LOT 11 WEST PARK **158 WEST PARK LANE** SANFORD, NC 27332 RIDGE VENT (TYP.) 0







 $\frac{\text{REAR ELEVATION}}{\text{SCALE: } 1/8" = 1'-0"}$



- 1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
- CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
- ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL SITE REQUIREMENTS.



ELECTRICAL: PIONEER



PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE **RESIDENTIAL BUILDING CODE.**



<u>8'-6" PORCH</u> PLATE HGT.



SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE A-4





SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE





ELECTRICAL LEGEND

ROUGH OPENINGS.

ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL

2.) VANITY LIGHTS TO BE SET @ 90/" A.F.F. (TYP.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE

4.) PLACE SWITCHES &" (MIN.) FROM

LOCATED BY ELECTRICIAN.

CELING FANS PER PLAN.

- 😑 110 y GFI OUTLET
- IV Y SWITCHED OUTLET
- BB 🕂 110 Y BASEBOARD OUTLET
- ₩ 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110Y GFI

- Ø 110 V DEDICATED CIRCUIT
- 120 Y DEDICATED CIRCUIT
- SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- PENDANT LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT

- \$ SWITCH
- \$_D DIMMER SWITCH

- TELEPHONE AND DATA
- TV- TV CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- 10 V SMOKE/ CM DETECTOR
- 5D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- $\overline{\mathbf{A}}$
- - CEILING FAN W/ LIGHT

LE E-1

ELECTRICAL

PLAN





SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE PLAN

E-2



ELECTRICAL LEGEND

ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.

2.) VANITY LIGHTS TO BE SET

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE

4.) PLACE SWITCHES 8" (MIN.) FROM

LOCATED BY ELECTRICIAN.

@ 90/" AFF. (TYP.)

ROUGH OPENINGS.

- 😑 110 y GFI OUTLET
- BB 🗢 110 Y BASEBOARD OUTLET
- + 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110Y GFI

- Ø 10 Y DEDICATED CIRCUIT
- 1 220 V DEDICATED CIRCUIT
- (240 Y, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- PENDANT LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT

- \$ SWITCH
- \$_D DIMMER SWITCH

- TV- TV CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- 110 Y SMOKE/ CO DETECTOR
- 5D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LVP LOW VOLTAGE PANEL
- ALARM ALARM PANEL
- - CEILING FAN W/ LIGHT



ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:

- 1. STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
- 2. FOR 120 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 6'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
- 3. FOR 130 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 4'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
- MEAN ROOF HEIGHT IS LESS THAN 30 FEET.
 EXTERIOR WALLS DESIGNED FOR 120 OR 130 MPH WINDS.
- 6. INSTALL 7/16" OSB SHEATHING ON ALL EXTERIOR WALLS OF ALL STORIES IN ACCORDANCE WITH SECTION R602.10.3 OF THE NCRC, 2018 EDITION.
- 7. ENERGY EFFICIENCY COMPLIANCE AND INSULATION VALUES OF THE BUILDING TO BE IN ACCORDANCE WITH CHAPTER 11 OF THE NCRC, 2018 EDITION.

ANCHO	HOR SPACING AND EMBEDMENT							
WIND ZONE	120 MPH	130 MPH						
SPACING	6'-0" O.C.	4'-0" O.C.						
EMBEDMENT	7"	7" INTO CONCRETE 15" INTO MASONRY						

NOTE: HORIZONTAL FOOTING REBAR REQUIRED IN HIGH WIND ZONES ONLY (140-150 MPH)





STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 4 (UNO).
- INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
 WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
- FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

BRACE WALL PANEL NOTES:

EXTERIOR WALLS: ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

GYPSUM: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

HD: 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1



—4 x 4 TRTD. POST MIN. (TYP.) RENAISSANCE RESIDENTIAL DESIGN, INC. RALEIGH, NC 27612 (919) 649-4128 WWW.RRDCAROLINA.COM ne art of transforming your vision into real RENAISSANCE RESIDENTIAL DESIGN, INC.. RESERVES THE RIGHT TO MAKE MODIFICATIONS TO FLOOR PLANS, DIMENSIONS, MATERIALS, AND SPECIFICATIONS WITHOUT NOTICE. THESE DRAWING ADE FOR THE THESE DRAWINGS ARE FOR THE PURPOSE OF CONVEYING AN ARCHITECTURAL CONCEPT ONLY. RENAISSANCE RESIDENTIAL DESIGN, INC. HERBY EXPRESSLY RESERVES ITS COMMON LAW COPYRIGHT AND OTHER PROPERTY RIGHTS IN THESE PLANS. THESE PLANS AND DRAWINGS ARE NOT TO BE REPRODUCED, CHANGED, OR COPIED IN ANY FORM OR MANNER WITHOUT EIGET ORTAINING THE EVERESS WITHOUT FIRST OBTAINING THE EXPRESS WRITTEN CONSENT OF RENAISSANCE RESIDENTIAL DESIGNS, INC. NOR ARE THEY TO BE ASSIGNED TO ANY THIRD PARTY WITHOUT FIRST OBTAINING SAID WITHTEN DERWINGON AND CONSTANT WRITTEN PERMISSION AND CONSENT [T] S T)

> RES. PROMOTIONS, INCENTIVES, FEATURES, OPTIONS ELOGR PLANS, ELEVATIONS, DESIGNS, MATERIALS AND MENSIONS ARE SUBJECT TO CHANGE WITHOUT NOTICE UMAR FOOTAGE AND DMICHSIONS ARE ESTIMATED AN V VARY IN ACTUAL CONSTRUCTION, ACTUAL POSITION V VARY IN ACTUAL CONSTRUCTION, ACTUAL POSITION AT PLAN, FLOOR PLANS AND ELEVATION RENDERINGS A BC ON LOT WILL BE DETEMBRIED N'THE SITE PLAN A AT PLAN, FLOOR PLANS AND ELEVATION RENDERINGS A RETO OF WEAVER HOMES ANY DELEVATION RENDERINGS A RETO OF WEAVER HOMES ANY DELEVATION TO PORTOCITIO OPERTY OF WEAVER HOMES ANY DELEVATION, OF REPOLUCTION OPERTY OF WEAVER HOMES ANY STRETTY PROHIBITED. SEE NEW HOME SALES CONSULTANT FOR UNDERVIDENT DETALS. COPYRIGHT © 2020 WEAVER HOMES



REV.:	
SCALE: 1/4" = 1'-0"	
DRAWN BY: WG	
ENGINEERED BY:	
REVIEWED BY:	

FRAMING PLAN

S-2

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN	Maximum Stud : (Per Tabl	SPACING (INCHES) E R602.3(5)
(. == .)	16	24
UP TO 3'	1	1
4'	2	1
8'	3	2
12'	5	3
16'	6	4

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

S-3

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



ATTIC VENT CALCULATION:

1040 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 6.9 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

- ALL FRAMING LUMBER TO BE #2 SPF (UNO).
 HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- 5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



WEAVER HOMES CAROLINA COLLECT HICKORY DRIVE LEFT

DATE: AUGUST 25, 2020
REV.:

SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

ROOF PLAN

S-4

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

ATTIC VENT CALCULATION:

1116 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 7.4 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).



8' 0"

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32

2. O.

4			1			1
Products Ples Net Qty Fab Type BM1 12'0" 1-3/4"x 16" LVL Kerto-S 2 2 FF BM2 15'0" 1-3/4"x 16" LVL Kerto-S 2 2 FF BM3 12'0" 2x10 SPF No.2 2 2 FF GDH 12'0" 2x12 SPF No.2 2 2 FF	Weaver Development Co. Inc.	Lot 11 West Park	Hickory "B"			J0921-5302
Dimension Notes 1. All exterior wall dimensions are to face of frame wall unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to russ dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to russ dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to russ dimensions are to face of frame wall unless noted otherwise Marked School Action Roof Area = 1468.1 sq.ft. Hip Line = 52.07 ft. Hip Line = 51.07 ft. Hip Line = 0 ft. Horizoff = 98.57 ft. Roded H = 159.04 ft. Drop Beam Manuel Manuel Quy Supported Member HuS26 USP 4 NA 16d/3-1/2" HUS26 USP 4	CITY / CO. Sanford / Lee	ADDRESS Lot 11 West Park	MODEL Roof	DATE REV. 09/07/21	DRAWN BY David Landry	SALES REP. Lenny Norris
	Bearing deemed requirem attached requirem attached requirem attached requirem attached requirem attached reactions. Tables. / retained reactions. Signatur S	AD CH. (3435E) (345	Contraction of the support of the su	A FLU & FLU & B ndustr e, N.C. 0) 864-4 nor equa e prescrip of shall re- stude requa- e stude requa- port system of shall re- port system of Lan OR JAC STRONG FUNCTION STRONG	CLEAN COOF EAN ial Par 28309 -8787 444 I to 3000 4444 I to 3000 inter a ional shale infor any clean for all andry CLEAN	Image: second

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

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ATIGE			Intension Notes 1. All exterior wall to wall dimensions are to face of sharing unless noted otherwise 2. All interior wall unless noted otherwise 3. All exterior wall unless noted otherwise 3. All Walls Shown Are Considered Load Bearing Roof Area = 1465.1 sqlt. Higge Line = 52:07.1. Higge Line = 52:07.1. Hoz. Of 1996.57.1. Decking = 50 sheets Image Line = 159.04.1. Dorp Beam Connector Information Nail Information Sym< Product Manuf Qty	TY / CO. Sanford / Lee 1200 10500 111000 1200 1200		ART F(CONTABLE		CK STL CAUN BY 1 Set Even of Links 340 1 1 360 1 1 360 1 1 360 1 1 360		
			HUS26 USP 4 NA 16d/3-1/2" 16d/3-1/2" Products Products Plies Net Qty Fab Type BM1 12'0" 1-3/4"x 16" LVL Kerto-S 2 2 BM2 15'0" 1-3/4"x 16" LVL Kerto-S 2 2 BM3 12'0" 2x10 SPF No.2 2 2 GDH 12'0" 2x12 SPF No.2 2 FF	Weaver Development Co. Inc. CI	AE Lot 11 West Park AD	Hickory "B" MC	TE	DR	J0921-5302 SA	
	ł			BUILDER	JOB NAM	PLAN	SEAL DAT	QUOTE #	JOB #	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

Trenco RE: J0921-5302 818 Soundside Rd Lot 11 West Park Edenton, NC 27932 Site Information: Customer: Weaver Homes Project Name: J0921-5302 Lot/Block: 11 Model: Hick Model: Hickory Address: 158 West Park Lane Subdivision: West Park State: NC City: Sanford General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.3 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf This package includes 25 individual, dated Truss Design Drawings and 0 Additional Drawings. No. Seal# Truss Name Date No. Seal# Truss Name Date 3/3/2021 E15459286 A1 3/3/2021 21 E15459306 V3 1 2 E15459287 A1GE V4 3/3/2021 3/3/2021 22 E15459307 3 E15459288 A2 3/3/2021 23 E15459308 V5 3/3/2021 4 E15459289 A3 3/3/2021 E15459309 V6 3/3/2021 24 5 E15459290 A4 3/3/2021 25 E15459310 V7 3/3/2021 6 A5 3/3/2021 E15459291 7 E15459292 A5GE 3/3/2021 8 E15459293 B1 3/3/2021 B1GE 9 E15459294 3/3/2021 10 E15459295 B2 3/3/2021 E15459296 C1 3/3/2021 11

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

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E15459297

E15459298

E15459299

E15459300

E15459301

E15459302

E15459303

E15459304 E15459305

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

C1GE

D1-GR

D1GE

M1GE

M2-GR

V1GE

M1

M2

V2

3/3/2021

3/3/2021

3/3/2021

3/3/2021

3/3/2021

3/3/2021

3/3/2021

3/3/2021

3/3/2021

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Gilbert, Eric

March 03, 2021

	10-2-8	22-2-8	<u>32-5-0</u>
	10-2-8	12-0-0	10-2-8
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. DEFL. in (loc) TC 0.28 Vert(LL) -0.34 10-13 BC 0.64 Vert(CT) -0.47 10-13 WB 0.27 Horz(CT) 0.05 8 Matrix-S Wind(LL) 0.05 2-13	I/defi L/d PLATES GRIP >999 360 MT20 244/190 >824 240 n/a n/a >999 240 Weight: 208 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to

33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22,

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

March 3,2021

MARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

	10-2-8	16-2-8	19-2-8 21-1	-8 24-2-8	30-1-8	8 32-5-0	
	10-2-8	6-0-0	3-0-0 1-11	-0 3-1-0	5-11-0	0 2-3-8	
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0-4	-0,Edge], [12:0-2-0,0-1-4], [[14:0-4-8,0-1-4], [15:0-	3-8,0-4-12]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.75 BC 0.67 WB 0.70 Matrix-S	DEFL. ir Vert(LL) -0.21 Vert(CT) -0.38 Horz(CT) 0.09 Wind(LL) 0.17	n (loc) l/defl 17 >999 3 17 >999 9 10 n/a 7 2-17 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 247 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SI BOT CHORD 2x10 S 10-15: WEBS 2x4 SI REACTIONS. (siz Max H	P No.1 SP No.1 *Except* 2x6 SP 2400F 2.0E P No.2 te) 2=0-3-8, 10=0-3-8 forz 2=-110(LC 10) bit 2 = 0.00(LC 12)		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood Rigid ceiling dir 1 Row at midpt	d sheathing dire rectly applied or 5-	ectly applied or 4-0-8 c r 10-0-0 oc bracing. 7	oc purlins.
FORCES. (lb) - Max. TOP CHORD 2-4= BOT CHORD 2-17 WEBS 4-17	Srav 2=1393(LC 2), 10=1353(LC 2) . Comp./Max. Ten All forces 250 (lb) or -2217/403, 4-5=-1870/483, 7-9=-2258/5 =-193/1848, 15-17=-195/1860, 13-15=-3 =-29/402, 7-15=-114/967, 9-15=-1075/2	less except when shown. 19, 9-10=-2889/551 71/2525, 10-13=-380/2525 32, 9-13=0/616, 5-7=-1955/4	459				
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-10; MWFRS (envelope)	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=103 and C-C Exterior(2) -0-8-10 to 3-8-3, Ini	sign. nph; TCDL=6.0psf; BCDL= erior(1) 3-8-3 to 16-2-8, Ex	6.0psf; h=15ft; Cat. II; terior(2) 16-2-8 to 20-7	Exp C; Enclosed 7-5, Interior(1) 20-	; -7-5 to		

33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

L	9-11-8	18-11-8	20-10-8	3 23-11-8	8 29-10-8	32-2-0	
	9-11-8	9-0-0	1-11-0	3-1-0	5-11-0	2-3-8	
Plate Offsets (X,Y)	[1:0-9-6,0-1-2], [5:0-3-0,Edge], [9:0-4-0,	Edge], [11:0-2-0,0-1-4], [13:0-	4-4,0-1-4], [14:0-3-	8,0-4-12]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.76 BC 0.66 WB 0.70 Matrix-S	DEFL. i Vert(LL) -0.2' Vert(CT) -0.3f Horz(CT) 0.09 Wind(LL) 0.1f	n (loc) 1 16 : 5 16 : 9 9 5 1-16 :	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 243 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 S BOT CHORD 2x10 S 9-14: 2x4 S	P No.1 SP No.1 *Except* 2x6 SP 2400F 2.0E P No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structura Rigid cei 1 Row at	al wood sheathing dir lling directly applied o t midpt 4	rectly applied or 3-10-0 or 10-0-0 oc bracing. -6	oc purlins.
REACTIONS. (siz Max Max Max	ze) 1=Mechanical, 9=0-3-8 Horz 1=-111(LC 8) Jplift 1=-76(LC 12), 9=-90(LC 13) Grav 1=1345(LC 2), 9=1347(LC 2)						
FORCES. (lb) - Max TOP CHORD 1-3= BOT CHORD 1-16 WEBS 6-14	. Comp./Max. Ten All forces 250 (lb) or 2189/401, 3-4=-1853/491, 6-8=-2237/5 5=-198/1827, 14-16=-200/1839, 12-14=-3 I=-117/966, 3-16=-53/392, 4-6=-1931/474	less except when shown. 17, 8-9=-2874/549 75/2511, 9-12=-384/2511 4, 8-14=-1081/228, 8-12=0/620	0				
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-10:	re loads have been considered for this de Vult=130mph (3-second aust) Vasd=103	sign. mph: TCDL=6.0psf: BCDL=6.0)psf: h=15ft: Cat. II:	Exp C: En	closed:		

MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.

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Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

0.05

12 >999 240

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 4-11-9 oc purlins.

Weight: 204 lb

FT = 20%

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BCDL

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

10.0

2x4 SP No 2 WFBS

REACTIONS. 1=Mechanical, 7=0-3-8 (size) Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13) Max Grav 1=1278(LC 1), 7=1331(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2015/TPI2014

TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-S

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

March 3,2021

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE 🛕 WARNING - Verify design pa Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

	10-2-8 10-2-8	22-2-8 12-0-0		+	32-5-0 10-2-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. DEFL. TC 0.29 Vert(L BC 0.65 Vert(C WB 0.27 Horz(C Matrix-S Wind(I	in (loc)) -0.34 9-12 [) -0.47 9-12 T) 0.05 7 L) 0.05 12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	PLATES MT20 Weight: 206 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to

33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

a) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

March 3,2021

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Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22, 21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.

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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 12 except (jt=lb) 15=181, 11=169.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.

March 3,2021

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	2-0-8 2-0-8	5-11-8 3-11-0	<u> </u>	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.17 BC 0.23 WB 0.18 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 8-9 >999 360 Vert(CT) -0.04 8-9 >999 240 Horz(CT) 0.00 6 n/a n/a Wind(LL) 0.01 8-9 >999 240	PLATES GRIP MT20 244/190 Weight: 184 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x6 SP No.1 *Except*
	1-2: 2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1 *Except*
	4-8,3-8: 2x4 SP No.2
SLIDER	Right 2x4 SP No.2 -H 3-9-13

REACTIONS. (size) 10=0-3-8, 6=0-3-8 Max Horz 10=-131(LC 8) Max Grav 10=859(LC 1), 6=555(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-10=-363/0, 1-2=-630/0, 3-4=-478/103, 4-6=-578/69 TOP CHORD BOT CHORD 9-10=0/631, 8-9=0/325, 6-8=0/334 WEBS 4-8=0/359, 2-9=-497/2, 2-3=-404/132

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
- Top chords connected as follows: 2x6 2 rows staggered at 0-9-0 oc, 2x4 1 row at 0-9-0 oc.
- Bottom chords connected as follows: 2x6 2 rows staggered at 0-9-0 oc.
- Webs connected as follows: 2x4 1 row at 0-9-0 oc, 2x6 2 rows staggered at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 2-4-12, Interior(1) 2-4-12 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 5) Provide adequate drainage to prevent water ponding.6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified.
- Building designer must review loads to verify that they are correct for the intended use of this truss.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-260, 3-4=-60, 4-7=-60, 6-10=-20

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 1-2.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Lot 11 West Park	
					E1545	59295
J0921-5302	B2	ROOF SPECIAL	1	2		
				_	Job Reference (optional)	
Comtech, Inc, Fayette	rille, NC - 28314,			8.330 s Oo	t 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:08 2021 Page	2

8.330 s Oct 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:08 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-U1cahM0RHI?pvFZkRA8yTkvRiguogdGmvs8Oo1zevvb

LOAD CASE(S) Standard
 Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
Vert: 1-2=-250, 3-4=-50, 4-7=-50, 6-10=-20 3) Dead + Lininhabitable Attic Without Storage: Lumber Increase 1 25 Plate Increase 1 25
Uniform Loads (plf)
Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-40 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 3-4=-39, 4-12=47, 6-12=39, 6-7=32
 Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-170, 3-4=35, 4-11=27, 6-11=35, 6-7=58, 6-10=-12
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=-51, 6-10=-20
Horz: 3-4=38, 4-6=-38, 6-7=-31
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=11, 6-10=-20 Horz: 3-4=-38, 4-6=-38, 6-7=31
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=-179, 3-4=-13, 4-6=11, 6-7=4, 6-10=-12
Horz: 3-4=1, 4-6=23, 6-7=16
Uniform Loads (plf)
Vert: 1-2=-191, 3-4=11, 4-6=-13, 6-7=2, 6-10=-12 Horz: 3-4=-23, 4-6=-1, 6-7=14
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (pir) Vert: 1-2=-201, 3-4=-35, 4-6=-11, 6-7=-4, 6-10=-20
Horz: 3-4=15, 4-6=9, 6-7=16 11) Dead + 0.6 MWERS Wind (Neg. Internal) Bight: Lumber Increase=1.60. Plate Increase=1.60.
Uniform Loads (plf)
Vert: 1-2=-213, 3-4=-11, 4-6=-35, 6-7=-28, 6-10=-20 Horz: 3-4=-9, 4-6=-15, 6-7=-8
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12
Horz: 3-4=-33, 4-6=21, 6-7=14 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12 Horz: 3-4=-21, 4-6=33, 6-7=26
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12
Horz: 3-4=-33, 4-6=21, 6-7=14 15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 3-4=-21, 4-6=33, 6-7=26
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (olf)
Vert: 1-2=-201, 3-4=-1, 4-6=-13, 6-7=-6, 6-10=-20
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 3-4=-7, 4-6=19, 6-7=26
 Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf)
Vert: $1-2=-220$, $3-4=-20$, $4-7=-20$, $6-10=-20$ 19) Dead + 0.75 Proof Live (ha) + 0.75(0.6 MW/EPS Wind (Neg. lat) Left): Lumber Increase-1.60. Plate Increase-1.60
Uniform Loads (plf)
vert: 1-2=-236, 3-4=-61, 4-6=-43, 6-7=-38, 6-10=-20 Horz: 3-4=11, 4-6=7, 6-7=12
20) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-245, 3-4=-43, 4-6=-61, 6-7=-56, 6-10=-20
Horz: 3-4=-7, 4-6=-11, 6-7=-6

21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidal truss event and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Lot 11 West Park
					E15459295
J0921-5302	B2	ROOF SPECIAL	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,			8.330 s Oo	t 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:08 2021 Page 3

'age ID:1yUksKymplk2404ufZYCrxyoKUD-U1cahM0RHI?pvFZkRA8yTkvRiguogdGmvs8Oo1zevvb

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-36, 4-6=-45, 6-7=-40, 6-10=-20

Horz: 3-4=-14, 4-6=5, 6-7=10

22) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-245, 3-4=-45, 4-6=-36, 6-7=-31, 6-10=-20 Horz: 3-4=-5, 4-6=14, 6-7=19

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-260, 3-4=-60, 4-7=-20, 6-10=-20

- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-220, 3-4=-20, 4-7=-60, 6-10=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-250, 3-4=-50, 4-7=-20, 6-10=-20 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-50, 6-10=-20

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			0-3-0		
	1		8-3-8		
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.84 BC 0.24 WB 0.00 Matrix-P	DEFL. ir Vert(LL) -0.05 Vert(CT) -0.10 Horz(CT) 0.00 Wind(LL) 0.10	n (loc) I/defl L/d 5 2-5 >999 360 0 2-5 >969 240 0 5 n/a n/a 0 2-5 >886 240	PLATES GRIP MT20 244/190 Weight: 37 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x6 SF	P No.1 P No.1 P No.2 P No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing o except end verticals. Rigid ceiling directly applied	directly applied or 5-3-4 oc purlins, d or 10-0-0 oc bracing.

0 2 0

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss expletens, see **AVSUPPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

			8-3-8		
			8-3-8		I
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [12:0-2-0,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	DEFL. ir Vert(LL) 0.09 Vert(CT) -0.08 Horz(CT) -0.00	n (loc) l/defl L/d 9 10-11 >999 240 8 10-11 >999 240 9 10-11 >999 240 0 8 n/a n/a	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 41 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP OTHERS 2x4 SP 8-12: 2	P No.1 P No.1 P No.2 P No.2 *Except* x6 SP No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8) Max Uplift 2=-216(LC 8), 8=-188(LC 8) Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.

Waren 5,20

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4x4 ||

Scale = 1:28.1

		<u>3-11-8</u> 3-11-8	+ 7	7-11-0 3-11-8				
Plate Offsets (X,Y)	[1:0-0-4,0-0-5], [1:0-0-8,0-3-6],	[3:0-0-4,0-0-5], [3:0-0-8,0-3-6]					1	
LOADING (psf)	SPACING- 2-0	-0 CSI .	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.1	5 TC 0.38	Vert(LL) -0.02	3-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.1	5 BC 0.57	Vert(CT) -0.04	3-4	>999	240		
BCLL 0.0 *	Rep Stress Incr N	O WB 0.39	Horz(CT) 0.01	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	4 Matrix-P	Wind(LL) 0.01	3-4	>999	240	Weight: 100 lb	FT = 20%
LUMBER-			BRACING-					

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No 2 WFBS WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=91(LC 24) Max Uplift 1=-191(LC 8), 3=-180(LC 9) Max Grav 1=2919(LC 1), 3=2779(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 1-2=-2418/177, 2-3=-2418/177 1-4=-100/1678, 3-4=-100/1678 BOT CHORD 2-4=-154/3142 WEBS

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 1-3=-20

🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

1-0-3

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job	Truss	Truss Type	Qty	Ply	Lot 11 West Park
J0921-5302	D1-GR	Common Girder	1	•	E15459298
				2	Job Reference (optional)
Comtech, Inc, Fayettev	rille, NC - 28314,			8.330 s Oo	ct 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:11 2021 Page 2
		ID:	1yUksKyn	plk2404uf	ZYCrxyoKUD-ucljJO2KagNOmjlJ6lif5MXvdurJtwsCbpN2OMzevvY

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)

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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x6 SP No.1 OP CHORD BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No 2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

- (lb) -Max Horz 2=-118(LC 10)
 - Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152.8=148.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

REACTIONS. All bearings 7-11-0.

			5-0-0		1
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code JPC2015/TPI2014	CSI. TC 0.28 BC 0.08 WB 0.00 Matrix-P	DEFL. in (loc) // Vert(LL) -0.01 2-4 > Vert(CT) -0.01 2-4 > Horz(CT) 0.00 Wind(LL) 0.01 2-4 >	/defl L/d 999 360 999 240 n/a n/a 999 240	PLATES GRIP MT20 244/190 Weight: 24 lb ET = 20%
DODL 10.0		IVIAU IA-1	Wild(EE) 0:01 2-4 >	333 240	Weight: 24 lb 11 = 20 %

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=63(LC 8) Max Uplift 2=-102(LC 8), 4=-79(LC 8)

Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

- capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.

Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

March 3,2021

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			5-0-0					1	
LOADING (psf) SPACING- TCLL 20.0 Plate Grip DOL TCDL 10.0 Lumber DOL BCLL 0.0 * Rep Stress Incr BCDL 10.0 Code IRC2015/T	2-0-0 1.15 1.15 YES PI2014	CSI. TC 0.09 BC 0.09 WB 0.02 Matrix-S	DEFL. Vert(LL) 0. Vert(CT) -0. Horz(CT) -0.	in (I)1)1)1)0	(loc) 8 8 6	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 27 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8)

Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This trues has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

- capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147, 6=115.

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Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 11 West Park
10004 5000		LL-KLP-			E15459302
J0921-5302	M2	нагнр	3	1	Job Reference (optional)
Comtech, Inc, Fay	etteville, NC - 28314,			8.330 s O	ct 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:13 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:13 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-r_PTk34a6ld6?0ShEjk7AndHJhbvKwNV37s8TEzevvW

LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 9=-350
3) Dead + Orlinitabilable Attic Without Storage. Lumber increase=1.25, Plate increase=1.25
Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40
Concentrated Loads (lb)
Vert: 9=-300
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=70, 2-3=26, 3-4=153, 5-6=12, 2-6=52, 8-10=115, 7-10=52
1012. 1-202, 2-370, 3-4100, 3-300
Vert: 9=548
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52
H012: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
Vart 0–566
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
Concentrated Loads (ID)
Volt. 3=420 7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
Concentrated Loads (lb)
Vell. 9=-420 8) Dead + 0.6 MWFPS Wind (Pos. Internal) eff: Lumber Increase - 1.60. Plate Increase - 1.60.
Uniform Loads (olf)
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10
Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
Concentrated Loads (lb)
Vert: 9=154
9) Dead + 0.5 MWPKS WITH (FOS. Internal) Right. Lumber Increase=1.00, Plate Increase=1.00
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
Concentrated Loads (lb)
Vert: 9=43
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber increase=1.60, Plate increase=1.60
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
Concentrated Loads (lb)
Vert: 9=-339
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Unition Loads (pin) Vert: 1-2=-2 2-3=-9 3-4=-2 5-6=-21 2-7=-20
Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
Concentrated Loads (Ib)
Vert: 9=-234
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (pii)
Har: $1-2-16$, $2-3-2$, $3-3-14$, $-20-11$, $2-1-12$
Concentrated Loads (lb)
Vert: 9=43
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
veri. 1-2=2, 2-3=3, 3-4=2, 3-0=1, 2-7=-12 Horr: 1.2=-14, 2:3=-21, 3:4=-27
Concentrated Loads (h)
Vert: 9=43
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12
пиг. 1-2=-20, 2-3=-33, 3-4=-20, 3-3=-39 Concentrated Loads (Ih)
Vert: 9=43
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60

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Job	Truss	Truss Type	Qty	Ply	Lot 11 West Park
1000 / 5000					E15459302
J0921-5302	M2	Half Hip	3	1	Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,			8.330 s Oo	t 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:13 2021 Page 3

8.330 s Oct 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:13 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-r_PTk34a6ld6?0ShEjk7AndHJhbvKwNV37s8TEzevvW

LOAD CASE(S) Standard	
Uniform Loads (plf)	
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12	
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27	
Vert 9=43	
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-7=-20	
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12	
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20	
Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0	
Concentrated LOads (Ib)	
18) Dead: Lumber Increase=0.90. Plate Increase=0.90 Plt. metal=0.90	
Uniform Loads (plf)	
Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20	
Concentrated Loads (lb)	
Vert: 9=-200 (1) Dead Live (hel.) + 0.75 Attic Floor + 0.75(0.6 MWEDS Wind (blog lat) Left) Lumber Jacobeo (1.60 Plate Jacobeo (1.60	
19) Dead + 0.75 KOOL Live (Dal.) + 0.75 Attic FIOOL + 0.75(0.6 MWYRKS WING (Neg. Int) Leit). Lumber increase=1.60, Flate increase=1.60 Uniform Loads (olf)	
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3	
Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26	
Concentrated Loads (lb)	
Verit: 9=-454 20) Dead - 0.75 Roof Live (bal) + 0.75 Attic Floor + 0.75/0.6 MW/ERS Wind (Neg. Int) Right): Lumber Increase-1.60 Plate Increase-1.60	
Uniform Loads (olf)	
Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20	
Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0	
Concentrated Loads (Ib)	
Vert: 9=-3/5	60
Uniform Loads (plf)	00
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20	
Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9	
Concentrated Loads (Ib)	
22) Dear 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWERS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60 Plate Increase=1	60
Uniform Loads (plf)	
Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20	
Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0	
Concentrated Loads (Ib)	
23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15. Plate Increase=1.15	
Uniform Loads (plf)	
Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20	
Concentrated Loads (lb)	
Vert: 9=-400 24) 2nd Pad + Roof Live (unbalanced): Lumber Increase=1.15. Plate Increase=1.15	
Uniform Loads (plf)	
Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20	
Concentrated Loads (lb)	
Vert: 9=-400 25) 2d Dead + 0.27 Read i in (unbelanced) - unber lances a 4.5.	
23) Sid Dead + 0.75 Kool Live (unbalanced). Lumber inclease=1.15, Plate inclease=1.15 Uniform Loads (olf)	
Vert: 1-3=-50, 3-4=-50, 5-6=-100, 2-7=-20	
Concentrated Loads (lb)	
Vert: 9=350	
20) 4m Dead + 0.75 Koot Live (unbalanced): Lumber increase=1.15, Plate increase=1.15 I bitiom Loads (off)	
Vert: 1-3-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20	
Concentrated Loads (lb)	
Vert: 9=-350	

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Job Truss	Truss Type	Qty PI	Ply	Lot 11 West Park
J0921-5302 M2-GR	HALF HIP	1	2	E15459303
			-	Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-400 8.330 s Oct 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:14 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-JAzryP4CtblzdA1uoQFMj?9Uy5yf3NdfHnbi?hzevvV

	Uniform Loads (plf)
	Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20
	Concentrated Loads (lb)
~	Vert: 9=-350
3)	Dead + Uninnabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
	Vert: 1.320. 3-420. 5-6160. 2-740
	Concentrated Loads (Ib)
	Vert: 9=-300
4)	Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
,	Uniform Loads (plf)
	Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52
	Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
	Concentrated Loads (lb)
C)	Vert: 9=548
5)	Lipiform Loads (nif)
	Vert: 1-2=51 2-3=58 3-4=51 5-6=-78 2-8=52 8-10=115 7-10=52
	Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
	Concentrated Loads (Ib)
	Vert: 9=566
6)	Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
	Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
	Vort: 9-420
7)	Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60 Plate Increase=1.60
• /	Uniform Loads (plf)
	Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
	Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
	Concentrated Loads (Ib)
~	Vert: 9=-420
8)	Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2-36 2-3-21 3-4-14 5-6-131 2-8-10 8-10-33 7-10-10
	Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
	Concentrated Loads (Ib)
	Vert: 9=154
9)	Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12
	Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
	Vort: 0-43
10) Dead + 0.6 MWERS Wind (Neg. Internal) Left: Lumber Increase=1.60. Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
	Concentrated Loads (Ib)
	Vert: 9=-339
11) Dead + 0.6 MWFRS wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2=-2 2-3=-9 3-4=-2 5-6=-141 2-7=-20
	Horz: 1-2=-18. 2-3=-11. 3-4=-18. 3-5=-0
	Concentrated Loads (Ib)
	Vert: 9=-234
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
	H0[Z: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
	Vert: 9=43
13) Dead + 0.6 MWERS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase=1.60
.0	Uniform Loads (plf)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
	Concentrated Loads (lb)
<i>,</i> .	Vert: 9=43
14) Dead + 0.6 MIVERS WIND (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60

2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

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Job	Truss	Truss Type	Qty	Ply	Lot 11 West Park
					E15459303
J0921-5302	M2-GR	HALF HIP	1	2	lob Reference (entionel)
					Job Relefence (optional)
Comtech, Inc, Fayette	/ille, NC - 28314,			8.330 s Oo	t 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:14 2021 Page 3

8.330 s Oct 7 2020 MiTek Industries, Inc. Tue Mar 2 15:48:14 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-JAzryP4CtblzdA1uoQFMj?9Uy5yf3NdfHnbi?hzevvV

LOAD CASE(S) Standard
Uniform Loads (plf)
Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
nuiz. i+z==z0, z->=-33, 3-4=-20, 3-5=-39 Concentrated Leads (Ib)
Vert: 9=43
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
H01Z: 1-Z=-14, Z-3=-Z1, 3-4=-14, 3-5=-27
Vert 9=43
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20
H072: 1-2=-20, 2-3=-19, 3-4=-20, 3-5=-12 Concentrated Lads (Ih)
Vert 9=-234
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20
H01Z: 1-Z=-14, Z-3=-7, 3-4=-14, 3-5=-0
Vert 9234
18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20
19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3
Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
Concentrated Loads (ID)
20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20
Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
Vert 9375
21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20
noiz. 1-219, 2-314, 3-419, 3-39 Concentrated Loads (Ib)
Vert: 9=-375
22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-40, 2-3=-40, 3-4=-40, 5-3=-200, 6-3=-236, 2-7=-20
Concentrated Loads (/b)
Vert: 9=-375
23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (pit) Vort $2 = 60, 2, 4 = 60, 5, 6 = 160, 2, 7 = 20$
Vent. 1-3-00, 3-4-00, 3-0-100, 2-7-=-20 Concentrated Loads (Ib)
Vert: 9=-400
24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Ver: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20
Vert 9=-400
25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20
26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20
Concentrated Loads (Ib)

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3x4 =

Plate Offsets (X,Y)	[7:0-3-11,Edge], [9:0-1-6,0-1-0], [10:0-2	-0,0-0-10]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 PCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code JPC/2015/TPI/2014	CSI. TC 0.06 BC 0.03 WB 0.08 Matrix S	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 7 n/a n/a	
LUMBER-		Matrix-S	BRACING-	

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No 2 OTHERS

REACTIONS. All bearings 12-10-9.

Max Horz 1=231(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12), 8=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-295/189

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

March 3,2021

MARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

	[1.0 0 0,0 0 0]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.14 BC 0.15 WB 0.09 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 5 n/a n/a Weight: 64 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF	' No.1		BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1

2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. All bearings 14-10-0.

(lb) - Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)

2-8=-338/247, 4-6=-338/247

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135. 6=135.

MARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Construints - Strange delivery design parameters and READ NOTES ON THIS AND INCLODED INTERFERENCE PAGE MIT-1473 BIV 5192/2021 BEFORE DSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see ANS/LTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 12-5-4.

(lb) - Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS 2-8=-312/241, 4-6=-312/241

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.

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TOP CHORD BOT CHORD

BRACING-

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=10-0-7, 3=10-0-7, 4=10-0-7

Max Horz 1=-92(LC 8)

Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

March 3,2021

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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10

Max Horz 1=68(LC 9)

Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

March 3,2021

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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD

2x4 SP No.2 OTHERS

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13

Max Horz 1=-44(LC 8)

Max Uplift 1=-15(LC 13), 3=-19(LC 13) Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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818 Soundside Road Edenton, NC 27932

3x4 🥢

3x4 📎

			2-10-0
Plate Offsets (X,Y)	[2:0-2-0,Edge]		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.01	Vert(LL) n/a - n/a 999 MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) n/a - n/a 999
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Weight: 8 lb FT = 20%
LUMBER-		I	BRACING-

TOP CHORD

BOT CHORD

2-10-0

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=2-10-0, 3=2-10-0

Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13)

Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

March 3,2021

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Image: Note of the product in the spectral product product in the spectral product in the spect	,			ME		ATE	*
Image: contract of the set of the s	51 0"	3. Adjust spacing as needed not to exceed 24"oc. Connector Information Sym Product Manuf Qty Supported Member Header Truss HUS410 USP 14 NA 16d/3-1/2" 16d/3-1/2" HUS410 USP 14 NA 16d/3-1/2" 16d/3-1/2" Member Products Products Products Product Pleis Net Qty Fab Type BM1 12'0" 1-3/4"x 16" LVL Kerto-S 2 2 FF BM2 15'0" 1-3/4"x 16" LVL Kerto-S 2 2 FF BM3 12'0" 2x10 SPF No.2 2 2 FF GDH 12'0" 2x12 SPF No.2 2 2 FF	Weaver Development Co. Inc.	Lot 11 West Park	Hickory "B"	: Seal Date	Ounte #
D Titlehel Table (dirived tom the memory predention) D D <t< td=""><td>32' 6" AEI 21</td><td>Dimension Notes 1. All exterior wall downline mosions are to face of transmissions are to face of transmissione are to face of transmissione are to face of transwork of the wall of truss dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise Mil Walls Shown Are considered Load Bearing Plumbing Drop Notes 1. Plumbing drop locations shown are NOT exact. 2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses. 3. All station to extend 24°oc.</td><td>CITY / CO. Sanford / Lee</td><td>ADDRESS Lot 11 West Park</td><td>MODEL Floor</td><td>DATE REV. 09/07/21</td><td>DRAWN BY David Landry</td></t<>	32' 6" AEI 21	Dimension Notes 1. All exterior wall downline mosions are to face of transmissions are to face of transmissione are to face of transmissione are to face of transwork of the wall of truss dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise Mil Walls Shown Are considered Load Bearing Plumbing Drop Notes 1. Plumbing drop locations shown are NOT exact. 2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses. 3. All station to extend 24°oc.	CITY / CO. Sanford / Lee	ADDRESS Lot 11 West Park	MODEL Floor	DATE REV. 09/07/21	DRAWN BY David Landry
attached Tables (derived from the pre-			require size an reaction 15000# retainer reaction Tables, retainer reaction Signatu LO M Signatu 1700 3400 5100 6800 8500 1020X 1190X 1360X 1530X	ments) to d number ns greatel . A registe d to design n that exc. A registe d to design ns that exc ure)AD CH (aast meEk of) 2) 4) 2) 3) 4) 5) 6) 7 0 8 0 9	o determin of wood r than 300 ered design in the sup- ceeds tho ered design Davi Davi Davi IART F ED CN TAR ACK STUD HEADE Z55 5100 765 1020 1275 1530	ine the ministructure results respecting profession profession of the result of the re	ali

▲ = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing deemed requiren attachec requiren size and reaction 15000#. retained reaction Tables.	ROC RUS Reilly F Fayet Phon Fax reactions to complete to design that excla A register	OF & SES Road In teville e: (910) s less that e contract derived f determin of wood a than 300 red design the sup eeds those	Te & FL & FL & B adustr b, N.C. 0) 864 1 864-4 n or equa b prescript to result trom the p e the min studs req 0 [#] but no n profess port syste e specific n profess	CIP COOF EAN rial Par 28309 -8787 1444 I to 3000Å refer to th prescriptiv imum focu uired to s t greater fa imum focu uired to s t greater fa	k k k k
retained reaction Signatu	to design s that exe re	n the sup ceed 1500 Davi	port syste 10#. d La	andr	y
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²⁰ (2) 40 1700 3400 5100 6800 8500 10200 11900 13600 15300	804 900 19 40 20 4 5 6 7 8 9 9	HEADS	NOTROFE 004501 014501 014501 01501 01501 01501	NOLL7924 072 340 680 102/ 136/	00 00 00 00
Sanford / Lee	Lot 11 West Park	Floor	09/07/21	David Landry	
CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	
Weaver Development Co. Inc.	Lot 11 West Park	Hickory "B"			
BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	

Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.

Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Info	ormation			
Sym	Product	Header	Truss			
	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM3	12' 0"	2x10 SPF No.2	2	2	FF
GDH	12' 0"	2x12 SPF No.2	2	2	FF

Truss Placement Plan

components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

	-	(Client:	Weaver De	velopment		Da	ate:	9/7/2021				Page 1 of 8
1.	Destau	F	Project:				In	put by:	David Lan	dry			
IS	Design	A	Address:	158 West	Park Lane		Ja Pr	b Name	e: Lot 11 We	st Park I3			
BM1 L	(orto_S I)	// 1	750" ¥	16.00	רי∩ר_				Level: Level				
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									2 011				
				11'11	"							,	´ 3 1/2"
1				11'11						1			
Member In	formation		A		F lava		Reaction	ns UNI	PATTERNI	D lb (Uplift))		
Plies:	2		Design N	/lethod:	ASD		Brg 1	LIVE	e Dea) 286	a Snow	v	vina 0	Const 0
Moisture Cond	lition: Dry		Building	Code:	IBC/IRC 2015		2	C	286	i9 2079		0	0
Deflection LL:	480		Load Sh	aring:	No Not Observed								
Importance:	360 Normal		Deck: Ceilina:		Not Checked Gypsum 1/2"								
Temperature:	Temp <= 100)°F	l semig		ojpoun n=								
							Bearings	5					
							Bearing	Length	n Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF 2 - SPF	3.500"	95% 95%	2869/2079 2869/2079	4948 4948	L	D+S D+S
Analysis Re	sults						, <u> </u>						
Analysis	Actual	Location A	Allowed	Capacity	Comb.	Case							
Unbraced	13679 π-lb 13679 ft-lb	5'11 1/2" 3	3695 ft-lb	0.344 (34	.%) D+S D+S	L 1							
ensidedd				(100%)									
Shear	3659 lb	1'6 5/8" 1 5'11 1/2" (3739 lb	0.266 (27	%) D+S	L							
TL Defl inch	0.164 (L/840)	5'11 1/2" 0).383 (L/360)) 0.430 (43	%) D+S	L							
Desian Not	es				,		1						
1 Fasten all p	lies using 3 rows of	f 10d Box nails	s (.128x3") a	t 12" o.c. N	aximum end di	stance not	1						
2 Refer to las	t page of calculation	ns for fastene	rs required for	or specified	loads.								
3 Girders are	designed to be sup	ported on the	bottom edg	e only.									
5 Top must be	e laterally braced at	t a maximum o	of 8'8 1/4" o.	с.									
6 Lateral slen	derness ratio base	d on single ply	width.		Sida	Dead 0.0		1 900	WAY 1 15 1	Vind 1.6 Const	1 25	Commont	<u> </u>
	Loau Type Uniform	L		nin Anarij	Тор	120 PI F		- 3110	0 PLF	0 PLF	0 PI F	Wall	0
2	Uniform				Тор	349 PLF	0 PLF	- 3	349 PLF	0 PLF	0 PLF	A2	
	Self Weight					12 PLF							
Neter		ohomioo	10		6 For fla	t roofo provido p	ronar drainana ta	provent	Manufacturer	Info	Con	ntech, Inc.	
Calculated Structured	Designs is responsible only	of the Handling	& Installatio	n	o. Por fla ponding	, piuvide p 9	isher grannade (o	PLOABUL	Metsä Wood	uilding Ord Elect	100 Fay USA	1 S. Reilly Road, etteville, NC \	Suite #639
design criteria and responsibility of the c	loadings shown. It is ustomer and/or the contrac	the 2 Refer tor to regarding	to manufacturer installation	or annea 's product in requirements,	formation multi-ply				Norwalk, CT 0	onnung, ∠nd Floor 6851 ∩	283 910	14 -864-TRUS	
application, and to veri Lumber	fy the dimensions and loads.	fastening approval 3. Damade	g details, beam s' s d Beams must not	trength values, a be used	and code				www.metsawc	od.com/us			
1. Dry service conditi 2. LVL not to be treat	ons, unless noted otherwise ted with fire retardant or corr	4. Design a 5. Provide lateral di	Issumes top edge lateral support at splacement and m	is laterally restrai bearing points tation	to avoid	4			-00-L0. EOR-			con	тесн
Version 20.40.075	Den and the form with	iaterai Qi	-processions and 10		This	uesign is valid	untii 4/24/2023	ა					

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	Client:	Weaver Developm	ent		Date:	9/7/2021	Page 2 of 8
TisDesign	Project: Address	158 West Park	lano		Input by: Job Name	David Landry :: Lot 11 West Park	
	, idai eesti	100 West Faik	Lane		Project #:	J0921-5303	
BM1 Kerto-S L	VL 1.750"	X 16.000"	2-Ply	- PASSI	ED ^I	Level: Level	
• • •	• •	• •	•	• •	•	••	$\overline{\mathbf{M}}$ 1
			•	•	•	• 112	1'4"
			•		•	<u> </u>	M L.
1 SPF							
<u>/</u>		11'11"				ł	3 1/2"
/		11'11"					
Multi-Ply Analysis							
Fasten all plies using 3 rov	ws of 10d Box nails	(.128x3") at 12"	o.c Maxim	um end dis	stance no	ot to exceed 6"	
Capacity Load	0.0 % 0.0 PLF						
Yield Limit per Foot	245.6 PLF						
Yield Limit per Fastener Yield Mode	81.9 lb. IV						
Edge Distance	1 1/2"						
Min. End Distance	3"						
Load Combination	1 00						
Notes Calculated Structured Designs is responsible onl	chemicals y of the Handling & Installat	tion	6. For flat roofs pro	wide proper drainag	e to prevent	Manufacturer Info Metsä Wood	Comtech, Inc. 1001 S, Reilly Road, Suite #639 Fayetteville, NC
structural adequacy of this component based design criteria and loadings shown. It responsibility of the customer and/or the contri-	on the 1. LVL beams must not be is the 2. Refer to manufactur actor to regarding installation	cut or drilled rer's product information requirements, multi-ply				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622 5850	28314 910-864-TRUS
application, and to verify the dimensions and load	Is fastening details, beam approvals 3 Damaged Beams must	n strength values, and code				www.metsawood.com/us ICC-ES: ESR-3633	
 Dry service conditions, unless noted otherwis LVL not to be treated with fire retardant or control 	e 4. Design assumes top ed 5. Provide lateral support lateral displacement and	ge is laterally restrained : at bearing points to avoid I rotation	This design is	valid until 4/24/	2023		соттесн
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	Client:	Weaver Development	Date:	9/7/2021	Page 4 of 8
La Da stars	Project:		Input by:	David Landry	
IsDesign	Address:	158 West Park Lane	Job Nam Brainet #	e: Lot 11 West Park	
BM2 Karta S LV/L	4 750" \	16 000" 2 DI		Level: Level	
BIVIZ NERTO-S LVL	1.750 /	16.000 Z-PIy	- PASSED		
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1 SPF End Grain				2 SPF End 0	
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/ <u>/</u>		1/17 1/2"			
		147 1/2			I
Mariti Dha Ana hasia					
	(10 d D	120.20) - (120 Mari			
Fasten all plies using 3 rows of Capacity	r IUd Box halls (.	128x3") at 12" O.C Maxi	mum end distance n	ot to exceed 6"	
Load 19	6.0 PLF				
Yield Limit per Foot 24	5.6 PLF				
Yield Mode IV	.9 10.				
Edge Distance 1	1/2"				
Min. End Distance 3" Load Combination D+	۰L				
Duration Factor 1.0	00				
Notes Calculated Structured Designs is responsible only of the	chemicals Handling & Installatic	6, For flat roofs n ponding	provide proper drainage to prevent	Manufacturer Info Metsä Wood	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to	1. LVL beams must not be cu 2. Refer to manufacturer	tordrilled 's product information		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the component suitability of the intended application, and to verify the dimensions and loads.	regarαing installation fastening details, beam s approvals	requirements, multi-ply trength values, and code		(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service conditions, unless noted otherwise	3. Damaged Beams must not 4. Design assumes top edge 5. Provide lateral support	be used is laterally restrained t bearing points to avoid		ICC-ES: ESR-3633	loom to out
2. LVL not to be treated with fire retardant or corrosive	lateral displacement and ro	tation This design	n is valid until 4/24/2023		соптесн

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	/	Client:	Weaver Developm	ient	Date:	9/7/2021 David Landar	Page 6 of 8
1	isDesign	Address:	158 West Park	Lane	Job Name	e: Lot 11 West Park	
BM3	S-P-F #2	2.000" X	10.000"	2-Ply - PA	SSED	Level: Level	
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	• •	•	• •	• •	•	• • • •	
				12'			
<i> </i>				12'			
M. 4: D	(Analyze's						
Fasten all	plies using 2 row	s of 10d Box nails	(.128x3") at 12"	o.c Maximum	end distance no	ot to exceed 6"	
Capacity Load		0.0 % 0.0 PLF					
Yield Limit pe Yield Limit pe	er Foot er Fastener	157.4 PLF 78.7 lb.					
Yield Mode Edge Distand	ce	IV 1 1/2"					
Min. End Dis	stance	3"					
Duration Fac	tor	1.00					
					[Manufacturer Info	Comtech, Inc.
							Fayetteville, NC USA 28314
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L				This design is value			

Protection Protection <th></th> <th></th> <th></th> <th>Client:</th> <th>Weaver Dev</th> <th>elopment</th> <th></th> <th></th> <th>Date:</th> <th>9/7/202</th> <th>1</th> <th></th> <th></th> <th></th> <th>Page 7 of</th>				Client:	Weaver Dev	elopment			Date:	9/7/202	1				Page 7 of
Type Output Output Output Output GDH S.P.F.#2 2.000" X 12.000" 2.PIy - PASSED [Intel Lower] GDH S.P.F.#2 2.000" X 12.000" 2.PIy - PASSED [Intel Lower] GDH S.P.F.#2 2.000" X 12.000" 2.PIy - PASSED [Intel Lower] GDH S.P.F.#2 2.000" X 12.000" 2.PIy - PASSED [Intel Lower] GDH S.P.F.#2 0.000" X 12.000" [Intel Lower] [Intel Lower] S.P.F.#2 O Passe [Intel Lower] [Intel Lower] S.P.F.#2 O Passe [Intel Lower] [Intel Lower] Contract Controls to 4 Passe Passe [Intel Lower] [Intel Lower] Desk Not Checked Desk Not Checked [Intel Lower] [Intel Lower] Desk Not Checked Desk Not Checked [Intel Lower] [Intel Lower] Desk Not Checked Desk Not Checked [Intel Lower] [Intel Lower] [Intel Lower] Desk Not C	is	Design		Project: Address:	158 West	Park Lane			Input by: Job Name:	David L Lot 11 V	andry Vest Park				
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1 SPF End Grain 2 SPF End Grain 8107 8107 Aember Information Reactions UNPATTERNED Ib (Uplifs) Time 2 Deficient Li 40 Deck Not Checked Caling: Gypsum 162* Deck Not Checked Caling: Gypsum 162* Deck Not Checked Caling: Gypsum 162* Deck State Li Deck Difficient Li 40 Deck Difficient Li 40 Analysis Actual Location Allowed Capacity Comb Case Moment 1480711b Location Networks (1650 Ors) Deficient Comb Streat lang angerd actualistos for theteres requerid to tables	•		•	•	•	and the second	and the second	•			•••			L	
Professor Professor <t< td=""><td>1 SPF E</td><td>nd Grain</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>2 SPF E</td><td>nd Grain</td><td>_</td><td></td><td></td><td></td></t<>	1 SPF E	nd Grain								2 SPF E	nd Grain	_			
1 B10° 1 Aember Information Reactions UNPATTERNED Ib (Uplift) Type:: Great Design Methics 1 ASD Design Methics Application: Floor Design Methics Application: Application: Importance: Normal Design Methics Application: Temperature: Normal Design Methics Column Calling: Optimized Capacity Column Column Analysis Actual Location Allowed Capacity Comb Moment 1400 Hb 45° 1458 Hb 0.202 (20%) D Unition 2 - SPF 3.300° 19% 751 / 88 839 L D + S Analysis Actual Location Allowed Capacity Comb Capacity D Unition 2 - SPF 3.300° 19% 751 / 88 839 L D + S Brind Ontic 455 Hb 12° 2734 Hb 0.202 (20%) D Unition Capacity D + S L Design Motes 1 2 - SPF 3.300° 19% 751 / 88 639 L D + S The bit physication at the support of the budge onthy 1 S and the physication a]				8'	10"]		1]3 "
Adember Information Reactions UNPATTERNED Ib (Uplift) Type Groat Application: Floor Despin Method: ASD Modistry Condition: Dry Dusking Codie: IBC/IRC 2015 ISD ISD Defection IL: 390 Deck: Not Checked 2 0 751 80 0 0 Defection IL: 390 Deck: Not Checked Celling: Groat Easting: No 2 0 751 80 0 0 0 Analysis Actual Location Allowed Capacity Comb. Case Grain -	1				8'	10"						1			
Type: Girder Application: Floar Brig Live Dead Snow Wind Const Pleas: 2 Design Method: ASD Dublice Condition 1 0 751 88 0 0 Deficition LL: 450 Design Method: ASD Design Method: ASD 0 0 Design Method: 1 0 751 88 0 0 Design Method: 1:0 Design Method: 2:0 751/88 839 L Design Method: 2:0 S51/87 Design Method: 2:0 Design Method: Design Method: Design Method: Design Method: Design Method: <t< td=""><td>Member In</td><td>formation</td><td></td><td></td><td></td><td></td><td></td><td>Reactio</td><td>ons UNP</td><td>ATTER</td><td>NED Ib (L</td><td>Jplift)</td><td></td><td></td><td></td></t<>	Member In	formation						Reactio	ons UNP	ATTER	NED Ib (L	Jplift)			
Piles: 2 Design Method: ASD 1 0 751 88 0 0 Dedection L1: 480 Design Method: No Design Method: No 2 0 751 88 0 0 Importance: Normal So 0 Design Method: No Design Method:	Туре:	Girder		Applicat	tion:	loor		Brg	Live	D	ead	Snow	Wind		Const
Deflection LL: 480 Load Sharing: No. No. Construction Construction <t< td=""><td>Plies: Moisture Conc</td><td>2 lition: Drv</td><td></td><td>Design Building</td><td>Method:</td><td>ASD BC/IRC 2015</td><td></td><td>1</td><td>0</td><td></td><td>751</td><td>88</td><td>0</td><td></td><td>0</td></t<>	Plies: Moisture Conc	2 lition: Drv		Design Building	Method:	ASD BC/IRC 2015		1	0		751	88	0		0
Deficient T1: 340 Temperature: Temp ≈ 100°F Temperature: Temp ≈ 100°F Ceiling: Gypsum 1/2" Bearing: Length Cap. React D/L lb Total Ld Case Ld. Comb. 1 - SPF 3.500° 19% 751 / 88 89 L D-S End Grain 2 - SPF 3.500° 19% 751 / 88 89 L D-S End Grain 2 - SPF 3.500° 19% 751 / 88 89 L D-S End Grain 2 - SPF 3.500° 19% 751 / 88 99 L D-S End Grain 2 - SPF 3.500° 100 Societ 3	Deflection LL:	480		Load St	naring:	No		2	0		751	00	0		0
Importance: Normal Ceaming: Gypsuin 1/2* Temperature:	Deflection TL:	360		Deck:	I	Not Checked									
Image: Security Bearings Analysis Actual Location Allowed Capacity Comb 1 Sign Cap React D/L Ib Total Ld. Case	Temperature:	Temp <= 100°	F	Celling:		Sypsum 1/2"									
matrix Bearing Length Cap. React DU, Ib Total La Case Ld. C	·	·						Bearing	gs						
Inalysis Actual Location Allowed Capacity Comb Casa Moment 1490 Pt-b 475 4153 Pt-b 0.359 (36%) D Uniform Shear 553 1b 112 2734 lb 0.421 (42%) D Uniform Shear 553 1b 112 2734 lb 0.421 (42%) D Uniform LD eff inch 0.004 (L/280) 475 11/16 0.202 (20%) D Uniform LD eff inch 0.004 (L/280) 475 11/16 0.279 (U360) 0.150 (15%) D+S LD eff inch 0.042 (L/281) 45 11/16 0.279 (U360) 0.150 (15%) D+S 1 Definich 0.042 (L/281) 45 11/16 0.279 (U360) 2 Refore to last page of calulations for fasteners required for specified loads. 3 3 Griders Top 60 PLF 0 PLF 0 PLF 0 PLF 0 PLF 3 Tie-in 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0 PSF 0 PSF 0 PSF 3 Tie-in 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0 PSF 0 PSF 0 PSF 0 PSF 3 Tie-in 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0 PSF 0 PSF 0 PSF 0 PSF								Bearing	g Length	Ca	p. React D)/L lb	Total Ld.	Case	Ld. Comb.
Grain Grain Caracy is Actual Moment Location Allowed Capacity Comb Case in degrain Sind 19% 751 / 88 839 L D *S Moment 1400 ft-b 475 4153 ft-b 0.421 (42%) D Uniform 1 1 0.202 (20%) D Uniform 1 1 1 0.202 (20%) D Uniform 1 1 1 1 0.202 (20%) S L 1 1 1 1 1 0.202 (20%) S L 1								1 - SPI End	- 3.500"	19	% 75	1/88	839 L		D+8
Analysis Actual Location Allowed Capacity Comb Casa Dist Dis Dist <thdist< th=""> <thd< td=""><td>Analysis Re</td><td>sults</td><td></td><td></td><td></td><td></td><td></td><td>Grain</td><td>= 3.500"</td><td>19</td><td>% 75[.]</td><td>1 / 88</td><td>839 </td><td></td><td>D+S</td></thd<></thdist<>	Analysis Re	sults						Grain	= 3.500"	19	% 75 [.]	1 / 88	839		D+S
Minimite Hold Rick Hold Rick Grain Contract 4490 fbb 45° 3530 fbb 0.202 (22%) D Uniform LL Defineh 0.004 45° 11° 0.209 (U480) 0.020 (2%) S L LL Defineh 0.004 45° 11° 0.279 (U380) 0.150 (15%) D+S L Design Note:	Analysis Momont	Actual	Location	Allowed	Capacity	Comb.	Case	End	0.000	15	// //0	1700	000 L		0.0
Shear 553 b 12° 2734 b 0.202 (20%) D Uniform LL Deflinch 0.004 4'5 1/16' 0.202 (20%) D L VIC22822) 4'5 1/16' 0.202 (20%) D L The Deflinch 0.004 (L/2281) 4'5 1/16' 0.202 (20%) D L Control Lic22821) 1 Fasten all piles using 2 rows of 10d Box nails (.128/37) at 12' o.c. Maximum end distance not to exceed 0''. 2 Refer to last page of calculations for fasteners required for specified loads. 3 3 Girders are dissigned to be supported on the bottom edge on single ply width. Top bode the barings. 1 D Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Top 60 PLF 0 PLF 0 PLF 0 PLF Wind 1.6 Const. 1.25 Comments 3 Tie-lin 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0 P	Unbraced	1490 ft-lb	4'5"	3539 ft-lb	0.421 (429	6) D	Uniform	Grain							
LL Defi inch 0.004 4'5 1/16' 0.209 (L/480) 0.020 (2%) S L TL Defi inch 0.042 (L/2821) 4'5 1/16' 0.279 (L/360) 0.150 (15%) D+S L Design Note: 1 Fasten all plies using 2 rows of 10d Box nais (.128x3'') at 12" o.c. Maximum end distance not to exceed 0'. 2 Refer to last page of calculations for fasteners required for specified loads. 3 Girders are designed to be supported on the bottom edge only. 4 Top loads must be supported on the bottom edge only. 4 Top loads must be supported equally by all plies. 5 Top Draced at bearings. 6 Lateral Botemess ratio based on single ply width. 10 Load Type Loadtion Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Jantom Top 60 PLF 0.PLF 0.PLF 0.PLF 0.PLF Vali 2 Uniform 0.0-0 to 8-10-0 1.0-0 Top 90 PLF 0.PLF 0.PLF 0.PLF 0.PLF 0.PLF B16E 3 Tie-In 0.0-0 to 8-10-0 1.0-0 Top 20 PSF 0.PSF 20 PSF 0.PSF	Shear	553 lb	1'2"	2734 lb	0.202 (209	6) D	Uniform								
The Define 0.042 (U2381) 45 1/16" 0.279 (U380) 0.150 (15%) D+S L Design Notes 1 1 Fasten all piles using 2 rows of 10d Box nalis (128x3') at 12" o.c. Maximum end distance not to exceed 6'. Image: Colspan="6">Comments are designed to be supported on the bottom edge only. 3 Girders are designed to be supported on the bottom edge only. Top load must be supported equally by all piles. Top load must be supported equally by all piles. Comments 1 Lateral slendemess ratio based on single ply width. Top 0 0 PLF 0 PLF 0 PLF 0 PLF Valit 2 Uniform Top 0 90 PLF 0 PLF 0 PLF 0 PLF 0 PLF Valit 3 Tie-ln 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0 PSF	LL Defl inch	0.004 (L/22622)	4'5 1/16"	0.209 (L/480	0) 0.020 (2%) S	L								
Design Notes 1 fasten all piles using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6'. 2 Refer to last page of calculations for fasteners required for specified loads. 3 Griders are designed to be supported on the bottom edge only. 4 Top loads must be supported equality by all piles. 5 Top loads must be supported equality by all piles. 6 Lateral stendermess ratio based on single ply width. ID Load Type Location 1 Uniform Top 60 PLF 0 PLF 0 PLF 0 PLF 0 PLF VIIF 2 Uniform Top 90 PLF 0 PLF 0 PLF 0 PLF 0 PLF BIGE 3 Tie-In 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0	TL Defl inch	0.042 (L/2381)	4'5 1/16"	0.279 (L/360	0) 0.150 (159	%) D+S	L								
	Design Not	es													
2 Refer to last page of calculations for fasteners required for specified loads. 3 Girders are designed to be supported on the bottom edge only. 4 Top loads must be supported equally by all plies. 5 Top braced at bearings. 6 Lateral stendermess ratio based on single ply width. ID Load Type Location 1 Uniform Top 60 PLF 0 PLF 0 PLF 0 PLF 0 PLF Wind 1.6 Comments 2 Uniform Top 60 PLF 0 PLF 0 PLF 0 PLF 0 PLF B Tege 3 Tie-In 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0 SPSF 0 SPSF <td>1 Fasten all p to exceed 6</td> <td>lies using 2 rows of [·] ".</td> <td>10d Box nai</td> <td>ls (.128x3")</td> <td>at 12" o.c. Ma</td> <td>aximum end di</td> <td>stance not</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	1 Fasten all p to exceed 6	lies using 2 rows of [·] ".	10d Box nai	ls (.128x3")	at 12" o.c. Ma	aximum end di	stance not								
Top lades must be supported equally by all plies. Sore that is used on single ply width. ID Load Type Location Trib Windt Side Dead O.9 Live I Sore	2 Refer to las	t page of calculation	s for fastene	ers required	for specified I	oads.									
5 Top braced at bearings. 6 Lateral sleerings. 1 Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Top 60 PLF 0 PLF 0 PLF 0 PLF 0 PLF 0 PLF BIGE 3 Tie-In 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0 PSF 0 PSF 0 PSF 0 PSF 0 PSF 0 PSF Roof Load Manufacturer Info Contech Intell This design is valid until 4/24/2023	4 Top loads n	nust be supported eq	ually by all	plies.	ge only.										
ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Top 60 PLF 0 PLF <	5 Top braced 6 Lateral slen	at bearings. derness ratio based	on single p	ly width.											
1 Uniform Top 60 PLF 0	ID	Load Type		Location	Trib Width	Side	Dead 0.9	Live	e 1 Snov	v 1.15	Wind 1.6	Const.	1.25 Cor	nments	
2 Uniform Top 90 PLF 0 PLF 0 PLF 0 PLF 0 PLF B1GE 3 Tie-In 0-0-0 to 8-10-0 1-0-0 Top 20 PSF 0 PSF 20 PSF 0 PSF 0 PSF Roof Load	1	Uniform				Тор	60 PLF	0 F	LF	0 PLF	0 PLF	0	PLF Wal	l	
S Ine-III 0-0-0 t0 0-10-0 I-0-0 top 20 PSF 0 PSF 0 PSF 0 PSF Kot Load Manufacturer Info Contech, Inc., 1001 S, Reilly Road, Suite #039 USA This design is valid until 4/24/2023	2	Uniform Tio. In	0.0.0	to 9 10 0	100	Тор	90 PLF	0 F	°LF	0 PLF	0 PLF	0	PLF B1G	5E flood	
Manufacturer Info Contech, Inc. 1001 S, Reilly Road, Suite #639 Fayetteville, NO USA 28514 - TRUS This design is valid until 4/24/2023 This design is valid until 4/24/2023	0		0-0-0	10 0-10-0	1-0-0	юр	20 494	υP	JF Z	U F 3F	UFOF	0	1 JF K00	LUAU	
Manufacturer Info Comtech. Inc. 1001 S. Reily Road, Suite #639 1001 S. Reily Road, Suite #639 1001 S. Reily Road, Suite #639 USA 28314 910-864-TRUS 28314 ersion 20.40.075 Powered by iStruct™ CONTECH															
Manufacturer Info Contech, Inc. 101 S, Reily Road, Suite #039 This design is valid until 4/24/2023 Contech, Inc. 101 S, Reily Road, Suite #039 This design is valid until 4/24/2023 Contech, Inc. 101 S, Reily Road, Suite #039 resion 20.40.075 Powered by iStruct™ Contech, Inc. 101 S, Reily Road, Suite #039															
Manufacturer Info Contech, Inc. 1001 S. Relly Road, Suite #039 Fayetteville, NO USA 28314 910-864-TRUS This design is valid until 4/24/2023 USA 28314 910-864-TRUS															
This design is valid until 4/24/2023 Figure 20.40.075 Powered by iStruct™ Figure 20.40.075 Powered PoweredPowered Powered Powered Powered PoweredPo									П	Manufactu	rer Info		Comtech,	Inc.	
This design is valid until 4/24/2023 CODI CODI CODI CODI CODI CODI CODI CODI									_				Fayettevill USA	eilly Road, S e, NC	Suite #639
This design is valid until 4/24/2023 Contech													28314 910-864-T	RUS	
This design is valid until 4/24/2023 CONTECH ersion 20.40.075 Powered by iStruct™ CSDI 874															
ersion 20.40.075 Powered by iStruct™						This	design is valid	until 4/24/20	023				C	om	тесн
	/ersion 20.40.075	Powered by iStruct™							<u>L</u>				CSD	DESIGN	

	/	Client:	Weaver Development	Date:	9/7/2021	Page 8 of 8
1	isDesign	Project: Address:	158 West Park Lane	Input by: Job Name Project #:	David Landry : Lot 11 West Park .10921-5303	
GDH	S-P-F #2	2.000" X	12.000" 2-Ply	- PASSED	Level: Level	
•	•	• •	• •	• •		M T
						11 1/4"
	• E End Grain	• •	• •	• •		
			8'10"			3"
<i> </i>			8'10"		1	
	• • •					
Fasten all	plies using 2 row	/s of 10d Box nails (128x3") at 12" o.c Maxim	num end distance nc	ot to exceed 6"	
Capacity Load	<u> </u>	0.0 % 0.0 PLF				
Yield Limit pe Yield Limit pe	r Foot r Fastener	157.4 PLF 78.7 lb.				
Yield Mode		IV 1.1/2"				
Min. End Distance	ance	1 1/2" 3"				
Load Combin	ation	1.00				
				F	Manufacturer Info	Comtech, Inc. 1001 S. Relly Road, Suite #639 Fayetteville, NC
						28314 910-864-TRUS
			This design is	s valid until 4/24/2023		соттесн

RE: J0921-5303 Lot 11 West Park Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Weaver Homes Project Name: J0921-5303 Lot/Block: 11 Model: Hick Address: 158 West Park Lane City: Sanford

Model: Hickory Subdivision: West Park State: Nc

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.3 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 8 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7	Seal# E15459375 E15459376 E15459377 E15459378 E15459379 E15459380 E15459381	Truss Name ET1 ET2 ET3 F1 F2 F3 F4	Date 3/3/2021 3/3/2021 3/3/2021 3/3/2021 3/3/2021 3/3/2021
6 7	E15459380 E15459381 E15450282	F3 F4 E5	3/3/2021 3/3/2021
0	E10409302	гu	3/3/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

			8-7-0			
Plate Offsets (X,Y)	[5:0-1-8,Edge], [17:0-1-8,0-1-8], [18:0-1-	·8,0-1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.00 WB 0.05 Matrix-P	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 9 n/a n/a	PLATES MT20 Weight: 54 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S OTHERS 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,

8-7-0

REACTIONS. All bearings 8-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb) Vert: 3=-71 6=-71 19=-71 20=-77

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

			11-1-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [14:0-1-8,Edge]		11-1-0			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.01 WB 0.04 Matrix-S	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 10 n/a n/a	PLATES MT20 Weight: 52 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S OTHERS 2x4 S	SP No.1(flat) SP No.1(flat) SP No.3(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,

REACTIONS.

DNS. All bearings 11-1-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

March 3,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD2x4 SP No.1 (flat)WEBS2x4 SP No.3 (flat)OTHERS2x4 SP No.3 (flat)

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

 		14-7-12 14-7-12					26-5-	-0	
Plate Offsets (X	(,Y)	[4:0-1-8,Edge], [15:0-1-8,Edge], [16:0-1	I-8,Edge], [21:0-1-8,Edge]					•	
LOADING (psf TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0)))))	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.55 BC 0.59 WB 0.53 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.17 21-22 -0.25 21-22 0.03 14	l/defl >999 >697 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 129 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS.	2x4 SP 2x4 SP 2x4 SP (size Max G	P No.1(flat) No.1(flat) P No.3(flat) e) 22=0-3-8, 17=0-3-8, 14=0-3-8 rav 22=728(LC 10), 17=1669(LC 1), 14	4=562(LC 7)	BRACING- TOP CHOR BOT CHOR	D Struc excep D Rigid 6-0-0	tural wood ot end veri ceiling dir oc bracin	l sheathing dir icals. ectly applied o g: 17-19,16-17	rectly applied or 6-0-0 o or 10-0-0 oc bracing, 1 7.	oc purlins, Except:
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1987/0, 3-4=-1987/0, 4-5=-1740/0, 5-6=-1740/0, 6-8=0/1282, 8-9=0 9-10=-1183/0, 10-11=-1183/0, 11-12=-1183/0 BOT CHORD 21-22=0/1314, 20-21=0/1987, 19-20=0/1987, 17-19=-191/818, 16-17=-31 15-16=0/1183, 14-15=0/954 WEBS 8-17=-284/0, 2-22=-1440/0, 2-21=0/745, 3-21=-260/0, 6-17=-1781/0, 6-1 4-19=-646/0, 9-17=-1465/0, 9-16=0/917, 10-16=-329/0, 12-14=-1044/0, 1				=0/1282, 366/574, -19=0/1122, , 12-15=-86/311					
NOTES-									

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling of individual truss expletens, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

		10-10-0			
Plate Offsets (X,Y) [8:0-1-8,Edge]					
LOADING (psf) SPACING- 2-0-0 TCLL 40.0 Plate Grip DOL 1.00 TCDL 10.0 Lumber DOL 1.00 BCLL 0.0 Rep Stress Incr YES BCDL 5.0 Code IRC2015/TPI2014	CSI. TC 0.42 BC 0.35 WB 0.30 Matrix-S	DEFL. in Vert(LL) -0.08 Vert(CT) -0.11 Horz(CT) 0.02	i (loc) l/defi L/d 9-10 >999 480 9-10 >999 360 9-10 n/a n/a	PLATES MT20 Weight: 56 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,

Max Grav 10=576(LC 1), 7=576(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1234/0, 3-4=-1234/0, 4-5=-1234/0

BOT CHORD 9-10=0/981, 8-9=0/1234, 7-8=0/982

WEBS 2-10=-1073/0, 5-7=-1075/0, 5-8=0/485, 2-9=0/478, 3-9=-255/0, 4-8=-266/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

 Plate checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oucling of individual truss expletens, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

			11-11-0			
· · ·			11-11-0			
Plate Offsets (X,Y)	[8:0-1-8,Edge], [9:0-1-8,Edge]					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ir	(loc) l/defl L/d	PLATES	GRIP
TCDL 40.0	Lumber DOL 1.00	BC 0.56	Vert(LL) -0.19 Vert(CT) -0.29	9-10 >740 480 9-10 >490 360	MT20	244/190
BCLL 0.0 BCDL 5.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.34 Matrix-S	Horz(CT) 0.02	. 7 n/a n/a	Weight: 59 lb	FT = 20%F, 11%E
LUMBER-		· ·	BRACING-			
TOP CHORD 2x4 SF BOT CHORD 2x4 SF	² No.1(flat) ² No.1(flat)		TOP CHORD	Structural wood sheathing di except end verticals.	rectly applied or 6-0-0	oc purlins,
WEBS 2x4 SF	PNo.3(flat)		BOT CHORD	Rigid ceiling directly applied	or 10-0-0 oc bracing.	
REACTIONS. (size	e) 10=0-3-8, 7=0-3-8					

11-11-0

Max Grav 10=635(LC 1), 7=635(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1508/0, 3-4=-1508/0, 4-5=-1508/0

BOT CHORD 9-10=0/1112, 8-9=0/1508, 7-8=0/1121

2-10=-1219/0, 2-9=0/558, 5-7=-1228/0, 5-8=0/655, 4-8=-353/0 WEBS

NOTES-

1) Unbalanced floor live loads have been considered for this design.
2) Plates checked for a plus or minus 1 degree rotation about its center.
3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

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10

1.5x3 ||

9

3x6 =

			<u>14-6-0</u> 14-6-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [11:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.61 BC 0.84 WB 0.46 Matrix-S	DEFL. in Vert(LL) -0.20 Vert(CT) -0.25 Horz(CT) 0.03	(loc) l/defl L/d 9-10 >838 480 9-10 >684 360 8 n/a n/a	PLATES MT20 Weight: 73 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP	P No.1(flat) No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
REACTIONS. (size	e) 12=0-3-8, 8=Mechanical					

Max Grav 12=778(LC 1), 8=784(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2243/0, 3-4=-2243/0, 4-5=-2186/0, 5-6=-2186/0

BOT CHORD 11-12=0/1424, 10-11=0/2243, 9-10=0/2243, 8-9=0/1429

2-12=-1561/0, 2-11=0/958, 3-11=-303/0, 6-8=-1573/0, 6-9=0/836, 5-9=-271/41, WEBS 4-9=-428/186

NOTES-

12

3x6 =

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

11

3x4 =

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

8

3x6 =

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			11-7-8			
			11-7-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8,	Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.60 BC 0.51 WB 0.33 Matrix-S	DEFL. in Vert(LL) -0.16 Vert(CT) -0.25 Horz(CT) 0.02	(loc) l/defi L/d 9-10 >846 480 9-10 >540 360 7 n/a n/a	PLATES GRIP MT20 244/190 Weight: 58 lb FT = 20%F. 11	%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	2 No.1(flat) 2 No.1(flat) 2 No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied of	rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.	
REACTIONS. (siz Max G	e) 10=Mechanical, 7=0-3-8 3rav 10=626(LC 1), 7=619(LC 1)					

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1441/0, 3-4=-1441/0, 4-5=-1441/0

BOT CHORD 9-10=0/1081, 8-9=0/1441, 7-8=0/1087

WEBS 2-10=-1190/0, 2-9=0/515, 5-7=-1190/0, 5-8=0/606, 4-8=-323/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

March 3,2021

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