

RE: Pinehurst B Vault Master Pinhurst B Vault Master Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: D.R. HORTON - RAL - 055Project Name: Pinehurst B Vault Master<br/>Model: PINEHURST / BAddress:Subdivision:<br/>State: NC

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 9 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	147723291	А	9/1/2021
2	147723292	A1	9/1/2021
3	147723293	A2	9/1/2021
4	147723294	A3	9/1/2021
5	147723295	A3E	9/1/2021
6	147723296	AE	9/1/2021
7	147723297	BE	9/1/2021
8	147723298	BGR	9/1/2021
9	147723299	CE	9/1/2021

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by 84 Components - #2383.

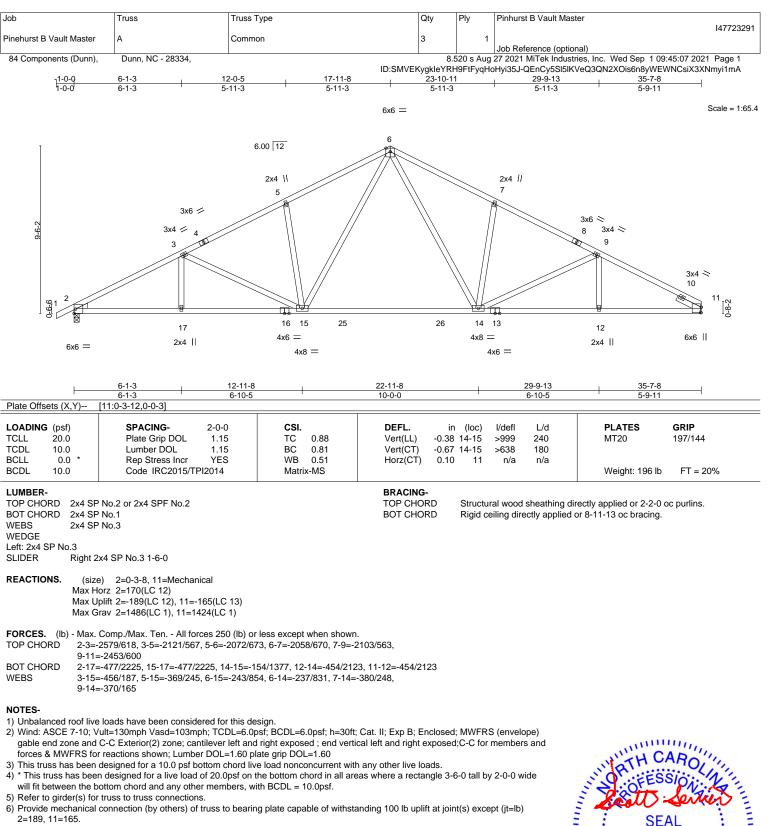
Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

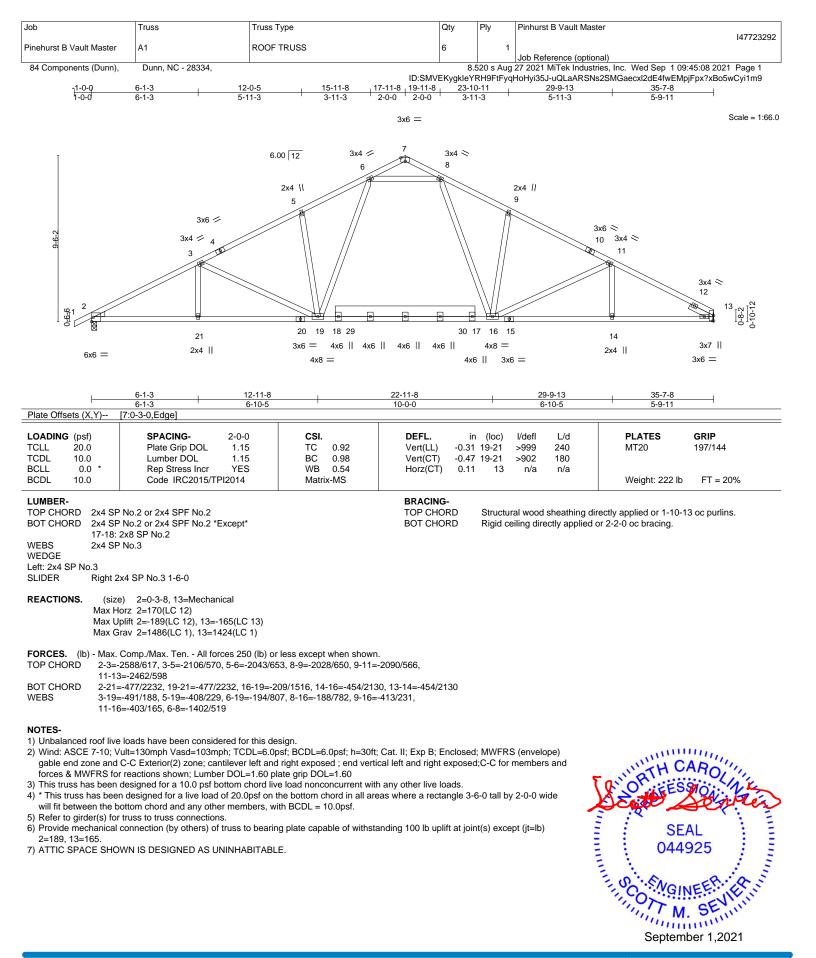






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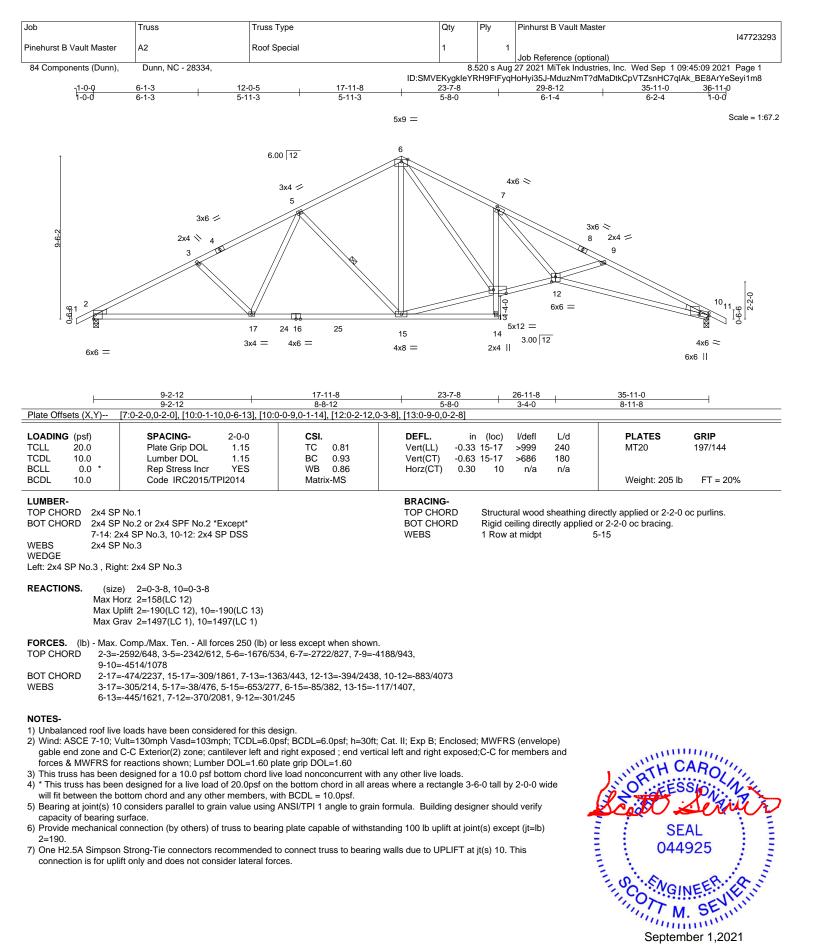
ENGINEERING BY **TRENCO** A Mi Tek Atfiliate 818 Soundside Road Edenton, NC 27932



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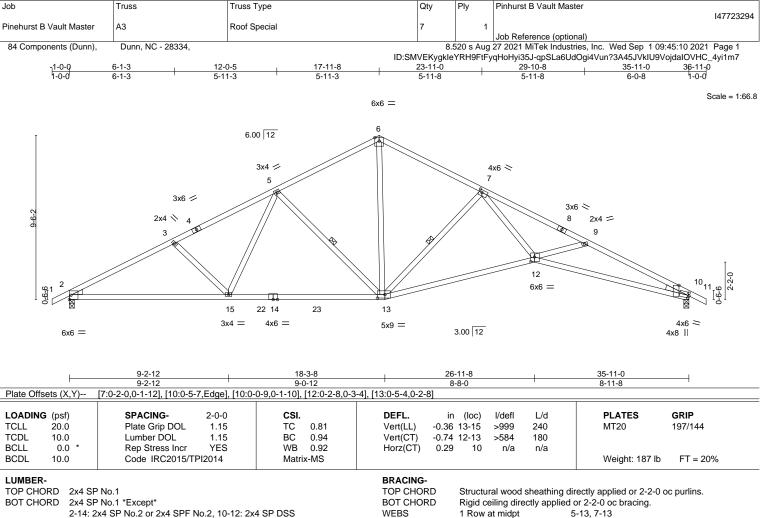
Edenton, NC 27932



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2-14: 2x4 SP No.2 or 2x4 SPF No.2, 10-12: 2x4 SP DSS WEBS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS.	(size)	2=0-3-8, 10=0-3-8
	Max Horz	2=158(LC 16)
	Max Uplift	2=-190(LC 12), 10=-190(LC 13)
	Max Grav	2=1497(LC 1), 10=1497(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2594/648, 3-5=-2344/611, 5-6=-1652/530, 6-7=-1700/543, 7-9=-4218/935, 9-10=-4523/1073
- 2-15=-473/2238, 13-15=-310/1859, 12-13=-436/2488, 10-12=-878/4080 BOT CHORD
- WFBS 3-15=-304/215, 5-15=-33/491, 5-13=-664/275, 6-13=-307/1107, 7-13=-1432/424, 7-12=-347/2220, 9-12=-272/236

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface

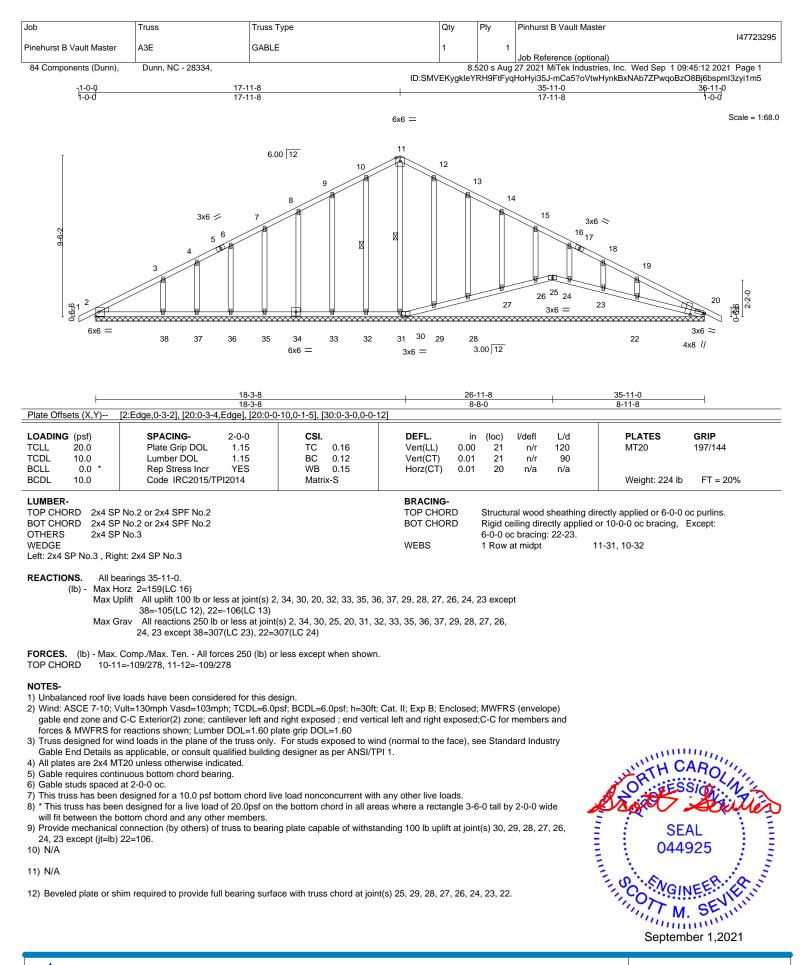
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=190

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 10. This connection is for uplift only and does not consider lateral forces.



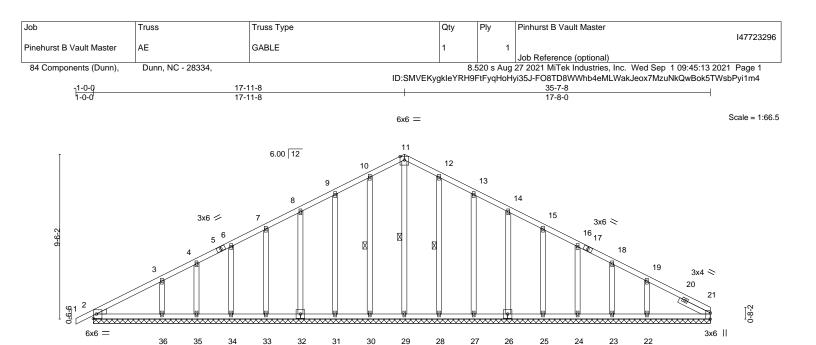
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			<u>35-7-8</u> 35-7-8				
Plate Offsets (X,Y)	[2:Edge,0-3-2], [21:0-4-4,Edge]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.16 BC 0.12 WB 0.12 Matrix-S	DEFL. in Vert(LL) -0.00 Vert(CT) 0.01 Horz(CT) 0.01	( ,	L/d 120 90 n/a	PLATES MT20 Weight: 235 lb	<b>GRIP</b> 197/144 FT = 20%
	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3		BRACING- TOP CHORD BOT CHORD WEBS		rectly applied	rectly applied or 6-0-0 o or 10-0-0 oc bracing. 11-29, 10-30, 12-28	oc purlins.

6x6 =

Left: 2x4 SP No.3

SLIDER Right 2x4 SP No.3 1-11-15

## REACTIONS. All bearings 35-7-8.

(lb) - Max Horz 2=163(LC 12)

 Max Uplift
 All uplift 100 lb or less at joint(s) 2, 30, 31, 32, 33, 34, 35, 28, 27, 26, 25, 24, 23 except 36=-105(LC 12), 22=-108(LC 13)

 Max Grav
 All reactions 250 lb or less at joint(s) 2, 21, 29, 30, 31, 32, 33, 34, 35,

28, 27, 26, 25, 24, 23 except 36=306(LC 23), 22=285(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 10-11=-120/282, 11-12=-120/282

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

6x6 =

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

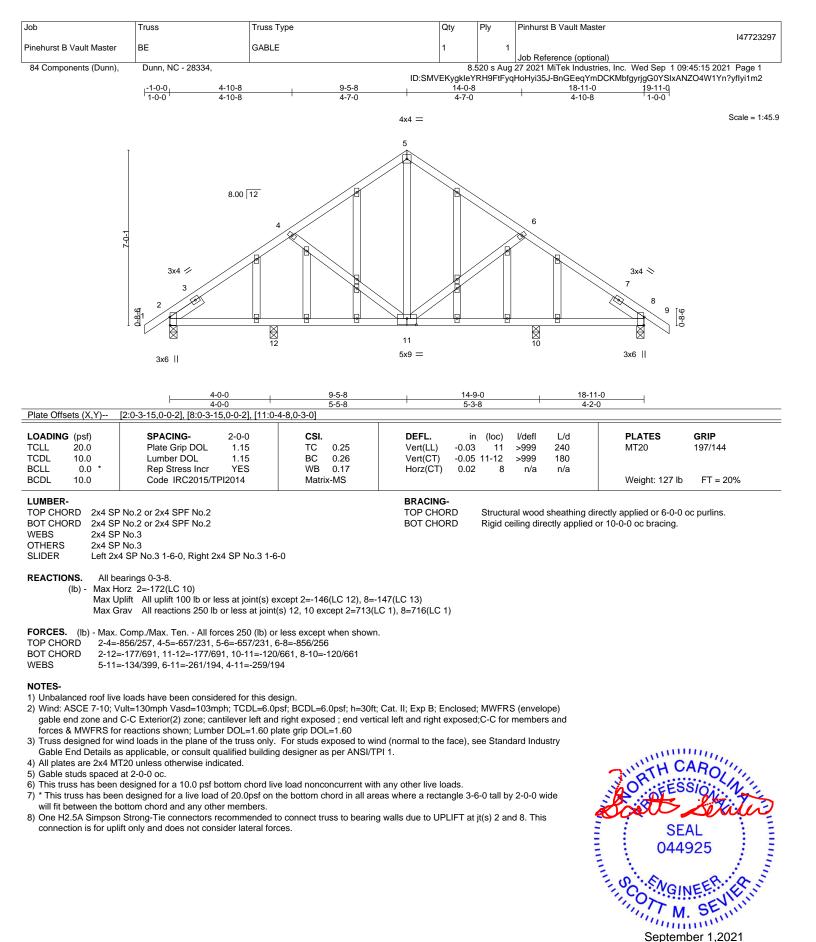
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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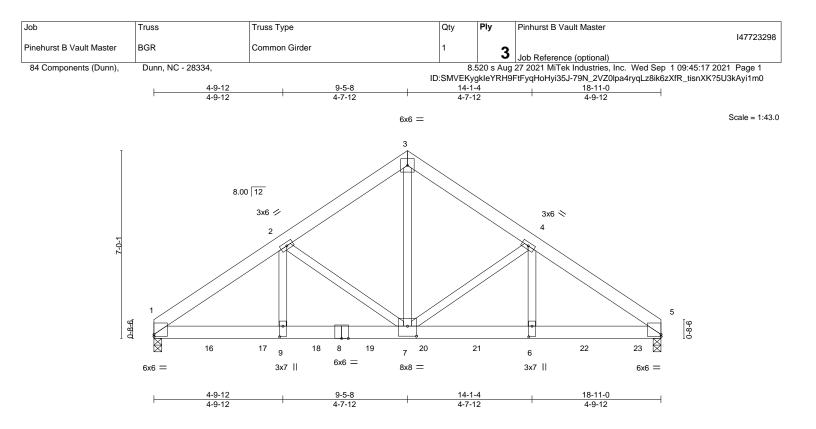


Plate Offsets (X,Y)	[1:0-0-0,0-0-14], [5:0-0-0,0-0-14], [6:0-4	-12,0-1-8], [7:0-4-0,0-4-8	], [9:0-4-12,0-1-8]	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	<b>CSI.</b> TC 0.19 BC 0.99 WB 0.97 Matrix-MS	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.07         7-9         >999         240           Vert(CT)         -0.14         7-9         >999         180           Horz(CT)         0.05         5         n/a         n/a	PLATES         GRIP           MT20         244/190           Weight: 397 lb         FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF WEBS 2x4 SF	P No.2 P No.3		BRACING- TOP CHORD Structural wood sheathin BOT CHORD Rigid ceiling directly appl	ng directly applied or 6-0-0 oc purlins. lied or 10-0-0 oc bracing.
Max H Max U	e) 1=0-3-8, 5=0-3-8 lorz 1=156(LC 34) lplift 1=-827(LC 12), 5=-930(LC 13) irav 1=6668(LC 1), 5=7484(LC 1)			
TOP CHORD 1-2=- BOT CHORD 1-9=-	Comp./Max. Ten All forces 250 (lb) or 9659/1276, 2-3=-6754/960, 3-4=-6760/ 1033/7945, 7-9=-1033/7945, 6-7=-998/ 930/7023, 4-7=-3014/499, 4-6=-351/32	961, 4-5=-9721/1286 3031, 5-6=-998/8031		
<ul> <li>Top chords connect Bottom chords conn Webs connected as</li> <li>2) All loads are conside ply connections have</li> <li>3) Unbalanced roof live</li> <li>4) Wind: ASCE 7-10; V gable end zone and forces &amp; MWFRS fo</li> <li>5) This truss has been</li> <li>6) * This truss has been</li> <li>6) * This truss has been</li> <li>7) Two H2.5A Simpsor connection is for upl</li> <li>8) Hanger(s) or other 2-0-12, 1404 lb dow lb down and 185 lb up</li> </ul>	e been provided to distribute only loads a loads have been considered for this de /ult=130mph Vasd=103mph; TCDL=6.0 C-C Exterior(2) zone; cantilever left and r reactions shown; Lumber DOL=1.60 p designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on t vottom chord and any other members. n Strong-Tie connectors recommended t ift only and does not consider lateral for sonnection device(s) shall be provided su n and 185 lb up at 4-0-12, 1404 lb dow	0-9-0 oc. d at 0-4-0 oc. f noted as front (F) or bac noted as (F) or (B), unles isign. bsf; BCDL=6.0psf; h=30ft right exposed ; end verti ate grip DOL=1.60 e load nonconcurrent wit the bottom chord in all are o connect truss to bearin ces. ufficient to support conce n and 185 lb up at 6-0-12 up at 12-0-12, 1404 lb d	; Cat. II; Exp B; Enclosed; MWFRS (envelope) ical left and right exposed;C-C for members and h any other live loads. eas where a rectangle 3-6-0 tall by 2-0-0 wide g walls due to UPLIFT at jt(s) 1 and 5. This ntrated load(s) 1404 lb down and 185 lb up at 2, 1404 lb down and 185 lb up at 8-0-12, 1404 lown and 185 lb up at 14-0-12, and 1404 lb	SEAL 044925 WGINEEP
	dard design parameters and READ NOTES ON THIS ANI by with MITek® connectors. This design is based a			ENGINEERING BY

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 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Pinhurst B Vault Master
					147723298
Pinehurst B Vault Master	BGR	Common Girder	1	3	
				J	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Wed Sep 1 09:45:17 2021 Page 2
		ID	SMVEKy	kleYRH9	-tFyqHoHyi35J-79N_2VZ0lpa4ryqLz8ik6zXfR_tisnXK?5U3kAyi1m0

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

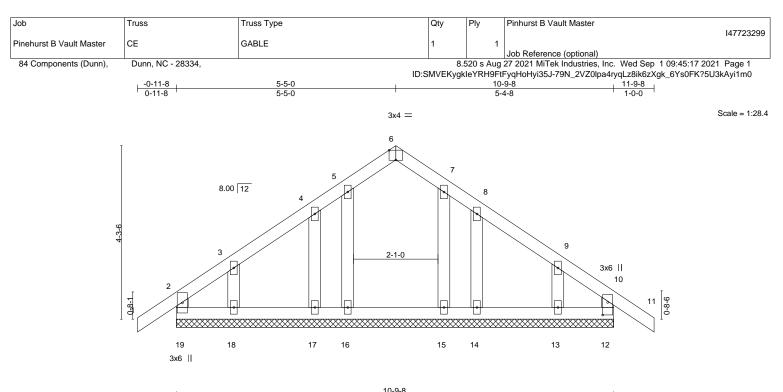
Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 10-13=-20

Concentrated Loads (lb)

Vert: 6=-1404(B) 16=-1404(B) 17=-1404(B) 18=-1404(B) 19=-1404(B) 20=-1404(B) 21=-1404(B) 22=-1404(B) 23=-1406(B)

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OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.11	Vert(LL)	-0.00	11	n/r	120	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	-0.01	11	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	12	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix	x-R						Weight: 59 lb	FT = 20%

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 except end verticals. WEBS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. OTHERS 2x4 SP No.3

REACTIONS. All bearings 10-9-8.

Max Horz 19=122(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 19, 12, 17, 18, 14, 13 Max Grav All reactions 250 lb or less at joint(s) 19, 12, 16, 15, 17, 18, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing. 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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