

RE: J0921-5297 Weaver / 12 West Park / Harnett Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0921-5297 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 7 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E16133653	A1	9/7/2021
2	E16133654	A1GE	9/7/2021
3	E16133655	A2	9/7/2021
4	E16133656	B1	9/7/2021
5	E16133657	B1GE	9/7/2021
6	E16133658	P1	9/7/2021
7	E16133659	P1GE	9/7/2021

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

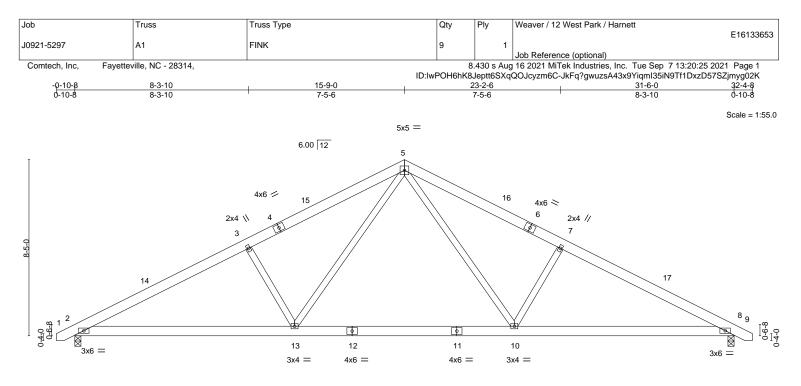
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





	10-6-0 10-6-0	+ <u>21-0-0</u> 10-6-0	<u>31-6-0</u> 10-6-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. DEFL. in (loc) TC 0.28 Vert(LL) -0.20 10-13 BC 0.48 Vert(CT) -0.29 10-13 WB 0.23 Horz(CT) 0.05 8 Matrix-S Wind(LL) 0.05 2-13	I/defl L/d PLATES GRIP >999 360 MT20 244/190 >999 240 MT20 244/190 n/a n/a Plates Series >999 240 FT = 20% Series	, 0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=107(LC 11) Max Uplift 2=-87(LC 12), 8=-87(LC 13) Max Grav 2=1299(LC 1), 8=1299(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2188/486, 3-5=-1990/517, 5-7=-1990/517, 7-8=-2188/486

BOT CHORD 2-13=-311/1914. 10-13=-102/1258. 8-10=-320/1873

WEBS 3-13=-466/285, 5-13=-144/843, 5-10=-144/843, 7-10=-466/285

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-6 to 3-8-7, Interior(1) 3-8-7 to 15-9-0, Exterior(2) 15-9-0 to 20-1-13, Interior(1) 20-1-13 to 32-2-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

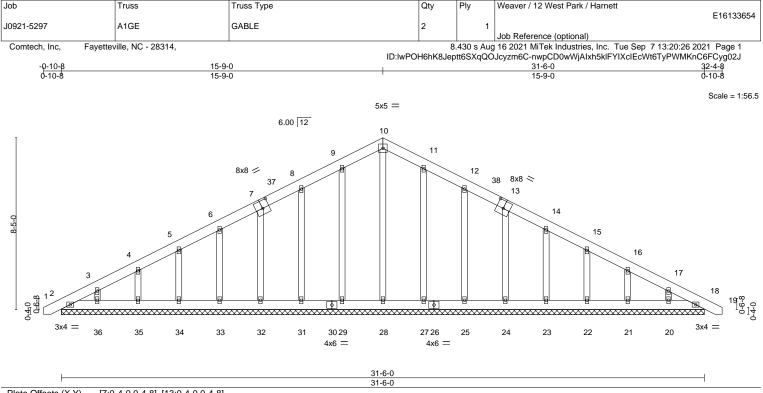


Structural wood sheathing directly applied or 4-11-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see Ma/S/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MiTek Affili 818 Soundside Road Edenton, NC 27932



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc	;) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) 0).00 1	8 n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT) (0.00 1	8 n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.14	Horz(CT) (0.00 1	8 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 246 lb	FT = 20%
LUMBER-			BRACING-					
TOP CHORD 2x6 SF	P No.1		TOP CHORD	Stru	ctural wood	I sheathing dir	ectly applied or 6-0-0	oc purlins.
BOT CHORD 2x6 SF	P No.1		BOT CHORD	Rigio	d ceiling dir	ectly applied of	or 10-0-0 oc bracing.	
OTHERS 2x4 SF	PNo 2			Ū	0		0	

REACTIONS. All bearings 31-6-0.

(lb) - Max Horz 2=166(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 2, 18, 29, 31, 32, 33, 34, 35, 36, 27, 25, 24, 23, 22, 21, 20 Max Grav All reactions 250 lb or less at joint(s) 2, 18, 28, 29, 31, 32, 33, 34, 35, 36, 27, 25, 24, 23, 22, 21, 20

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 8-9=-94/277, 9-10=-110/321, 10-11=-110/323, 11-12=-94/280

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-6 to 3-9-0, Exterior(2) 3-9-0 to 15-9-0, Corner(3) 15-9-0 to 20-1-13, Exterior(2) 20-1-13 to 32-2-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

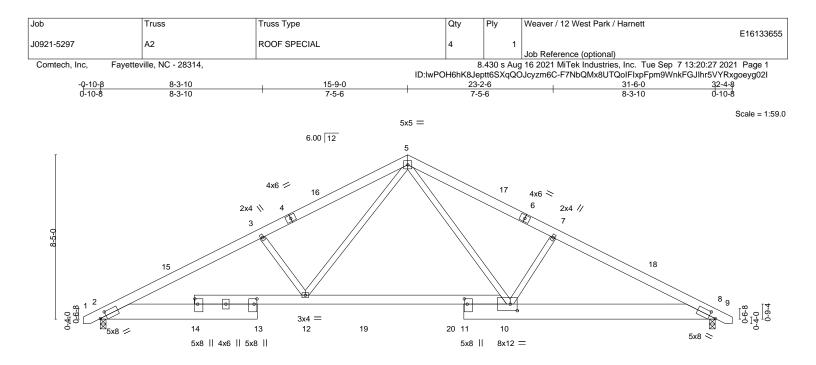
8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 18, 29, 31, 32, 33, 34, 35, 36, 27, 25, 24, 23, 22, 21, 20.



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	6-0-8 6-0-8	8-0-8 2-0-0	10-6-0 2-5-8	18-7-8 8-1-8		21-0-				31-6-0 10-6-0	
Plate Offsets (X,Y)	[2:0-3-15,0-2-10], [8:0-3-	15,0-2-10]	, [10:0-4-4,0-4-0]	, [11:0-3-4,0-1-1	2], [13:0-3-4,0-	1-12], [14	4:0-3-4	,0-1-12]		1	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TI	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matri:	0.24 0.62 0.25 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.17 10 -0.28 10 0.06 0.06 10	0-12 8	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 244 lb	GRIP 244/190 FT = 20%
10-14:	P No.1 SP No.1 *Except* : 2x6 SP No.1 P No.2				BRACING- TOP CHOR BOT CHOR				0	lirectly applied or 4-8-15 I or 10-0-0 oc bracing.	oc purlins.

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=107(LC 11) Max Uplift 2=-88(LC 12), 8=-88(LC 13) Max Grav 2=1299(LC 1), 8=1299(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2535/552, 3-5=-2263/555, 5-7=-2167/507, 7-8=-2403/507
- BOT CHORD 2-12=-363/2197, 10-12=-123/1378, 8-10=-344/2077
- WEBS 5-12=-183/1008, 5-10=-120/928, 7-10=-468/286, 3-12=-460/283

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-6 to 3-8-7, Interior(1) 3-8-7 to 15-9-0, Exterior(2) 15-9-0 to 20-1-13, Interior(1) 20-1-13 to 32-2-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

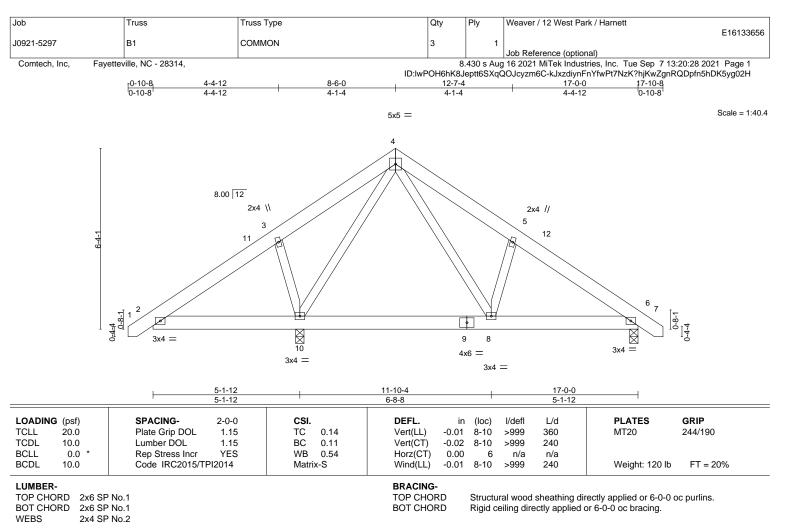
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

SEAL 036322 September 7,2021

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REACTIONS. (size) 10=0-3-8, 6=0-3-8 Max Horz 10=-147(LC 10) Max Uplift 10=-66(LC 12), 6=-43(LC 13)

Max Grav 10=1033(LC 1), 6=451(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-371/395, 3-4=-269/441, 4-5=-465/125, 5-6=-488/47

BOT CHORD 2-10=-264/368 6-8=0/341

WEBS 3-10=-327/231, 4-10=-707/371, 4-8=-109/465, 5-8=-288/202

NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-12 to 3-8-1, Interior(1) 3-8-1 to 8-6-0, Exterior(2) 8-6-0 to 12-8-1, Interior(1) 12-8-1 to 17-8-12 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

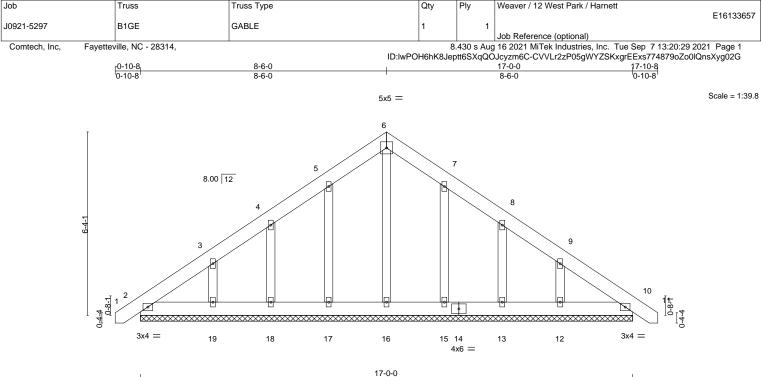
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 6.



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LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. i	n (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.02	Vert(LL) 0.00	· · ·	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT) 0.00) 10	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.06	Horz(CT) 0.00) 10	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	()				Weight: 126 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 17-0-0.

Max Horz 2=-184(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 17, 18, 15, 13 except 19=-119(LC 12), 12=-116(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 10, 16, 17, 18, 19, 15, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-12 to 3-8-1, Exterior(2) 3-8-1 to 8-6-0, Corner(3) 8-6-0 to 12-10-13, Exterior(2) 12-10-13 to 17-8-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 17, 18, 15, 13 except (jt=lb) 19=119, 12=116.

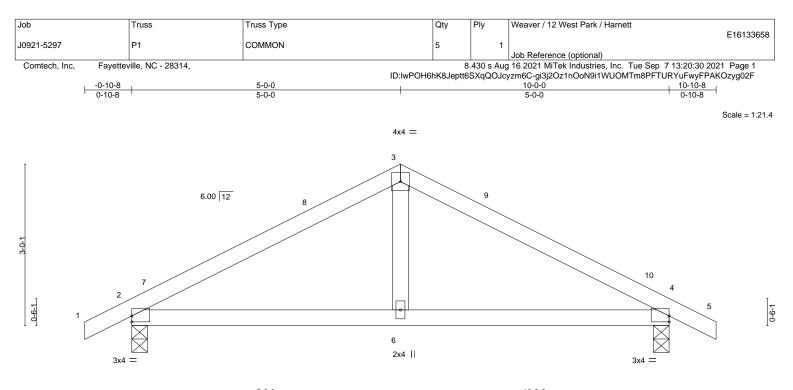


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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	5-0					10-0			
Plate Offsets (X,Y) [2	<u>5-0</u> ::0-0-0,0-1-5], [4:0-0-0,0-1-5]	-0				5-0	-0		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.24	Vert(LL)	0.04	2-6	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.20	Vert(CT)	-0.03	4-6	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT)	0.01	4	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 38 lb	FT = 20%
BOT CHORD 2x4 SP N WEBS 2x4 SP N			BOT CHOR	RD	Rigid c	eiling dir	ectly applied o	or 9-8-3 oc bracing.	
	2=0-3-8, 4=0-3-8 z 2=38(LC 11) ift 2=-89(LC 9), 4=-89(LC 8)								

BOT CHORD 2-6=-392/392, 4-6=-392/392

WEBS 3-6=-311/234

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 5-0-0, Exterior(2) 5-0-0 to 9-4-13, Interior(1) 9-4-13 to 10-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

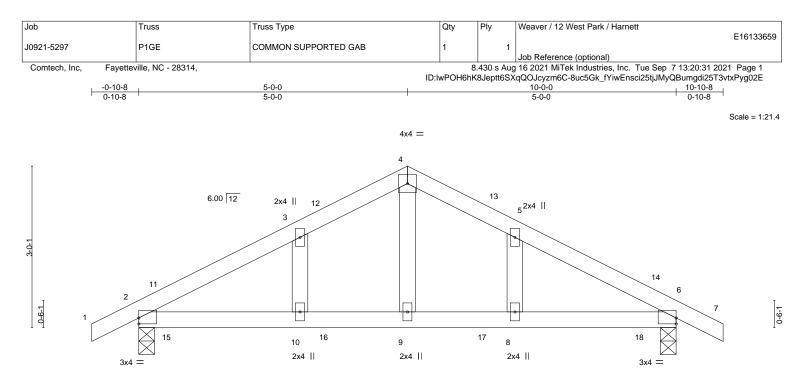
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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1	Δ	Δ	Δ	

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.24	Vert(LL) -0.02 10 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.27	Vert(CT) -0.03 2-10 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.06	Horz(CT) -0.01 6 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05 2-10 >999 240	Weight: 43 lb FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
OTHERS	2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=59(LC 12) Max Uplift 2=-117(LC 9), 6=-117(LC 8) Max Grav 2=450(LC 1), 6=450(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-514/746, 3-4=-448/794, 4-5=-448/794, 5-6=-514/746
- BOT CHORD 2-10=-515/397, 9-10=-515/397, 8-9=-515/397, 6-8=-515/397

WEBS 4-9=-435/200

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 5-0-0, Corner(3) 5-0-0 to 9-4-13, Exterior(2) 9-4-13 to 10-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=117, 6=117.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 8-1-12 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



