

RE: Penwell BFK Penwell BFK Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: D.R. HORTON - RAL - 055Project Name: Penwell BFK
Model: PENWELL BFKAddress:Subdivision:
State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.5 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 8 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	147712039	А	9/1/2021
2	147712040	A1	9/1/2021
3	147712041	A1E	9/1/2021
4	147712042	A2	9/1/2021
5	147712043	AE	9/1/2021
6	147712044	BE	9/1/2021
7	147712045	BGR	9/1/2021
8	147712046	CE	9/1/2021

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by 84 Components - #2383.

Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Sevier, Scott

September 01, 2021



4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 19 and 179 lb uplift

at joint 12.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions

shown; Lumber DOL=1.60 plate grip DOL=1.60 All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 18 and 163 lb uplift at joint 11.



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A MiTek Aff 818 Soundside Road Edenton, NC 27932



7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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TREERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

September 1,2021



	4-9-0	0-10-3	13-4-13		13-6-0	20-3-0	
	4-9-0	2-1-3	6-6-11		2-1-3	4-9-0	1
Plate Offsets (X,Y)	[2:0-2-0,0-1-12], [4:0-2-0,0-0-4], [6:0-2-0,0-1-12], [13:	:0-1-8,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0 Plate Grip DOL 1. Lumber DOL 1. Rep Stress Incr. VE	-0 CSI. 15 TC 15 BC	0.41 DEFL. 0.46 Vert(LL) 0.48 Herr(CT)	in (loc) -0.06 11-12 : -0.14 11-12 :	l/defl L/d >999 240 >914 180	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI201	4 Matrix	-S	0.02 8	11/a 11/a	Weight: 147 lb	FT = 20%
LUMBER-			BRACING-				

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-11-0 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 5-0-8 except (jt=length) 14=0-3-8, 10=0-3-8. (lb) - Max Horz 16=-86(LC 10)

Max Holz 16=-86(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 15, 9, 14, 10 except 16=-117(LC 12), 8=-119(LC 13) Max Grav All reactions 250 lb or less at joint(s) 15, 9, 14, 10 except 16=716(LC 1), 8=716(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-316/185, 3-4=-883/316, 4-5=-883/318, 5-6=-316/186, 2-16=-299/178,
- 6-8=-299/178
- BOT CHORD 15-16=-188/829, 14-15=-188/829, 12-14=-188/829, 11-12=-57/594, 10-11=-186/829, 9-10=-186/829, 8-9=-186/829
- WEBS 4-11=-81/297, 4-12=-79/297, 3-16=-712/127, 5-8=-712/126

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 9, 14, 10 except (jt=lb) 16=117, 8=119.



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Plate Offsets (X,Y) [1:0-2-5,0-2-4], [7:0-2-5,0-2-4], [9:0-4-0,	,0-4-8]						
LOADING (psf) SPACING- 2-0-0 TCLL 20.0 Plate Grip DOL 1.15 TCDL 10.0 Lumber DOL 1.15 BCLL 0.0 * Rep Stress Incr NO BCDL 10.0 Code IRC2015/TPI2014	CSI. TC 0.69 BC 0.54 WB 0.97 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.08 9-11 >999 240 Vert(CT) -0.17 9-11 >999 180 Horz(CT) 0.05 7 n/a n/a	PLATES GRIP MT20 244/190 MT18HS 244/190 Weight: 419 lb FT = 20%				
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP DSS WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 2-8-2, Right 2x4 SP No.3 2-8 REACTIONS. (size) 1=0-3-8 (req. 0-3-13), 7=0-3-8 (req. Max Horz 1=90(LC 31) Max Uplift 1=-904(LC 8), 7=-1029(LC 9) Max Grav 1=7266(LC 1), 7=8261(LC 1)	2 q. 0-4-5)	BRACING- TOP CHORD Structural wood sheathing of BOT CHORD Rigid ceiling directly applied SUPPLEMENTARY BEARING PLATES, SPECIAL A OTHER MEANS TO ALLOW FOR THE MINIMUM R WIDTH (SUCH AS COLUMN CAPS, BEARING BLC ARE THE RESPONSIBILITY OF THE TRUSS MANN OR THE BUILDING DESIGNER.	firectly applied or 6-0-0 oc purlins. I or 10-0-0 oc bracing. INCHORAGE, OR EQUIRED SUPPORT CKS, ETC.) JFACTURER				
Max Uplift 1=-904(LC 8), 7=-1029(LC 9) Max Grav 1=7266(LC 1), 7=8261(LC 1) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-3=-11903/1480, 3-4=8-712/120, 4-5=-8709/1119, 5-7=-12757/1589 BOT CHORD 1-13-11903/1480, 3-4=8-712/120, 4-5=-8709/1119, 5-7=-12757/1589 BOT CHORD 1-11=-1301/10236, 9-11=-1301/10236, 8-9=-1307/10998, 7-8=-1307/10998 WEBS 4-9=-853/7034, 5-9=-3726/569, 5-8=-470/4302, 3-9=-2836/456, 3-11=-361/3440 NOTES- 1) 3-ply truss to be connected together with 10d (0.120°x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 3 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 3 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 3 rows staggered at 0-9-0 oc. 3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connected as follows: 2x6 - 1 row at 0-9-0 0C. 3) Uholanced roof live loads have been considered for this design. 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0pst; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone: end verical left and right exposed; Lumber DCL=1.60 5) All plates are MT20 plates unless otherwise indicated. 6) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other imembers. 9) WarkING: Required bearing size to joint(5) 1, 7 greater than input bearing size. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=904, 7=1029.							

Continued on page 2

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September 1,2021

Job	Truss	Truss Type	Qty	Ply	Penwell BFK	
					I47712	2045
Penwell BFK	BGR	Common Girder	1	2		
				J	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	520 s Aug	27 2021 MiTek Industries, Inc. Tue Aug 31 14:42:01 2021 Page 2	2
		ID:2okNGRrr?KSPRIXqb91IA?y8hTY-mmIBJ3NHAnelq?cAk1qc1DUypWHSiysbNvwG76yiIVq				q

NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1393 lb down and 183 lb up at 2-0-12, 1393 lb down and 183 lb up at 4-0-12, 1393 lb down and 183 lb up at 6-0-12, 1393 lb down and 183 lb up at 10-0-12, 1393 lb down and 183 lb up at 12-0-12, 1393 lb down and 183 lb up at 12-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 16-0-12, 1393 lb down and 183 lb up at 18-9-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 10=-1393(B) 9=-1393(B) 12=-1393(B) 13=-1393(B) 14=-1393(B) 15=-1393(B) 16=-1393(B) 17=-1393(B) 18=-1393(B) 19=-1393(B) 1

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LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

BRACING-TOP CHORD

RD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-9-0.

(lb) - Max Horz 16=57(LC 11)

 Max Uplift
 All uplift 100 lb or less at joint(s) 16, 10, 14, 12, 15, 11

 Max Grav
 All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 12, 15, 11

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 12,

0) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 12, 15, 11.



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