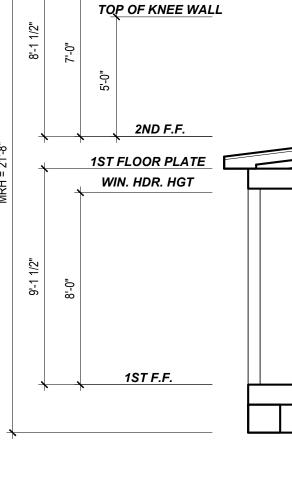
GENERAL NOTES:

- 1. IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY THAT ALL DIMENSIONS, ROOF PITCHES, AND SQUARE FOOTAGE ARE CORRECT PRIOR TO CONSTRUCTION. K&A HOME DESIGNS, INC. IS NOT RESPONSIBLE FOR ANY DIMENSIONING, ROOF PITCH, OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 2. ALL WALLS SHOWN ON THE FLOOR PLANS ARE DRAWN AT 4" UNLESS NOTED OTHERWISE.
- 3. ALL ANGLED WALL SHOWN ON THE PLANS ARE 45 DEGREES UNLESS NOTED OTHERWISE.
- 4. STUD WALL DESIGN SHALL CONFORM TO ALL NORTH CAROLINA STATE BUILDING CODE REQUIREMENTS.
- 5. DO NOT SCALE PLANS. DRAWING SCALE MAY BE DISTORTED DUE TO COPIER IMPERFECTIONS.
- 6. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH NORTH CAROLINA RESIDENTIAL STATE BUILDING CODE, 2018 EDITION.

SQUARE FOOTAGE

| <u>HEATED SQUARE FOOTAGE</u> | | <u>UNHEATED SQUARE F</u> | <u>OOTAGE</u> |
|------------------------------|------|--------------------------|---------------|
| FIRST FLOOR= | 1960 | CARPORT= | 542 |
| SECOND FLOOR= | 1207 | FRONT PORCH= | 319 |
| THIRD FLOOR= | N/A | CVD. PORCH= | 576 |
| BASEMENT= | N/A | BREEZEWAY= | 64 |
| | | STORAGE= | 123 |
| | | | |
| TOTAL HEATED= | 3167 | TOTAL UNHEATED | = <u>1624</u> |



2ND FLOOR PLATE

WIN.HDR.HGT

CRAWL SPACE VENTILATION CALCULATIONS

-VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON THE PLAN BUT SHOULD BE PLACED TO PROVIDE ADEQUATE VENTILATION AT ALL POINTS TO PREVENT DEAD AIR POCKETS.

-100% VAPOR BARRIER MUST BE PROVIDED WITH 12" MIN. LAP JOINTS.

-THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1500 AS LONG AS **REQUIRED OPENINGS ARE PLACED SO AS TO PROVIDE CROSS-VENTILATION OF THE SPACE.** THE INSTALLATION OF OPERABLE LOUVERS SHALL NOT BE PROHIBITED. (COMPLY WITH NC CODE MIN. WITH REGARD TO VENT PLACEMENT FROM CORNERS)

| 1960 | SQ. FT. OF CRA | WL SPACE/1500 |
|--------------|----------------|--------------------------------|
| 1.31 | SQ. FT. OF REC | QUIRED VENTILATION |
| PROVIDED BY: | 3 | VENTS AT 0.45 SQ. FT. NET FREE |

VENTILATION EACH= 1.35 SQ. FT. OF VENTILATION

**FOUNDATION DRAINAGE- WATERPROOFING PER SECTIONS 405 & 406.

ATTIC VENTILATION CALCULATIONS

- CALCULATIONS SHOWN BELOW ARE BASED ON VENTILATORS USED AT LEAST 3 FT. ABOVE THE CORNICE VENTS WITH THE BALANCE OF VENTIALTION PROVIDED BE EAVE VENTS.

- CATHEDRAL CEILINGS SHALL HAVE A MIN. 1" CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.

3444 SQ. FT. OF ATTIC/300= 11.48

EACH OF INLET AND OUTLET REQUIRED.

*WALL AND ROOF CLADDING DESIGN VALUES

- WALL CLADDING IS DESIGNED FOR A 24.1 SQ. FT. OR GREATER POSITIVE AND NEGATIVE PRESSURE.

- ROOF VALUES BOTH POSITVE AND NEGATIVE SHALL BE AS FOLLOWS:

45.5 LBS. PER SQ. FT. FOR ROOF PITCHES OF 0/12 TO 2.25/12

34.8 LBS. PER SQ. FT. FOR ROOF PITCHES OF 2.25/12 TO 7/12

21 LBS. PER SQ. FT. FOR ROOF PITCHES OF 7/12 TO 12/12

** MEAN ROOF HEIGHT 30' OR LESS

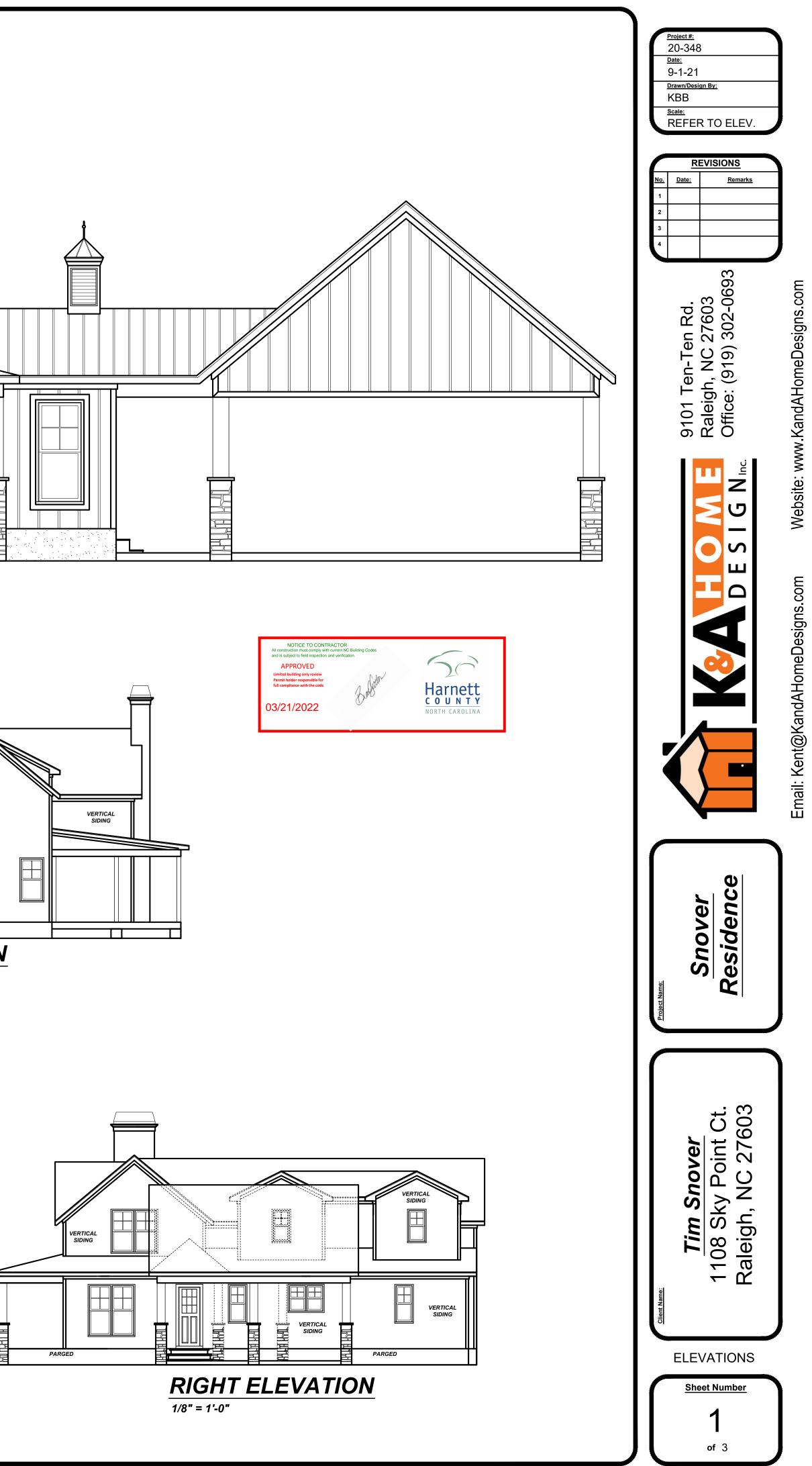


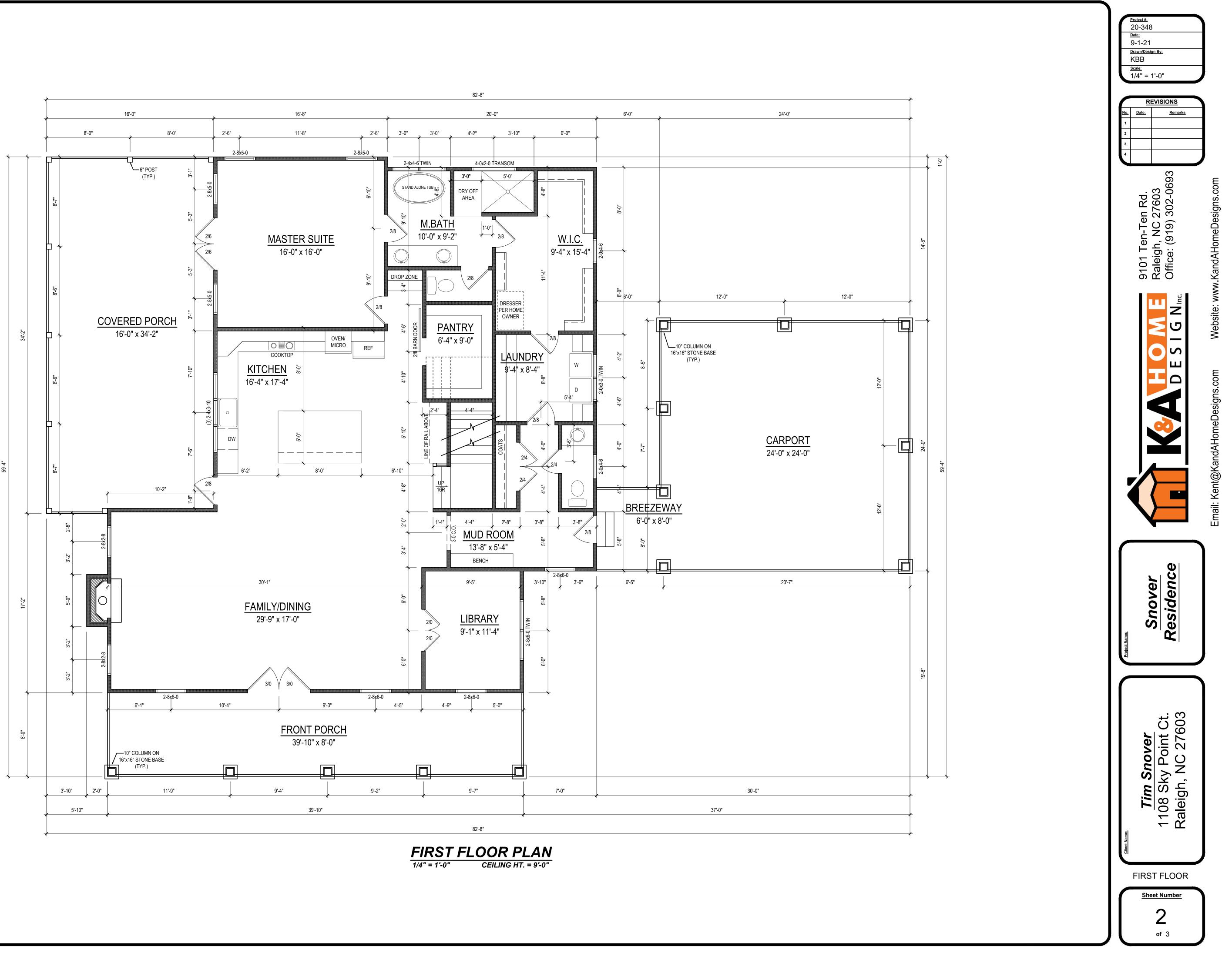
















SECOND FLOOR PLAN 1/4" = 1'-0" CEILING HT. = 8'-0"

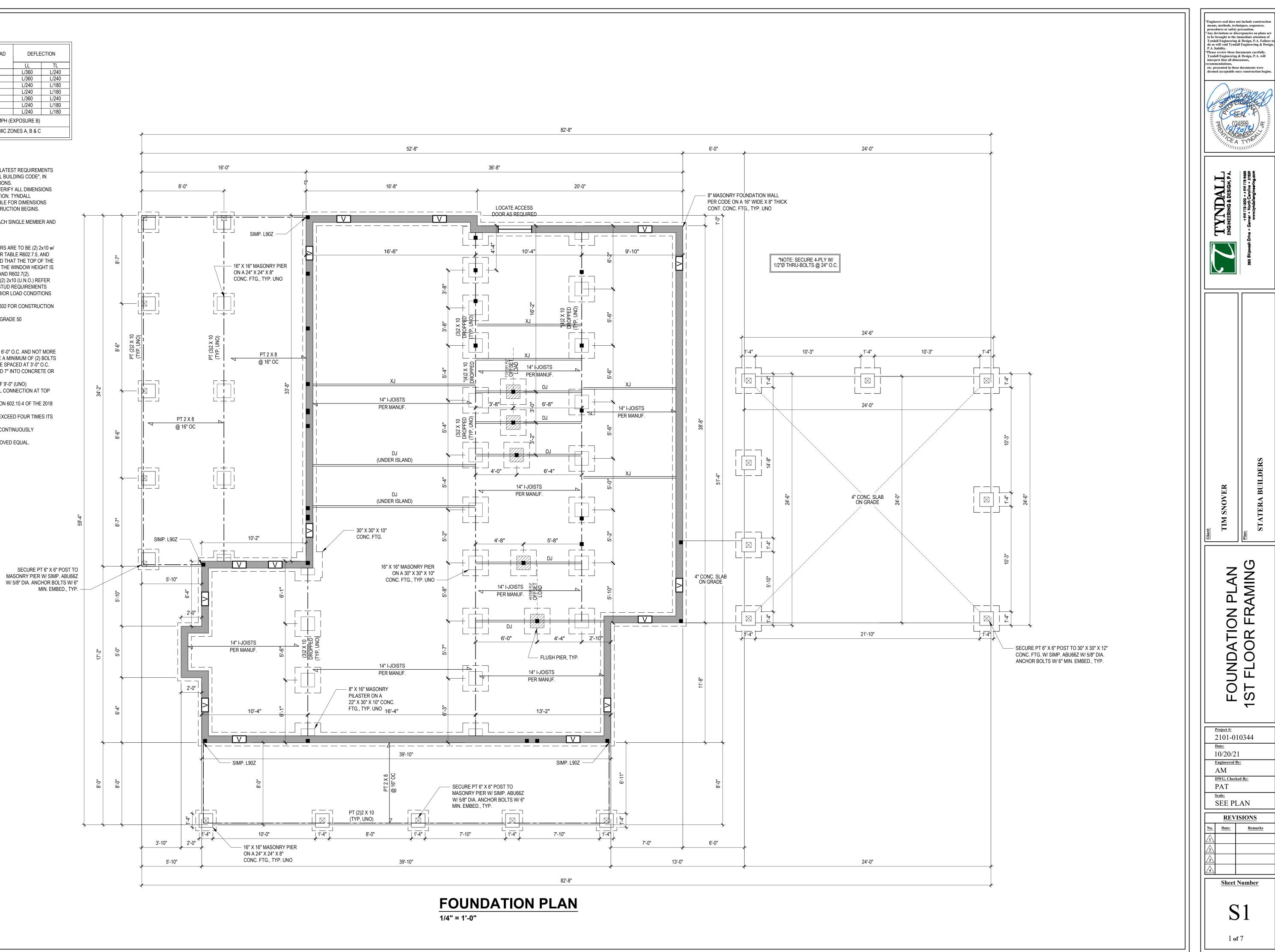
| Project #: 20-348 Date: 9-1-21 Drawn/Design By: KBB | |
|---|--|
| Scale: 1/4" = 1'-0" REVISIONS No. Date: Remarks 1 | |
| 9101 Ten-Ten Rd. Baleigh, NC 27603 Office: (919) 302-0693 | Email: Kent@KandAHomeDesigns.com Website: www.KandAHomeDesigns.com |
| | (ent |
| Snover | Email: Kent@ |
| Poiet Name | Email: Kent@ |
| Internet Int | Email: Kent@ |
| Tim Snover 1108 Sky Point Ct. Raleigh, NC 27603 | Email: Kent@ |

DESIGN LOADS

| | LIVE LOAD (PSF) | DEAD LOAD (PSF) | DEFLE | CTION | |
|--------------------|---------------------------------|--------------------|-------------|-------|--|
| | | | LL | TL | |
| FLOOR (primary) | 40 | 10 | L/360 | L/240 | |
| FLOOR (secondary) | 40 | 10 | L/360 | L/240 | |
| ATTIC (w/ storage) | 20 | 10 | L/240 L/180 | | |
| ATTIC (no access) | 10 | 5 | L/240 L/180 | | |
| EXTERNAL BALCONY | 40 10 L/360 | | | | |
| ROOF | 20 | 10 | L/240 | L/180 | |
| ROOF TRUSS | 20 | 20 | L/240 | L/180 | |
| WIND LOAD | BASED ON 120 MPH (EXPOSURE B) | | | | |
| SEISMIC | BASED ON SEISMIC ZONES A, B & C | | | | |

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 3) ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ 4) (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER 5) TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 7)
- Fy = 50 KSI MIN. (UNO) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF 10) 1) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
- MASONRY. 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
- 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION. 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



DESIGN LOADS

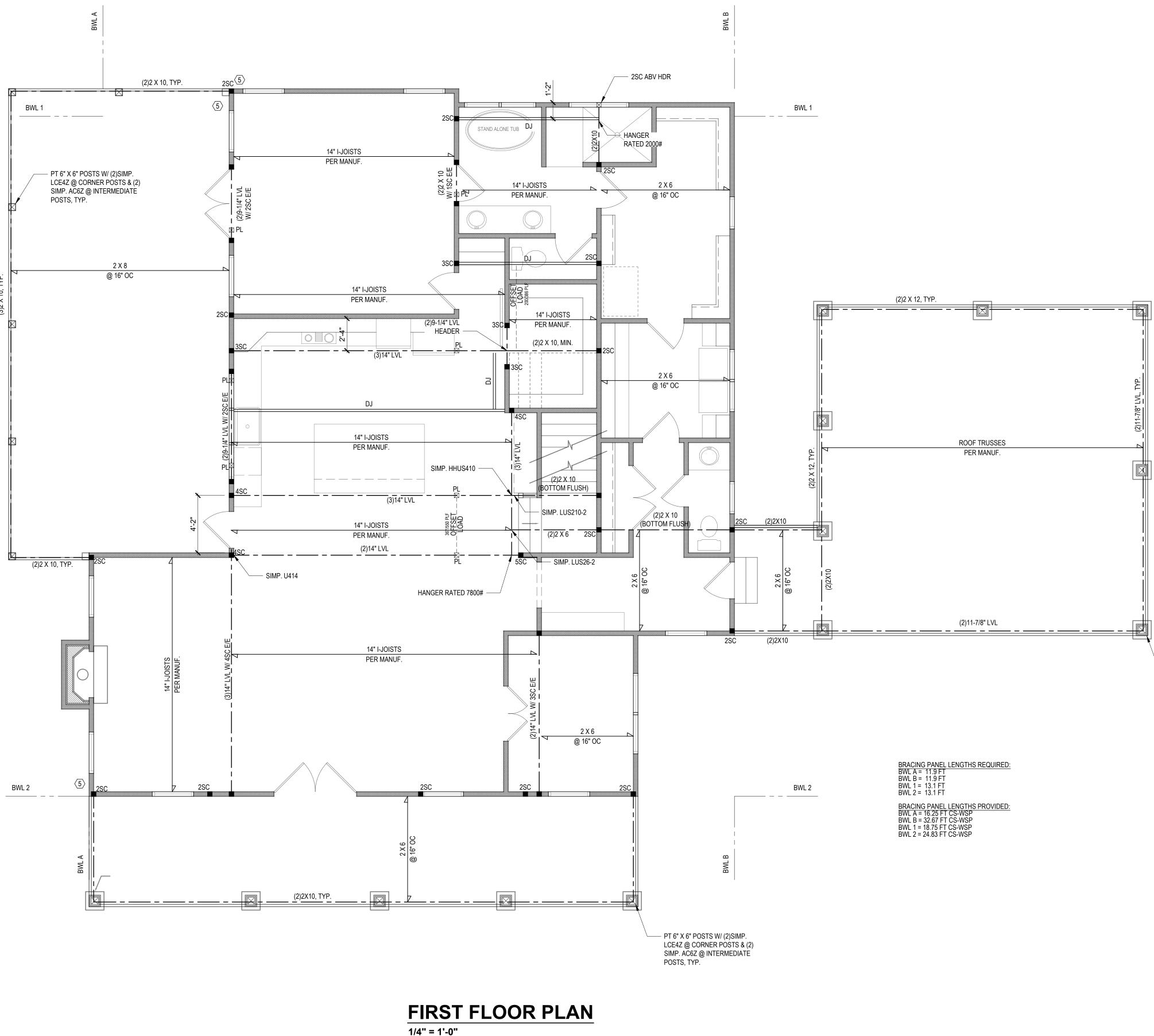
| | LIVE LOAD (PSF) | DEAD LOAD (PSF) | DEFLE | CTION | |
|--------------------|---------------------------------|--------------------|-------|-------|--|
| | · · · · | , | LL | TL | |
| FLOOR (primary) | 40 | 10 | L/360 | L/240 | |
| FLOOR (secondary) | 40 | 10 | L/360 | L/240 | |
| ATTIC (w/ storage) | 20 | 10 | L/240 | L/180 | |
| ATTIC (no access) | 10 5 L/240 L | | | | |
| EXTERNAL BALCONY | 40 | 10 | L/360 | L/240 | |
| ROOF | 20 | 10 | L/240 | L/180 | |
| ROOF TRUSS | 20 | 20 | L/240 | L/180 | |
| WIND LOAD | BASED ON 120 MPH (EXPOSURE B) | | | | |
| SEISMIC | BASED ON SEISMIC ZONES A, B & C | | | | |

STRUCTURAL NOTES:

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- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS. 3) ALL LUMBER SHALL BE SYP #2 (UNO)
- ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ 4) (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER 5) TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 7) Fy = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- IRC. 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
- LEAST HORIZONTAL DIMENSION. 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION. 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- LESS. 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF
- THE 2018 NCRC. 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- $\langle 1 \rangle$ REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- $\langle 2 \rangle$ 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN
 - 67% OF WALL HEIGHT - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
 - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- $\langle 4 \rangle$ SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- $\langle 5 \rangle$ MINIMUM 800# HOLD-DOWN DEVICE



PM

3:31

| means, methods, tech procedures or safety *Any deviations or dis to be brought to the i Tyndall Engineering do so will void Tynda P.A. liability. *Please review these do Tyndall Engineering interpret that all dim recommendations, etc. presented in these | *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, | | | | | |
|--|---|--|--|--|--|--|
| ALL Design, PA. | 4899 4999 49 4 | | | | | |
| TYNDAL ENGINEERING & DESIGN 7 919 778-1200 = # 919 77 250 Shipwash Drive = Garner = North Carolina = www.tyndailenginearti | | | | | | |
| <u>client:</u> TIM SNOVER | Plan: STATERA BUILDERS | | | | | |
| 1ST FLOOR HEADER | FLOOR HEADER FLOOR FRAMING | | | | | |
| Project #: 2101-010344 Date: 10/20/21 Engineered By: AM DWG. Checked By: PAT Scale: SEE PLAN REVISIONS No. Date: Remarks | | | | | | |
| No. Date: Remarks 1 | | | | | | |

- PT 6" X 6" POSTS W/ (2)SIMP. LCE4Z @ CORNER POSTS & (2) SIMP. AC6Z @ INTERMEDIATE POSTS, TYP.

DESIGN LOADS

| | LIVE LOAD (PSF) | DEAD LOAD (PSF) | DEFLECTION | | | |
|--------------------|---------------------------------|--------------------|-------------|-------|--|--|
| | (-) | (-) | LL | TL | | |
| FLOOR (primary) | 40 | 10 | L/360 | L/240 | | |
| FLOOR (secondary) | 40 | 10 | L/360 | L/240 | | |
| ATTIC (w/ storage) | 20 | 10 | L/240 L/180 | | | |
| ATTIC (no access) | 10 5 L/240 L/* | | | | | |
| EXTERNAL BALCONY | 40 10 L/360 L | | | | | |
| ROOF | 20 | 10 | L/240 | L/180 | | |
| ROOF TRUSS | 20 | 20 | L/240 | L/180 | | |
| WIND LOAD | BASED ON 120 MPH (EXPOSURE B) | | | | | |
| SEISMIC | BASED ON SEISMIC ZONES A, B & C | | | | | |

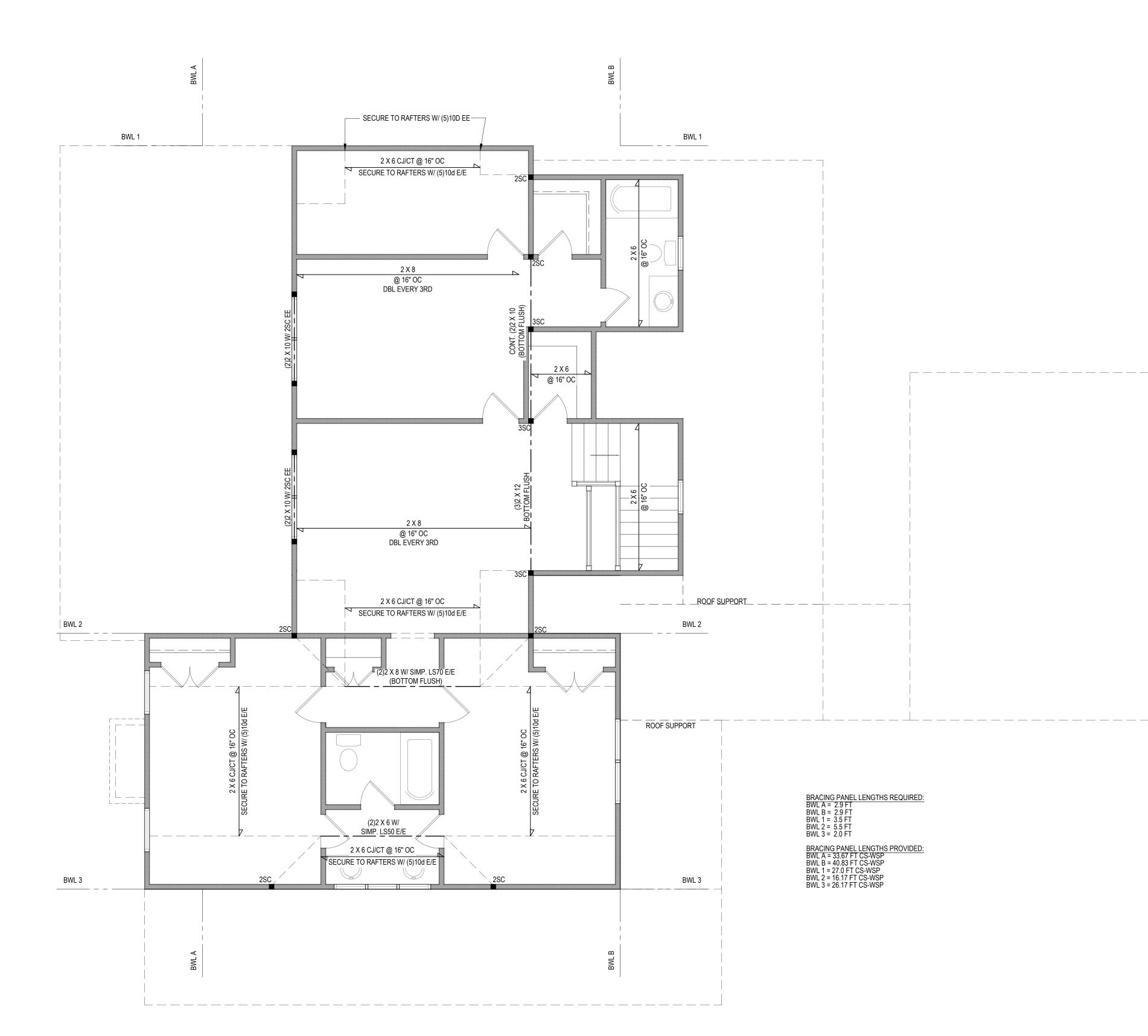
STRUCTURAL NOTES:

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 ALL LUMBER SHALL BE SYP #2 (UNO)
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- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 9) ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
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- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
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STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- LESS. 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF
- THE 2018 NCRC.
 BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3.
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- 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:

 - 24" ADJACENT TO OPENINGS NOT MORE THAN
 67% OF WALL HEIGHT
 - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
 - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- $\langle \overline{4} \rangle$ SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
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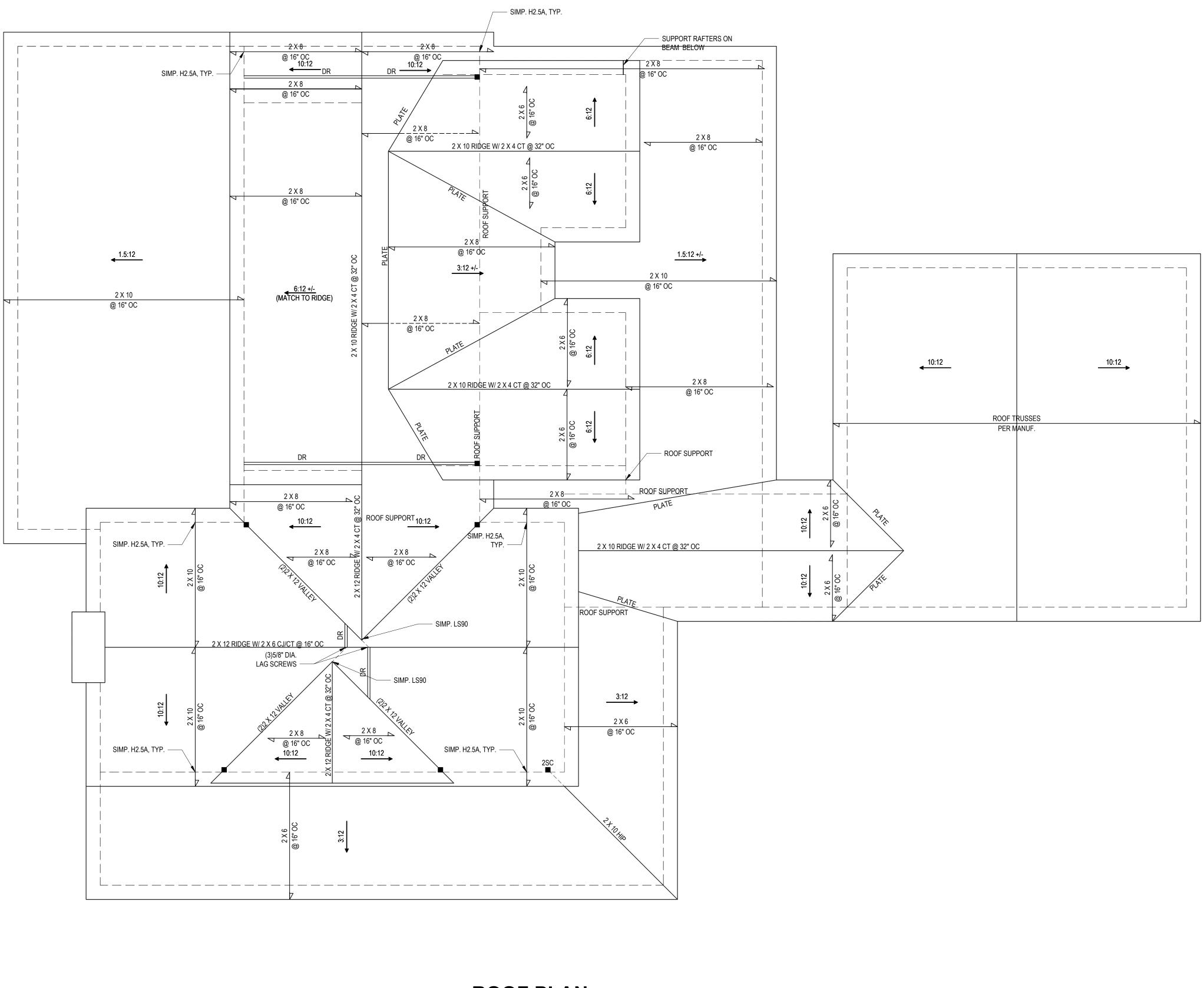
PM

3:31

SECOND FLOOR PLAN

1/4" = 1'-0"

| *Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. *Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins. | | | | | | | |
|---|---|--|--|--|--|--|--|
| TYNDALL ENGINEERING & DESIGN, P.A. | ± 919 773-1200 = # 919 773-9668 250 Shipwash Drive = Garner = North Carolina = 27329 www.tyndallengineering.com | | | | | | |
| client: TIM SNOVER | Plan: STATERA BUILDERS | | | | | | |
| 2ND FLOOR HEADER 2ND FLR. CLG. FRAMING | | | | | | | |
| Project #: 2101-010344 Date: 10/20/21 Engineered By: AM DWG. Checked By: PAT Scale: SEE PLAN REVISIONS No. Date: Remarks 1 2 | | | | | | | |
| <u>Sheet N</u> | | | | | | | |



ROOF PLAN

1/4" = 1'-0"

| *Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. *Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins. | | | | | | |
|---|--|--|--|--|--|--|
| TYNDALL ENGINEERING & DESIGN, P.A. | 778-1200 = F 919-778-1200 = F 919-778-9488 250 Shipwash Drive = Garner = Novah Carolina = 27529 www.tyndallanginearing.com | | | | | |
| client: TIM SNOVER | Plan: STATERA BUILDERS | | | | | |
| ROOF PLAN | | | | | | |
| Project #: 2101-010344 Date: 10/20/21 Engineered By: AM DWG. Checked By: PAT Scale: SEE PLAN REVISIONS <u>Atternal</u> <u>1</u> <u>2</u> <u>3</u> <u>4</u> | | | | | | |
| S 4 of | - | | | | | |

|--|

1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.

2) DESIGN LOADS:

| | LIVE LOAD (PSF) | DEAD LOAD (PSF) | DEFLE | CTION | |
|---------------------------|-------------------------------|--------------------|-------------|-------|--|
| | | | LL | TL | |
| ALL FLOORS | 40 | 10 | L/360 | L/240 | |
| ATTIC (w/ walk up stairs) | 30 | 10 | L/360 | L/240 | |
| ATTIC (pull down access) | 20 | 10 | L/240 | L/180 | |
| ATTIC (no access) | 10 | 5 | L/240 L/180 | | |
| EXTERNAL BALCONY | 40 | 10 | L/360 L/240 | | |
| ROOF | 20 | 10 | L/240 | L/180 | |
| ROOF TRUSS | 20 | 20 | L/240 | L/180 | |
| WIND LOAD | BASED ON 120 MPH (EXPOSURE B) | | | | |
| SEISMIC | SEISMIC ZONES A, B & C | | | | |

3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF

4) CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE. (U.N.O.)

5) MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.

- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO.
 ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.)
 ALL LSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.6M PSI (U.N.O.)
 ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- 7) ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2"Ø x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- 10) PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- 11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.

12) WALL AND ROOF CLADDING VALUES:
WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:
39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12
36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12
18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12
**MEAN ROOF HEIGHT 30'-0" OR LESS

- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- 14) REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 15) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- 19) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 20) MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 21) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

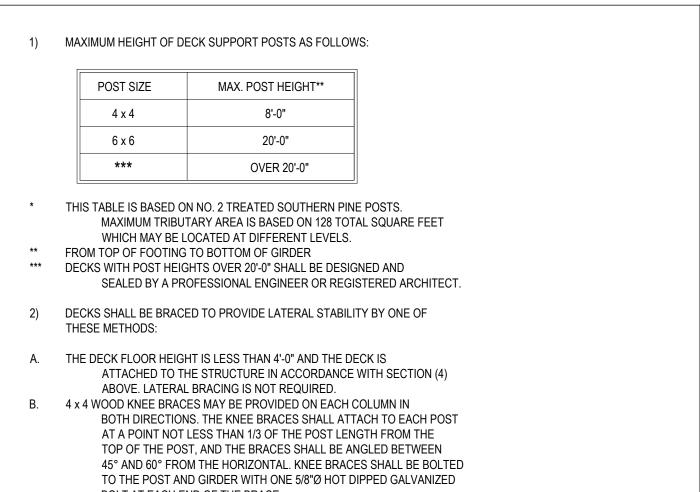
| CLIMATE ZONES | FENESTRATION U-FACTOR | SKYLIGHT ^b U-FACTOR | GLAZED FENESTRATION SHGC ^{b,<u>k</u>} | CEILING ^m R-VALUE | WOOD FRAMED WALL R-VALUE | MASS WALL R-VALUE | FLOOR R-VALUE | BASEMENT ^{c,Q} WALL R-VALUE | SLAB ^d R-VALUE AND DEPTH | CRAWL SPACE ^C WALL R-VALUE |
|------------------|--------------------------|-----------------------------------|--|---|---|--|------------------------|--|---|---|
| 3 | 0.35 | 0.55 | 0.30 | <u>38 or 30</u> <u>cont</u> | 15 or 13 + 2.5 | <u>5/13 or</u> 5/10 cont | 19 | <u>5/13</u> ^f | 0 | 5/13 |
| 4 | 0.35 | 0.55 | 0.30 | 38 or 30 cont ^j | 15 or 13 + <u>2.5</u> ^h | <u>5/13 or</u> 5/10 cont | 19 | <u>10/15</u> | 10 | <u>10/15</u> |
| 5 | <u>0.35</u> | 0.55 | NR | $\frac{38 \text{ or } 30}{\text{cont}}$ | $^{n}\frac{19, \text{ or } 13 + 5}{0 \text{ or } 15 + 3}$ | 13/17 <u>or</u> <u>13/12.5 cont</u> | 30 ^g | <u>10/15</u> | 10 | <u>10/19</u> |

| * | TABLE N1102.1 CLIMATE ZONES 3-5 |
|----------|--|
| NO SCALE | a. R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE. |
| | b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT |
| | (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION. |
| | c. 10/15" MEANS R-10 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OF THE HOME |
| | OR R-15 CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL. |
| | d. FOR MONOLITHIC SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM |
| | OF THE FOOTING OR A MAXIMUM OF 24" BELOW GRADE WHICHEVER IS LESS. FOR FLOATING SLABS, INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24". WHICHEVER IS LESS. R-5 SHALL BE |
| | ADDED TO THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS. |
| | e. <u>DELETED</u> |
| | f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY FIGURE N1101.7 AND TABLE N1101.7. |
| | g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM. |
| | h. THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO "13+5" MEANS R-13 CAVITY INSULATION PLUS R-5 INSULATED |
| | SHEATHING. "15+3" MEANS R-15 CAVITY INSULATION. PLUS R-3 INSULATED SHEATHING. IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR, |
| | INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT |
| | OF THE EXTERIOR, SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2. "13 + 2.5" MEANS R-13 CAVITY |
| | INSULATION PLUS R-2.5 SHEATHING. |
| | i. FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WALL. |
| | i. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO GREATER THAN 0.55 SHALL BE |
| | PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY. |
| | k. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.70 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY. |
| | I. R-30 SHALL BE DEEMED TO SATISFY THE CEILING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF UNCOMPRESSED R-30 INSULATION EXTENDS OVER THE WALL TOP PLATE |
| | AT THE EAVES. OTHERWISE R-38 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN 1 INCH |
| | |
| | m, TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF; THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE. n. R -19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL 2 × 6 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED |
| | AND INSTALLED IN A 2X4 WALL IS NOT DEEMED TO COMPLY. |
| | 0. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT. |
| | |

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| | | DEFINITIONS FOR C | | | |
|-------|---|-----------------------|-------|---|------------------------|
| | | | | | <u> 2113</u> |
| ALT | = | ALTERNATE | MAX | = | MAXIMUM |
| CANT | = | CANTILEVER | MIN | = | MINIMUM |
| CJ | = | CEILING JOIST | NOM | = | NOMINAL |
| CMU | = | CONCRETE MASONRY UNIT | 0.C. | = | ON CENTER |
| COL | = | COLUMN | PL | = | POINT LOAD |
| CONC | = | CONCRETE | PT | = | PRESSURE TREATED |
| CONT | = | CONTINUOUS | REINF | = | REINFORCED |
| СТ | = | COLLAR TIE | REQD | = | REQUIRED |
| DBL | = | DOUBLE | RJ | = | ROOF JOIST |
| DIA | = | DIAMETER | RS | = | ROOF SUPPORT |
| DJ | = | DOUBLE JOIST | SC | = | STUD COLUMN |
| DR | = | DOUBLE RAFTER | SCH | = | SCHEDULE |
| EA | = | EACH | SPEC | = | SPECIFIED |
| EE | = | EACH END | THK | = | THICK |
| FJ | = | FLOOR JOIST | TJ | = | TRIPLE JOIST |
| FND | = | FOUNDATION | TRTD | = | TREATED |
| FTG | = | FOOTING | TYP | = | TYPICAL |
| GALV | = | GALVANIZED | UNO | = | UNLESS NOTED OTHERWISE |
| HORIZ | = | HORIZONTAL | W | = | WIDE FLANGE BEAM |
| HT | = | HEIGHT | WWF | = | WELDED WIRE FABRIC |
| MANUF | = | MANUFACTURER | XJ | = | EXTRA JOIST |



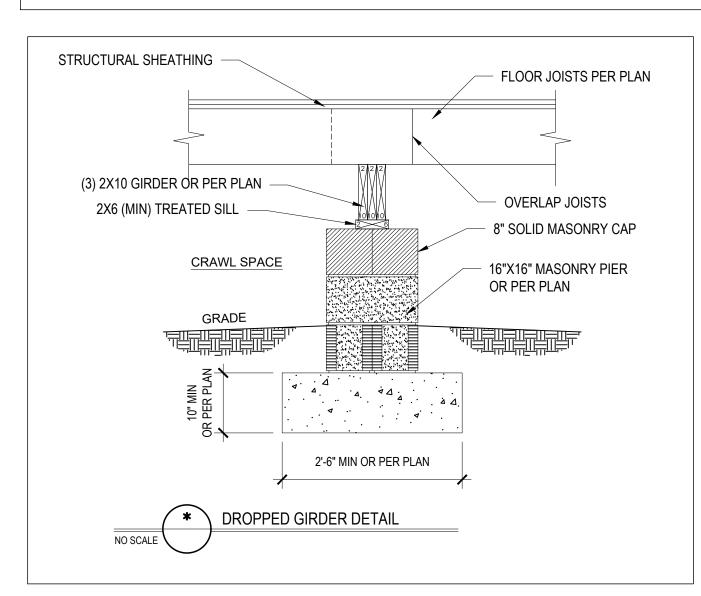
BOLT AT EACH END OF THE BRACE. C. FOR FREESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

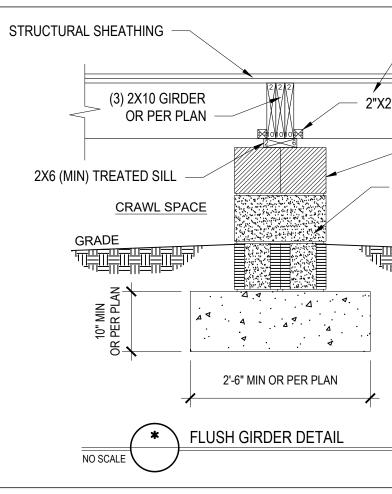
| Г | | | | | |
|---|-----------|------------------------|---------------------|--------------------|----------------------|
| | POST SIZE | MAX. TRIBUTARY AREA | Max. Post Height | EMBEDMENT DEPTH | CONCRETE DIAMETER |
| | 4 x 4 | 48 SQ. FT. | 4'-0" | 2'-6" | 1'-0" |
| | 6 x 6 | 120 SQ. FT. | 6'-0" | 3'-6" | 1'-8" |

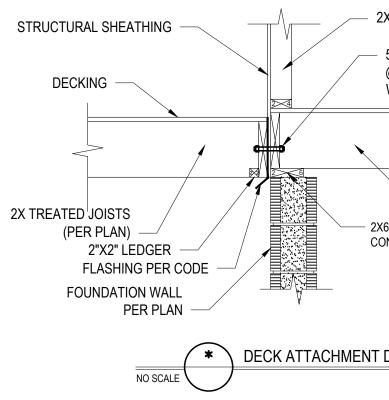
D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO

 (2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL
 TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS.
 THE 2 x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8"Ø HOT
 DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER.

 E. FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.

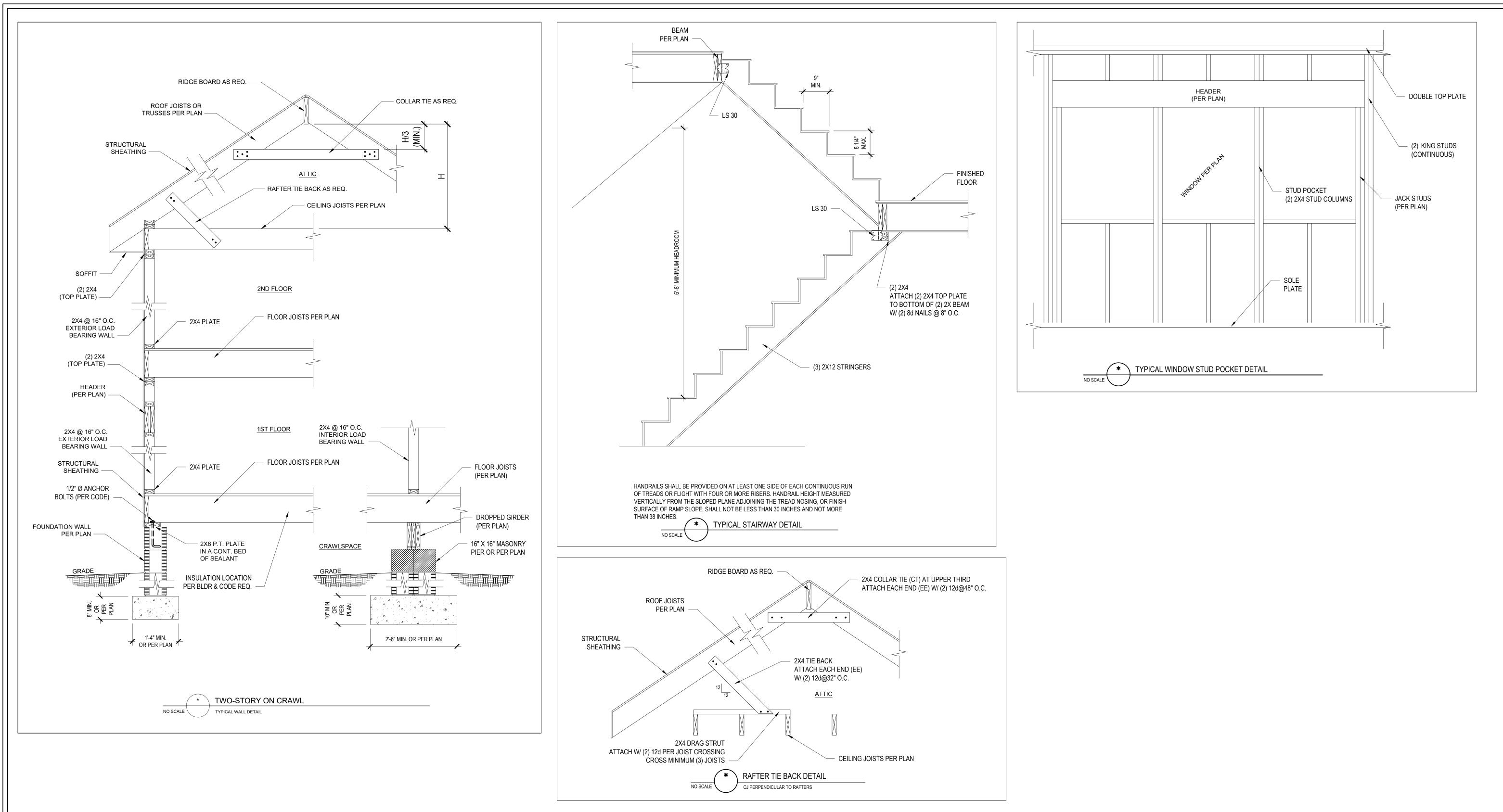




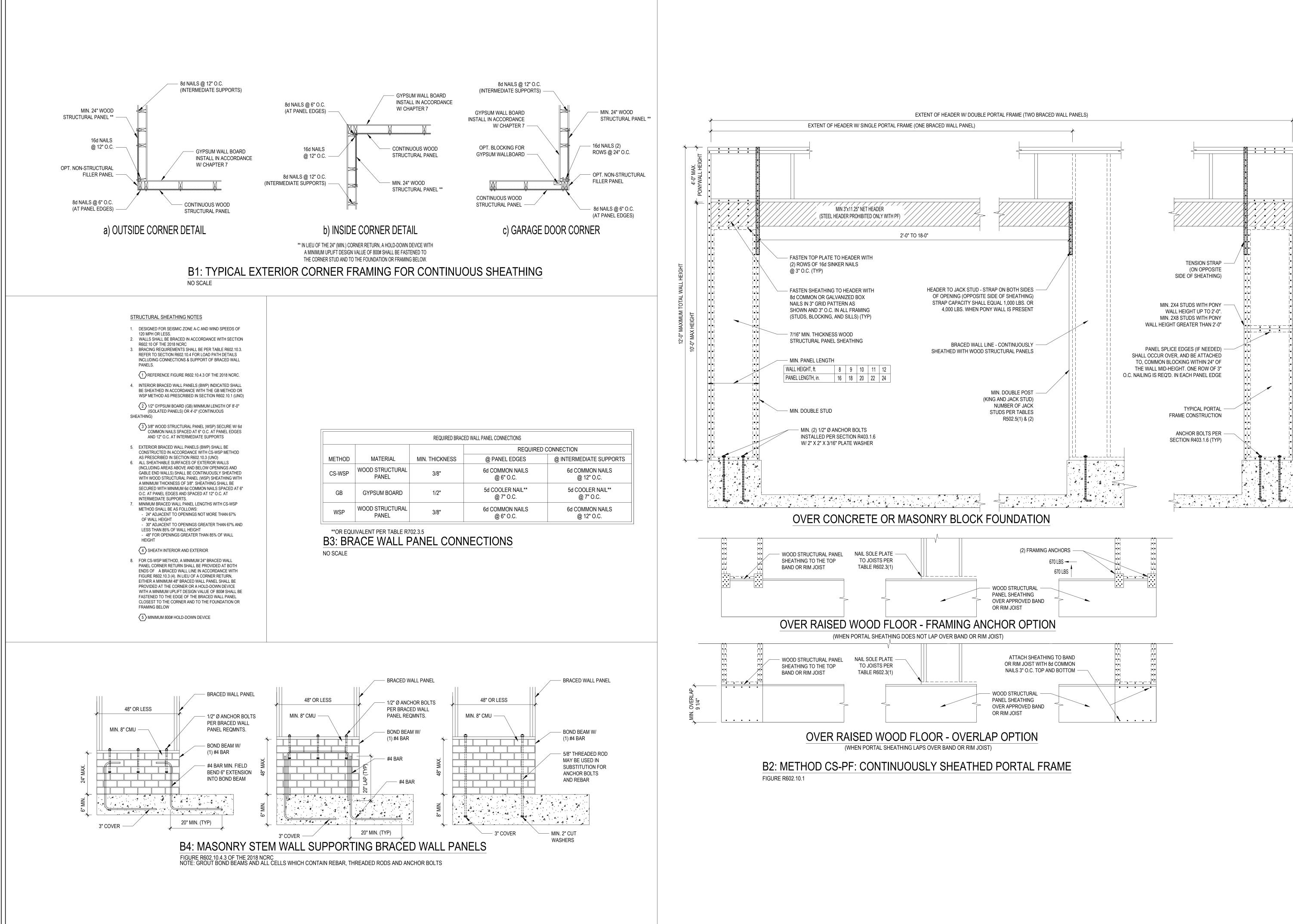


| 'X2" LEDGER | | |
|---|----|--|
| | | |
| 8" SOLID MASONRY CAP — 16"X16" MASONRY PIER | | |
| OR PER PLAN | | |
| | | |
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| <u>_</u> | | |
| | | |
| | | |
| | | |
| | | |
| 2X4@16" O.C. | | |
| – 5/8" Ø GALV. THRU-BOLT @ 20" O.C. (SIDING) OR 16" O.C. (BRIC W/ (3) 12d NAILS @ 6" O.C. | К) | |
| | | |
| <u></u> | | |
| - FLOOR JOISTS PER PLAN | | |
| 2X6 P.T. PLATE IN A CONT. BED OF SEALANT | | |
| | | |
| | | |
| T DETAIL | | |
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| | | |

| means, meth procedures o *Any deviatio to be brough Tyndall Engi do so will voi P.A. liability. *Please review Tyndall Engi interpret tha recommendati etc. presente deemed acce | *Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. *Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. Iability. *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins. | | | |
|--|--|---|--|--|
| TYNDALL | ENGINEERING & DESIGN, PA. | ≠ 919 778-1200 = 1 919 778-9488 250 Shipwash Drive = Garner = North Carolina = 27529 www.tyndallengineering.com | | |
| Client: TIM SNOVER | | Plan: STATERA BUILDERS | | |
| | STANDARD DETAILS | | | |
| 210 <u>Date:</u> 10/2 <u>Engine</u> AM <u>DWG</u> . PA <u>Scale:</u> SEH <u>F</u> | 10/20/21 Engineered By: AM DWG. Checked By: PAT Scale: SEE PLAN REVISIONS <u>No.</u> Date: Remarks 1 2 3 | | | |
| <u><u>S</u>I</u> | <u>Sheet Number</u> D1 5 of 7 | | | |



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| | ENGINEERING & DESIGN, P.A. | 7 919 778-1200 = # 919 778-1200 = # 919 778-9488 250 Shipwash Drive = Garner = North Carolina = 27529 www.tyndallanginaering.com | | | |
| Client: | TIM SNOVER | Plan: STATERA BUILDERS | | | |
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