 2x4 SP No.3

BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3

REACTIONS. (size) 1=4-1-12, 3=4-1-12 Max Horz 1=138(LC 9) Max Uplift 3=-65(LC 9)

Max Grav 1=165(LC 20), 3=179(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.



Structural wood sheathing directly applied or 4-2-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.17 BC 0.09 WB 0.00 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 12 lb FT = 20%
LUMBER-			BRACING-	

BOT CHORD

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS. (size) 1=2-7-12, 3=2-7-12 Max Horz 1=82(LC 9) Max Uplift 3=-39(LC 9) Max Grav 1=98(LC 20), 3=106(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.



Structural wood sheathing directly applied or 2-8-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MiTek Affilia 818 Soundside Road Edenton, NC 27932

