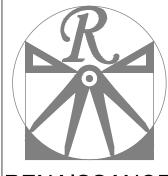
PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE.

LOT 12 WEST PARK 128 WESTPARK LANE SANFORD, NC 27332

GENERAL NOTES

- 1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
- 2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK, CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
- 3. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED.
- 4. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL



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LECTION WEAVER HOMES CAROLINA COLI MAGNOLIA

DATE: JUNE 30, 2020 REV.: SCALE: AS NOTED

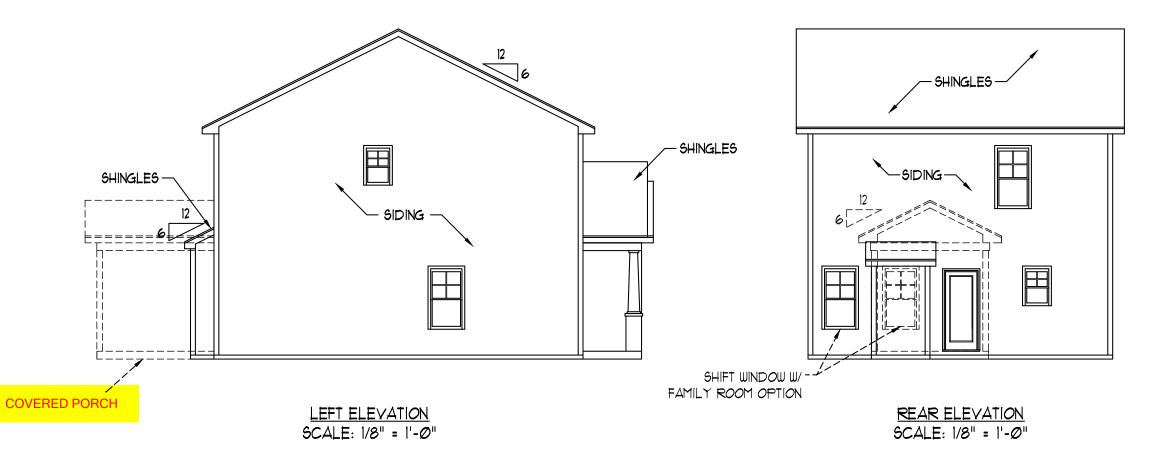
DRAWN BY: WG

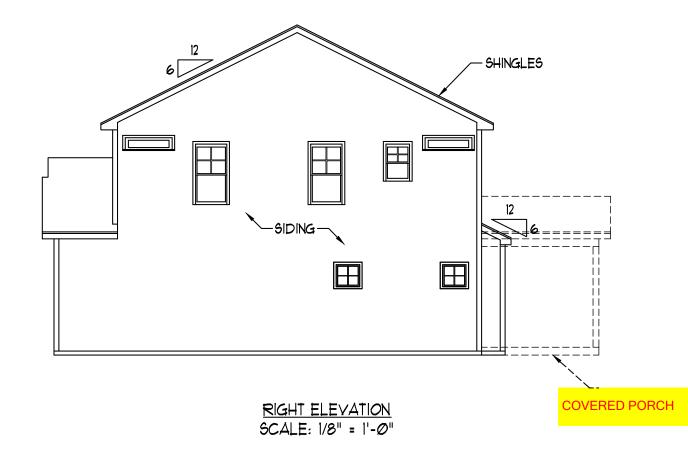
ENGINEERED BY: REVIEWED BY:

B - ELEVATIONS

A-2

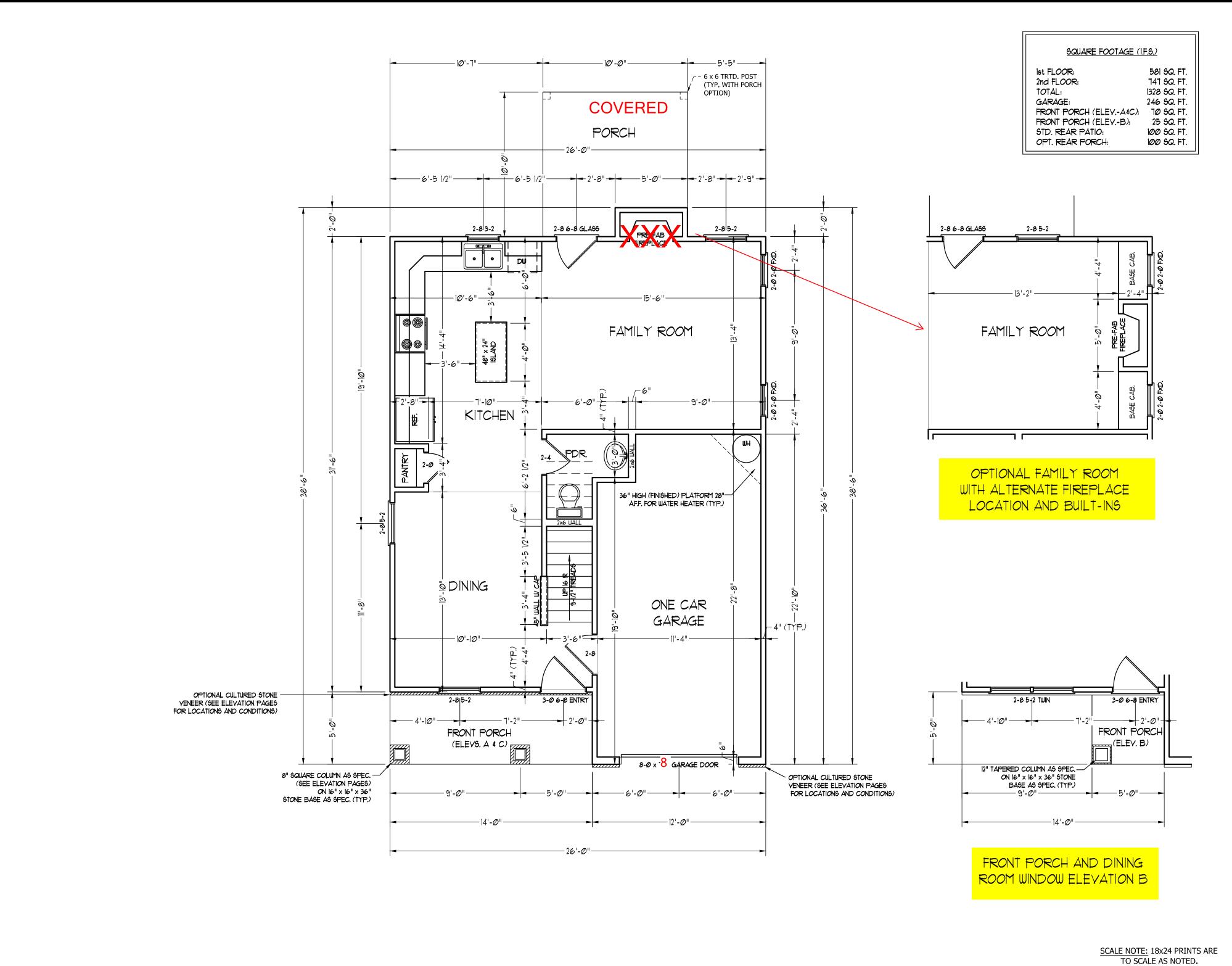






HVAC: MAINSTREAM PLUMBING: DOUBLE J **ELECTRICAL: JM POPE**

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



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DATE: JUNE 30, 2020 REV.:

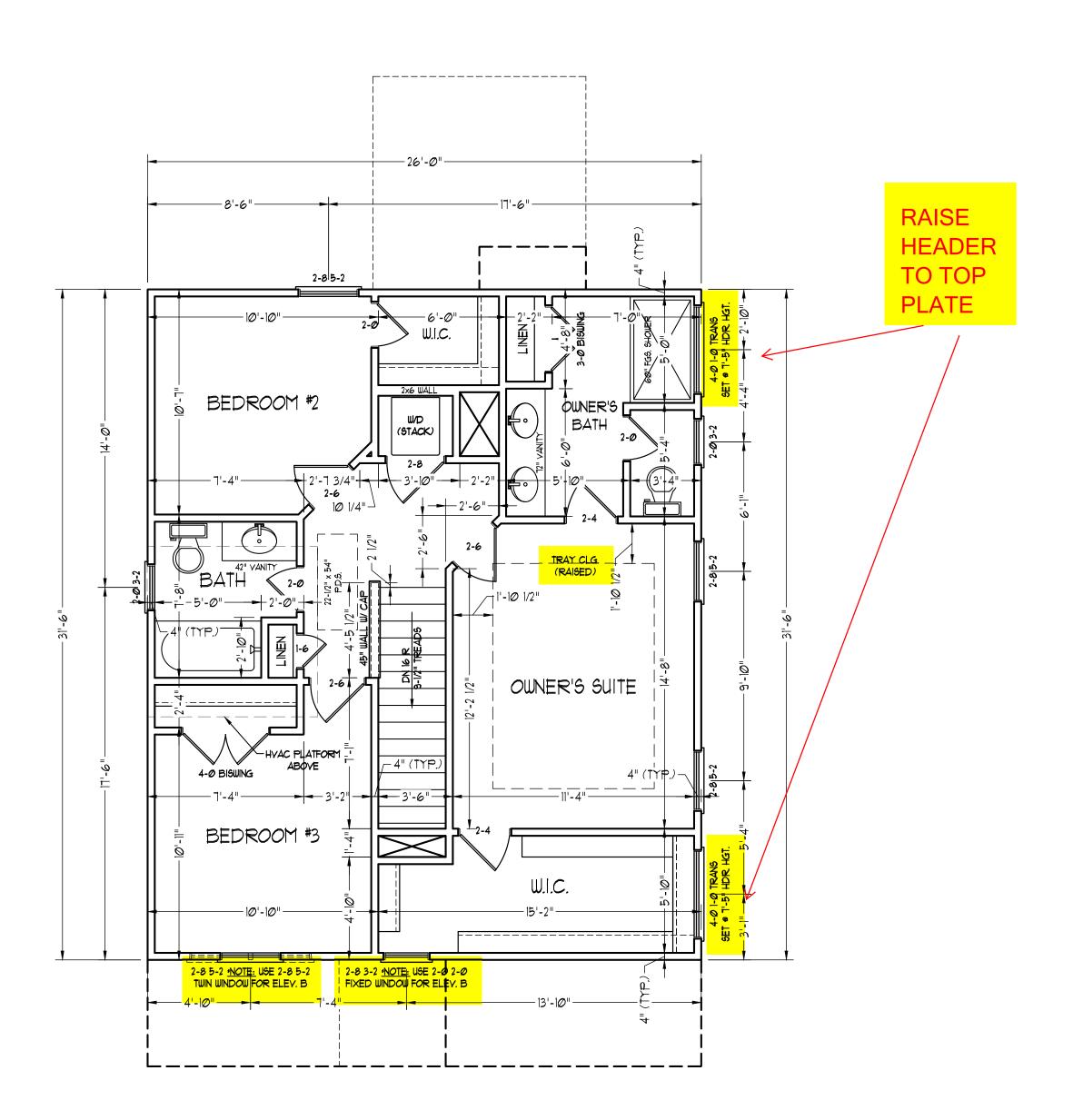
SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY:

REVIEWED BY:

FIRST FLOOR PLAN

A-4

11x17 PRINTS ARE NOT TO SCALE





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DATE: JUNE 30, 2020

SCALE: 1/4" = 1'-0"

DRAWN BY: WG ENGINEERED BY:

REVIEWED BY:

SECOND FLOOR

PLAN

A-5

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

ELECTRICAL LAYOUT NOTES:

- I.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET 9 90" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- → IIØ Y OUTLET
- ₩ 110 Y GFI OUTLET
- = 110 Y SWITCHED OUTLET
- $_{\mathrm{BB}}$ \Longrightarrow 110 $_{\mathrm{V}}$ BASEBOARD OUTLET
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 1100 GF1
- WEATHERPROOF
- 220 ∨ OUTLET
- Ø 110 V DEDICATED CIRCUIT
- # 220 Y DEDICATED CIRCUIT
- PH SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- -CEILING MOUNT LIGHT
- -P- PENDANT LIGHT
- RECESSED CAN LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT

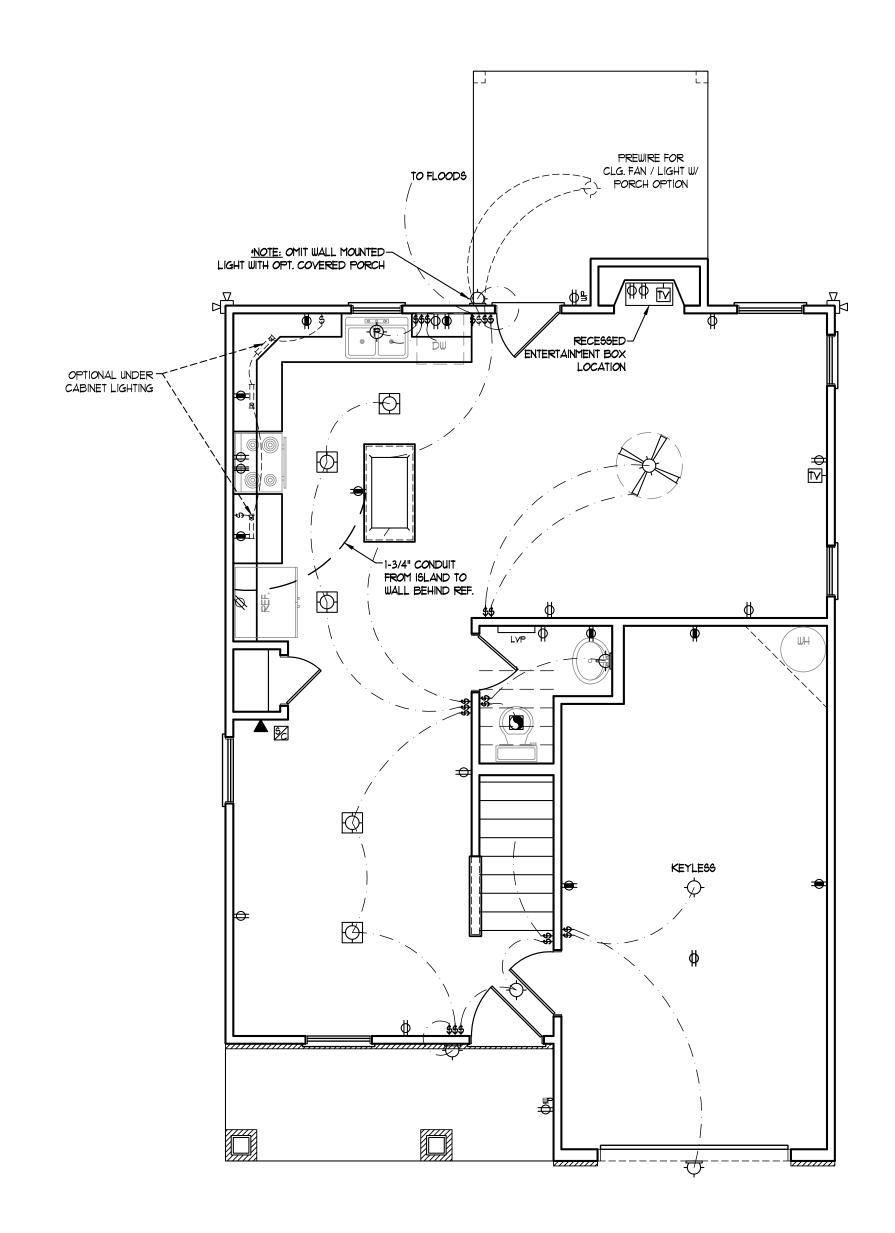
 FLUORESCENT LIGHT
- undercabinet light
- FLOOD LIGHT
- \$ SWITCH
- \$D DIMMER SWITCH
- ▲ TELEPHONE
- △ DATA
- TELEPHONE AND DATA
- TY- TY CONNECTION
- TV/ DATA
- D- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- MOKE/CM DETECTOR
- BD 110 Y SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM PANEL

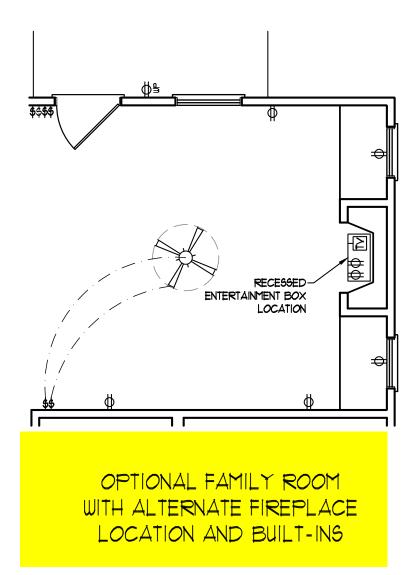


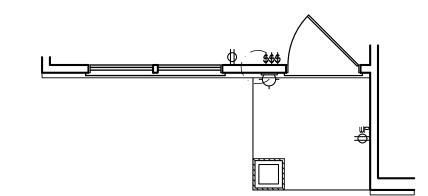
EILING FAN



CEILING FAN W/ LIGH

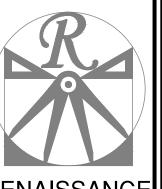






FRONT PORCH AND DINING ROOM WINDOW ELEVATION B

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DATE: JUNE 30, 2020

REV.:

SCALE: 1/4" = 1'-0"

DRAWN BY: WG

ENGINEERED BY:
REVIEWED BY:

FIRST FLOOR ELECTRICAL PLAN

E-1

ELECTRICAL LAYOUT NOTES:

- 1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET 9 90" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

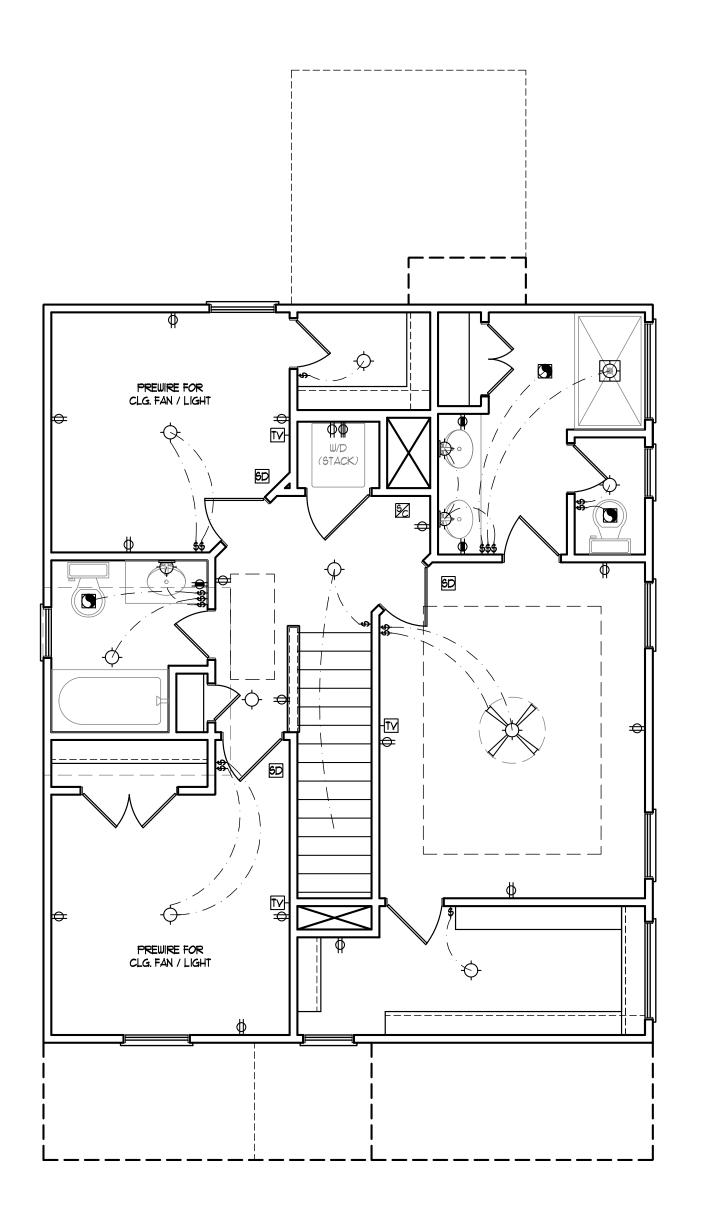
- → IIØ ∨ OUTLET
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- BB = 110 Y BASEBOARD OUTLET
- 4-PLEX
- COUNTER OR FLOOR MOUNTED
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- WEATHERPROOF
- **⇒** 22Ø ∨ OUTLET
- Ø 110 Y DEDICATED CIRCUIT
- # 220 Y DEDICATED CIRCUIT
- PH SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
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- MINI CAN LIGHT
- EYEBALL LIGHT

FLUORESCENT LIGHT

- undercabinet light
- FLOOD LIGHT
- SWITCH
- \$D DIMMER SWITCH
- ▲ TELEPHONE
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- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM PANEL









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DATE: JUNE 30, 2020 REV.:

SCALE: 1/4" = 1'-0"

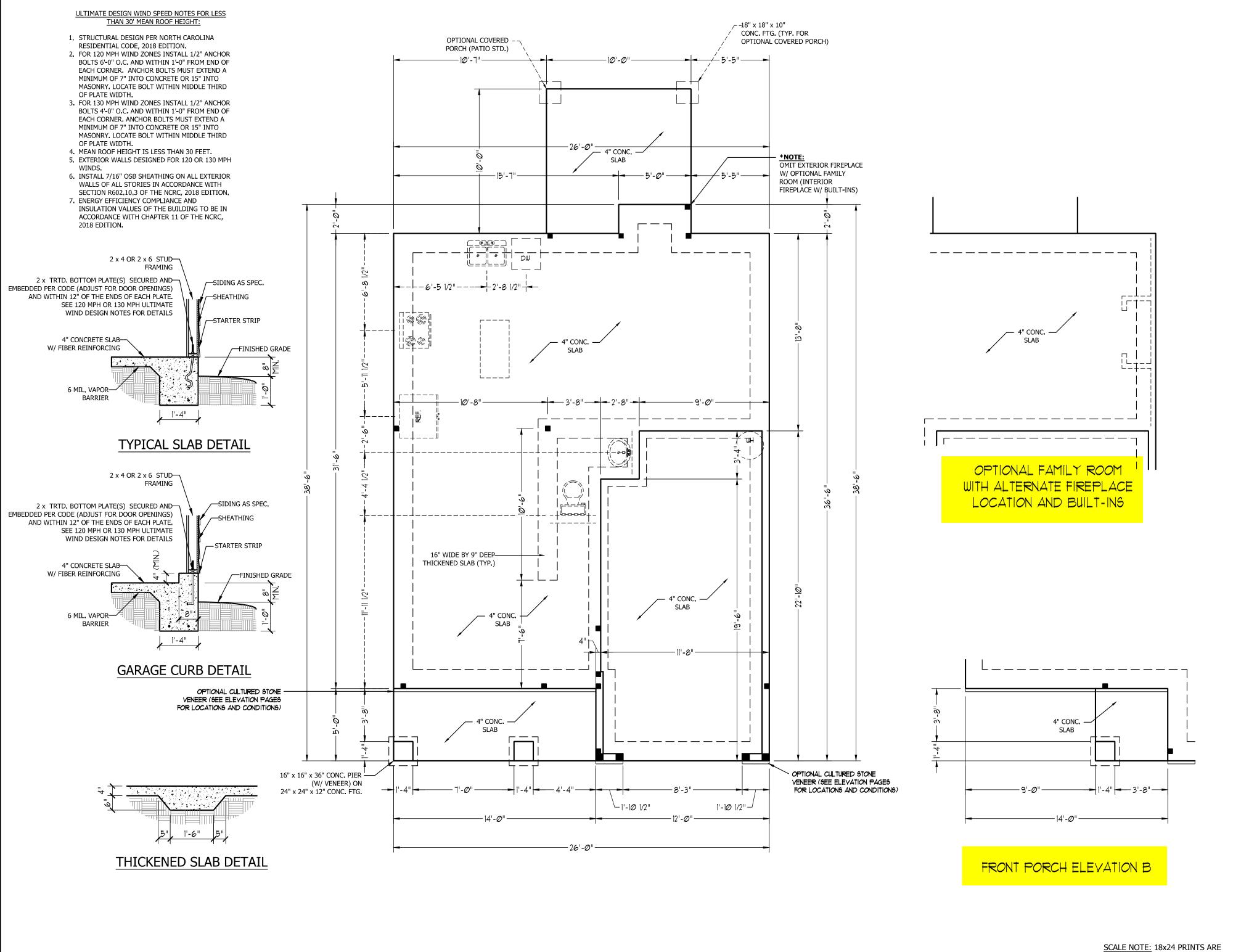
DRAWN BY: WG ENGINEERED BY:

REVIEWED BY:

SECOND FLOOR ELCTRICAL PLAN

SCALE NOTE: 18x24 PRINTS ARE E-2 11x17 PRINTS ARE NOT TO SCALE

TO SCALE AS NOTED.



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CAROLINA COLLECTION
MAGNOLIA

DATE: JUNE 30, 2020

REV.:

REV.: SCALE: 1/4" = 1'.0"

DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:

MONO SLAB FOUNDATION PLAN

S-1

TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE

STRUCTURAL NOTES:

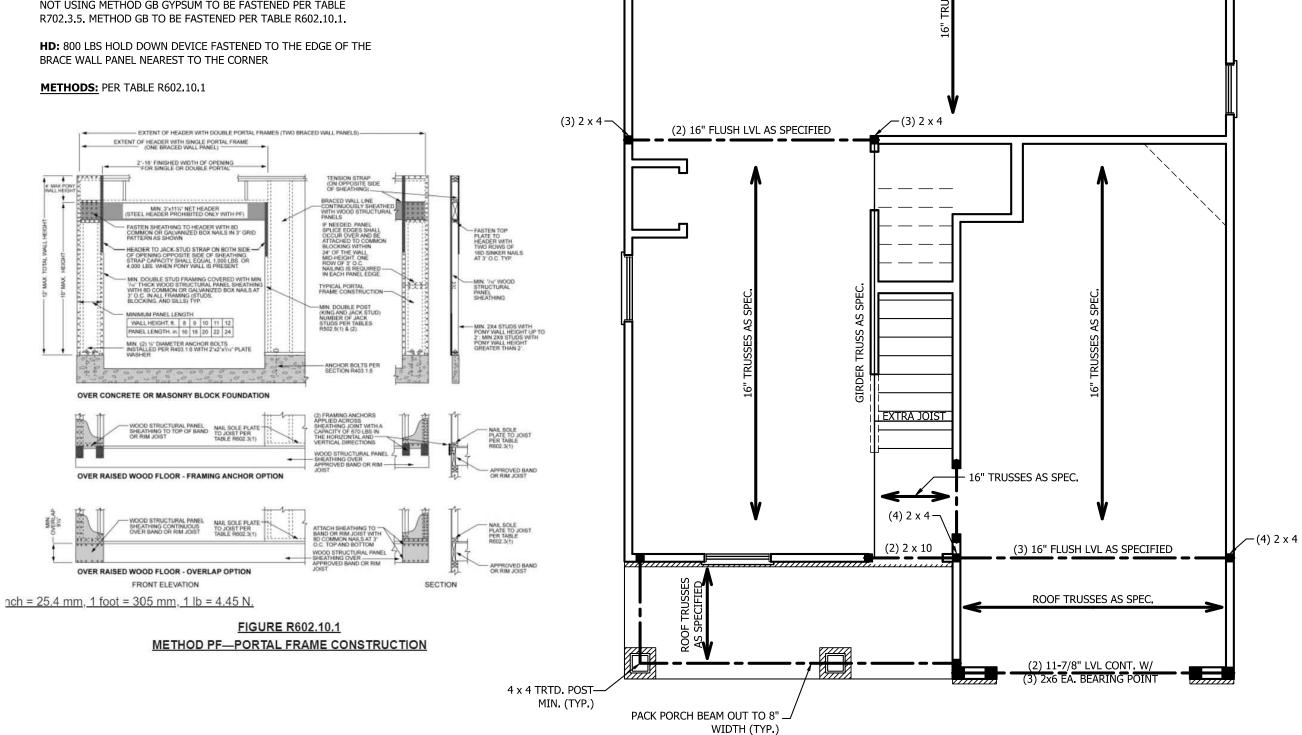
- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
- 4. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
- 7. FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

BRACE WALL PANEL NOTES:

EXTERIOR WALLS: ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH, METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH, METHOD PF CONTRIBUTES 1,5 TIMES ITS ACTUAL LENGTH.

GYPSUM: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE



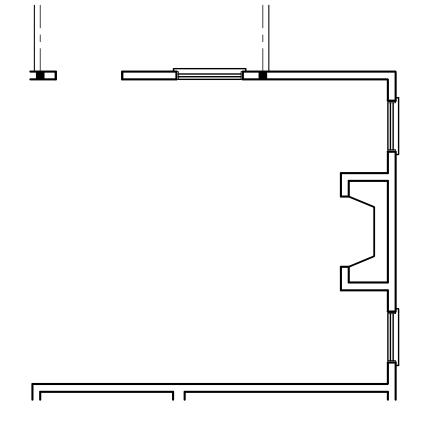
PACK PORCH BEAM OUT TO 8" WIDTH (TYP. W/ COVERED -

PORCH OPTION)

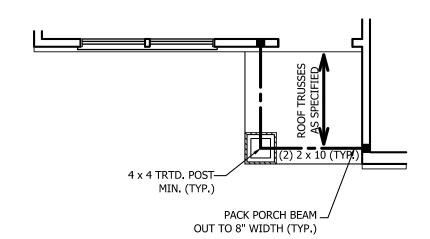
(2) 2x10

_ - 6 x 6 TRTD. POST (TYP. WITH PORCH

OPTION)



OPTIONAL FAMILY ROOM WITH ALTERNATE FIREPLACE LOCATION AND BUILT-INS



FRONT PORCH AND DINING ROOM WINDOW ELEVATION B

> SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



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DATE: JUNE 30, 2020 REV.:

SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY:

REVIEWED BY:

SECOND FLOOR FRAMING PLAN

S-2

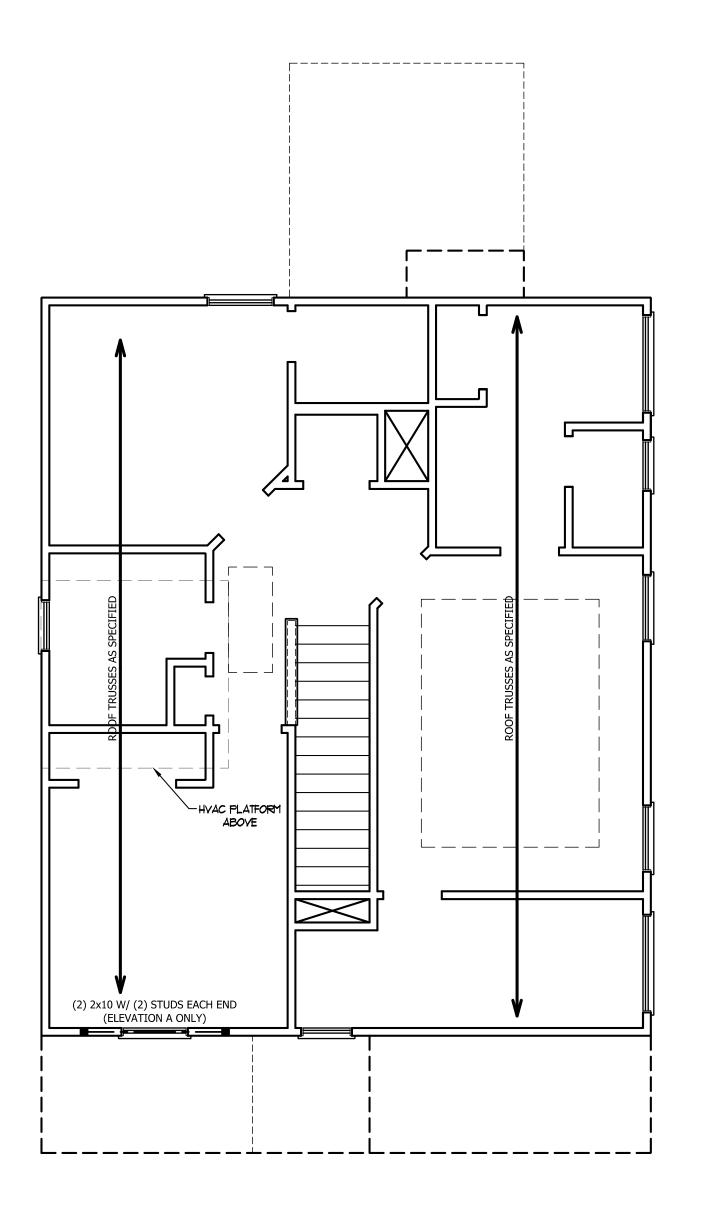


TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

AT EACH END OF TIEADERS IN EXTERIOR WA						
HEADER SPAN (FEET)		SPACING (INCHES) E R602.3(5)				
(. == .)	16	24				
UP TO 3'	1	1				
4'	2	1				
8'	3	2				
12'	5	3				
16'	6	4				

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



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WEAVER HOMES CAROLINA COLL MAGNOLIA

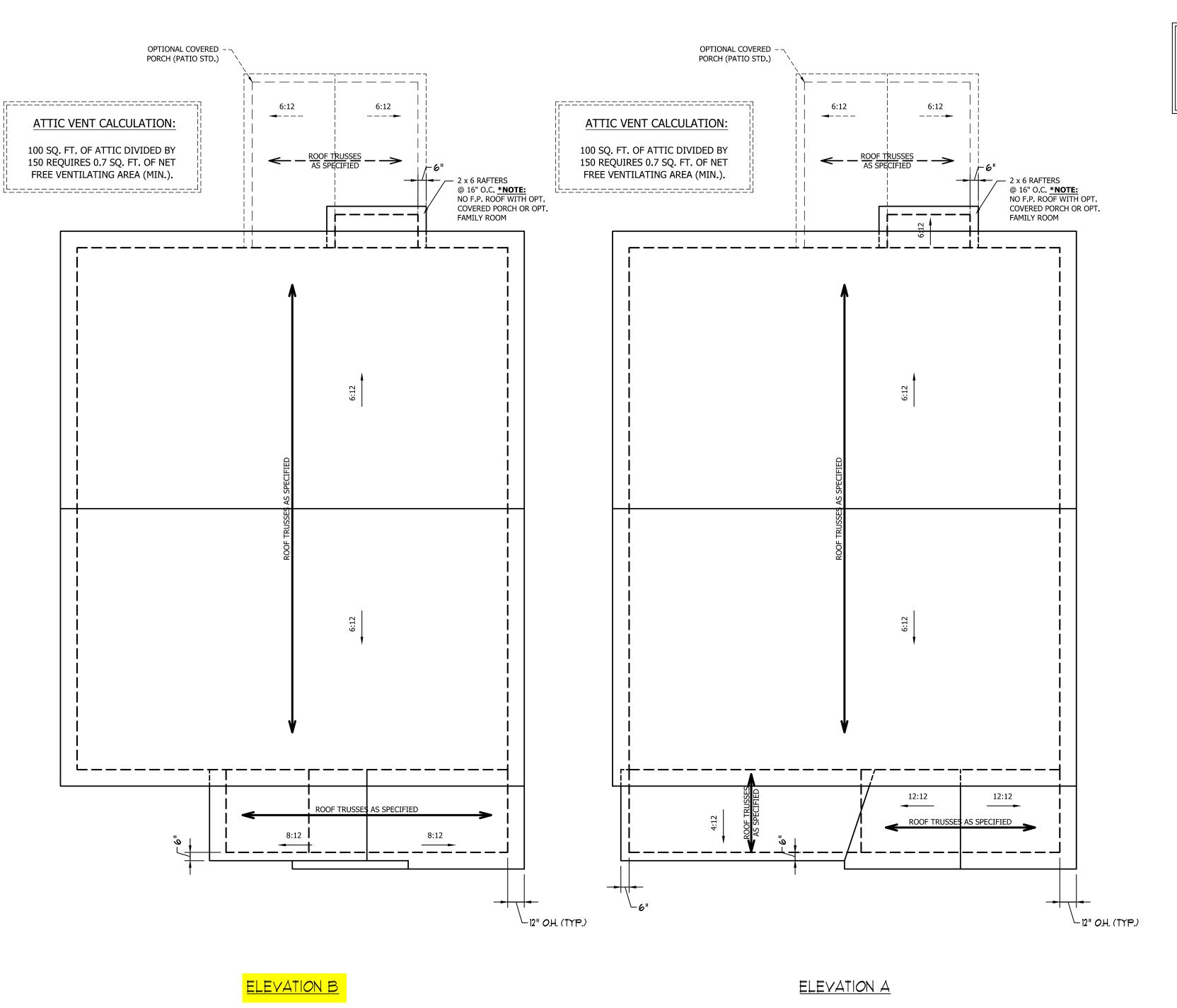
DATE: JUNE 30, 2020 SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY:

REVIEWED BY:

ATTIC FLOOR FRAMING PLAN

S-3

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



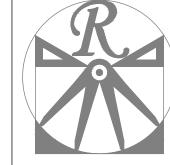
ATTIC VENT CALCULATION:

1077 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 7.2 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

- ALL FRAMING LUMBER TO BE #2 SPF (UNO).
 HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0".
 FASTEN MEMBERS WITH THREE ROWS OF 12d
- NAILS @ 16" O.C. (TYP.)

 3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/
 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND
 FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- 4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- 5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



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WEAVER HOMES CAROLINA COLLECTION MAGNOLIA

DATE: JUNE 30, 2020 REV.:

SCALE: 1/4" = 1'-0"

DRAWN BY: WG

DRAWN BY: WG
ENGINEERED BY:

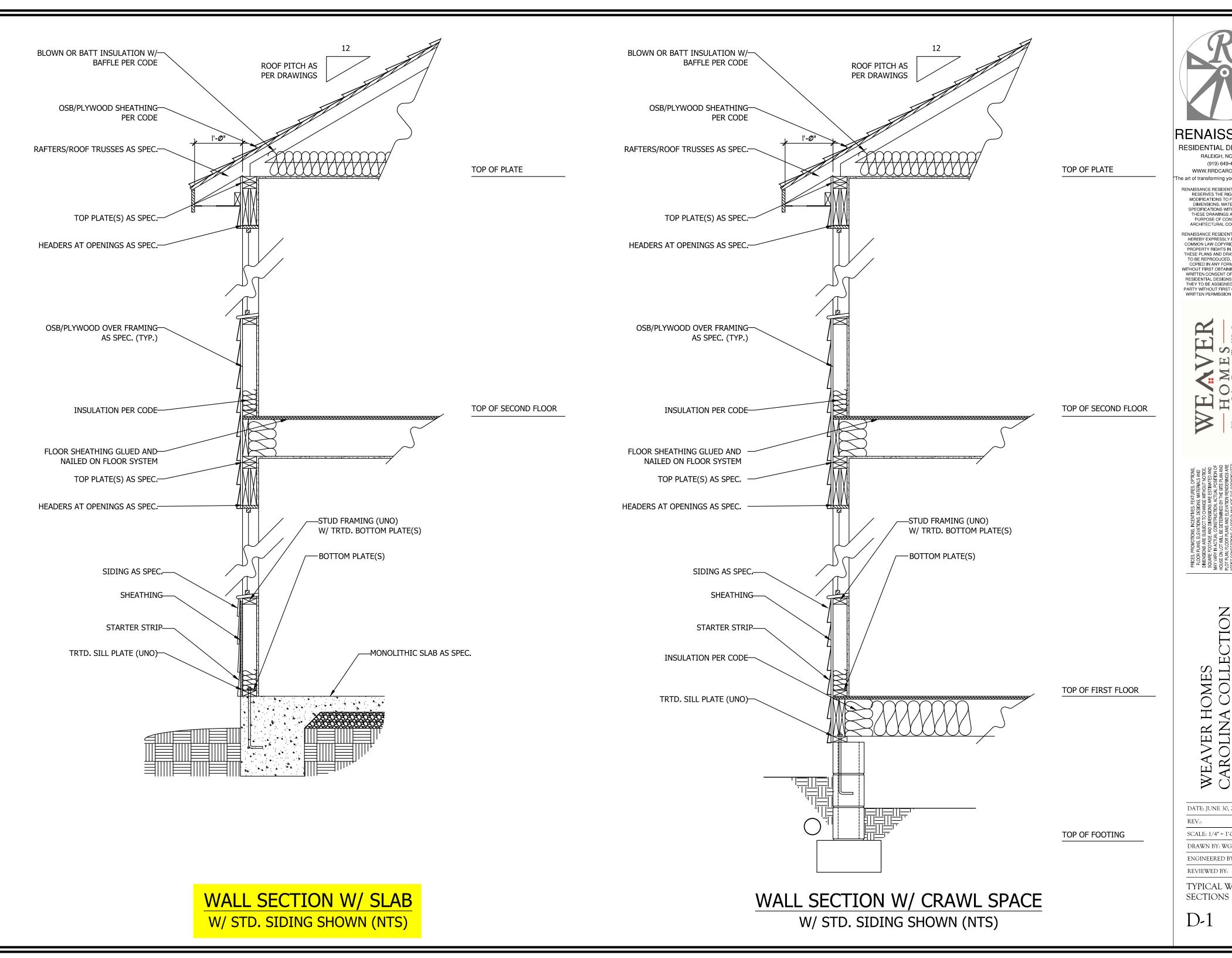
REVIEWED BY:
ROOF PLAN

ELEVATIONS A & B

S-3

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE



RENAISSANCE

RESIDENTIAL DESIGN, INC. RALEIGH, NC 27612

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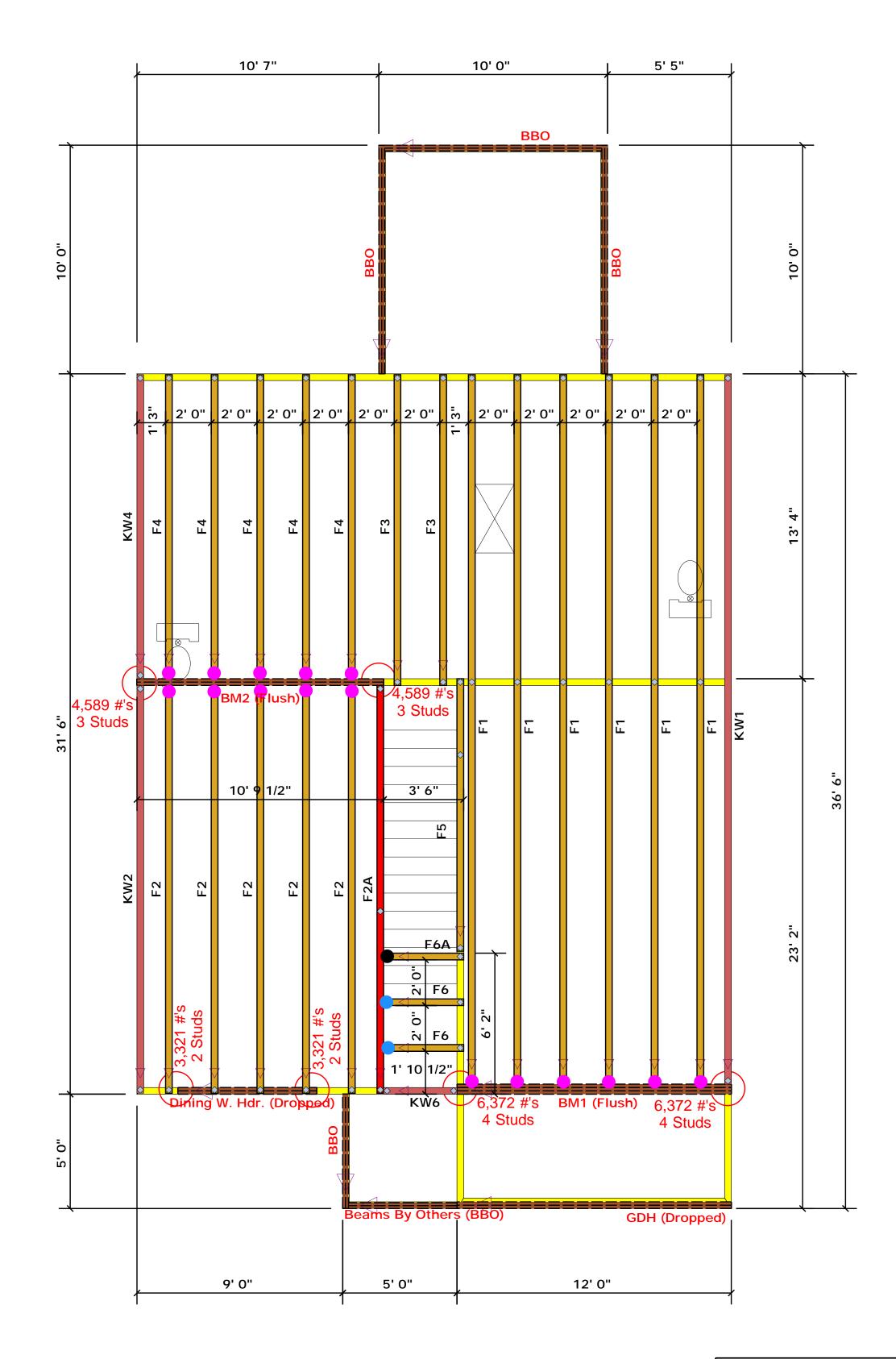
WEAVER HOMES CAROLINA COL

DATE: JUNE 30, 2020 REV.: SCALE: 1/4" = 1'-0"

DRAWN BY: WG ENGINEERED BY:

REVIEWED BY: TYPICAL WALL

D-1



= HUS410 (Qty. 16)
= MSH422 (Qty. 2)
= MSH422IF (Qty. 1)

Truss Placement Plan SCALE: NTS

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
Dining W. Hdr. (Dropped)	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH (Dropped)	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
BM1 (Flush)	12' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF
BM2 (Flush)	11' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF

L		CHART FO	_	ACK STUD	5
3	a. w u€e o	E JACK STUBS A FEAGERA		CONPCACUOSI i	
NOCTON B AND	SEC DISTUBBION CONTRIBUTED	PND PENCTION (LP TO)	MRQ IS STUDS FOR CIPAN - PARKIR	END REACTION (0.1.10)	REQUESTLOS FOR
170	0 1	2550	1	3400	1
340	0 2	5100	2	6800	2
510	0 3	7650	3	10200	3
680	0 4	10200	4	13600	4
850	0 5	12750	5	17000	5
1020	00 6	15300	6		
1190	0 7				
1360	8 00				
1530	00 9				

	BUILDER	Weaver Development	CITY / CO.	Sanford / Harnett	THIS IS These true the building
	JOB NAME	Lot 12 West Park	ADDRESS	128 West Park Lane	is respons the overall walls, and regarding
	PLAN	Magnolia Elev. B	MODEL	Floor	or online (
	SEAL DATE	Seal Date	DATE REV.	/ /	prescripti (derived foundatio than 3000 be retained
	QUOTE #	Quote #	DRAWN BY	Christine Shivy	specified retained t
-	JOB#	J0921-5298	SALES REP.	Lenny Norris	Signat

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual designer sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BcSi-B1 and BcSi-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Christine Shivy

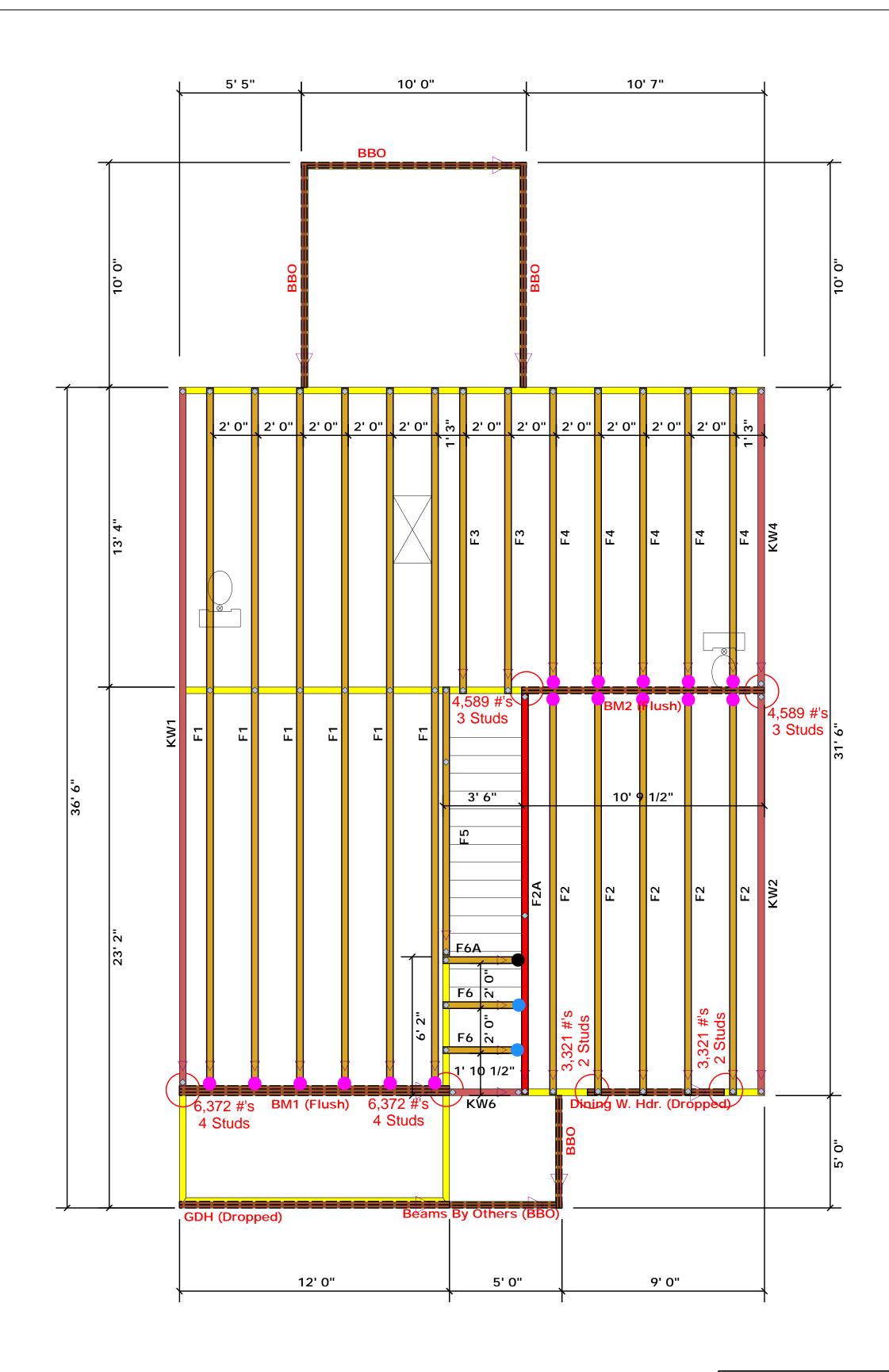
Christine Shivy

TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

соттесн

ROOF & FLOOR



= HUS410 (Qty. 16) = MSH422 (Qty. 2) ●= MSH422IF (Qty. 1)

Truss Placement Plan SCALE: NTS

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)

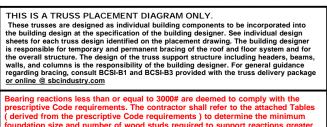
All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
Dining W. Hdr. (Dropped)	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH (Dropped)	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
BM1 (Flush)	12' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF
BM2 (Flush)	11' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF

(0455	ART FOR JACK STUD		BUILDER	Weaver Development
TO T	FEADER/FORR	-	JOB NAME	Lot 12 West Park
C PO SEC C PO SEC C PO SEC C PO SEC	Propercion (or up) (or up) (or up) (or up)	REQUESTLOS FOR (4) N.Y. HEADER	PLAN	Magnolia Elev. B
1700 1 3400 2 5100 3	2550 1 3400 5100 2 6600 7650 3 10200	3	SEAL DATE	Seal Date
6800 4 8500 5 10200 6	10200 4 13600 12750 5 17000 15300 6		QUOTE #	Quote #
11900 7 13600 8 15300 9			JOB #	J0921-5298

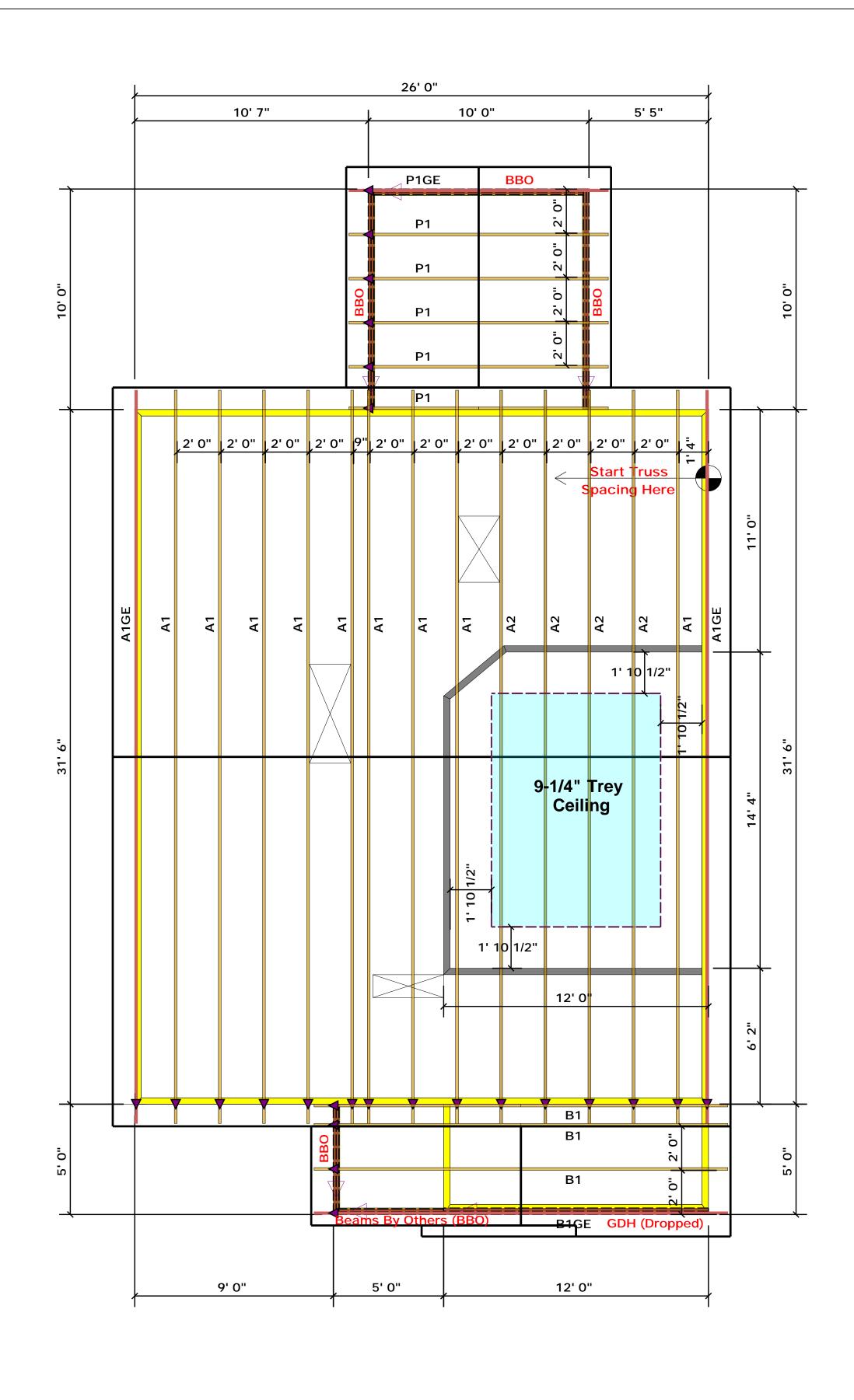
	BUILDER	Weaver Development	CITY / CO.	Sanford / Harnett
400	JOB NAME	Lot 12 West Park	ADDRESS	128 West Park Lane
	PLAN	Magnolia Elev. B	MODEL	Floor
	SEAL DATE	Seal Date	DATE REV.	/ /
	QUOTE #	Quote #	DRAWN BY	Christine Shivy
	JOB#	J0921-5298	SALES REP.	Lenny Norris



TRUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309 Christine Shivy Phone: (910) 864-8787 Fax: (910) 864-4444 **Christine Shivy**

соттесн

ROOF & FLOOR



All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

LOAD CHART FOR JACK STUDS

(BASED ON ABLES (802.51) A 6(1)

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2550 1 5100 2

7650 3

10200 4 12750 5

15300 6

3400

6800 2

10200 3

13600 4

17000 5

1700 1 3400 2

Truss Placement Plan SCALE: NTS

-inginicei c	a rrass brawing)	O O / \L	
BUILDER	Weaver Development	CITY / CO.	Sanford / Harnett
JOB NAME	Lot 12 West Park	ADDRESS	128 West Park Lane
PLAN	Magnolia Elev. B	MODEL	Roof
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Christine Shivy
JOB #	J0921-5297	SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

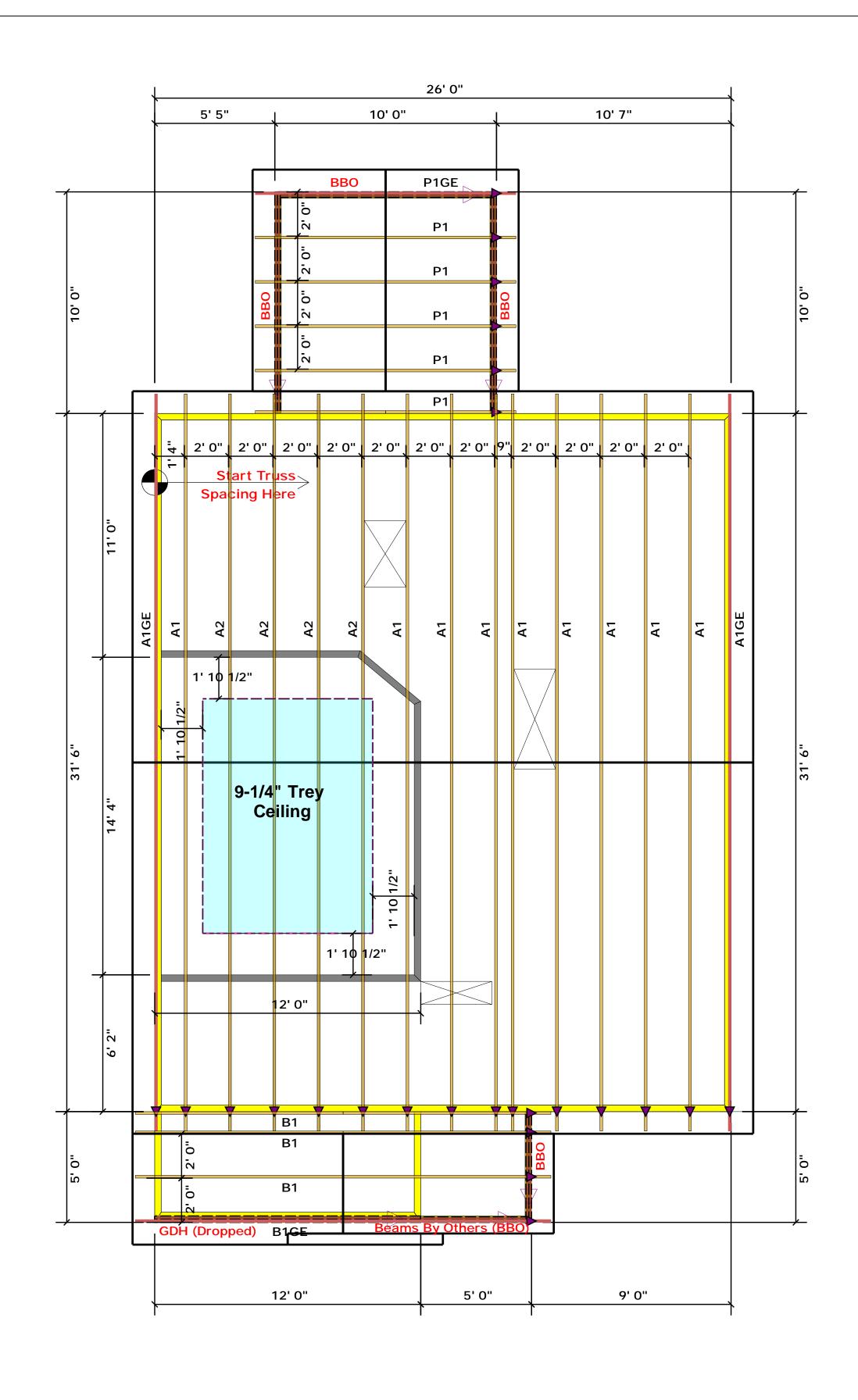
Bearing reactions less than or equal to 3000# are deemed to comply with the

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for any reaction that exceed 15000#.

christine Shivy
Christine Shivy



Fax: (910) 864-4444



All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

(Reference Engineered Truss Drawing)

Truss Placement Plan SCALE: NTS

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

▲ = Denotes Left End of Truss

			_		
	BUILDER	Weaver Development	CITY / CO.	Sanford / Harnett	
	JOB NAME	Lot 12 West Park	ADDRESS	128 West Park Lane	
	PLAN	Magnolia Elev. B	MODEL	Roof	ŀ
-	SEAL DATE	Seal Date	DATE REV.	/ /	
	QUOTE #	Quote #	DRAWN BY	Christine Shivy	١
-	JOB#	J0921-5297	SALES REP.	Lenny Norris	

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Christine Shivy **Christine Shivy**



Phone: (910) 864-8787 Fax: (910) 864-4444



Client: Project: Address:

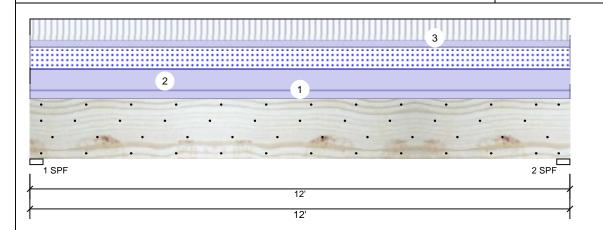
Weaver Homes Magnolia Elev. B Magnolia Elev. B Date: 9/7/2021

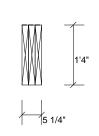
Input by: Christine Shivy Job Name: Magnolia Elev. B

Project #:

Kerto-S LVL 1.750" X 16.000" BM₁

Level: Level 3-Ply - PASSED





Page 1 of 1

Member Information Reactions UNPATTERNED Ib (Uplift) Туре: Girder Application: Floor Brg Wind Const Live Dead Snow ASD Plies: 3 Design Method: 1932 3460 1950 0 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 3460 0 2 1932 1950 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 3460 / 2912 6372 L D+0.75(L+S) 2 - SPF 3.500" 82% 3460 / 2912 6372 L D+0.75(L+S)

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	17746 ft-lb	6'	62010 ft-lb	0.286 (29%)	D+0.75(L+S)	L
Unbraced	17746 ft-lb	6'	17774 ft-lb	0.998 (100%)	D+0.75(L+S)	L
Shear	4571 lb	10'5 3/8"	17920 lb	0.255 (26%)	D+L	L
LL Defl inch	0.066 (L/2116)	6'	0.289 (L/480)	0.230 (23%)	0.75(L+S)	L
TL Defl inch	0.143 (L/967)	6'	0.385 (L/360)	0.370 (37%)	D+0.75(L+S)	L

Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 10'3 3/4" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Wall	
2	Uniform			Тор	325 PLF	0 PLF	325 PLF	0 PLF	0 PLF	A2	
3	Uniform			Far Face	108 PLF	322 PLF	0 PLF	0 PLF	0 PLF	F1	
	Self Weight				19 PLF						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

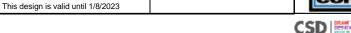
6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Client: Project: Address:

Weaver Homes Magnolia Elev. B Magnolia Elev. B Date: 9/7/2021

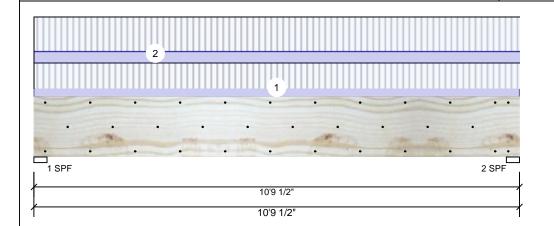
Input by: Christine Shivy Job Name: Magnolia Elev. B

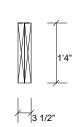
Project #:

Kerto-S LVL 2-Ply - PASSED 1.750" X 16.000" BM₂

Level: Level

Reactions UNPATTERNED Ib (Uplift)





Page 1 of 1

Member Information Туре: Girder Application: Floor Plies: 2 Design Method: ASD Moisture Condition: Dry **Building Code: IBC/IRC 2015** Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F

Brg	Live	Dead	Snow	Wind	Const
1	3389	1200	0	0	0
2	3389	1200	0	0	0

Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 4589 L D+L 1 - SPF 3.500" 1200 / 3389 2 - SPF 3.500" 88% 1200 / 3389 4589 L D+I

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	11397 ft-lb	5'4 3/4"	34565 ft-lb	0.330 (33%)	D+L	L
Unbraced	11397 ft-lb	5'4 3/4"	11746 ft-lb	0.970 (97%)	D+L	L
Shear	4386 lb	1'6 5/8"	11947 lb	0.367 (37%)	D+L	L
LL Defl inch	0.085 (L/1457)	5'4 3/4"	0.259 (L/480)	0.330 (33%)	L	L
TL Defl inch	0.115 (L/1076)	5'4 3/4"	0.345 (L/360)	0.330 (33%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6. Lateral slenderness ratio based on single ply width

o Edicial sicilaciness ratio based on single ply width.										
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	89 PLF	267 PLF	0 PLF	0 PLF	0 PLF	F4
2	Uniform			Near Face	121 PLF	361 PLF	0 PLF	0 PLF	0 PLF	F2
	Self Weight				12 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



CSD |



Client: Project: Address:

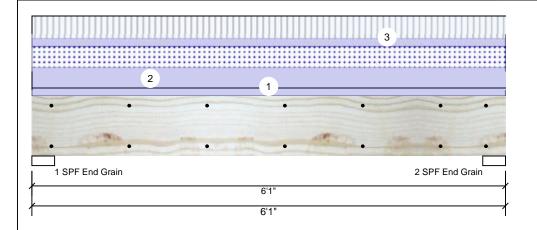
Weaver Homes Magnolia Elev. B Magnolia Elev. B Date: 9/7/2021 Input by: Christine Shivy

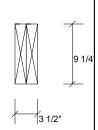
Job Name: Magnolia Elev. B Project #:

Dining W. Hdr. Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level

Decetions LINIDATTEDNIED Ib (Liniift)





Page 1 of 1

iviember information							
Type:	Girder						
Plies:	2						
Moisture Condition:	Dry						
Deflection LL:	480						
Deflection TL:	360						
Importance:	Normal						
Temperature:	Temp <= 100°F						

Mambar Information

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Re	Reactions diveat terived to (opint)										
Brç	g Live Dead	Snow	Wind	Const							
1	1098 1759	989	0	0							
2	1098 1759	989	0	0							

Analysis Results Analysis Actual Location Allowed Comb. Case Capacity 3' 1/2" 14423 ft-lb 0.300 (30%) D+0.75(L+S) L Moment 4322 ft-lb Unbraced 4322 ft-lb 3' 1/2" 10944 ft-lb 0.395 (39%) D+0.75(L+S) L 2231 lb 1' 7943 lb 0.281 (28%) D+0.75(L+S) L Shear LL Defl inch 0.032 (L/2087) 3' 1/2" 0.141 (L/480) 0.230 (23%) 0.75(L+S) TL Defl inch 0.069 (L/983) 3' 1/2" 0.188 (L/360) 0.370 (37%) D+0.75(L+S) L

Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1759 / 1565 1 - SPF 3.500" 3324 L D+0.75(L+S) End Grain 1759 / 1565 3324 L D+0.75(L+S) 2 - SPF 3.500"

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slanderness ratio based on single bly width

Self Weight

/ Lateral Sienderness ratio based on single ply width.										
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Load
2	Uniform			Тор	325 PLF	0 PLF	325 PLF	0 PLF	0 PLF	A1
3	Uniform			Тор	121 PLF	361 PLF	0 PLF	0 PLF	0 PLF	F2

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

7 PLF

End

Grain

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







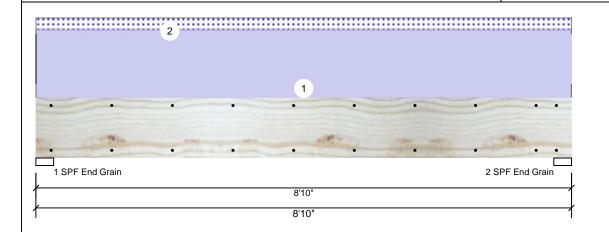
Client: Weaver Homes Project: Magnolia Elev. B Address: Magnolia Elev. B

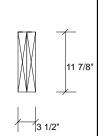
Date: 9/7/2021 Input by: Christine Shivy Job Name: Magnolia Elev. B

Project #:

2-Ply - PASSED **Kerto-S LVL** 1.750" X 11.875" **GDH**

Level: Level





Page 1 of 1

Member Information							
Type:	Girder						
Plies:	2						
Moisture Condition:	Dry						
Deflection LL:	480						
Deflection TL:	360						
Importance:	Normal						
Temperature:	Temp <= 100°F						

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Brg Wind Live Dead Snow Const 0 1101 177 0 0 1 0 2 0 1101 177 0

1101 / 177

12%

Bearings Bearing Length 1 - SPF 3.500"

Cap. React D/L lb Total Ld. Case Ld. Comb. 1101 / 177 D+S 1277 I

1277 L

D+S

Grain 2 - SPF 3.500" End Grain

End

Analysis Results Analysis Actual Location Allowed

Comb. Case Capacity Moment 0.122 (12%) D Uniform 2185 ft-lb 4'5" 17919 ft-lb Unbraced 2536 ft-lb 4'5" 10756 ft-lb 0.236 (24%) D+S 797 lb 7980 lb 0.100 (10%) D Uniform Shear 7'7 3/8" LL Defl inch 0.006 4'5 1/16" 0.209 (L/480) 0.030 (3%) S (L/18257) TL Defl inch 0.040 (L/2525) 4'5 1/16" 0.279 (L/360) 0.140 (14%) D+S

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

Self Weight

7 Lateral slenderness ratio based on single ply width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Loads	
2	Uniform			Тор	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	2'-0" Gable End	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

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