

RE: Winston B Vault Master Winston B Vault Master Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: D.R. HORTON - RAL - 055Project Name: Winston B Vault Master<br/>Model: WINSTON / B Vault MasterAddress:Subdivision:<br/>State: NC

## General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	l47933871	А	9/17/2021
2	147933872	A1	9/17/2021
3	147933873	A2	9/17/2021
4	147933874	A3	9/17/2021
5	147933875	A4	9/17/2021
6	147933876	A4A	9/17/2021
7	147933877	A5	9/17/2021
8	147933878	A5E	9/17/2021
9	147933879	AE	9/17/2021
10	147933880	В	9/17/2021
11	l47933881	BE	9/17/2021
12	147933882	BGR	9/17/2021
13	147933883	С	9/17/2021
14	147933884	CE	9/17/2021
15	147933885	D	9/17/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by 84 Components - #2383.

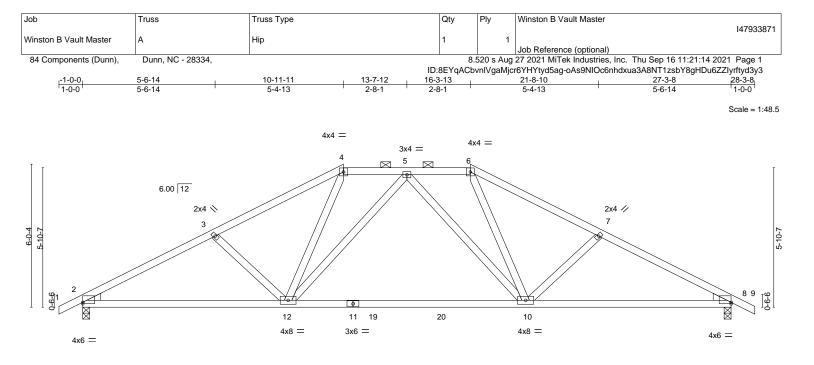
Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





	<u>8-7-12</u> 8-7-12		<u>18-7-12</u> 10-0-0			27-3-8 8-7-12	
Plate Offsets (X,Y)-	- [2:0-0-0,0-0-12], [8:Edge,0-0-12]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.48 BC 0.98 WB 0.26 Matrix-MS	Vert(CT) -0	in (loc) l/defl 0.31 10-12 >999 0.57 10-12 >571 0.07 8 n/a	L/d 240 180 n/a	<b>PLATES</b> MT20 Weight: 141 lb	<b>GRIP</b> 197/144 FT = 20%
BOT CHORD 2x4 WEBS 2x4 WEDGE	SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2 SP No.3 Right: 2x4 SP No.3		BRACING- TOP CHORD BOT CHORD	2-0-0 oc purlins	(5-2-0 max.): 4	ctly applied or 3-9-8 d 1-6. 2-2-0 oc bracing.	oc purlins, except
Ma Ma	size) 2=0-3-8, 8=0-3-8 x Horz 2=99(LC 12) x Uplift 2=-130(LC 12), 8=-130(LC 13) x Grav 2=1152(LC 1), 8=1152(LC 1)						
	ax. Comp./Max. Ten All forces 250 (lb) o		l.				

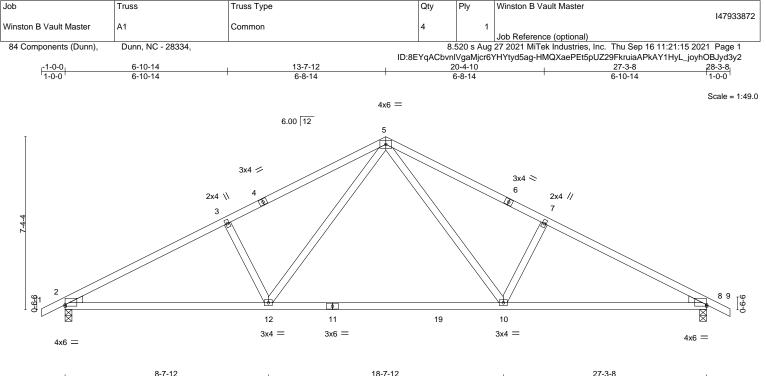
- TOP CHORD 2-3=-1909/468, 3-4=-1668/422, 4-5=-1239/394, 5-6=-1239/395, 6-7=-1668/422, 7-8=-1909/468
- BOT CHORD 2-12=-322/1638, 10-12=-171/1308, 8-10=-326/1638
- WEBS 3-12=-294/215, 4-12=-61/518, 6-10=-61/518, 7-10=-294/215

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=130, 8=130.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	-0.39 10-12	>840	240	MT20	197/144
CDL 10.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.62 10-12	>530	180		
CLL 0.0	Rep Stress Incr	YES	WB	0.28	Horz(CT)	0.06 8	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matrix-	MS					Weight: 128 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 2-2-0 oc bracing.

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 \*Except\* 8-11: 2x4 SP No.1 WEBS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8.8=0-3-8 Max Horz 2=122(LC 12) Max Uplift 2=-149(LC 12), 8=-149(LC 13) Max Grav 2=1152(LC 1), 8=1152(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1872/467, 3-5=-1694/505, 5-7=-1695/504, 7-8=-1873/467 BOT CHORD 2-12=-308/1595, 10-12=-105/1056, 8-10=-310/1596

WEBS 5-10=-152/681, 7-10=-392/266, 5-12=-153/677, 3-12=-393/265

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

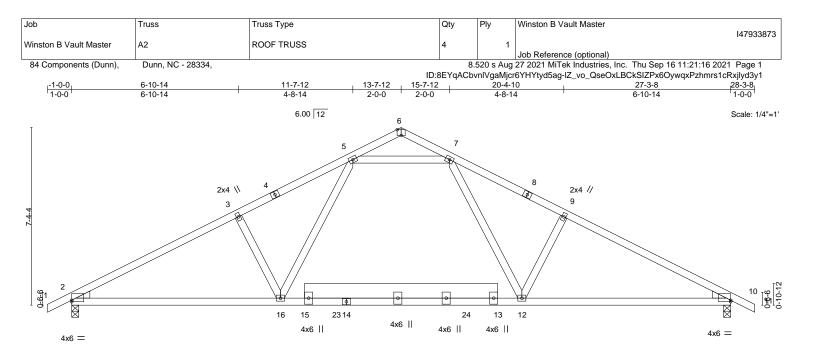
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=149, 8=149.

# ATTENDED IN SEAL 44925 mm September 17,2021



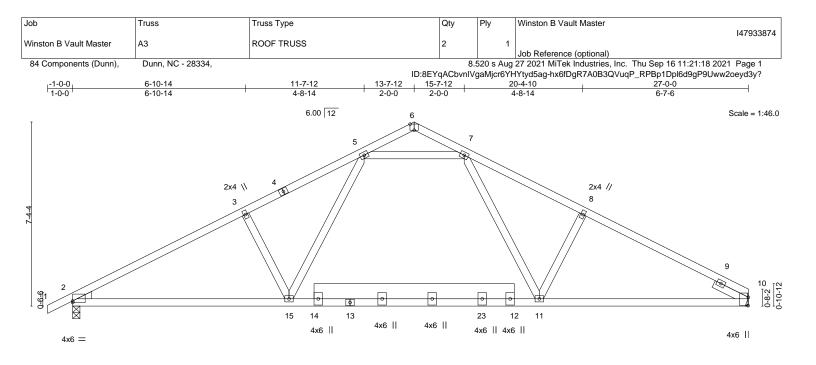


8-7-12	<u>18-7-12</u> 10-0-0	27-3-8 8-7-12
Plate Offsets (X,Y) [2:0-0-0,0-0-12], [6:0-2-0,Edge], [10:0-0-		07712
LOADING (psf)         SPACING-         2-0-0           TCLL         20.0         Plate Grip DOL         1.15           TCDL         10.0         Lumber DOL         1.15           BCLL         0.0 *         Rep Stress Incr         YES           BCDL         10.0         Code IRC2015/TPI2014	CSI.         DEFL.         in         (           TC         0.56         Vert(LL)         -0.20         16           BC         0.78         Vert(CT)         -0.31         16           WB         0.44         Horz(CT)         0.05           Matrix-MS          0.05         0.05	6-19 >999 240 MT20 197/144
LUMBER- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* 13-15: 2x8 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3 , Right: 2x4 SP No.3		f Structural wood sheathing directly applied or 3-8-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.
REACTIONS.         (size)         2=0-3-8, 10=0-3-8           Max Horz         2=122(LC 12)           Max Uplift         2=-149(LC 12), 10=-149(LC 13)           Max Grav         2=1152(LC 1), 10=-152(LC 1)           FORCES.         (lb) - Max. Comp./Max. Ten All forces 250 (lb) or I           TOP CHORD         2-3=-1846/462, 3-5=-1668/489, 7-9=-1668/489           BOT CHORD         2-16=-299/1568, 12-16=-157/1191, 10-12=-30	9, 9-10=-1846/462	
<ul> <li>WEBS 7-12=-103/596, 9-12=-394/235, 5-16=-103/596</li> <li>NOTES- <ol> <li>Unbalanced roof live loads have been considered for this des</li> <li>Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0pg gable end zone and C-C Exterior(2) zone;C-C for members a DOL=1.60</li> <li>All plates are 3x4 MT20 unless otherwise indicated.</li> <li>This truss has been designed for a 10.0 psf bottom chord live 5) * This truss has been designed for a load of 20.0psf on th will fit between the bottom chord and any other members, wit</li> <li>Provide mechanical connection (by others) of truss to bearing 2=149, 10=149.</li> <li>ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE</li> </ol> </li> </ul>	6, 3-16=-394/235, 5-7=-1123/424 sign. sf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; M and forces & MWFRS for reactions shown; Lumber D be load nonconcurrent with any other live loads. he bottom chord in all areas where a rectangle 3-6-0 th BCDL = 10.0psf. g plate capable of withstanding 100 lb uplift at joint(s)	DoL=1.60 plate grip

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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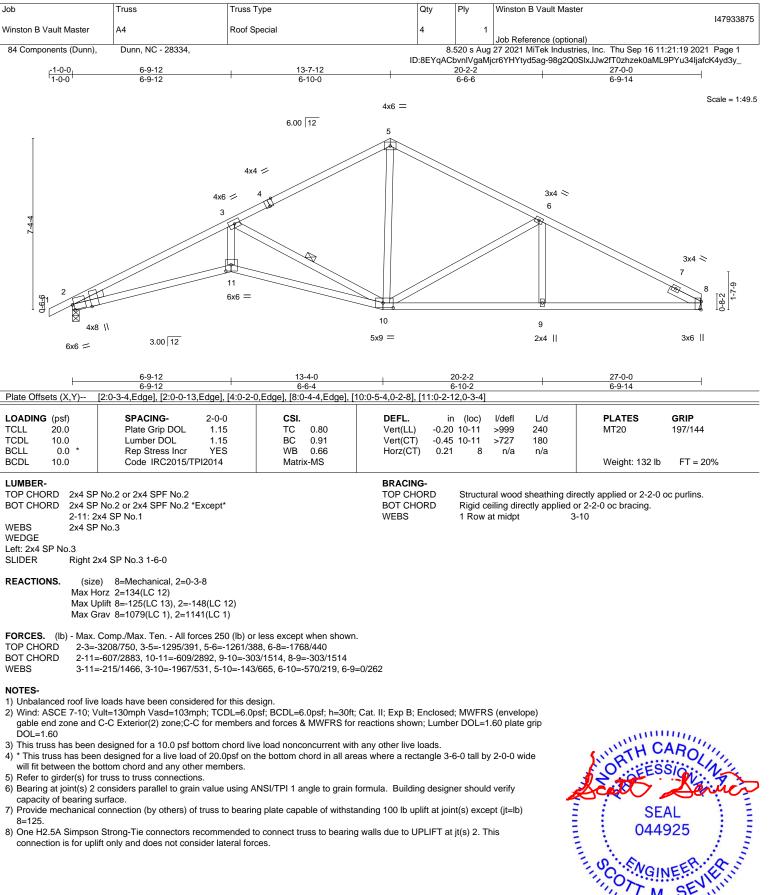


<b> </b>	8-7-12 8-7-12		<u>18-7-12</u> 10-0-0			27-0-0 8-4-4	
Plate Offsets (X,Y)	[2:0-0-0,0-0-12], [6:0-2-0,Edge], [10:0-4	-0,0-0-3]	10-0-0			8-4-4	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.72 BC 0.77 WB 0.43 Matrix-MS	Vert(LL) -0.20	) 15-22 > 2 15-22 >	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 151 lb	<b>GRIP</b> 197/144 FT = 20%
BOT CHORD 2x4 SP 12-14: WEBS 2x4 SP WEDGE Left: 2x4 SP No.3	2 No.2 or 2x4 SPF No.2 No.2 or 2x4 SPF No.2 *Except* 2x8 SP No.2 No.3 Xx4 SP No.3 1-6-0		BRACING- TOP CHORD BOT CHORD			rectly applied or 3-7-0 o or 10-0-0 oc bracing.	oc purlins.
REACTIONS. (size Max H Max U Max G FORCES. (lb) - Max. TOP CHORD 2-3=-	e) 10=Mechanical, 2=0-3-8 orz 2=134(LC 12) plift 10=-125(LC 13), 2=-149(LC 12) rav 10=1079(LC 1), 2=1141(LC 1) Comp./Max. Ten All forces 250 (lb) o 1824/458, 3-5=-1646/485, 7-8=-1601/4 324/1549, 11-15=-179/1167, 10-11=-2	76, 8-10=-1745/449					
	=-398/234, 5-15=-102/598, 7-11=-96/54		111/425				
NOTES-							
<ol> <li>Unbalanced roof live</li> <li>Wind: ASCE 7-10; V gable end zone and DOL=1.60</li> <li>All plates are 3x4 Mi</li> <li>This truss has been will fit between the b</li> <li>Refer to girder(s) for</li> <li>Provide mechanical 10=125, 2=149.</li> </ol>	e loads have been considered for this de /ult=130mph Vasd=103mph; TCDL=6.0 C-C Exterior(2) zone;C-C for members T20 unless otherwise indicated. designed for a 10.0 psf bottom chord lix n designed for a live load of 20.0psf on vottom chord and any other members, w russ to truss connections. connection (by others) of truss to bearin WN IS DESIGNED AS UNINHABITABL	osf; BCDL=6.0psf; h=30ft; and forces & MWFRS for re load nonconcurrent with the bottom chord in all are ith BCDL = 10.0psf. ng plate capable of withsta	reactions shown; Lumb h any other live loads. eas where a rectangle 3-	er DOL=1.6 6-0 tall by 2	0 plate grip 2-0-0 wide	SE 044	• •



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A MITek Atfilla 818 Soundside Road Edenton, NC 27932

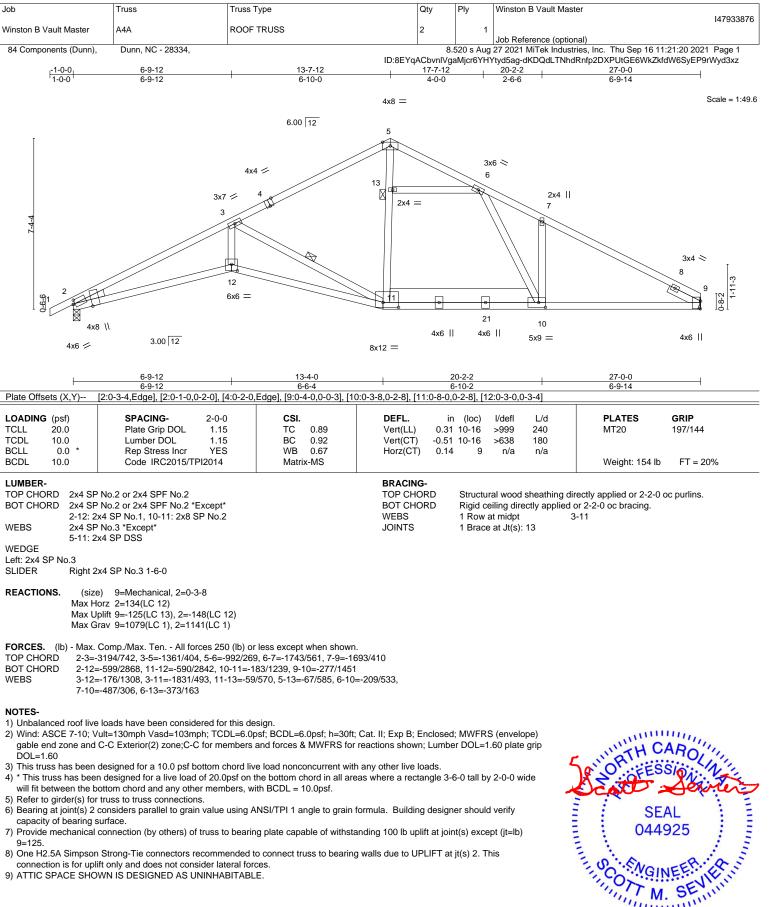


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### September 17,2021

818 Soundside Road

Edenton, NC 27932

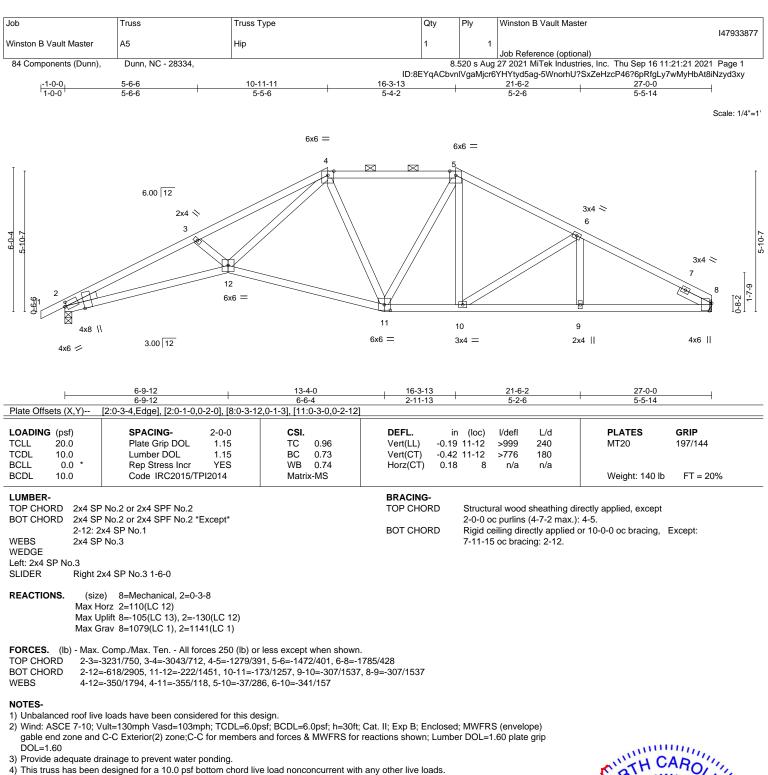


9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid to less only with with twe commendations. This besign is based only upon parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

 Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

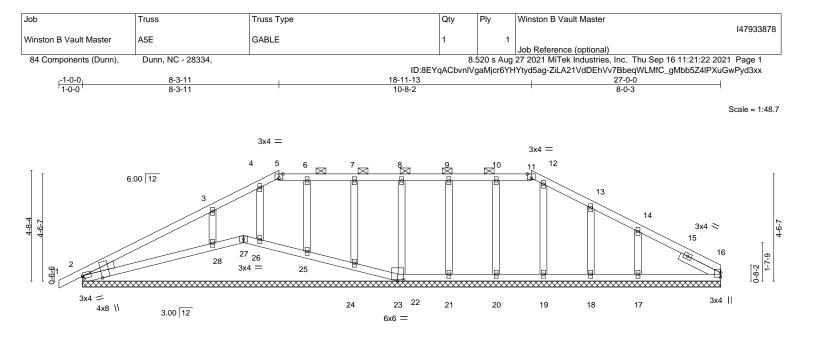
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=105.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



818 Soundside Road Edenton, NC 27932



<b> </b>	6-9-12 6-9-12	<u>13-4-0</u> 6-6-4		<u>27-0-0</u> 13-8-0	
Plate Offsets (X,Y)	[2:0-3-4,Edge], [2:0-0-6,0-1-6], [5:0-2-		6:0-2-4,0-0-3], [23:0-3-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.35 BC 0.25 WB 0.07 Matrix-S	DEFL.inVert(LL)0.00Vert(CT)0.02Horz(CT)0.00	(loc) l/defl L/d 1 n/r 120 1 n/r 90 16 n/a n/a	PLATES         GRIP           MT20         197/144           Weight: 137 lb         FT = 20%
BOT CHORD 2x4 S OTHERS 2x4 S WEDGE Left: 2x4 SP No.3	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3 2x4 SP No.3 1-10-13		BRACING- TOP CHORD BOT CHORD	2-0-0 oc purlins (6-0-0 max.):	ectly applied or 6-0-0 oc purlins, except 5-11. or 10-0-0 oc bracing, Except:
(lb) - Max H Max I	earings 27-0-0. Horz 2=81(LC 12) Jplift All uplift 100 lb or less at joint(s) except 27=-104(LC 3), 28=-148(I Grav All reactions 250 lb or less at join 19, 18 except 2=256(LC 1), 28=4	LC 12) ht(s) 16, 27, 22, 24, 25, 26, 2			
	. Comp./Max. Ten All forces 250 (lb) =-321/227	or less except when shown.			
,	e loads have been considered for this (	5			

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 24, 25 except (jt=lb) 27=104, 28=148.

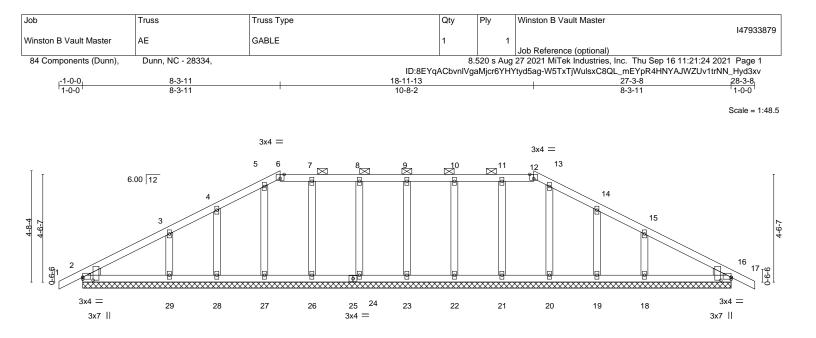
11) Ň/A

12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 27, 24, 25, 26, 28.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







					27-3-0						
I					27-3-8						
Plate Offsets (X,Y) [2	2:0-1-8,0-5-3], [2:0-0-0,0	)-1-0], [6:0-2-0,	,Edge], [12:0	-2-0,Edge], [	16:Edge,0-1-0], [16	:0-1-8,	0-5-3]				
LOADING (psf) FCLL 20.0 FCDL 10.0 SCLL 0.0 *	<b>SPACING-</b> Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.13 0.09 0.05	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.01 0.00	(loc) 17 17 16	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL 10.0	Code IRC2015/T	912014	Matri	x-S						Weight: 144 lb	FT = 20%
	No.2 or 2x4 SPF No.2 No.2 or 2x4 SPF No.2 No.3				BRACING- TOP CHOR BOT CHOR	_	2-0-0 o	c purlins	(6-0-0 max.):	rectly applied or 6-0-0 c : 6-12. or 10-0-0 oc bracing.	oc purlins, except

27-3-8

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 27-3-8.

(lb) - Max Horz 2=-76(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 2, 23, 24, 26, 27, 28, 29, 22, 21, 19, 18, 16

Max Grav All reactions 250 lb or less at joint(s) 2, 23, 24, 26, 27, 28, 22, 21, 20, 19, 16 except 29=279(LC 23), 18=279(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) N/A

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





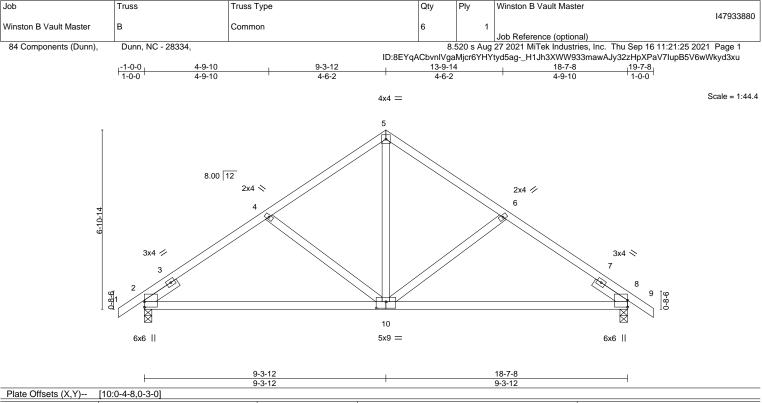


Plate Offsets (X,Y)				
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl	L/d PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.25	Vert(LL) -0.10 10-17 >999	240 MT20 197/144
TCDL 10.0	Lumber DOL 1.15	BC 0.71	Vert(CT) -0.21 10-17 >999	180
BCLL 0.0 *	Rep Stress Incr YES	WB 0.20	Horz(CT) 0.02 8 n/a	n/a
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS		Weight: 95 lb FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

 SLIDER
 Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-170(LC 10) Max Uplift 2=-100(LC 12), 8=-100(LC 13) Max Grav 2=805(LC 1), 8=805(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-839/204, 4-5=-745/182, 5-6=-745/182, 6-8=-839/204

BOT CHORD 2-10=-130/779, 8-10=-73/746

WEBS 5-10=-77/520, 6-10=-279/182, 4-10=-279/182

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

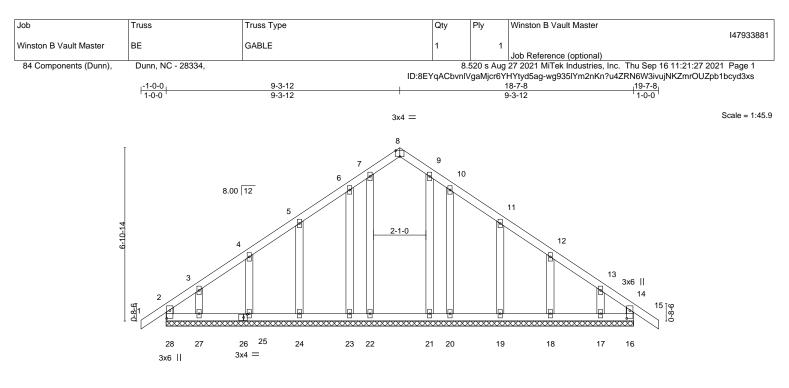
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.





			<u>18-7-8</u> 18-7-8		
Plate Offsets (X,Y)	[8:0-2-0,Edge], [14:0-3-12,0-1-8], [26:0-	1-12,0-1-8], [28:0-3-12,0-1	-8]		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.13 BC 0.07 WB 0.06 Matrix-R	<b>DEFL.</b> ir Vert(LL) -0.00 Vert(CT) -0.01 Horz(CT) 0.00	15 n/r 120 15 n/r 90	RIP 7/144 T = 20%
	No.2 or 2x4 SPF No.2 No.2 or 2x4 SPF No.2 No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	 ırlins,

REACTIONS. All bearings 18-7-8.

2x4 SP No.3

Max Horz 28=-170(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 28, 16, 23, 24, 25, 20, 19, 18 except 27=-125(LC 12), 17=-117(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 28, 16, 22, 21, 23, 24, 25, 27, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

OTHERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

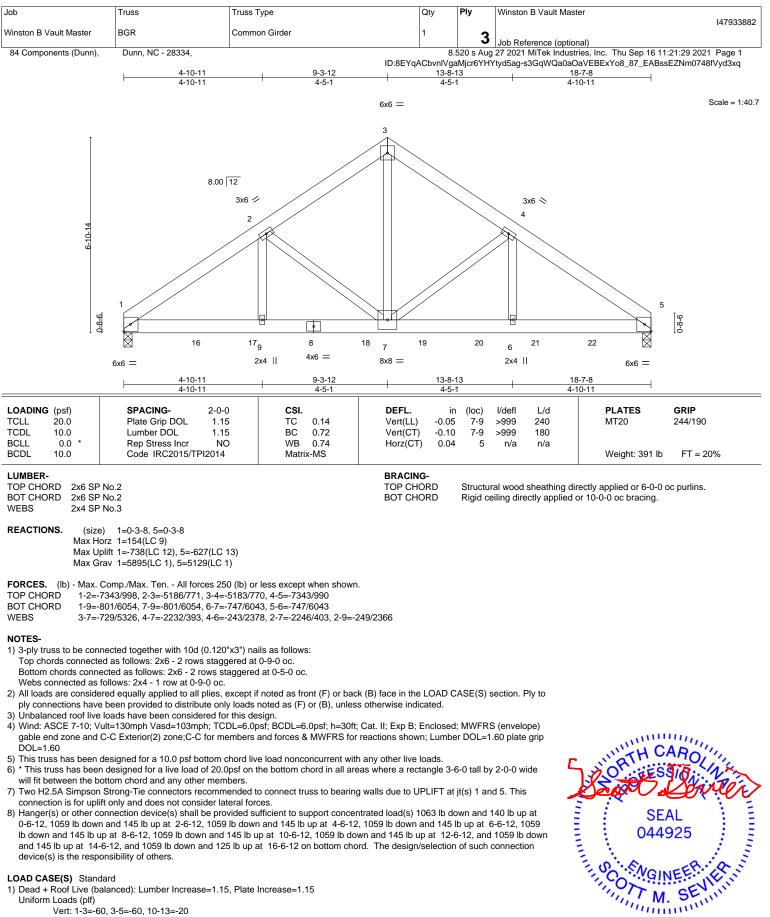
6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.







Vert: 1-3=-60, 3-5=-60, 10-13=-20

#### Continued on page 2

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road

Edenton, NC 27932

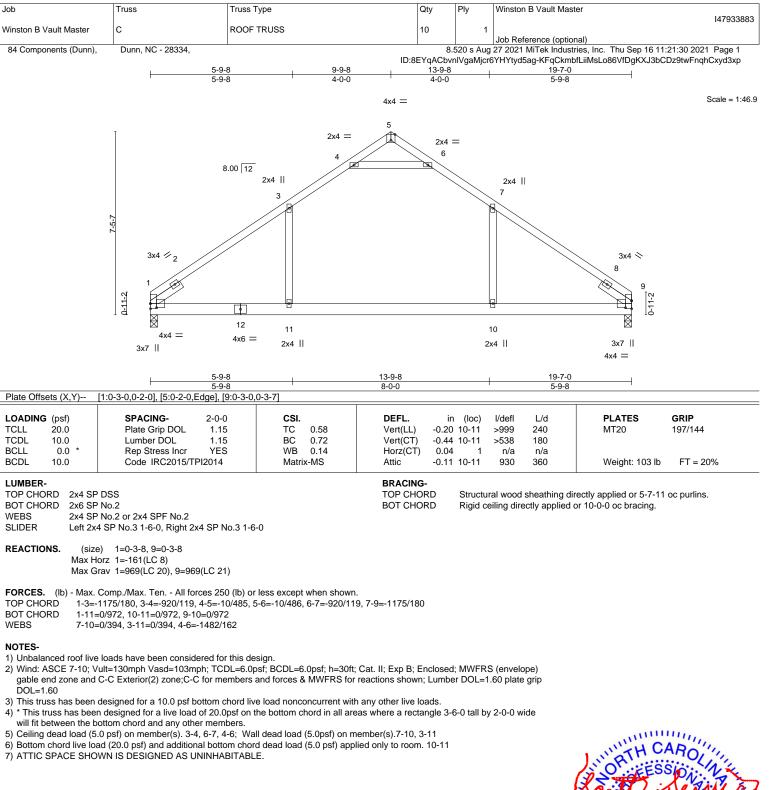
September 17,2021

Job	Truss	Truss Type	Qty	Ply	Winston B Vault Master
	200				147933882
Winston B Vault Master	BGR	Common Girder	1	3	Job Reference (optional)
				-	Job Relefence (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Thu Sep 16 11:21:29 2021 Page 2
		ID:8EYq/	ACbvnlVga	aMjcr6YHY	tyd5ag-s3GqWQa0aOaVEBExYo8_87_EABssEZNm0748fVyd3xq

#### LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 8=-1059(B) 12=-1063(B) 16=-1059(B) 17=-1059(B) 18=-1059(B) 19=-1059(B) 20=-1059(B) 21=-1059(B) 22=-1059(B)









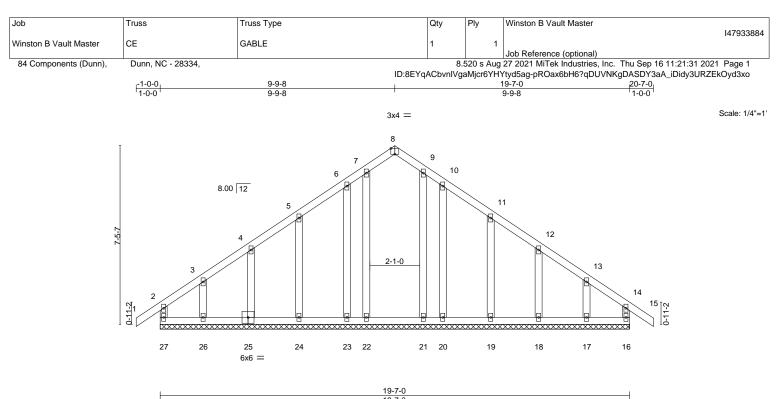


Plate Offsets (X,Y)	[8:0-2-0,Edge]		19-7-0				Γ	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.17 BC 0.09 WB 0.09 Matrix-R	<b>DEFL.</b> Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) 0.0	01 15	l/defl n/r n/r n/a	L/d 120 90 n/a	<b>PLATES</b> MT20 Weight: 130 lb	<b>GRIP</b> 197/144 FT = 20%
	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2		BRACING- TOP CHORD		ıral wood end verti	0	ectly applied or 6-0-0	oc purlins,

WEBS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. OTHERS 2x4 SP No.3

REACTIONS. All bearings 19-7-0.

Max Horz 27=-178(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 27, 16, 23, 24, 25, 20, 19, 18 except 26=-146(LC 12), 17=-133(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 27, 16, 22, 21, 23, 24, 25, 26, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

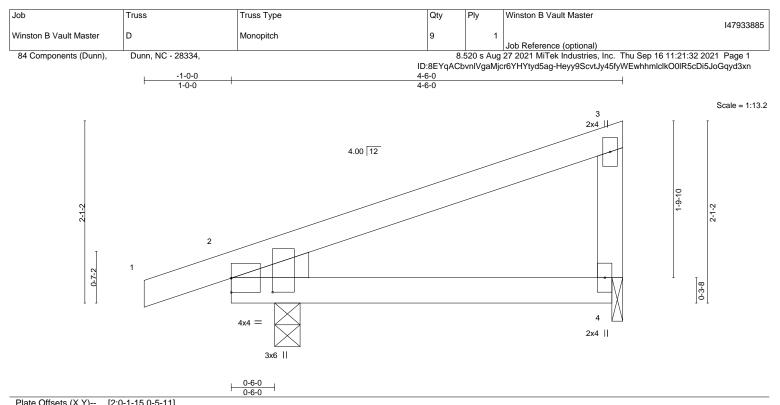
7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.







CDL 10.0	Code IRC2015/TPI2014	Matrix-MP		Weight: 19 lb FT = 20%
OADING         (psf)           CLL         20.0           CDL         10.0           CLL         0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.18 BC 0.14 WB 0.00	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.01         4-9         >999         240           Vert(CT)         -0.02         4-9         >999         180           Horz(CT)         0.00         2         n/a         n/a	PLATES GRIP MT20 197/144

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=73(LC 8) Max Uplift 2=-70(LC 8), 4=-37(LC 12) Max Grav 2=272(LC 1), 4=136(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

# Contraction of the State 1111111111 4925 mm September 17,2021



<sup>3) \*</sup> This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

