

RE: Wilmington B Wilmington B Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: D.R. HORTON - RAL - 055Project Name: Wilmington BLot/Block:Model: WILMINGTON / BAddress:Subdivision:City: FUQUAY-VARINAState: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 9 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	147901364	A	9/15/2021
2	147901365	A1	9/15/2021
3	147901366	A2	9/15/2021
4	147901367	A3	9/15/2021
5	147901368	A3E	9/15/2021
6	147901369	AE	9/15/2021
7	147901370	BE	9/15/2021
8	I47901371	BGR	9/15/2021
9	I47901372	D	9/15/2021

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

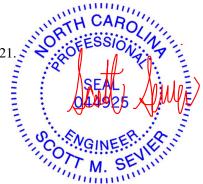
based on the parameters provided by 84 Components - #2383.

Truss Design Engineer's Name: Sevier, Scott

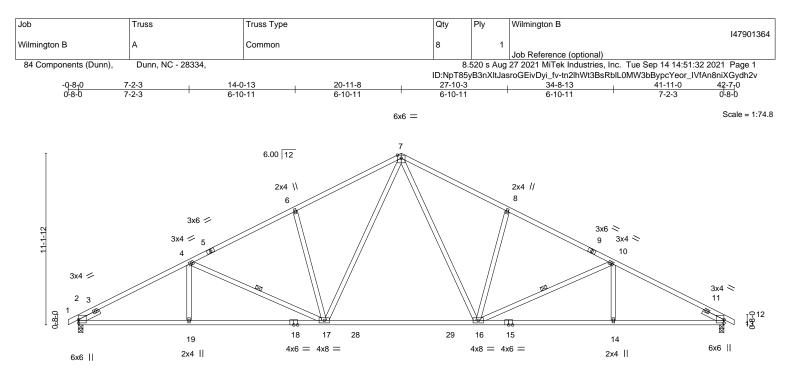
My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



September 15, 2021



	7-2-3 7-2-3	<u>15-11-8</u> 8-9-5		25-11-8 10-0-0			4-8-13 3-9-5	<u>41-11-0</u> 7-2-3)
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DC	L 1.15	TC 0.81	Vert(LL)	-0.46 16-17	>999	240	MT20	197/144
CDL 10.0	Lumber DOL	1.15	BC 0.80	Vert(CT)	-0.75 16-17	>671	180		
BCLL 0.0 *	Rep Stress In	cr YES	WB 0.69	Horz(CT)	0.14 12	n/a	n/a		
BCDL 10.0	Code IRC201	5/TPI2014	Matrix-AS					Weight: 234 lb	FT = 20%

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2 *Except* 1-5,9-13: 2x4 SP No.1
BOT CHORD	2x4 SP No.1
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

BRACING-TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied.Rigid ceiling directly applied.1 Row at midpt10-16, 4-17

REACTIONS. (size) 2=0-3-8, 12=0-3-8 Max Horz 2=181(LC 12) Max Uplift 2=-210(LC 12), 12=-210(LC 13) Max Grav 2=1717(LC 1), 12=1717(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2960/714, 4-6=-2433/664, 6-7=-2332/755, 7-8=-2332/755, 8-10=-2433/664, 10-12=-2960/714

 BOT CHORD
 2-19=-526/2560, 17-19=-526/2560, 16-17=-166/1629, 14-16=-527/2560, 12-14=-527/2560

 WEBS
 7-16=-252/905, 8-16=-418/270, 10-16=-533/205, 10-14=0/272, 7-17=-252/905, 6-17=-418/270, 4-17=-533/205, 4-19=0/272

NOTES-

1) Unbalanced roof live loads have been considered for this design.

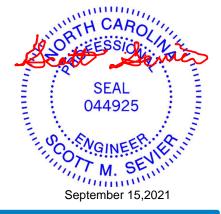
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

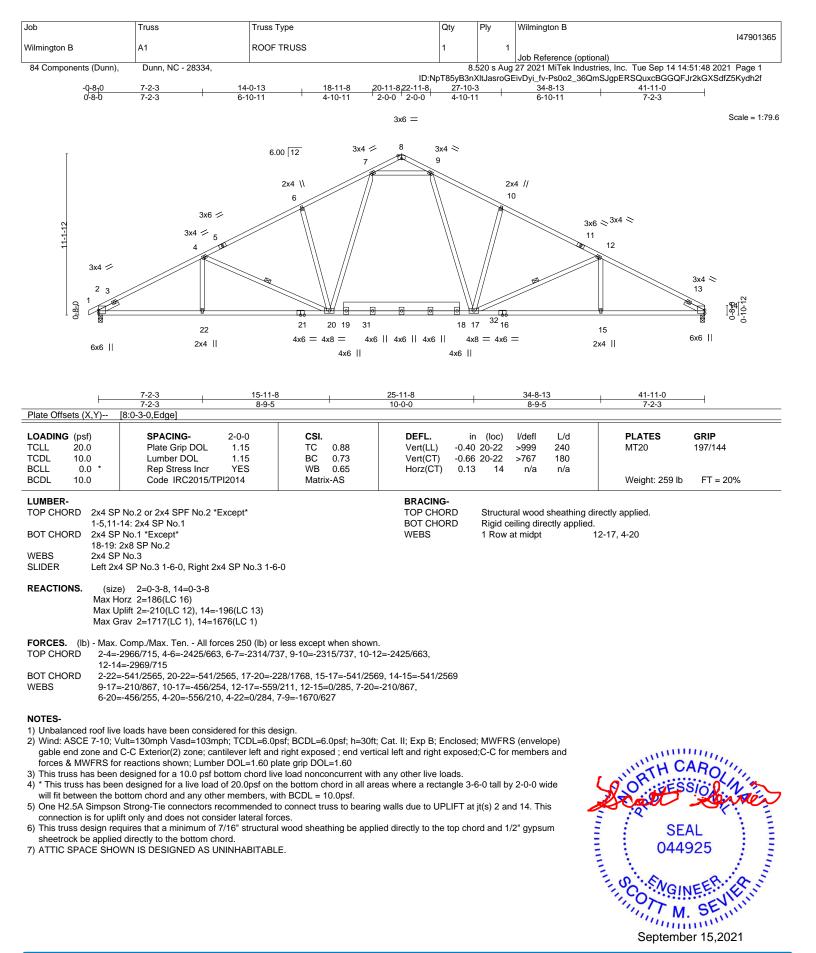
5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



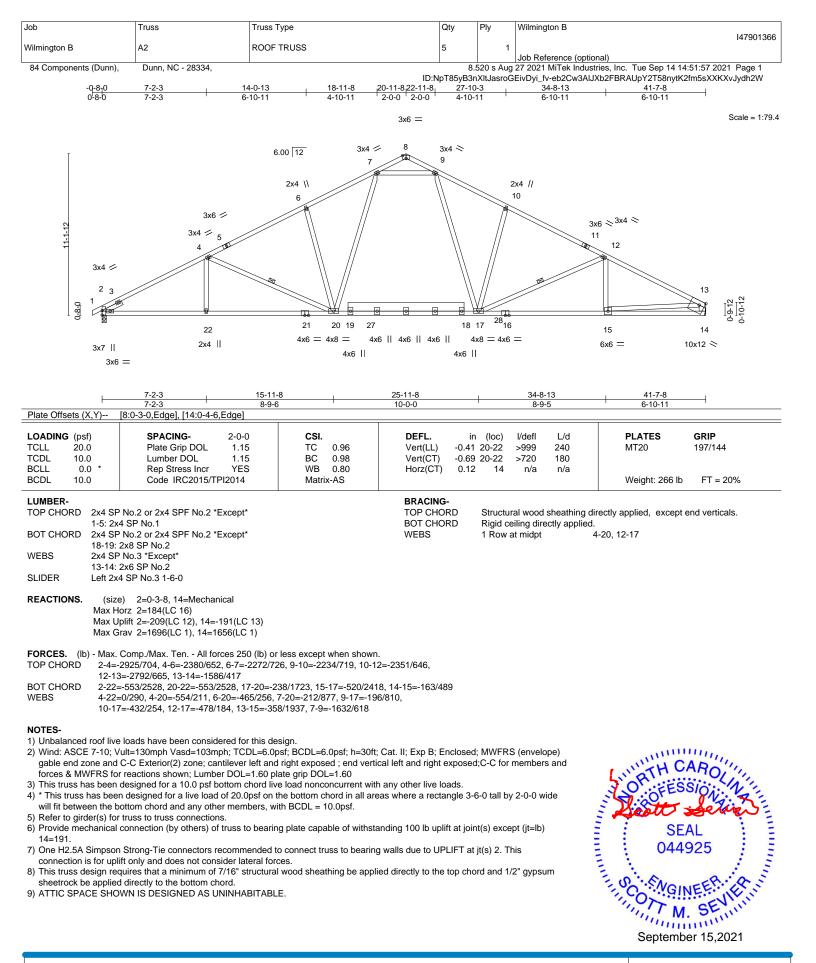
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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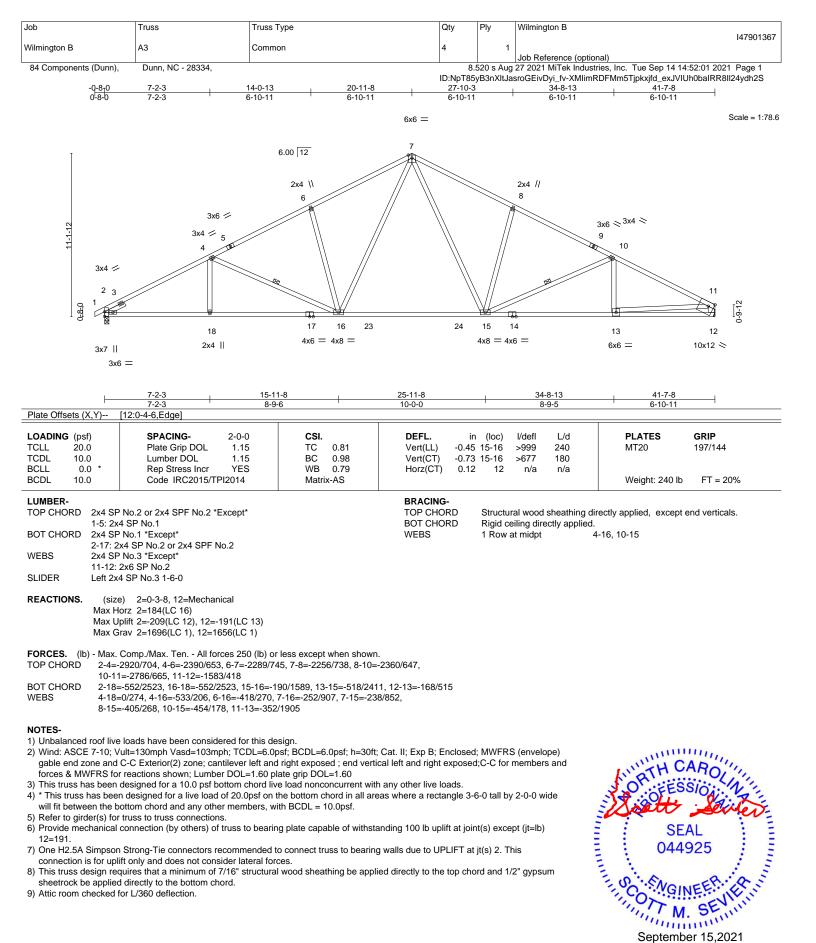
Edenton, NC 27932



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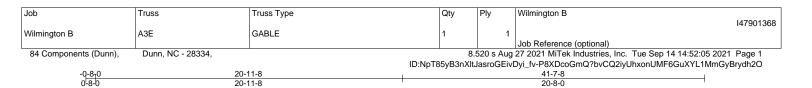
TRENCO

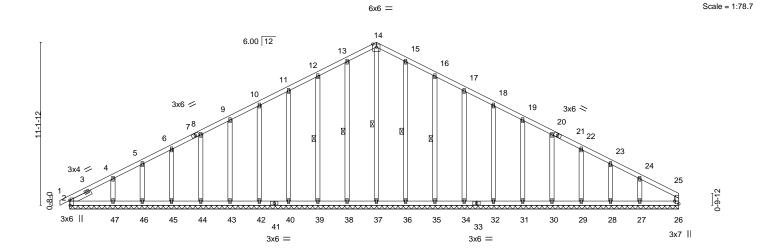
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DADING (psf)	SPACING- 2-0-0	CSI.		(,	/d PLATES GRIP	
CLL 20.0	Plate Grip DOL 1.15	TC 0.11	Vert(LL) -0.0		20 MT20 197/144	
DL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) 0.0		90	
CLL 0.0 *	Rep Stress Incr YES	WB 0.14	Horz(CT) 0.0	1 26 n/a r	n/a	
DL 10.0	Code IRC2015/TPI2014	Matrix-S			Weight: 301 lb FT = 20%	
	No.2 or 2x4 SPF No.2 No.2 or 2x4 SPF No.2		TOP CHORD	Structural wood she except end verticals	athing directly applied or 6-0-0 oc purlins,	
EBS 2x4 SF	No.3		BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.		
HERS 2x4 SF	No.3		WEBS	1 Row at midpt	14-37, 13-38, 12-39, 15-36, 16-35	
IDER Left 2x	4 SP No.3 1-7-2					

28 except 47=-110(LC 12), 27=-110(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 26, 37, 38, 39, 40, 42, 43, 44, 45, 46, 47, 36, 35, 34, 32, 31, 30, 29, 28, 27

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 11-12=-119/270, 12-13=-137/321, 13-14=-152/362, 14-15=-152/362, 15-16=-137/321, 16-17=-119/270

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

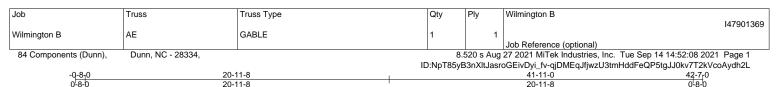
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- r) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

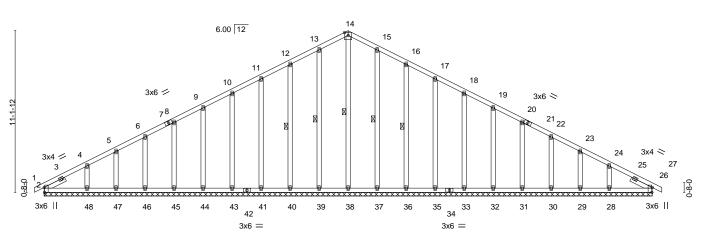


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—			<u>41-11-0</u> 41-11-0		I
Plate Offsets (X,Y)	2:0-4-1,Edge], [26:0-4-1,Edge]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.09 BC 0.06 WB 0.14 Matrix-S	DEFL. 0.0 Vert(LL) 0.0 Vert(CT) 0.0 Horz(CT) 0.0	0 27 n/r 90	PLATES GRIP MT20 197/144 Weight: 305 lb FT = 20%
	No.2 or 2x4 SPF No.2 No.2 or 2x4 SPF No.2 No.3		BRACING- TOP CHORD BOT CHORD WEBS	Rigid ceiling directly applied of	ectly applied or 6-0-0 oc purlins. or 10-0-0 oc bracing. 4-38, 13-39, 12-40, 15-37, 16-36

SLIDER Left 2x4 SP No.3 1-7-2, Right 2x4 SP No.3 1-7-2

REACTIONS. All bearings 41-11-0. (lb) - Max Horz 2=181(LC 12)

(ID) - IMAX HOIZ Z=101(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 39, 40, 41, 43, 44, 45, 46, 47, 37, 36, 35, 33, 32, 31, 30, 29, 28 except 48=-112(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 2, 26, 38, 39, 40, 41, 43, 44, 45, 46, 47, 48, 37, 36, 35, 33, 32, 31, 30, 29, 28

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 12-13=-120/287, 13-14=-136/329, 14-15=-136/329, 15-16=-120/287

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

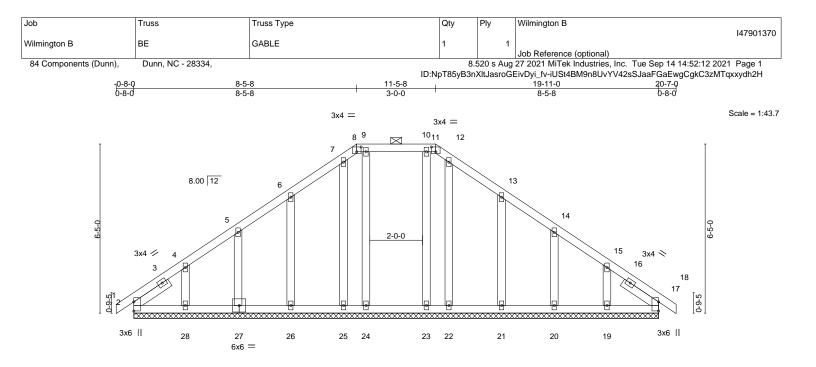
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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late Offse	ets (X,Y) [8:0-2-0,0-2-3], [11:0-2-0),0-2-3]			19-11-0					1	
LOADING TCLL TCDL BCLL	i (psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.06 0.04 0.07	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 0.00 0.00	(loc) 17 17 17	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code IRC2015/T		Matri		1012(01)	0.00	17	n/a	n/a	Weight: 132 lb	FT = 20%
LUMBER- TOP CHO BOT CHO OTHERS	RD 2x4 SP	No.2 or 2x4 SPF No.2 No.2 or 2x4 SPF No.2 No.3				BRACING- TOP CHOF BOT CHOF	RD	2-0-0 o	c purlins	(6-0-0 max.)	rectly applied or 6-0-0 o : 8-11. or 10-0-0 oc bracing.	oc purlins, except

19-11-0

REACTIONS. All bearings 19-11-0.

(lb) - Max Horz 2=-151(LC 8)

Max Horz 2=-151(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 24, 23, 25, 26, 27, 22, 21, 20, 19, 17 except 28=-103(LC 12)
 Max Grav All reactions 250 lb or less at joint(s) 2, 24, 23, 25, 26, 27, 28, 22, 21, 20, 19, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Left 2x4 SP No.3 1-7-8, Right 2x4 SP No.3 1-7-8

NOTES-

SLIDER

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

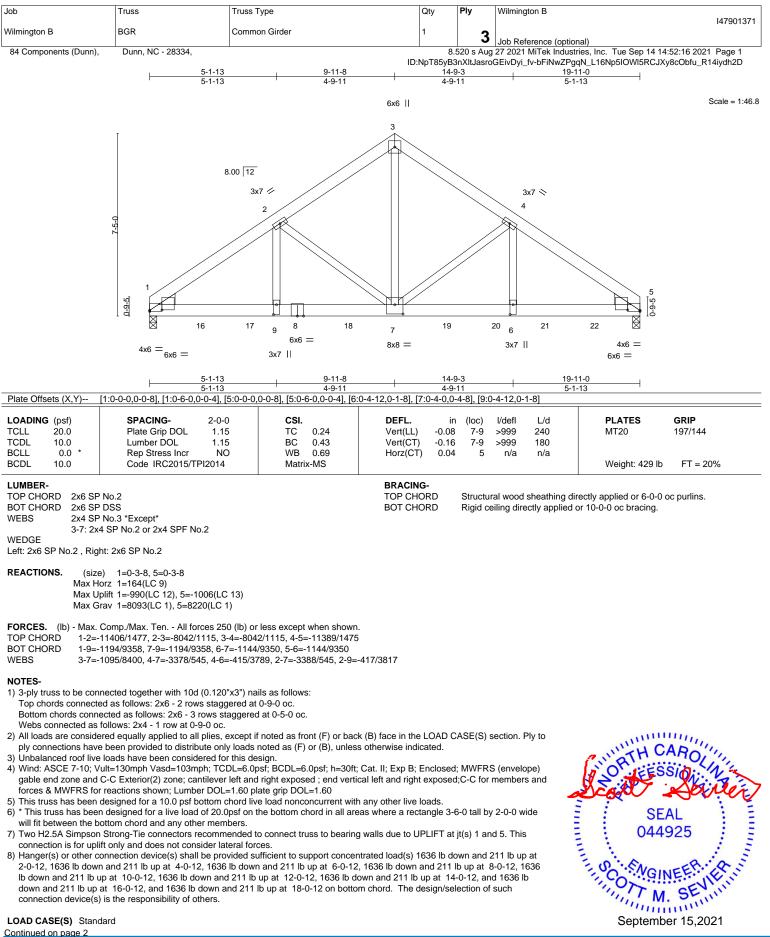
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) N/A

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Wilmington B
					I47901371
Wilmington B	BGR	Common Girder	1	3	
				U U	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Sep 14 14:52:16 2021 Page 2
		ID:	NpT85yB3	3nXltJasro	GEivDyi_fv-bFiNwZPgqN_L16Np5IOWI5RCJXy8cObfu_R14iydh2D

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

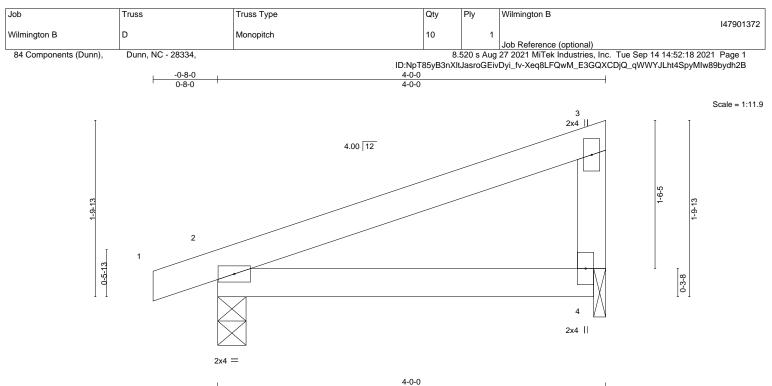
Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 10-13=-20

Concentrated Loads (lb)

Vert: 8=-1636(B) 7=-1636(B) 16=-1636(B) 17=-1636(B) 18=-1636(B) 19=-1636(B) 20=-1636(B) 21=-1636(B) 22=-1636(B)

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			4-0-0	
LOADING (psf)	SPACING- 2-0	0-0 CSI .	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.	.15 TC 0.21	Vert(LL) -0.01 4-7 >999 240	MT20 197/144
TCDL 10.0	Lumber DOL 1.	.15 BC 0.15	Vert(CT) -0.02 4-7 >999 180	
BCLL 0.0 *	Rep Stress Incr YE	'ES WB 0.00	Horz(CT) 0.00 2 n/a n/a	
BCDL 10.0	Code IRC2015/TPI201	14 Matrix-MP		Weight: 15 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD WEBS 2x4 SP No.3

TOP CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 2=0-3-8, 4=0-1-8 (size) Max Horz 2=64(LC 11) Max Uplift 2=-52(LC 8), 4=-33(LC 12) Max Grav 2=198(LC 1), 4=151(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



