

RE: Winston C Vault Bonus Winston vault Bonus Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: D.R. HORTON - RAL - 055Project Name: Winston C Vault BonusLot/Block:Model: WINSTON / C Vault BonusAddress:Subdivision:City: FUQUAY-VARINAState: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 13 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	147936825	A1	9/17/2021
2	147936826	A2	9/17/2021
3	147936827	A3	9/17/2021
4	147936828	A4	9/17/2021
5	147936829	A4A	9/17/2021
6	147936830	A4E	9/17/2021
7	147936831	AE	9/17/2021
8	147936832	В	9/17/2021
9	147936833	BE	9/17/2021
10	147936834	BGR	9/17/2021
11	147936835	С	9/17/2021
12	147936836	CE	9/17/2021
13	147936837	D	9/17/2021

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by 84 Components - #2383.

Truss Design Engineer's Name: Sevier, Scott

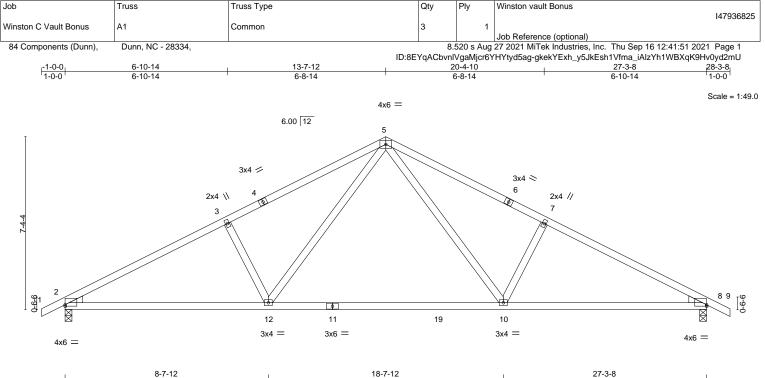
My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Sevier, Scott



	1	8-7-12				10-0-0				8-7-12	
Plate Offsets (2	X,Y)	[2:0-0-0,0-0-12], [8:0-0-0,0)-0-12]								
LOADING (ps	sf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.	.0	Plate Grip DOL	1.15	тс	0.62	Vert(LL)	-0.39 10-12	>840	240	MT20	197/144
TCDL 10.	.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.62 10-12	>530	180		
BCLL 0.	.0 *	Rep Stress Incr	YES	WB	0.28	Horz(CT)	0.06 8	n/a	n/a		
BCDL 10.	.0	Code IRC2015/TPI	2014	Matrix	k-MS					Weight: 128 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 2-2-0 oc bracing.

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2 *Except*

 & 8-11: 2x4 SP No.1
 WEBS

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS.	(size)	2=0-3-8, 8=0-3-8
	Max Horz	2=122(LC 12)
	Max Uplift	2=-149(LC 12), 8=-149(LC 13)
	Max Grav	2=1152(LC 1), 8=1152(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1872/467, 3-5=-1694/505, 5-7=-1695/504, 7-8=-1873/467

- BOT CHORD 2-12=-308/1595, 10-12=-105/1056, 8-10=-310/1596
- WEBS 5-10=-152/681, 7-10=-392/266, 5-12=-153/677, 3-12=-393/265

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

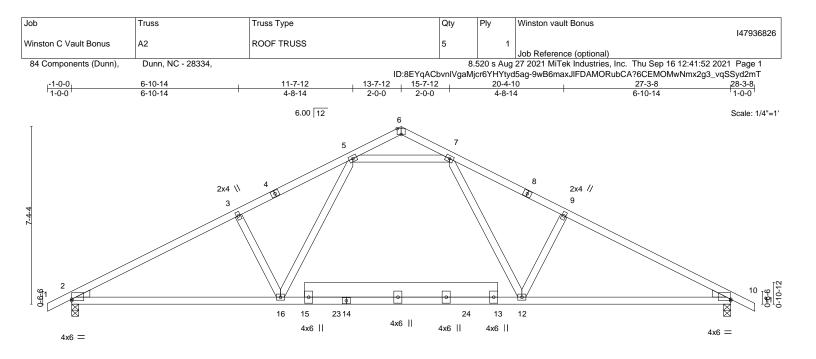
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=149, 8=149.





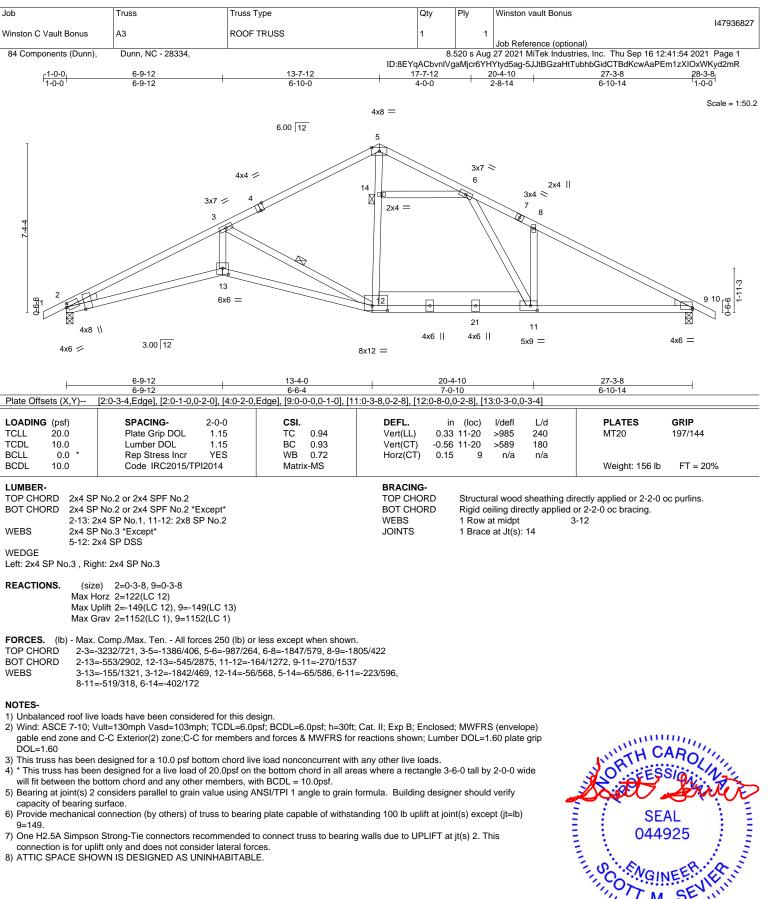


	8-7-12	+	18-7-12		+	27-3-8	
Plate Offsets (X,Y)	8-7-12 [2:0-0-0,0-0-12], [6:0-2-0,Edge], [10:0-0	-0,0-0-12]	10-0-0			8-7-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.78 WB 0.44 Matrix-MS	Vert(CT) -0.	20 16-19 >	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 153 lb	GRIP 197/144 FT = 20%
BOT CHORD 2x4 SI 13-15:	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 *Except* 2x8 SP No.2 P No.3 ght: 2x4 SP No.3		BRACING- TOP CHORD BOT CHORD			ectly applied or 3-8-15 or 10-0-0 oc bracing.	oc purlins.
Max H Max U Max C	e) 2=0-3-8, 10=0-3-8 Horz 2=122(LC 12) Jplift 2=-149(LC 12), 10=-149(LC 13) Grav 2=1152(LC 1), 10=1152(LC 1) . Comp./Max. Ten All forces 250 (lb) of	less except when shown					
TOP CHORD 2-3= BOT CHORD 2-16	-1846/462, 3-5=-1668/489, 7-9=-1668/4 =-299/1568, 12-16=-157/1191, 10-12=-3 =-103/596, 9-12=-394/235, 5-16=-103/5	39, 9-10=-1846/462 02/1568					
 2) Wind: ASCE 7-10; ' gable end zone and DOL=1.60 3) All plates are 3x4 M 4) This truss has been 5) * This truss has been 5) * This truss has been 6) Provide mechanical 2=149, 10=149. 	e loads have been considered for this de Vult=130mph Vasd=103mph; TCDL=6.0 I C-C Exterior(2) zone;C-C for members IT20 unless otherwise indicated. I designed for a 10.0 psf bottom chord live en designed for a live load of 20.0psf on sottom chord and any other members, w I connection (by others) of truss to bearin DWN IS DESIGNED AS UNINHABITABL	bsf; BCDL=6.0psf; h=30ft; and forces & MWFRS for re load nonconcurrent with the bottom chord in all are ith BCDL = 10.0psf. Ig plate capable of withsta	reactions shown; Lurr h any other live loads. eas where a rectangle	ber DOL=1.6	0 plate grip 2-0-0 wide	CONTRACT OF CONTRACT.	•



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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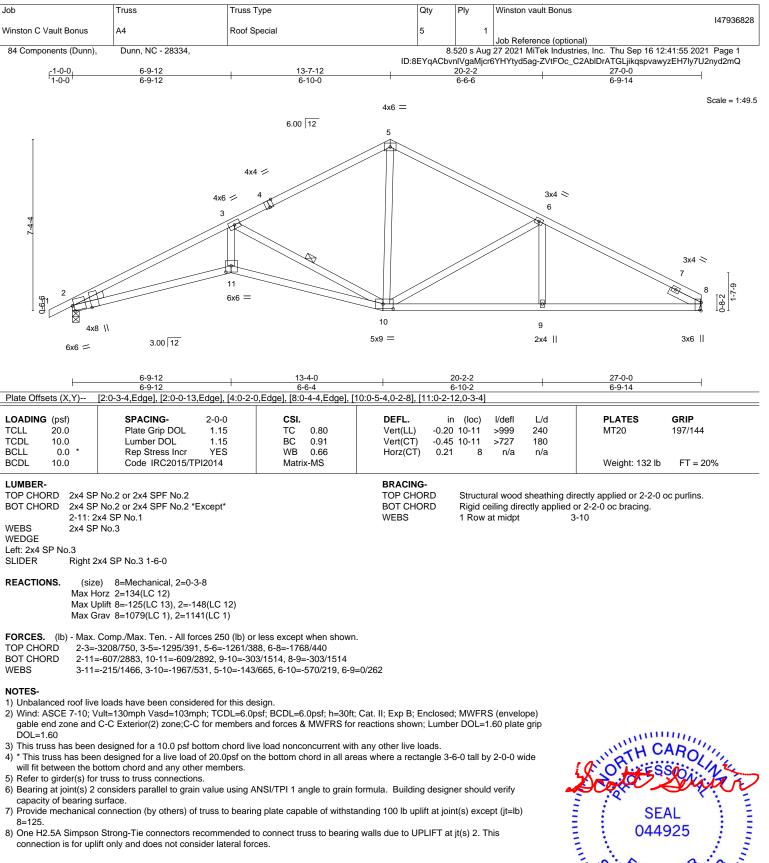
8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

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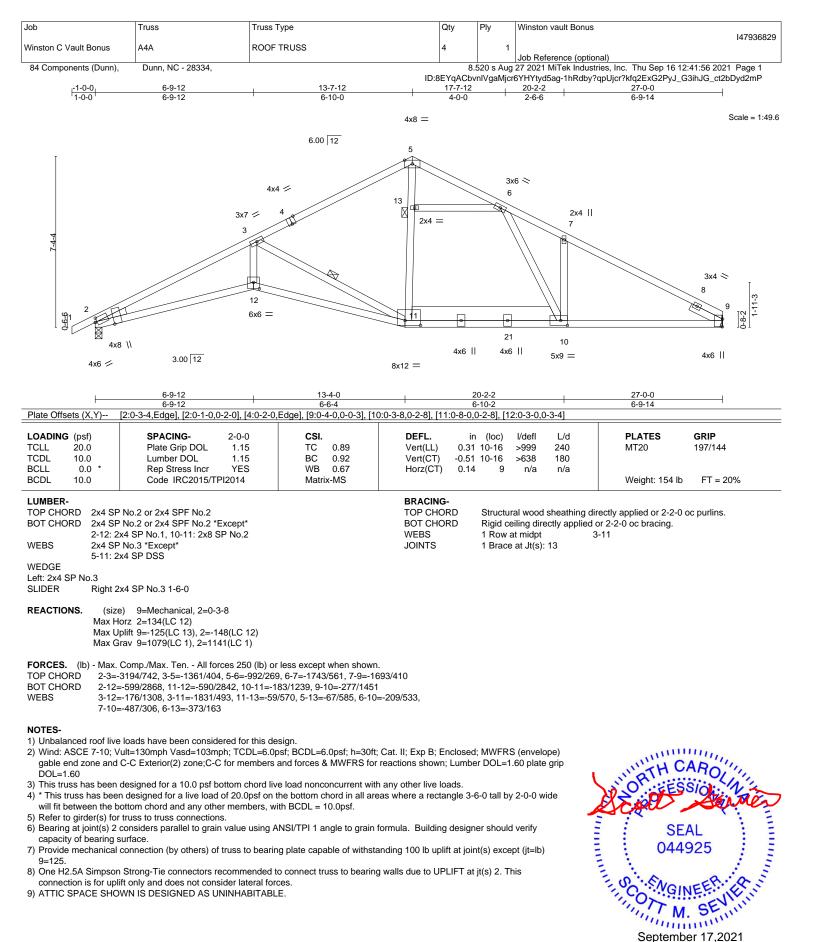






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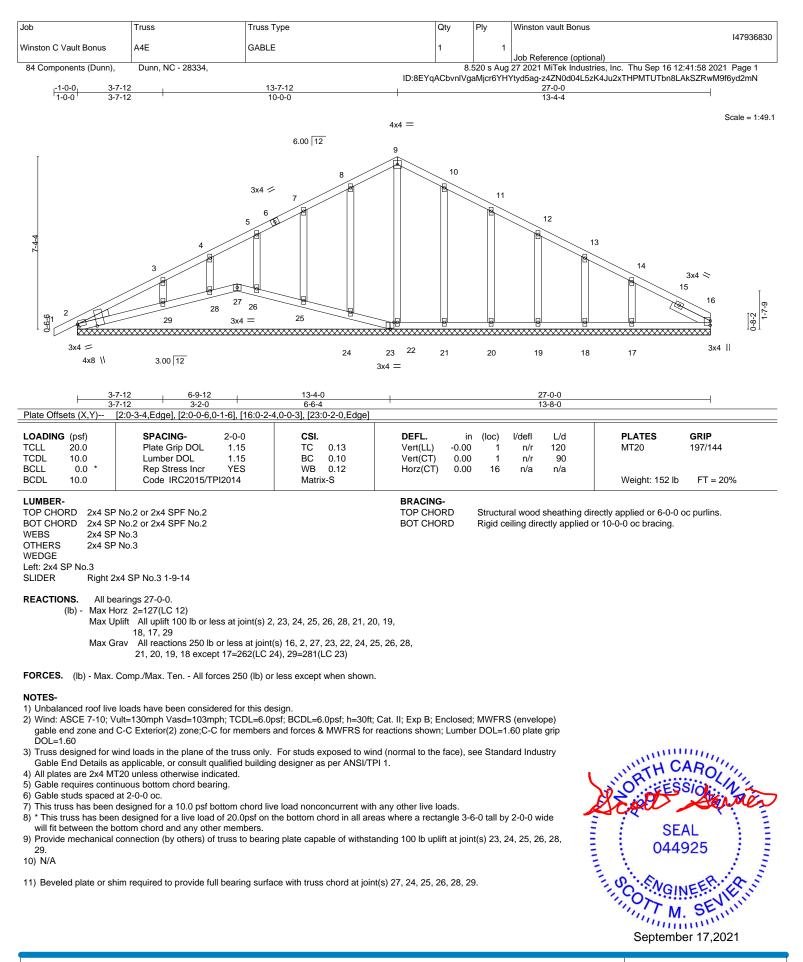




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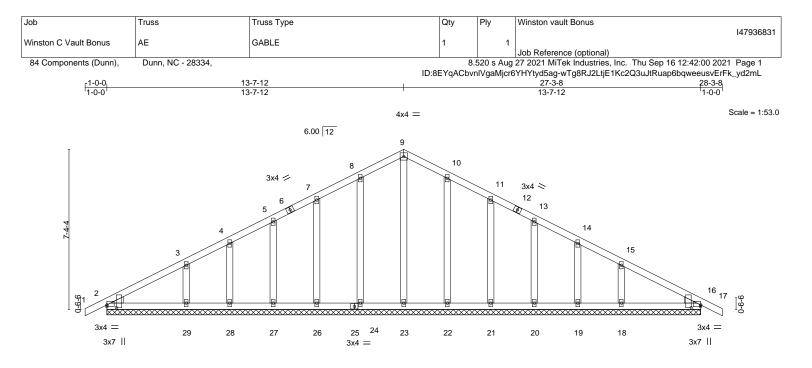
A MITIEK A 818 Soundside Road

Edenton, NC 27932



TRENCO AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932



late Offsets (X,Y) [2:0-0-0,0-1-0], [2:0-1-8,0-5-3], [16:E	dge,0-1-0], [16:0-1-8,0-5-3]						1	
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.13	Vert(LL)	0.00	17	n/r	120	MT20	197/144
TCDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT)	0.01	17	n/r	90		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.12	Horz(CT)	0.01	16	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 158 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

 IOP CHORD
 2x4 SP No.2 of 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 OTHERS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 27-3-8.

(lb) - Max Horz 2=122(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 2, 16, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18

Max Grav All reactions 250 lb or less at joint(s) 2, 16, 23, 24, 26, 27, 28, 22, 21, 20, 19 except 29=280(LC

23), 18=280(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

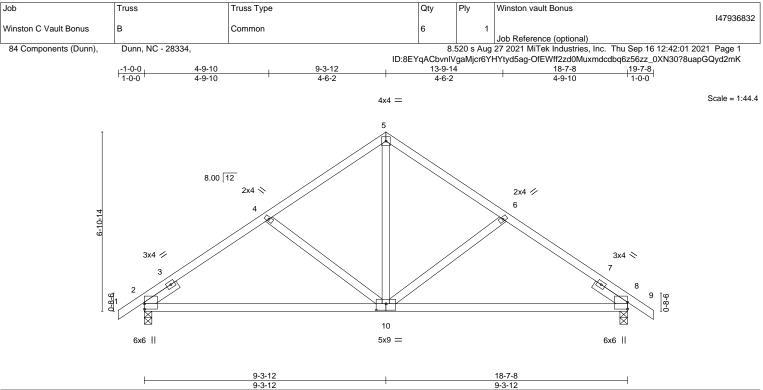
6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







	sets (X,Y)	[10:0-4-8,0-3-0]									
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.25	Vert(LL)	-0.10 10-17	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.71	Vert(CT)	-0.21 10-17	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.20	Horz(CT)	0.02 8	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MS					Weight: 95 lb	FT = 20%

LUMBER-

2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-170(LC 10) Max Uplift 2=-100(LC 12), 8=-100(LC 13) Max Grav 2=805(LC 1), 8=805(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-839/204, 4-5=-745/182, 5-6=-745/182, 6-8=-839/204 TOP CHORD

BOT CHORD 2-10=-130/779. 8-10=-73/746

WEBS 5-10=-77/520, 6-10=-279/182, 4-10=-279/182

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

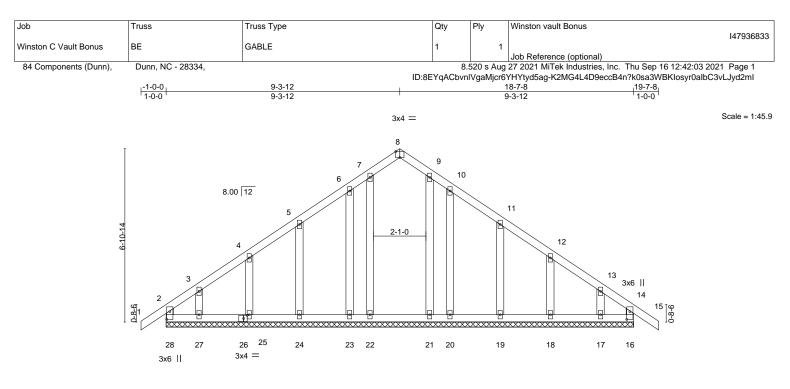
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.







			<u>18-7-8</u> 18-7-8		
Plate Offsets (X,Y)	[8:0-2-0,Edge], [14:0-3-12,0-1-8], [26:0-	1-12,0-1-8], [28:0-3-12,0-1	-8]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.13 BC 0.07 WB 0.06 Matrix-R	DEFL. ir Vert(LL) -0.00 Vert(CT) -0.01 Horz(CT) 0.00	15 n/r 120 15 n/r 90	PLATES GRIP MT20 197/144 Weight: 118 lb FT = 20%
	No.2 or 2x4 SPF No.2 No.2 or 2x4 SPF No.2 No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

REACTIONS. All bearings 18-7-8.

2x4 SP No.3

Max Horz 28=-170(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 28, 16, 23, 24, 25, 20, 19, 18 except 27=-125(LC 12), 17=-117(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 28, 16, 22, 21, 23, 24, 25, 27, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

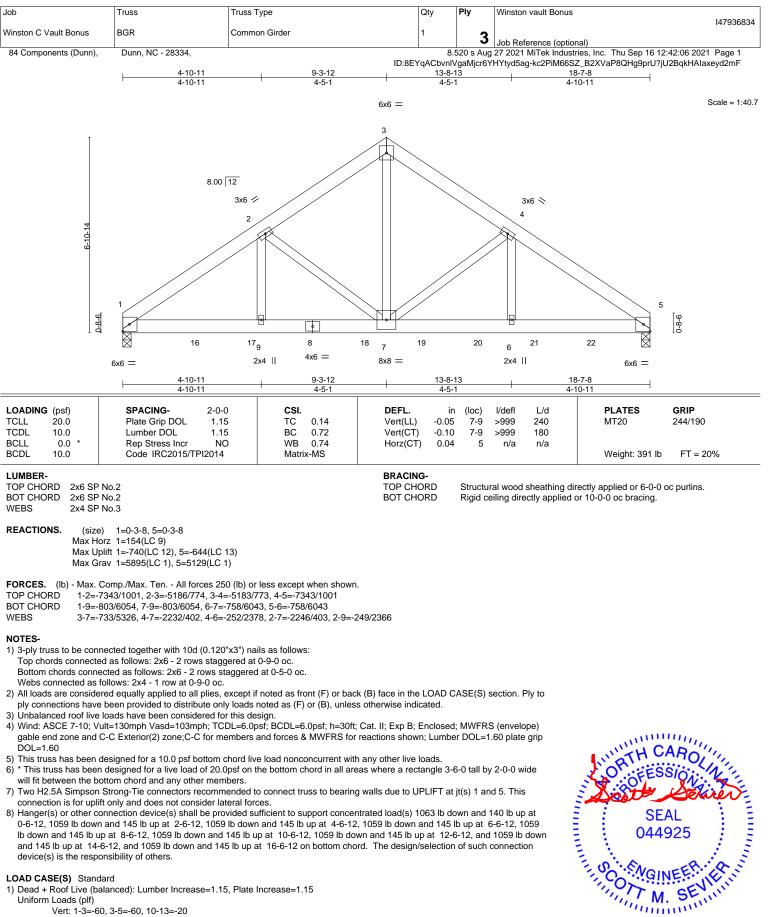
OTHERS

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.





¹⁾ Unbalanced roof live loads have been considered for this design.



Vert: 1-3=-60, 3-5=-60, 10-13=-20

Continued on page 2

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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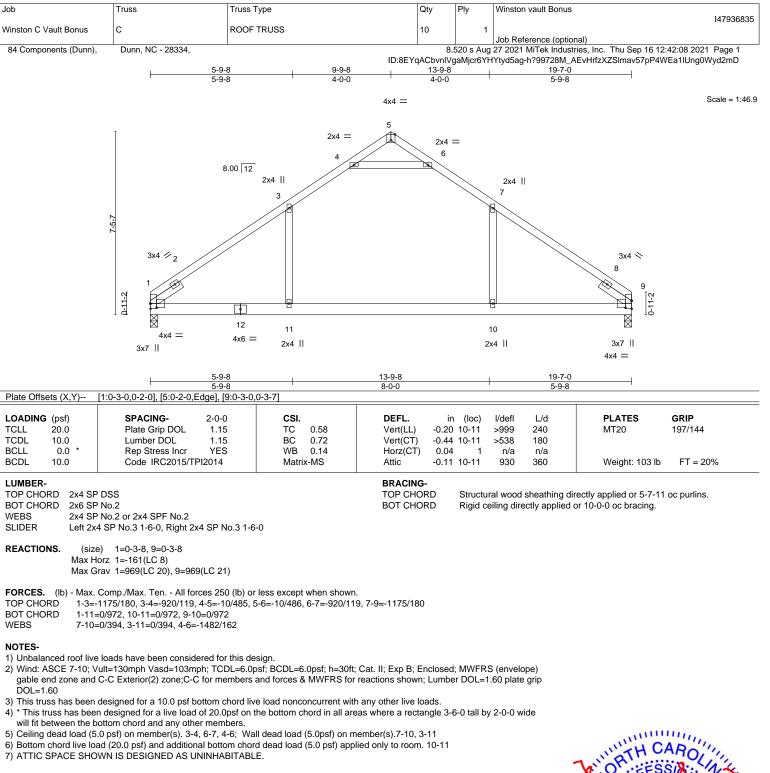
September 17,2021

Job	Truss	Truss Type	Qty	Ply		Winston vault Bonus
	505					147936834
Winston C Vault Bonus	BGR	Common Girder	1		3	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			8.520 s	s Aug	27 2021 MiTek Industries, Inc. Thu Sep 16 12:42:06 2021 Page 2
			ID:8EYqACI	vnlVgaM	/jcr6Yl	HYtyd5ag-kc2PiM66SZ_B2XVaP8QHg9prU?jU2BqkHAIaxeyd2mF

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 8=-1059(B) 12=-1063(B) 16=-1059(B) 17=-1059(B) 18=-1059(B) 19=-1059(B) 20=-1059(B) 21=-1059(B) 22=-1059(B)









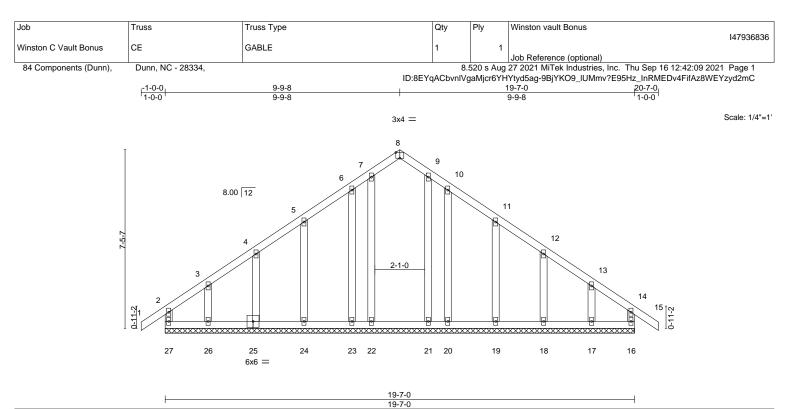


Plate Offsets (X,Y)	[8:0-2-0,Edge]				
.OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. i	n (loc) l/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.17	Vert(LL) -0.0	0 15 n/r 120	MT20 197/144
CDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) -0.0	1 15 n/r 90	
SCLL 0.0 *	Rep Stress Incr YES	WB 0.09	Horz(CT) 0.0	0 16 n/a n/a	
CDL 10.0	Code IRC2015/TPI2014	Matrix-R			Weight: 130 lb FT = 20%
UMBER-			BRACING-		
OP CHORD 2x4 SP	No.2 or 2x4 SPF No.2		TOP CHORD	Structural wood sheathing	directly applied or 6-0-0 oc purlins,
OT CHORD 2x4 SP	No.2 or 2x4 SPF No.2			except end verticals.	
/EBS 2x4 SP	No.3		BOT CHORD	Rigid ceiling directly applie	ed or 10-0-0 oc bracing.

OTHERS 2x4 SP No.3

REACTIONS. All bearings 19-7-0. (lb) - Max Horz 27=-178(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 27, 16, 23, 24, 25, 20, 19, 18 except 26=-146(LC 12), 17=-133(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 27, 16, 22, 21, 23, 24, 25, 26, 20, 19, 18, 17

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

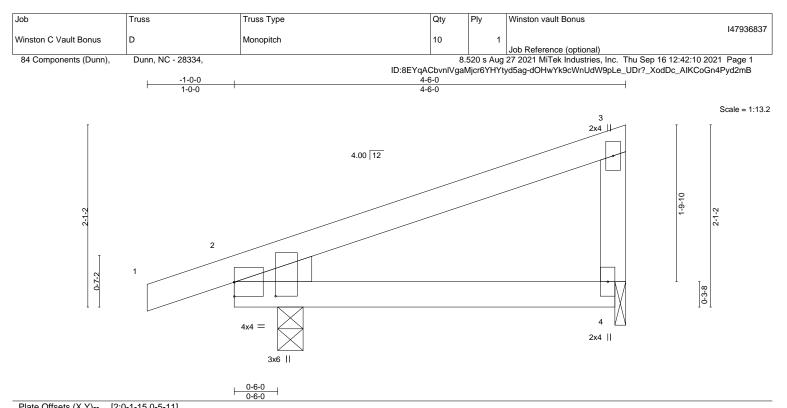
7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.







OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.18	Vert(LL) -0.01 4-9 >999 240	MT20 197/144
TCDL 10.0	Lumber DOL 1.15	BC 0.14	Vert(CT) -0.02 4-9 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 2 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP		Weight: 19 lb FT = 20%

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3
 BRACING

 TOP CHORD
 Structural wood sheathing directly applied or 4-6-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=73(LC 8) Max Uplift 2=-70(LC 8), 4=-37(LC 12) Max Grav 2=272(LC 1), 4=136(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.





