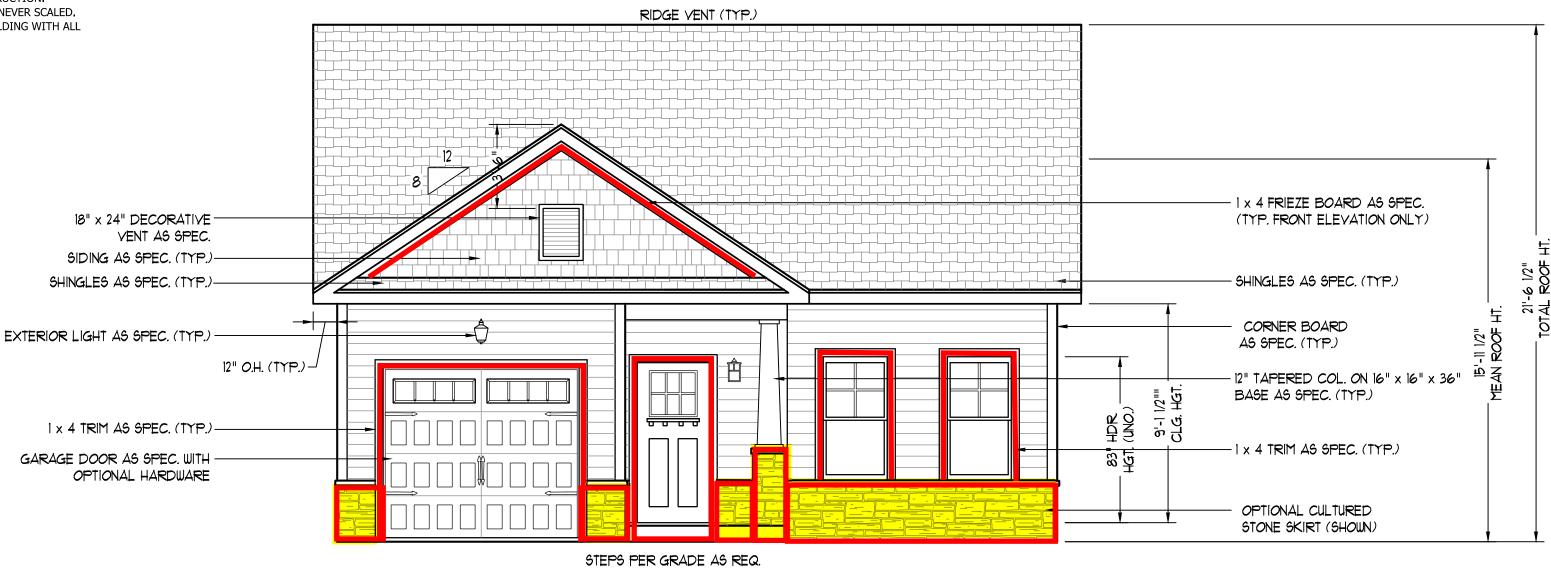
# **GENERAL NOTES** 1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS. 2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL SITE REQUIREMENTS.

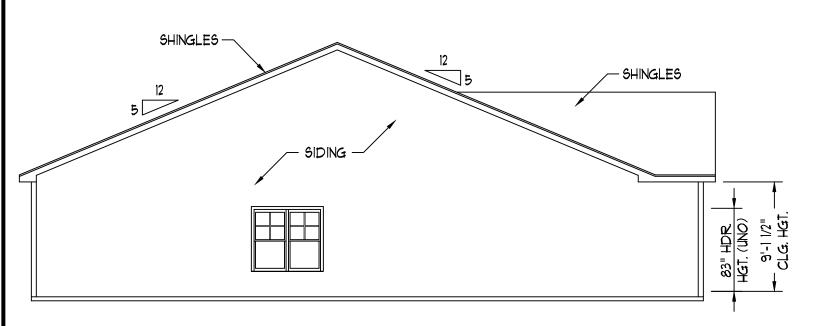


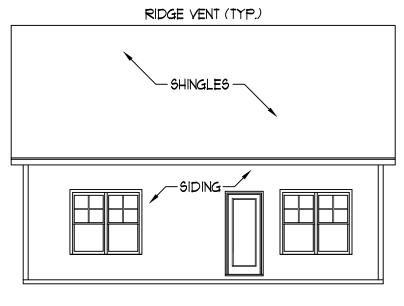


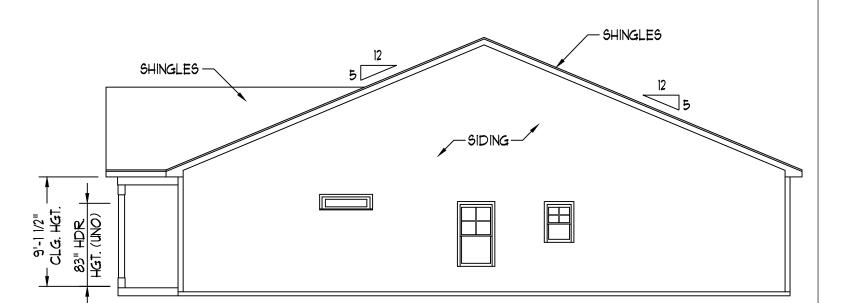


FRONT ELEVATION-A SCALE: 1/4" = 1'-0"

Harnett 09/29/2021





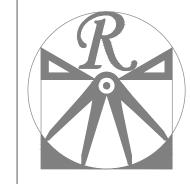


LEFT ELEVATION SCALE: 1/8" = 1'-0"

REAR ELEVATION SCALE: 1/8" = 1'-0" RIGHT ELEVATION SCALE: 1/8" = 1'-0"

PLUMBING: DOUBLE J **HVAC: MAINSTREAM ELECTRICAL: PIONEER** 

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



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WEAVER HOMES CAROLINA COLL LEYLAND DRIVE

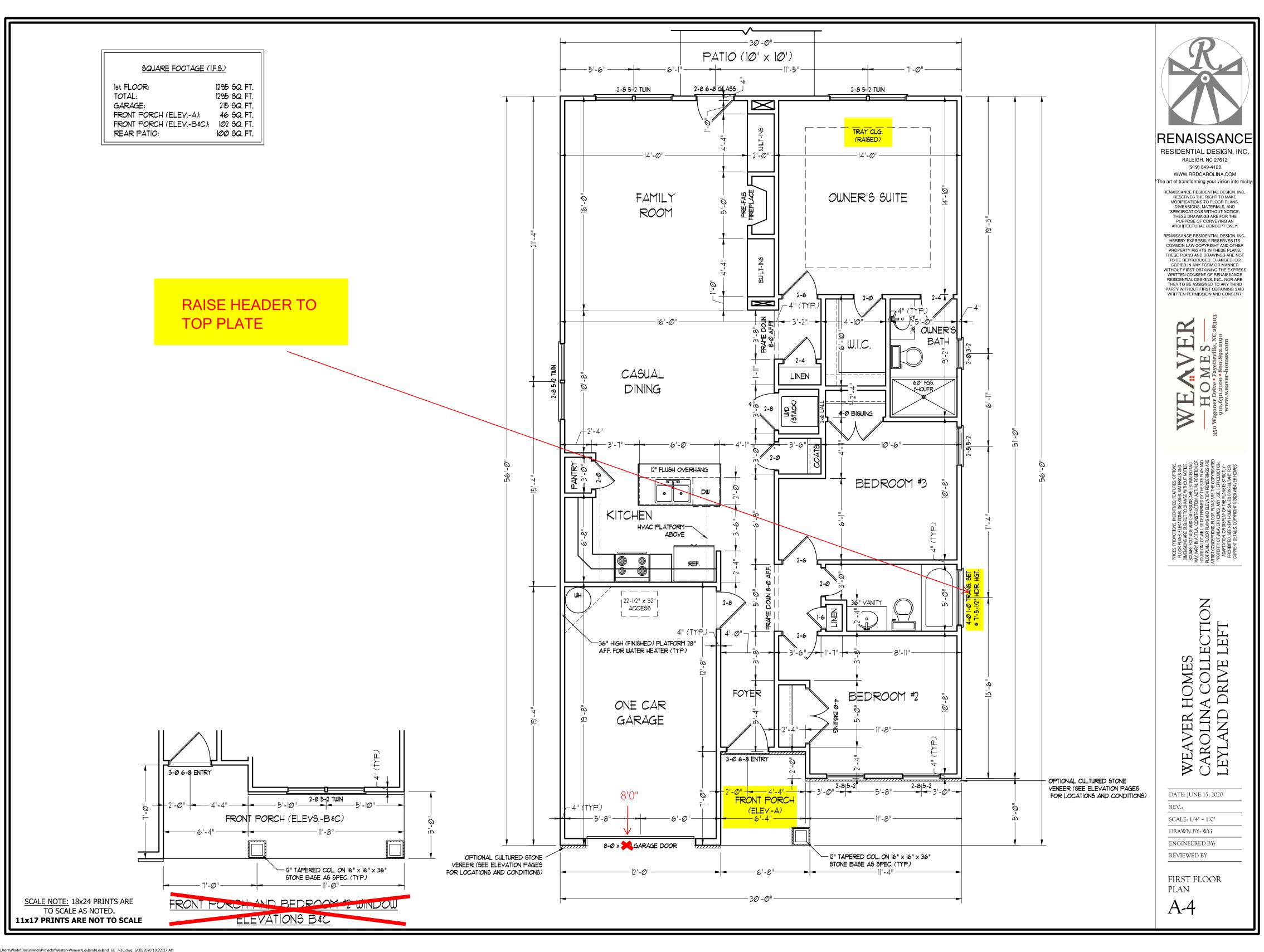
DATE: JUNE 15, 2020

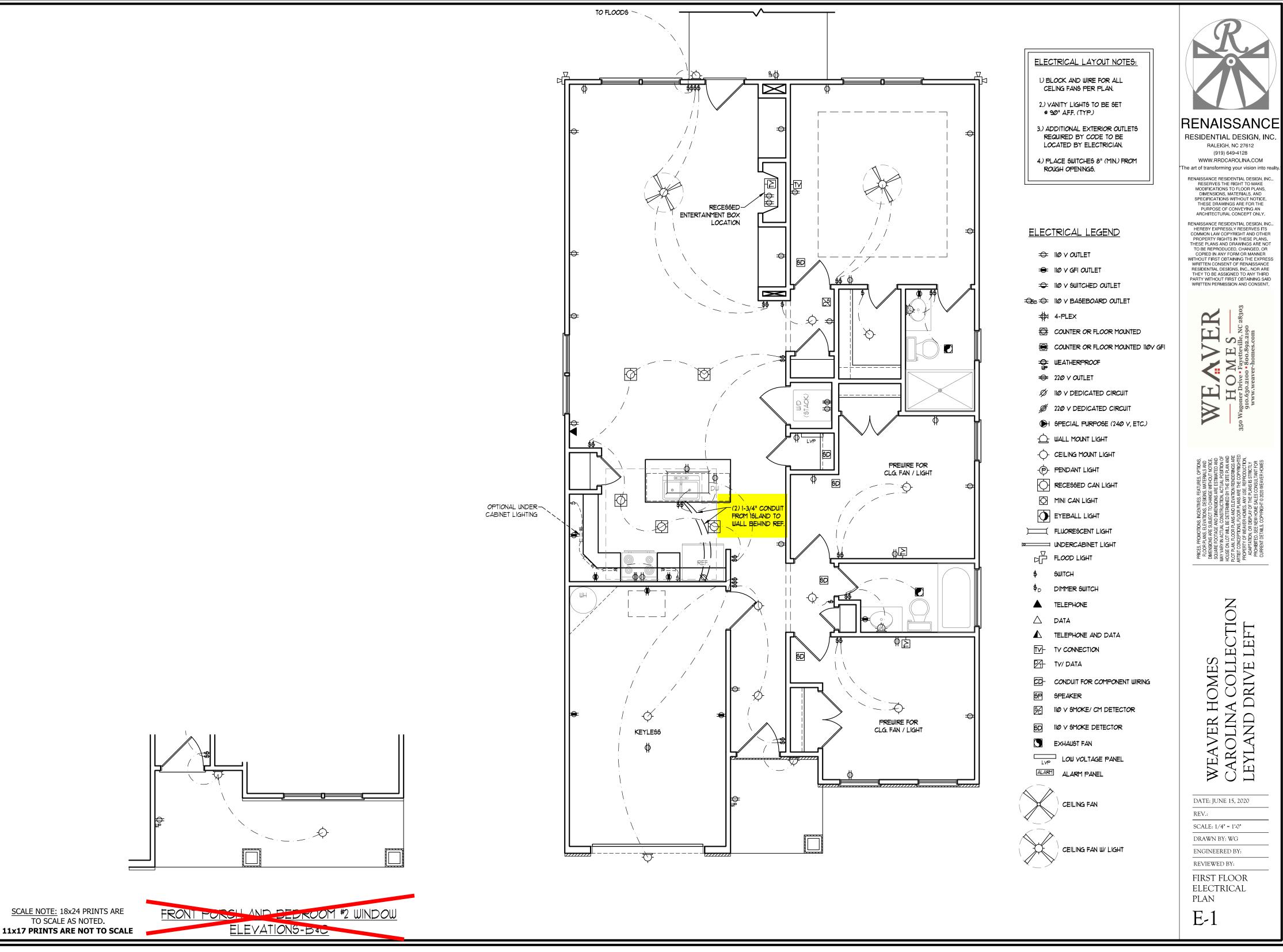
SCALE: AS NOTED DRAWN BY: WG

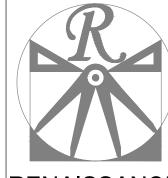
ENGINEERED BY: REVIEWED BY:

A - ELEVATIONS

A-1







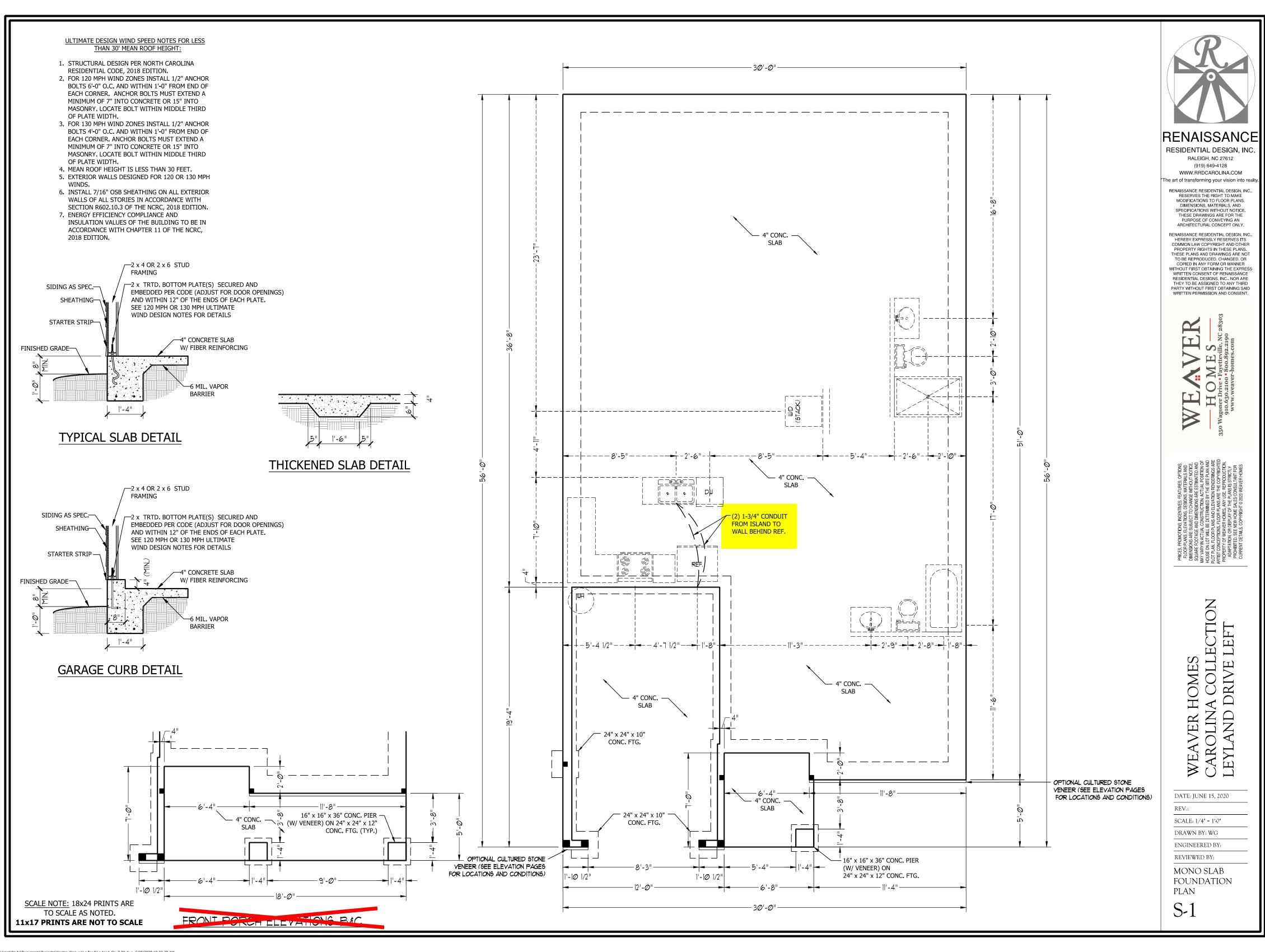
RENAISSANCE

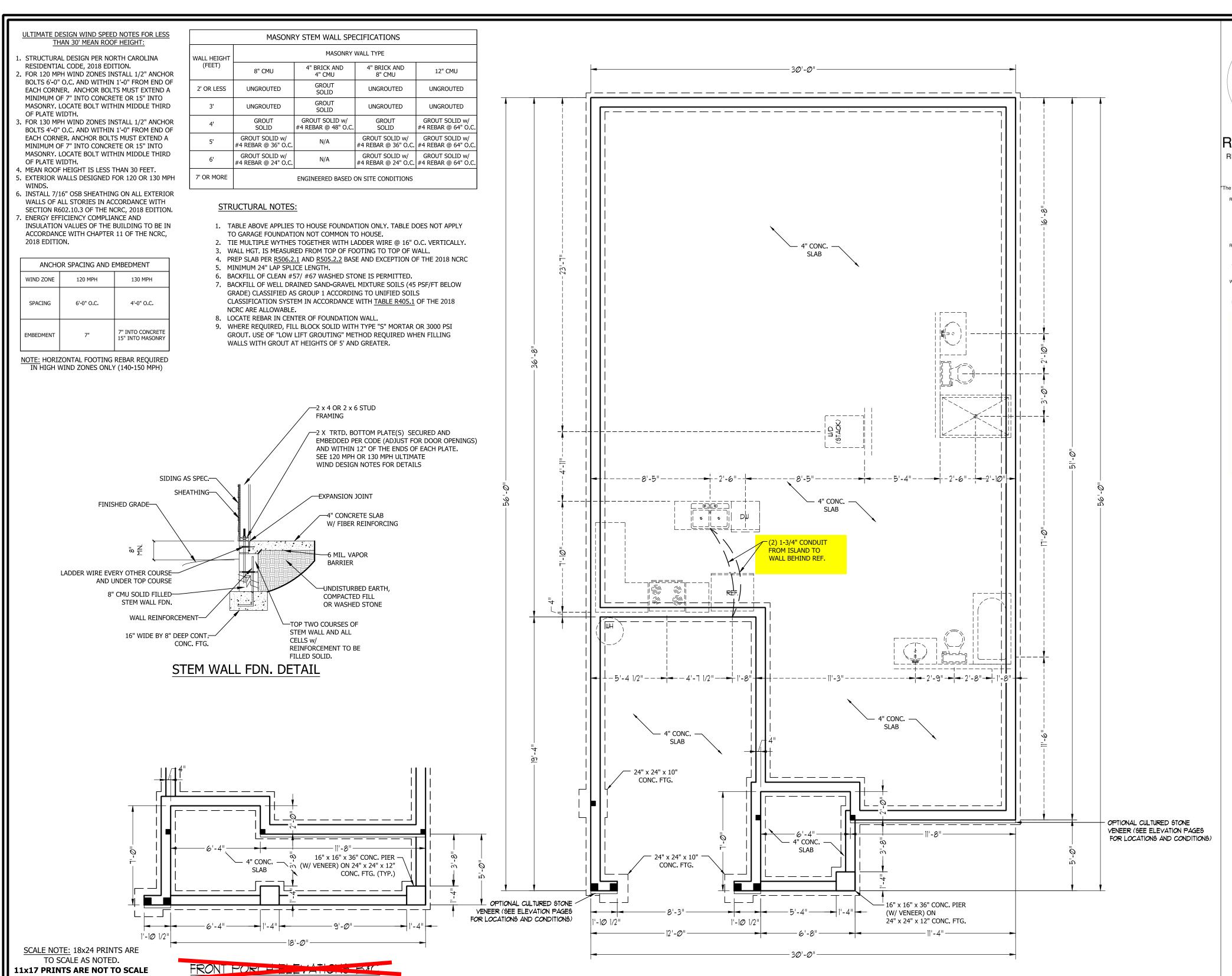
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WEAVER HOMES SAROLINA COLLECTION EYLAND DRIVE LEFT

DATE: JUNE 15, 2020

REV.:

SCALE: 1/4" = 1'-0"

DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:

STEMWALL SLAB FOUNDATION PLAN

S-1

 $C: \label{locuments} \label{locuments} C: \label{locuments} We aver \label{locuments} \label{locuments} AM \label{locuments} AM \label{locuments} \label{locuments} AM \label{locuments} \label{locuments} \label{locuments} AM \label{locuments} \l$ 

# **BRACE WALL PANEL NOTES:**

**EXTERIOR WALLS:** ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE,

REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

**GYPSUM:** ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

**HD:** 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

# **METHODS:** PER TABLE R602.10.1

1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2

**STRUCTURAL NOTES:** 

- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 4 (UNO).
- 3. INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
- 4. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
- 7. FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

(2) 9-1/4" LVL (2) 9<del>-</del>1/4" LVL  $\mathbf{x}$ /--(3) 2 x 4 (3) 2 x 4 — —(3) 2 x 4 GIRDER TRUSS AS SPEC. ROOF TRUSSES AS SP (2) 2 x 10 (TYP.) (2) 2x12 SYP #2 CONT. W/ (2) 2x6 EA. BEARING POINT POST MIN. (TYP.)



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DATE: JUNE 15, 2020

REV.:

SCALE: 1/4" = 1'-0"

DRAWN BY: WG

ENGINEERED BY: REVIEWED BY:

SECOND FLOOR

FRAMING PLAN

S-2

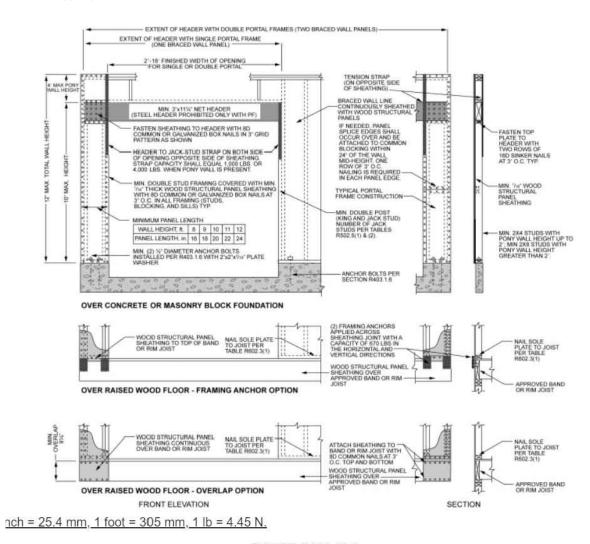
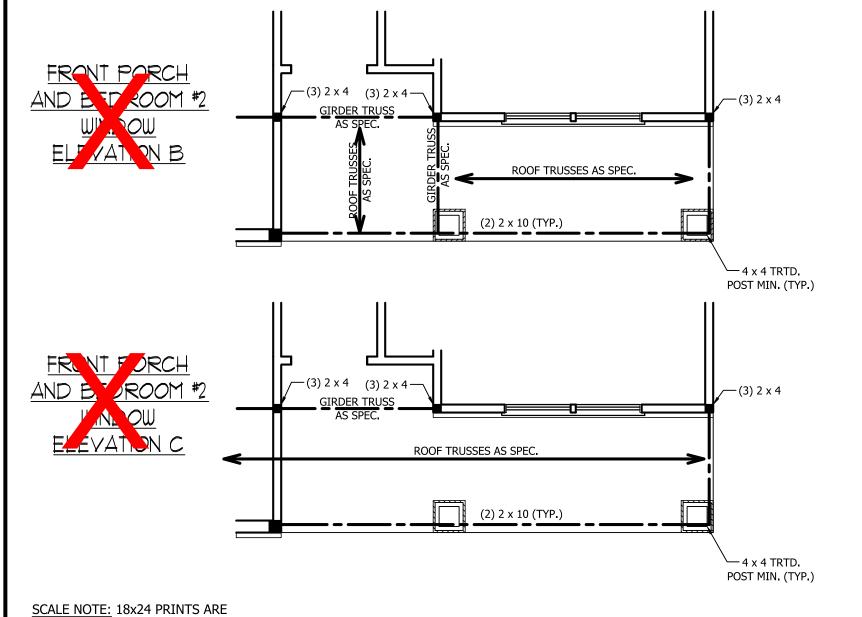


FIGURE R602.10.1

METHOD PF—PORTAL FRAME CONSTRUCTION

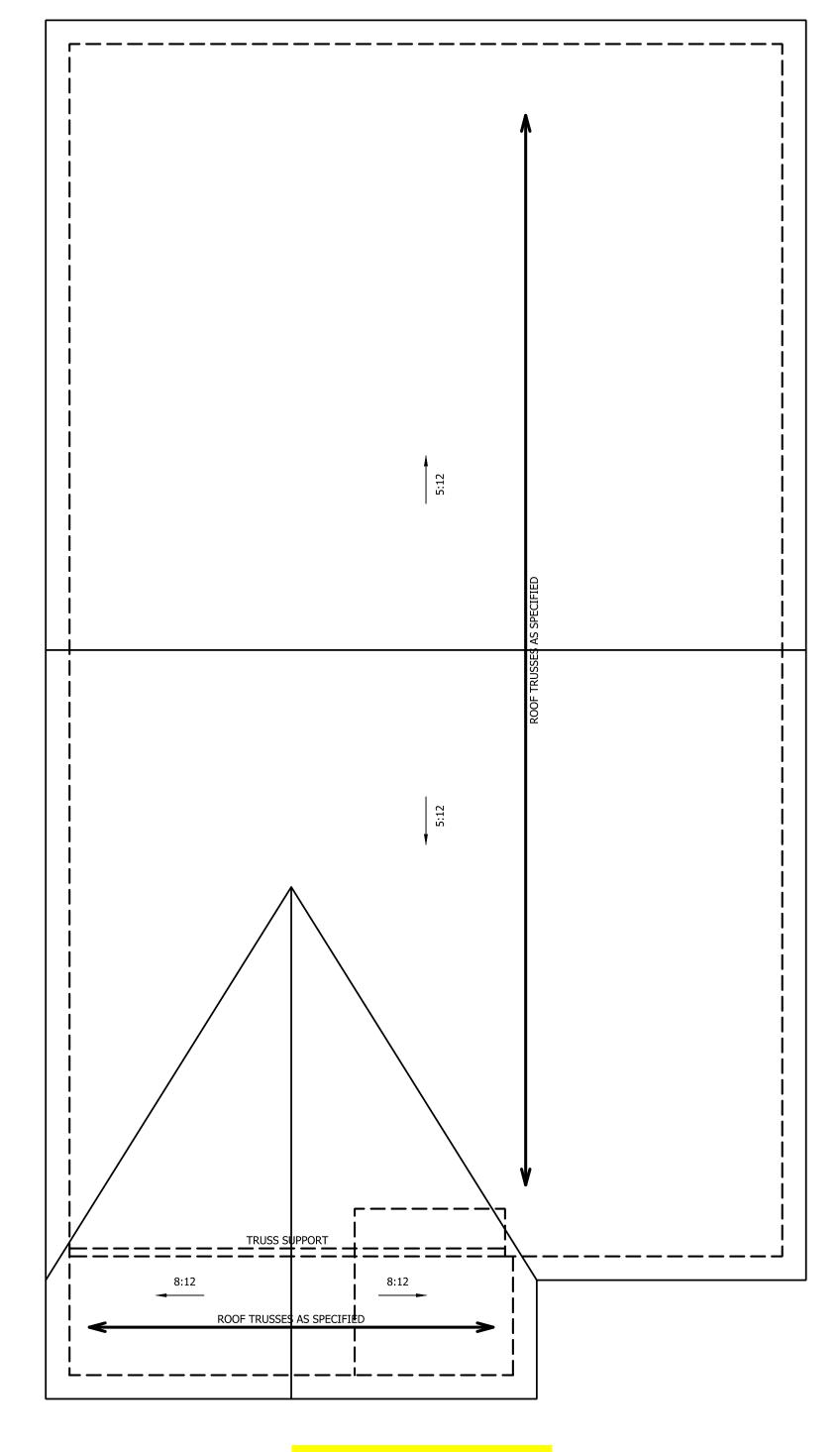


# ATTIC VENT CALCULATION:

1756 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 11.7 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

# STRUCTURAL NOTES:

- ALL FRAMING LUMBER TO BE #2 SPF (UNO).
- HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- 3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE ELEVATION A



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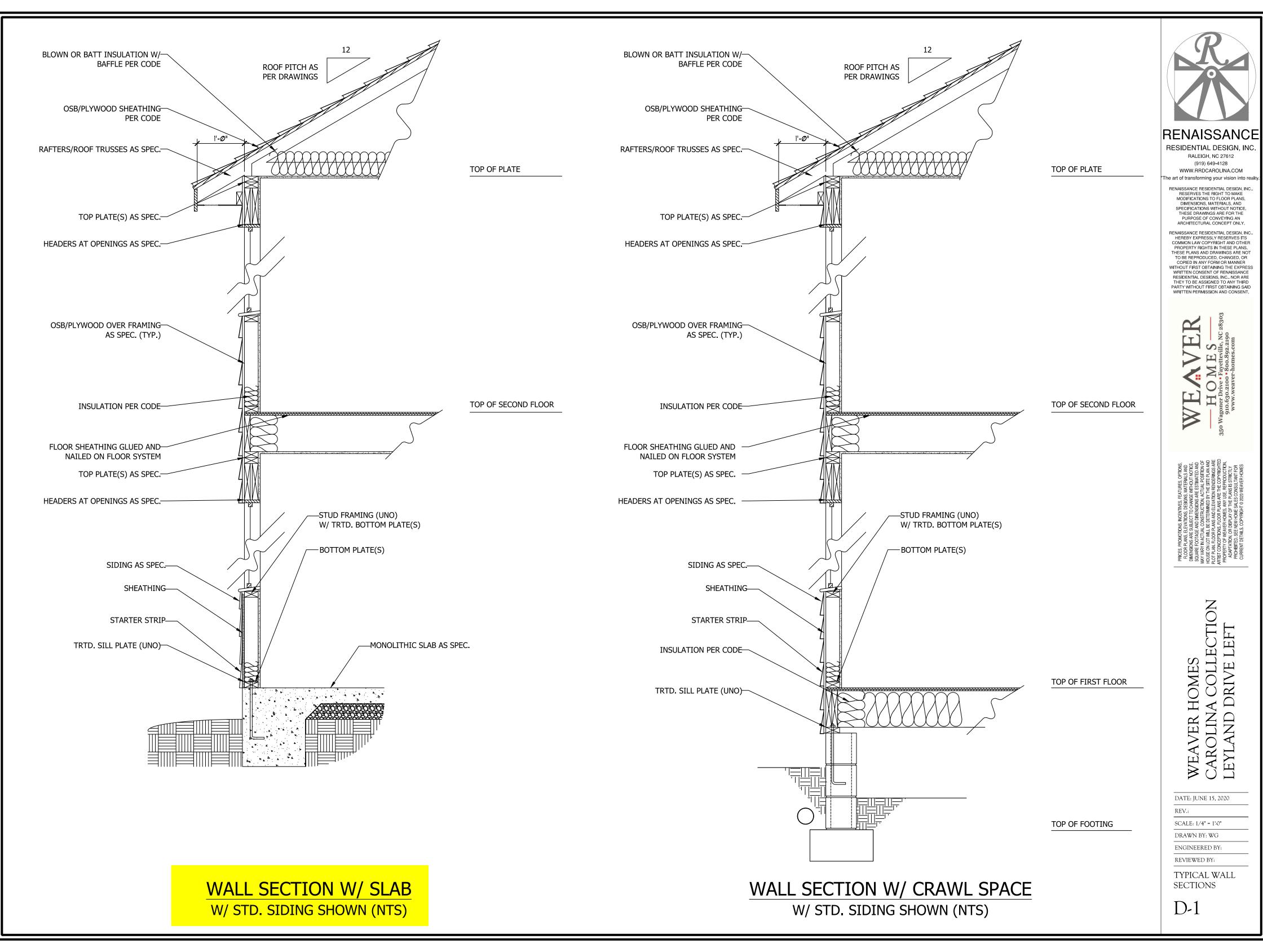
DATE: JUNE 15, 2020

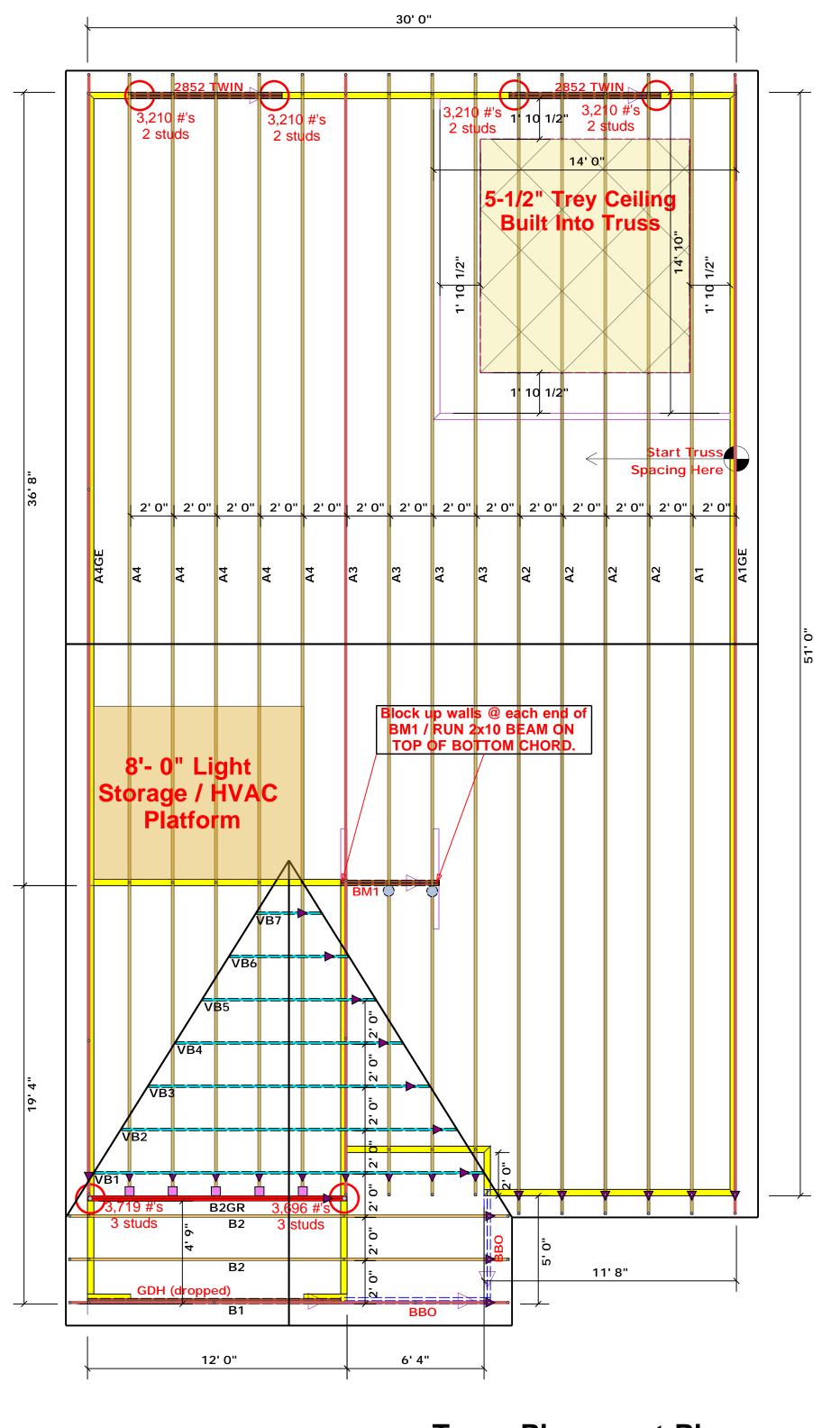
SCALE: 1/4" = 1'-0" DRAWN BY: WG

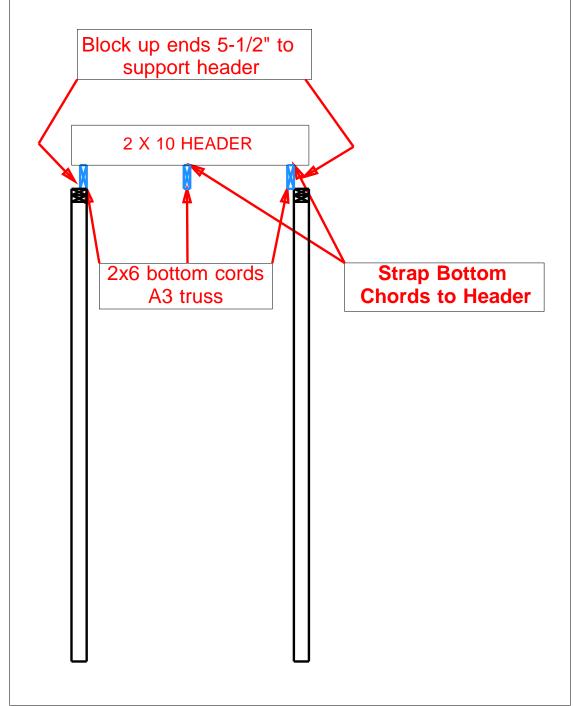
ENGINEERED BY: REVIEWED BY:

ROOF PLAN ELEVATION-A

S-3







HUS28 USP 5 16d/3-1/2 | 16d/3-1/2" USP 2 MSH422 10d/3" 10d/3"

Truss Placement Plan SCALE: 1/4" = 1'-0"

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)

Estimation Selection Formula Calculation Name Roof Area 1st Floor Roof Area 1981.44 1st Floor **Roof Decking** Roof Decking 68 sheets

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Truss Reactions are Less

than 3,000 lbs. Unless Noted Otherwise.

BEAM LEGEND								
PlotID	Length	Product	Plies	Net Qty	Fab T			
2852 TWIN	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4	FF			
GDH (dropped)	12' 0"	2x12 SPF No.2	2	2	FF			

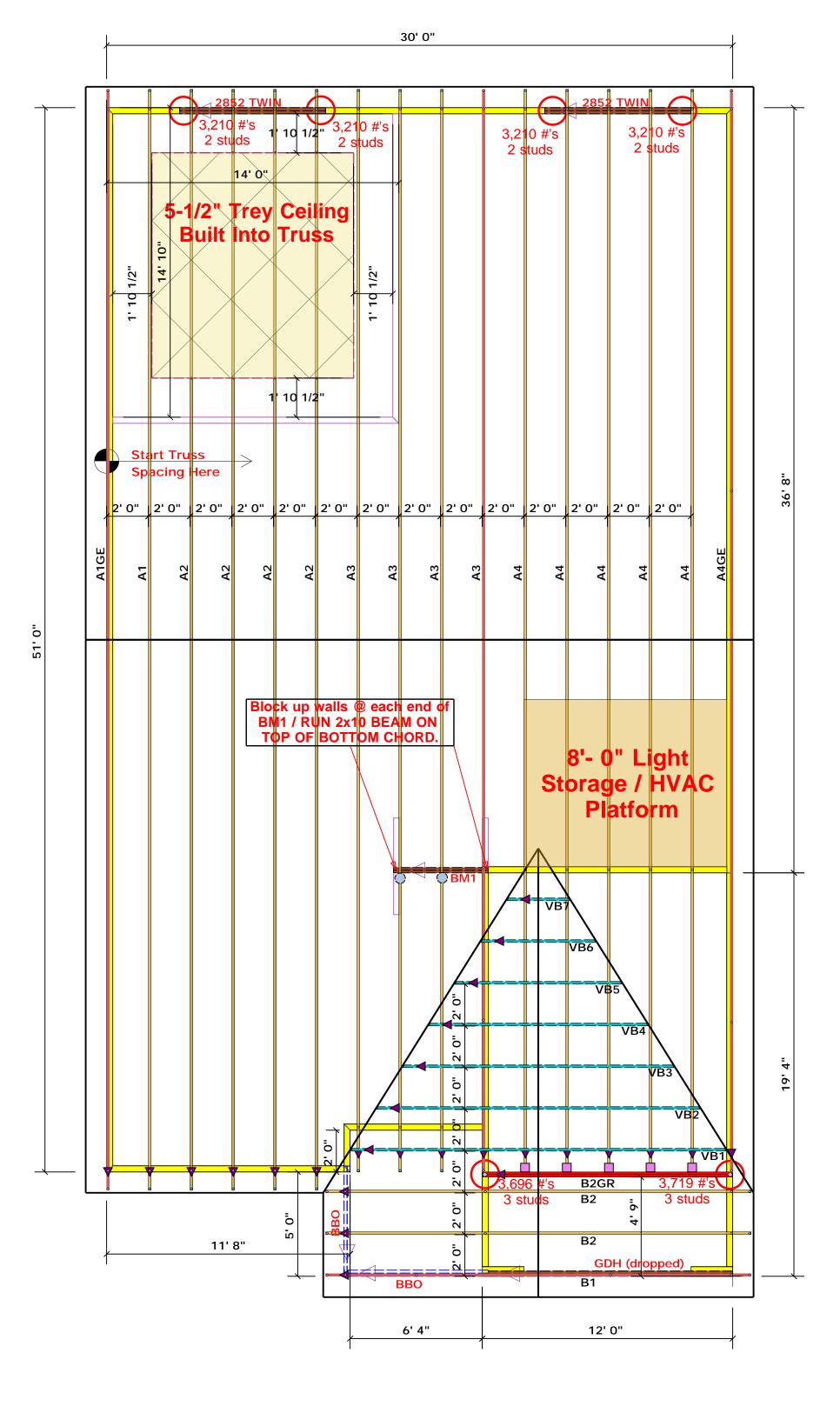
**ROOF & FLOOR TRUSSES & BEAMS** Reilly Road Industrial Park Fayetteville, N.C. 28309

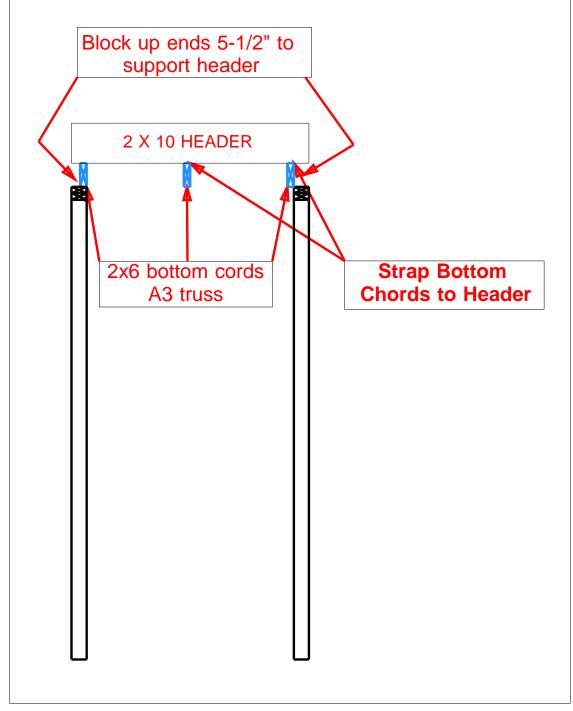
> Phone: (910) 864-8787 Fax: (910) 864-4444

соттесн

LOAD CHART FOR JACK STUDS  (BASED ON FAILER (SECS)) J (B)  MANUS OF JACK STUDS (CONTROL & CATOM OF		BUILDER	Weaver Development	CITY / CO.	Sanford / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.  These trusses are designed as individual building components to be incorporated the building design at the specification of the building designer. See individual des sheets for each truss design identified on the placement drawing. The building des			
	HAN BLACTOCH (1) TO WAS CONTROL OF TO WAS CONTROL OF TOWN WE CANTROL OF TOWN WE CONTROL OF TOWN WE CONTROL OF TOWN WE CONTROL O	FEADERSTROER Z SE	N 25 50 50 50 50 50 50 50 50 50 50 50 50 50	JOB NAME	Lot 8 West Park	ADDRESS	212 West Park Lane	is responsible for temporary and permanent bracing of the roof and floor system at the overall structure. The design of the truss support structure including headers, I walls, and columns is the responsibility of the building designer. For general guida regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery p	
		SADAGA CRAS UTS CORN F VARIE	Mary Mary H	PLAN	Leyland "A"	MODEL	ROOF	or online @ sbcindustry.com  Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached in the contractor shall refer to the con	
	1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	Seal Date	DATE REV.	/ /	( derived from the prescriptive Code requirements ) to determine the minim foundation size and number of wood studs required to support reactions gi than 3000# but not greater than 15000#. A registered design professional si be retained to design the support system for any reaction that exceeds tho	
	6800 4 8500 5 10200 6	0 5 12750 5 17000 0 6 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Lenny Norris	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000 Lenny Norris	
	11900 7 13600 8 15300 9			JOB #	J0921-5307	SALES REP.	Lenny Norris	Lenny Norris	

# 1700 1 3400 2 5100 3 6800 4 8500 5 10200 6 11900 7 13600 8 15300 9





HUS28 USP 5 16d/3-1/2 16d/3-1/2" MSH422 Varies 10d/3"

(045Fb ON 140LES R5025(1) & (b)) NUMBER OF DIACK STUDG REQUIRED IN CALEND OF PEADER/PERDER

2550 1 5100 2

7650 3

10200 4

12750 5

15300 6

3400

6800 2

10200 3

13600 4

17000 5

1700 1 3400 2

Estimation								
Name Selection Formula Calculation								
Roof Area	1st Floor	Roof Area	1981.44					
Roof Decking	1st Floor	Roof Decking	68					

JOB NAME

**SEAL DATE** 

QUOTE #

JOB #

PLAN

**BEAM LEGEND** PlotID 7' 2852 TWIN GDH (dropped)

Truss Placement Plan SCALE: 1/4" = 1'-0"

**ADDRESS** 

DATE REV.

DRAWN BY

SALES REP.

MODEL

Sanford / Harnett

212 West Park Lane

**ROOF** 

**Lenny Norris** 

Lenny Norris

/ /

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

PIOTID	Lengtr	n Product		Piles	Net Qty	гар гуре	
2852 TWIN	7' 0"	1-3/4"x 9-1	I/4" LVL Kerto-S	2	4	FF	
GDH (dropped)	12' 0"	2x12 SPF N	lo.2	2	2	FF	
LOAD CHART FOR JACK STUDS (0455 ON 1405 (8025)) 4 (6))		BUILDER	Weaver Develop	ment			CITY / CO.

Lot 8 West Park

Leyland "A"

Seal Date

Quote #

J0921-5307

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.  These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com						
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.						
Signature Lenny Norris						

Lenny Norris



Phone: (910) 864-8787

Fax: (910) 864-4444

isDesign

Client: WESTAN

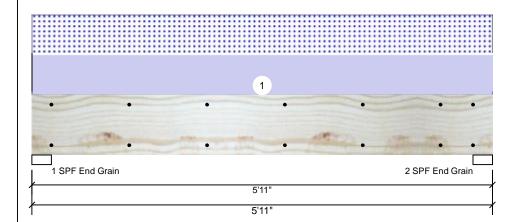
Project: Address: Date: Input by: 9/13/2021

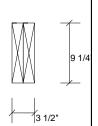
Lenny Norris Job Name: LEYLAND A

Project #:

**2852 TWIN Kerto-S LVL** 1.750" X 9.250" 2-Ply - PASSED

Level: Level





Page 1 of 1

Member Information							
Type:	Girder						
Plies:	2						
Moisture Condition:	Dry						
Deflection LL:	480						
Deflection TL:	360						
Importance:	Normal						
Temperature:	Temp <= 100°						

Application: Floor ASD Design Method: **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Brg Wind Live Dead Snow Const 0 1616 1595 0 0 1 0 2 0 1616 1595 0

### Analysis Results Analysis Actual Location Allowed 4166 ft-lb Moment 2'11 1/2" 14423 ft-lb

4166 ft-lb

2170 lb

Comb. Case Capacity 0.289 (29%) D+S L 2'11 1/2" 11027 ft-lb 0.378 (38%) D+S L 4'11 1/2" 7943 lb 0.273 (27%) D+S ī LL Defl inch 0.032 (L/2069) 2'11 1/2" 0.139 (L/480) 0.230 (23%) S TL Defl inch 0.065 (L/1028) 2'11 1/2" 0.185 (L/360) 0.350 (35%) D+S

Bearings			
Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF 3.000" End Grain	35% 1616 / 1595	3210 L	D+S
2 - SPF 3.000" End Grain	35% 1616 / 1595	3210 L	D+S

### **Design Notes**

Unbraced

Shear

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID Trib Width Side Dead 0.9 Load Type Location Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Top 539 PLF 0 PLF 539 PLF 0 PLF 0 PLF TRUSSES A2,A4 Self Weight 7 PLF

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 2/26/2023





Client: WESTAN

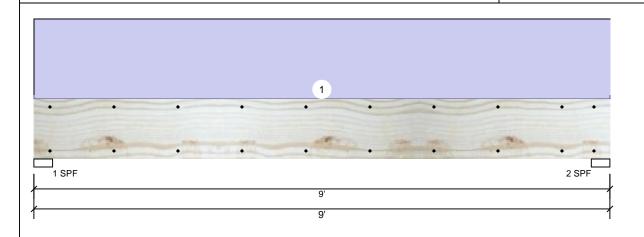
Project: Address: Date: Input by:

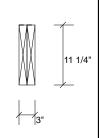
9/13/2021 Lenny Norris Job Name: LEYLAND A

Project #:

2.000" X 12.000" 2-Ply - PASSED S-P-F #2

Level: Level





Page 1 of 1

Member Information					Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	V	Vind	Const	
Plies:	2	Design Method:	ASD	1	0	1350	0		0	0	
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	0	1350	0		0	0	
Deflection LL:	480	Load Sharing:	No								
Deflection TL:	360	Deck:	Not Checked								
Importance:	Normal										
Temperature:	Temp <= 100°F										
				Bearing	gs						
				Bearing	g Length	Cap. Rea	act D/L lb	Total	Ld. Case	Ld. Comb.	
				1 - SPF	3.500"	30%	1350 / 0	1350	Uniform	D	
				2 - SPF	3 500"	30%	1350 / 0	1350	Uniform	D	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2736 ft-lb	4'6"	4153 ft-lb	0.659 (66%)	D	Uniform
Unbraced	2736 ft-lb	4'6"	3515 ft-lb	0.778 (78%)	D	Uniform
Shear	1000 lb	1'2"	2734 lb	0.366 (37%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.072 (L/1422)	4'6"	0.285 (L/360)	0.250 (25%)	D	Uniform

### **Design Notes**

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	300 PLF	0 PLF	0 PLF	0 PLF	0 PLF	GABLE WEIGHT

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