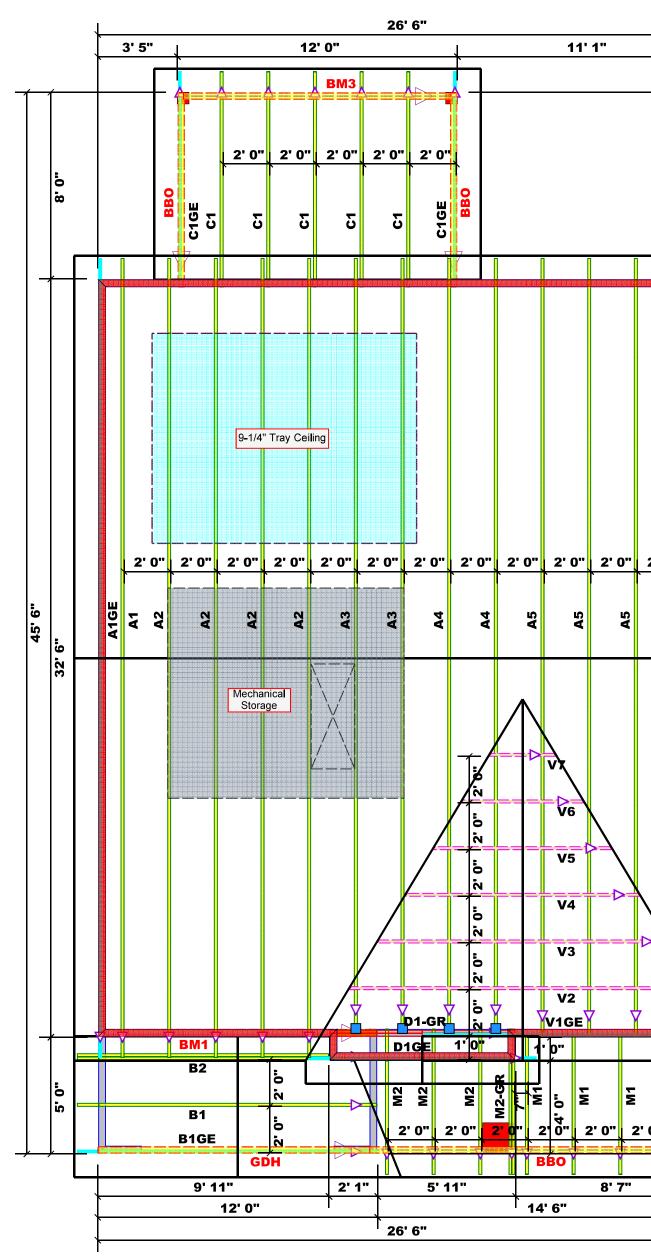


5"	8.0.		R Bearing deemed requiren size and reaction retained reaction Signatus Signatus LO7 NUR Signatus Signa	Fayet Phon Fax reactions to complements. The number s greater A registe to desig s that exc. A registe s of c/ s desig s f registe to desig s f registe s f registe s f registe s f registe s f registe s f registe f registe s f regist	Road I teville teville (910) s less that y with the e contract determin of wood than 300 red design n the sup ceeds thour red design David David David ART Fi b ON TABL SC STUDE PEADS 2550 5100 7650	OR JA OR JA ES REGISTRE STATE STATE STATE OR JA STATE STA	2830! 1-8787 4444 al to 3000 ptive Coc refer to t prescript nimum fo puired to t greater sional sh rem for al andry CK ST (0.3 (b)) b 8 54 55	09937
	32' 6"	Dimension Notes 1. All exterior wall to wall dimensions are to face of frame wall unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise BAIL Walls Shown Are Considered Load Bearing Match Legend Roof Area = 1468.1 sq.ft. Ridge Line = 52.07 ft. Hip Line = 0 ft. Hip Line = 0 ft. Hip Line = 0 ft. Hip Line = 50.3 ft. Baked OH = 159.04 ft. Decking = 50 sheets Image: Solid other wall to the state of the solid other wall to	CITY / CO. Sanford / Harnett	8	MODEL Roof	DATE REV . 08/3/21	DRAWN BY David Landry	
	5: 0"	ProductsPlotIDLengthProductPliesNet QtyFab TypeBM112'0"1-3/4"x 16" LVL Kerto-S22FFBM215'0"1-3/4"x 16" LVL Kerto-S22FFBM312'0"2x10 SPF No.222FFGDH12'0"2x12 SPF No.222FFColspan="4">Truss Placement PlanScale: $1/4"=1'$	BUILDER Weaver Development Co. Inc.	JOB NAME lot 77 Thomas Farm	PLAN Hickory "A"	SEAL DATE	QUOTE #	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards



A5 A5GE A5GE A5GE A5GE		32'6"	Interior Male Interior well underscripter data Interior well underscript	F Bearing deemed requiren attached requiren size and reaction 15000%, retained reaction Signatu LOr NUA Core Signatu 1700 3400 5100 6800 8500 10200 11900 13600	eilly R Fayet Phon Fax: reactions to comple- nents. The comple- nents. The comple- nents is greater A register to design that exceed A register to design that exceed a design th	toad Ir teville e: (911) : (910) : (91	Adustr (N.C. 0) 864-4 864-4 a prescriptor stude request promote shall b tor shall or shall in profession ad Loi DR JAC ES RESUZE CREQUENT STREER	I to 3000# tive Code trive Code refer to the innum fou- uirred to sta- ional shale em for all modery CK STU 0 8 E4 END 0 8 E4 E	rk * are eve Code vve Code vve Code vupport than Ube y JDS vof * are ve Code value val
			Ridge Line = 52.07 ft.Hip Line = 0 ft.Horiz. OH = 98.57 ft.Raked OH = 159.04 ft.Decking = 50 sheetsDrop Beam	Spring Lake	Bill Shaw	Roof	08/3/21	David Landry	Lenny Norris
			Connector Information Sym Product Manuf Qty Supported Member Header Truss HUS26 USP 4 NA 16d/3-1/2" 16d/3-1/2"	CITY / CO .	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
2'0" 2'0"	7 7 7	5.0.	ProductsPlotIDLengthProductPliesNet QtyFab TypeBM112' 0"1-3/4"x 16" LVL Kerto-S22FFBM215' 0"1-3/4"x 16" LVL Kerto-S22FFBM312' 0"2x10 SPF No.222FFGDH12' 0"2x12 SPF No.222FF	Weaver Development Co. Inc.	AE Lot 77 Thomas Farm	Hickory "A"	TE	*	J0721-4569
	*			BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	#

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Trenco RE: J0721-4569 818 Soundside Rd Lot 77 Thomas Farm Edenton, NC 27932 Site Information: Customer: Weaver Development Inc. Project Name: J0721-4569 Lot/Block: 77 Model: Hickory Address: Subdivision: City: State: General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.3 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 25 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17	Seal# E15727048 E15727050 E15727050 E15727051 E15727052 E15727053 E15727055 E15727056 E15727056 E15727057 E15727058 E15727060 E15727061 E15727062 E15727063 E15727064	Truss Name A1 A1GE A2 A3 A4 A5 A5GE B1 B1GE B2 C1 C1GE D1-GR D1GE M1 M1GE M2	Date 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021	No. 21 22 23 24 25	Seal# E15727068 E15727069 E15727070 E15727071 E15727072
-					
-		-			
18	E15727065	M2-GR	8/3/2021		
19	E15727066	V1GE	8/3/2021		
20	E15727067	V2	8/3/2021		

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



V3 V4

V5

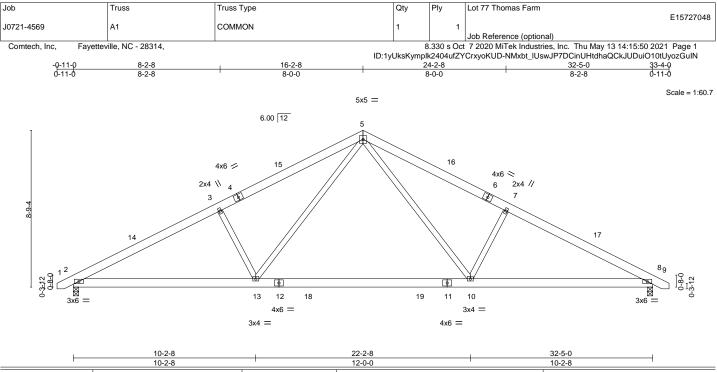
V6

V7

Truss Name Date 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021

Gilbert, Eric

August 03, 2021



		10-2-8		· ·		12-0-0		· ·		10-2-8	•
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	-0.34 10-13	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.64	Vert(CT)	-0.47 10-13	>824	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.27	Horz(CT)	0.05 8	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	2014	Matri	x-S	Wind(LL)	0.05 2-13	>999	240	Weight: 208 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 WEBS

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(LC 13)

Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at joint 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

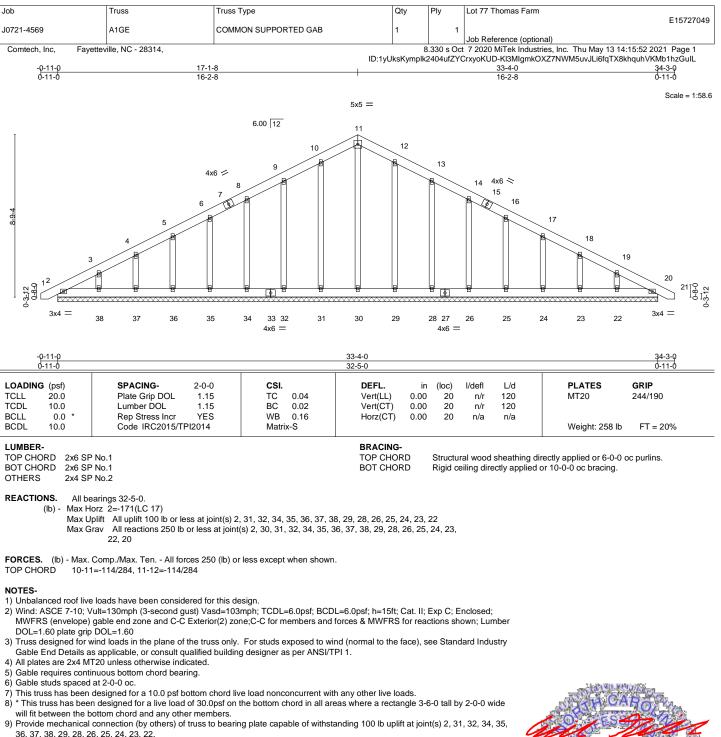


Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





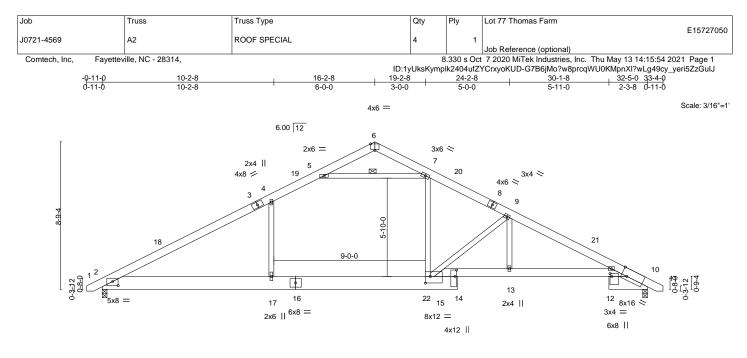
10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



May 14,2021

MARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTek® connectors. This skip included only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual Truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and provery damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPHI Quality Criteria, DSB-89 and BCSI Building Component Statery Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





ł	<u>10-2-8</u> 10-2-8	<u> </u>	19-2-8	21-1-8	24-2-8	<u> 30-1</u> 5-11		4
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0-4	-0,Edge], [12:0-2-0,0-1-4],	[14:0-4-8,0-1-4],	15:0-3-8	,0-4-12]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.75 BC 0.67 WB 0.70 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.21 -0.38 0.09	(loc) l/defl 17 >999 17 >999 10 n/a 2-17 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 247 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x10 10-15 WEBS 2x4 S REACTIONS. (siz Max Max	P No.1 SP No.1 *Except* : 2x6 SP 2400F 2.0E P No.2 ze) 2=0-3-8, 10=0-3-8 Horz 2=-110(LC 10) Uplift 2=-90(LC 12), 10=-90(LC 13) Grav 2=1393(LC 2), 10=1353(LC 2)		BRACING- TOP CHOR BOT CHOR WEBS	D R		ectly applied o	ectly applied or 4-0-8 (r 10-0-0 oc bracing. -7	oc purlins.
TOP CHORD 2-4= BOT CHORD 2-17	Comp./Max. Ten All forces 250 (lb) or 2217/403, 4-5=-1870/483, 7-9=-2258/5 7=-193/1848, 15-17=-195/1860, 13-15=-3 7=-29/402, 7-15=-114/967, 9-15=-1075/2	19, 9-10=-2889/551 371/2525, 10-13=-380/252						
	/e loads have been considered for this de Vult=130mph (3-second gust) Vasd=103		=6.0psf; h=15ft; C	at. II; Ex	p C; Enclosed	;		

MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8, Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8, Ext

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

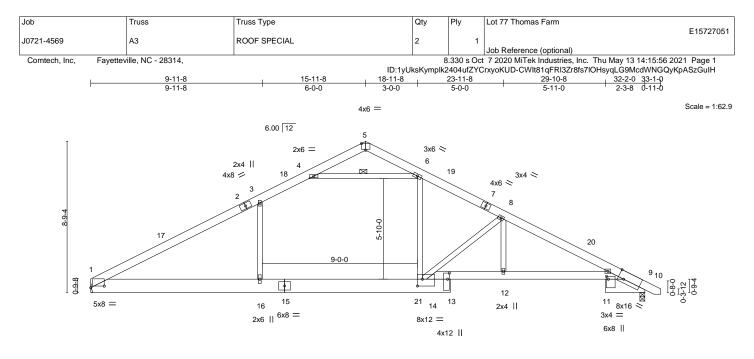
6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



BIS Soundside Road Edenton, NC 27932



L	9-11-8	18-11-8	20-10-				
	9-11-8	9-0-0	1-11-		•• ••	1-0 2-3-8	
Plate Offsets (X,Y)	[1:0-9-6,0-1-2], [5:0-3-0,Edge], [9:0-4-0,	Edgej, [11:0-2-0,0-1-4], [1	13:0-4-4,0-1-4], [14:0-3	-8,0-4-12			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.76 BC 0.66 WB 0.70 Matrix-S	Vert(LL) -0.2 Vert(CT) -0.3 Horz(CT) 0.0	6 16	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 243 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 S BOT CHORD 2x10 S 9-14: : WEBS 2x4 S			BRACING- TOP CHORD BOT CHORD WEBS	Structu Rigid d	ural wood sheathing o	directly applied or 3-10-0 d or 10-0-0 oc bracing. 4-6	
Max L Max C FORCES. (lb) - Max TOP CHORD 1-3= BOT CHORD 1-16	te) 1=Mechanical, 9=0-3-8 Horz 1=-111(LC 8) Jplift 1=-76(LC 12), 9=-90(LC 13) Grav 1=1345(LC 2), 9=1347(LC 2) . Comp./Max. Ten All forces 250 (lb) or -2189/401, 3-4=-1853/491, 6-8=-2237/5 :=-198/1827, 14-16=-200/1839, 12-14=-5 :=-197/966, 3-16=-53/392, 4-6=-1931/47	17, 8-9=-2874/549 875/2511, 9-12=-384/2511	l				
 2) Wind: ASCE 7-10; MWFRS (envelope to 32-10-10 zone;C 3) This truss has been 4) * This truss has been 	e loads have been considered for this de Vult=130mph (3-second gust) Vasd=103) and C-C Exterior(2) 0-0-12 to 4-5-9. Int -C for members and forces & MWFRS fo a designed for a 10.0 psf bottom chord liv en designed for a live load of 30.0psf on i	mph; TCDL=6.0psf; BCDI erior(1) 4-5-9 to 15-11-8, I or reactions shown; Lumber re load nonconcurrent with the bottom chord in all are	Exterior(2) 15-11-8 to 2 er DOL=1.60 plate grip n any other live loads.	0-4-5, Int DOL=1.6	erior(1) 20-4-5 60		1.07 Simon

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) Refer to girder(s) for truss to truss connections.

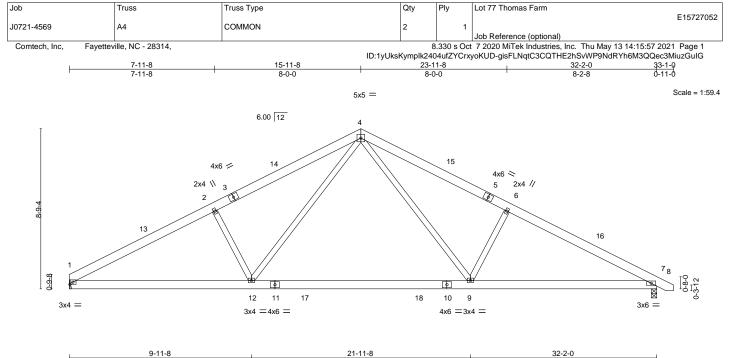
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss evaluat and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rector and bracing of trusses and truss systems, see **ANSTPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







		9-11-8		1		12-0-0			1		10-2-8	
Plate Offsets	5 (X, Y)	[1:0-1-14,0-1-8]										
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 2	20.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	-0.34	9-12	>999	360	MT20	244/190
TCDL 1	0.0	Lumber DOL	1.15	BC	0.64	Vert(CT)	-0.47	9-12	>822	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.27	Horz(CT)	0.05	7	n/a	n/a		
BCDL 1	0.0	Code IRC2015/TF	PI2014	Matri	x-S	Wind(LL)	0.05	12	>999	240	Weight: 204 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 7=0-3-8 Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13) Max Grav 1=1278(LC 1), 7=1331(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



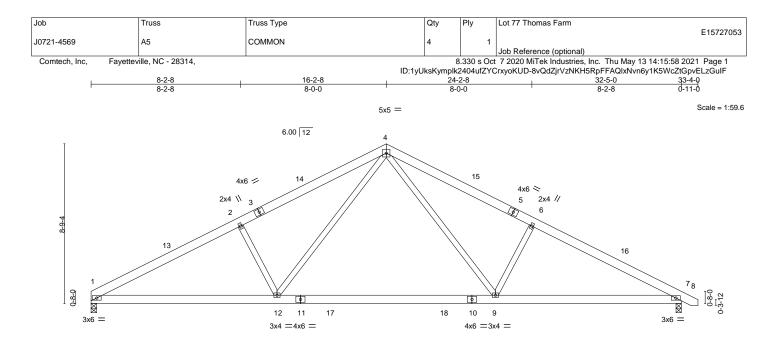
Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





F	10-2-8 10-2-8				22-2-8 32-5-0 12-0-0 10-2-8						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	0 Plate Grip DOL 0 Lumber DOL 0 * Rep Stress Incr	2-0-0 1.15 1.15 YES 'PI2014	CSI. TC BC WB Matrix	0.29 0.65 0.27 -S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.34 -0.47 0.05 0.05		l/defl >999 >822 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 206 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 WEBS

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



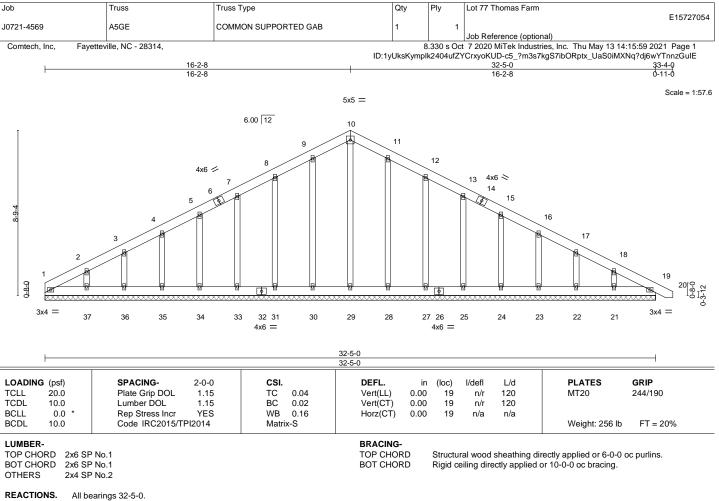
Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





- Max Horz 1=-175(LC 17) (lb) -
 - Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except 37=-101(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22,
 - 21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

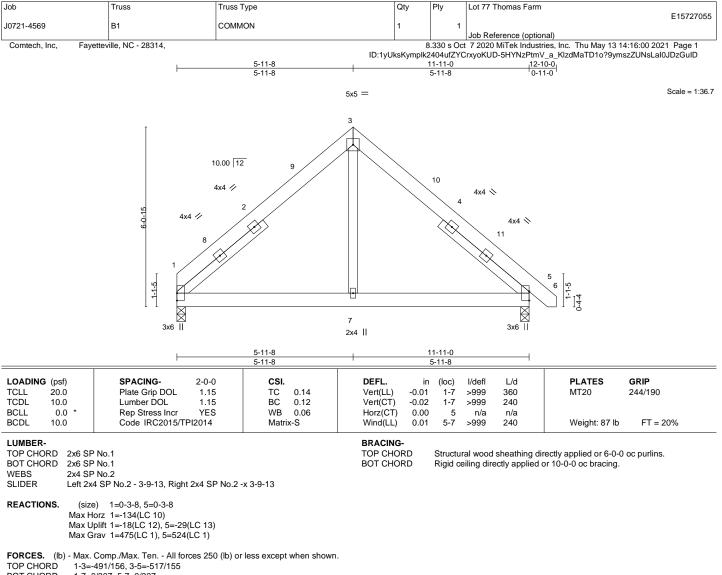
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.



May 14,2021

🛕 WARNING - Verify design pa neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid of use only with MTek® connectors. This sket intoCLOBED with REFERENCE FAGE MIF 4/3 FeV, 519/2020 BEFORE 052. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Braching indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BOT CHORD 1-7=0/307, 5-7=0/307

WEBS 3-7=0/277

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

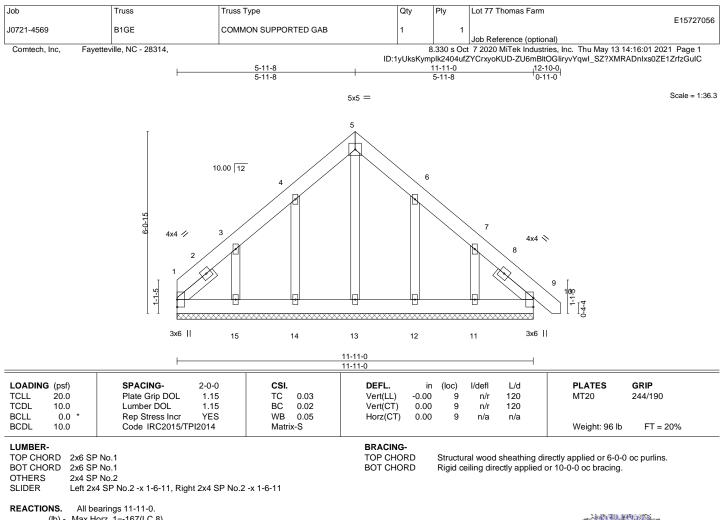
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



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(lb) - Max Horz 1=-167(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 12 except 15=-181(LC 12), 11=-169(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

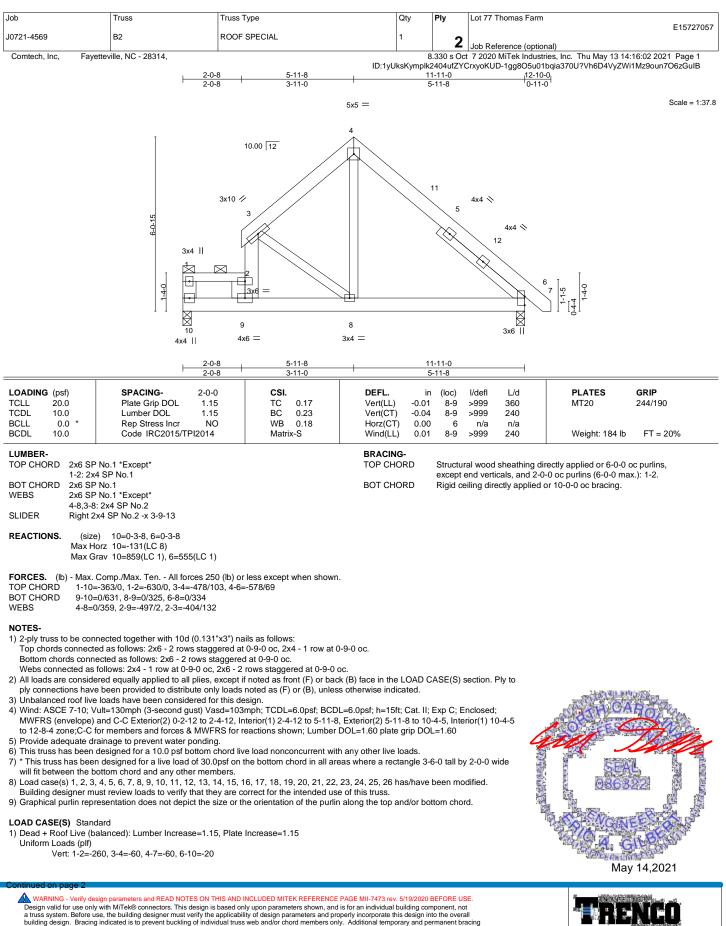
NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 12 except (it=lb) 15=181, 11=169.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.



May 14,2021

🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 818 Soundside Road Edenton, NC 27932



818 Soundside Road Edenton, NC 27932

Design valid for use only with MTerk® connectors. This design is back into ALD DED will the REFERENCE FAGE MIF/473 few. 519/2020 beFVRE USE. Design valid for use only with MTerk® connectors. This design is backed only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Compon Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
J0721-4569	P2	ROOF SPECIAL			E15727057
JU721-4569	B2	ROOF SPECIAL	1	2	Job Reference (optional)
Comtech, Inc, F	ayetteville, NC - 28314,	1			t 7 2020 MiTek Industries, Inc. Thu May 13 14:16:02 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 14:16:02 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-1gg8O5u01bqia370U?Vh6D4VyZWi1Mz9oun7O6zGuIB

	ID. IYOKSKYIIIPIK2404ul2 FCI
LOAD CASE(S) Standard	
 Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) 	
Vert: 1-2=-250, 3-4=-50, 4-7=-50, 6-10=-20	
 Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) 	
Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-40 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-156, 3-4=27, 4-12=35, 6-12=27, 6-7=20, 6-10=-12 Horz: 3-4=-39, 4-12=47, 6-12=39, 6-7=32	
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf) Vert: 1-2=-170, 3-4=35, 4-11=27, 6-11=35, 6-7=58, 6-10=-12	
Horz: 3-4=-47, 4-11=39, 6-11=47, 6-7=70	
 Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) 	
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=-51, 6-10=-20	
Horz: 3-4=38, 4-6=-38, 6-7=-31 7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=11, 6-10=-20 Horz: 3-4=38, 4-6=-38, 6-7=31	
 B) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) 	
Vert: 1-2=-179, 3-4=-13, 4-6=11, 6-7=4, 6-10=-12	
Horz: 3-4=1, 4-6=23, 6-7=16 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-191, 3-4=11, 4-6=-13, 6-7=2, 6-10=-12 Horz: 3-4=-23, 4-6=-1, 6-7=14	
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60	1
Uniform Loads (plf) Vert: 1-2=-201, 3-4=-35, 4-6=-11, 6-7=-4, 6-10=-20	
Horz: 3-4=15, 4-6=9, 6-7=16	20
 Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.6 Uniform Loads (plf) 	
Vert: 1-2=-213, 3-4=-11, 4-6=-35, 6-7=-28, 6-10=-20 Horz: 3-4=-9, 4-6=-15, 6-7=-8	
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase	e=1.60
Uniform Loads (plf) Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12	
Horz: 3-4=-33, 4-6=21, 6-7=14	
 Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increa Uniform Loads (plf) 	se=1.60
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12	
Horz: 3-4=-21, 4-6=33, 6-7=26 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase	se=1.60
Uniform Loads (plf)	
Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12 Horz: 3-4=-33, 4-6=21, 6-7=14	
 Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase Uniform Loads (plf) 	se=1.60
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12	
Horz: 3-4=-21, 4-6=33, 6-7=26 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase	se=1 60
Uniform Loads (plf)	
Vert: 1-2=-201, 3-4=-1, 4-6=-13, 6-7=-6, 6-10=-20 Horz: 3-4=-19, 4-6=7, 6-7=14	
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increa	ise=1.60
Uniform Loads (plf) Vert: 1-2=-213, 3-4=-13, 4-6=-1, 6-7=6, 6-10=-20	
Horz: 3-4=-7, 4-6=19, 6-7=26	
 Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf) 	
Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-20 19) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.	60 Plate Increase-1.60
Uniform Loads (plf)	
Vert: 1-2=-236, 3-4=-61, 4-6=-43, 6-7=-38, 6-10=-20 Horz: 3-4=11, 4-6=7, 6-7=12	
20) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=	1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=-245, 3-4=-43, 4-6=-61, 6-7=-56, 6-10=-20	
Horz: 3-4=-7, 4-6=-11, 6-7=-6	
21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Incre	ease=1.60, Plate Increase=1

21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

WARNIG - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



1	Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
						E15727057
	J0721-4569	B2	ROOF SPECIAL	1	2	
					_	Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:02 2021 Page 3

ID:1yUksKymplk2404ufZYCrxyoKUD-1gg8O5u01bqia370U?Vh6D4VyZWi1Mz9oun7O6zGulB

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-36, 4-6=-45, 6-7=-40, 6-10=-20 Horz: 3-4=-14, 4-6=5, 6-7=10

22) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

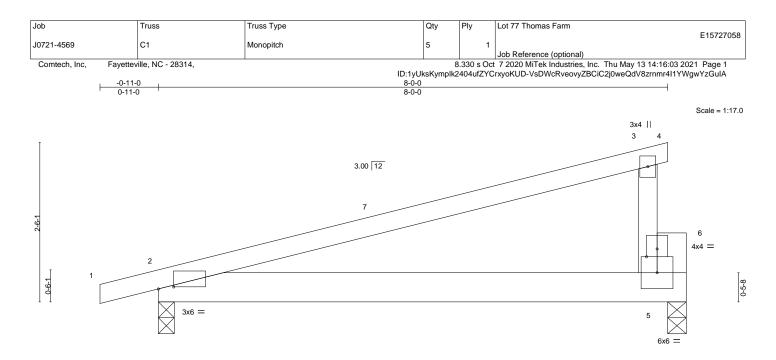
- Uniform Loads (plf)
 - Vert: 1-2=-245, 3-4=-45, 4-6=-36, 6-7=-31, 6-10=-20 Horz: 3-4=-5, 4-6=14, 6-7=19
- 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf) Vert: 1-2=-260, 3-4=-60, 4-7=-20, 6-10=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-220, 3-4=-20, 4-7=-60, 6-10=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-250, 3-4=-50, 4-7=-20, 6-10=-20
- 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-50, 6-10=-20

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			0-3-0				
			8-3-8				
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]						
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. i	n (loc) l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.84	Vert(LL) -0.05	(,	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.24	Vert(CT) -0.10) 2-5 >969	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00) 5 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.10	0 2-5 >886	240	Weight: 37 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x4 SP	' No.1		TOP CHORD	Structural woo	d sheathing di	rectly applied or 5-3-4	l oc purlins,
BOT CHORD 2x6 SP	' No.1			except end ve	rticals.		
WEBS 2x4 SP	No.2		BOT CHORD	Rigid ceiling d	irectly applied	or 10-0-0 oc bracing.	
OTHERS 2x6 SP	No.1						

8-3-8

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

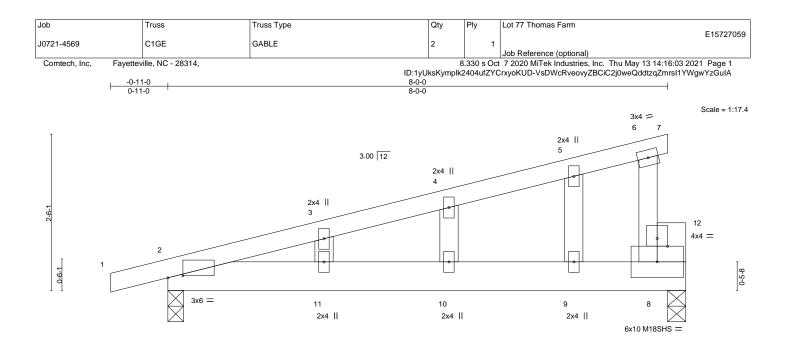
NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.



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	 		<u>8-3-8</u> 8-3-8		I
Plate Offsets (X,Y) [[2:0-2-14,0-0-6], [12:0-2-0,0-1-8]		0-0-0		-
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	Vert(LL) 0.09	n (loc) I/defl L/d 9 10-11 >999 240 8 10-11 >999 240 9 8 n/a n/a	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 41 lb FT = 20%
	No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8) Max Uplift 2=-216(LC 8), 8=-188(LC 8) Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

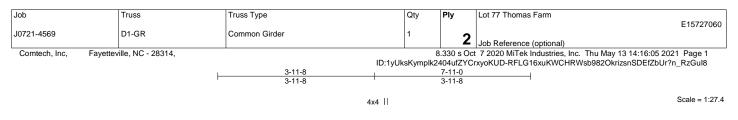
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.

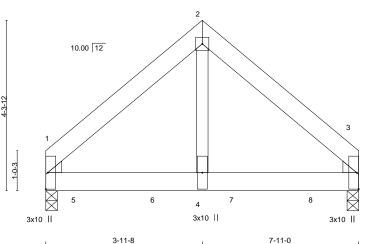


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component</u> **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932





				3-11	-8		3	8-11-8		1		
Plate Offse	ets (X,Y)	[1:0-0-4,0-0-5], [1:0-0-8,0-3	3-6], [3:0-0-4	4,0-0-5], [3:0-0	-8,0-3-6]							
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.02	3-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.57	Vert(CT)	-0.04	3-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.39	Horz(CT)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI	2014	Matrix	-P	Wind(LL)	0.01	3-4	>999	240	Weight: 100 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No 2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz	1=91(LC 24)
Max Uplift	1=-191(LC 8), 3=-180(LC 9)
Max Grav	1=2919(LC 1), 3=2779(LC 2)

FORCES. (lb)	- Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.
TOP CHORD	1-2=-2418/177, 2-3=-2418/177
BOT CHORD	1-4=-100/1678, 3-4=-100/1678
WEBS	2-4=-154/3142

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20

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May 14,2021

BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

1-0-3

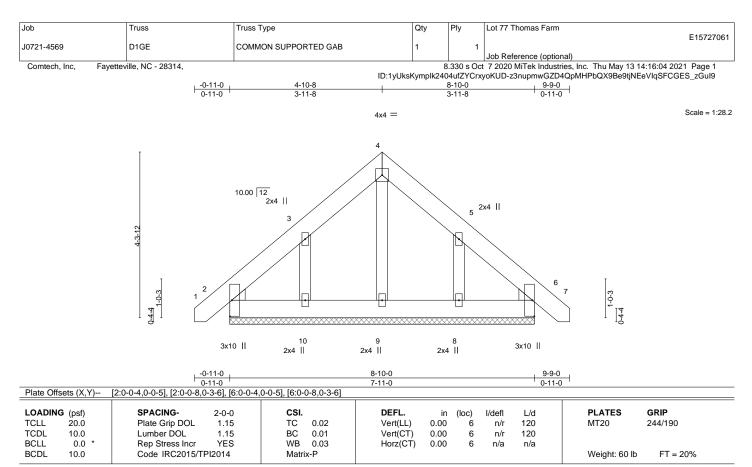
Job		Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
						E15727060
J0721-4569		D1-GR	Common Girder	1	2	
					_	Job Reference (optional)
Comtech, Inc,	Fayettev	ille, NC - 28314,		6	3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:05 2021 Page 2
			ID:1yUI	ksKymplk2	404ufZYC	rxyoKUD-RFLG16xuKWCHRWsb982OkrizsnSDEfZbUr?n_RzGul8

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)

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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No 2

WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. All bearings 7-11-0.

(lb) - Max Horz 2=-118(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152.8=148.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



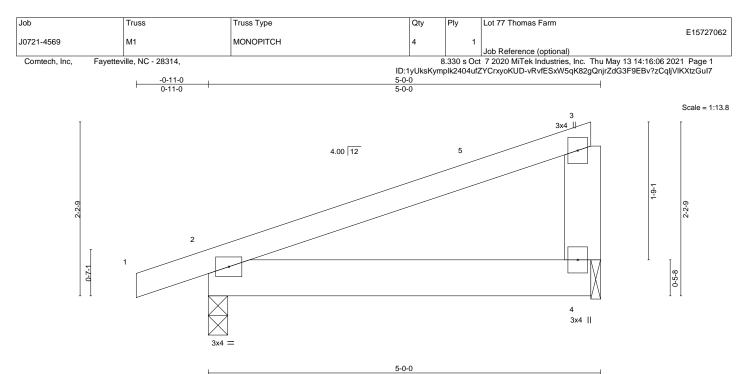
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021

MARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTE k® connectors. This sket on LCULCUL MIT Interstep Texpect Proceed MIT-1473 (eV. 319/2/20 betrofte Use. Design valid for use only with MTE k® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Comport Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			5-0-0				I	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.01	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) -0.01	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00		n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.01	2-4	>999	240	Weight: 24 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=63(LC 8)

Max Uplift 2=-102(LC 8), 4=-79(LC 8) Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.



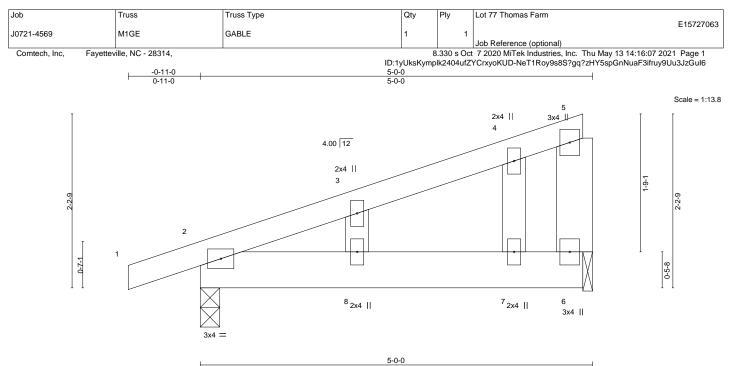
Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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			5-0-0		
LOADING (psf)	SPACING- 2-0-0	CSI. DI	FL. in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.09 Ve	rt(LL) 0.01 8	>999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09 Ve	rt(CT) -0.01 8	>999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02 Ho	rz(CT) -0.00 6	i n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	()		Weight: 27 lb FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8)

Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

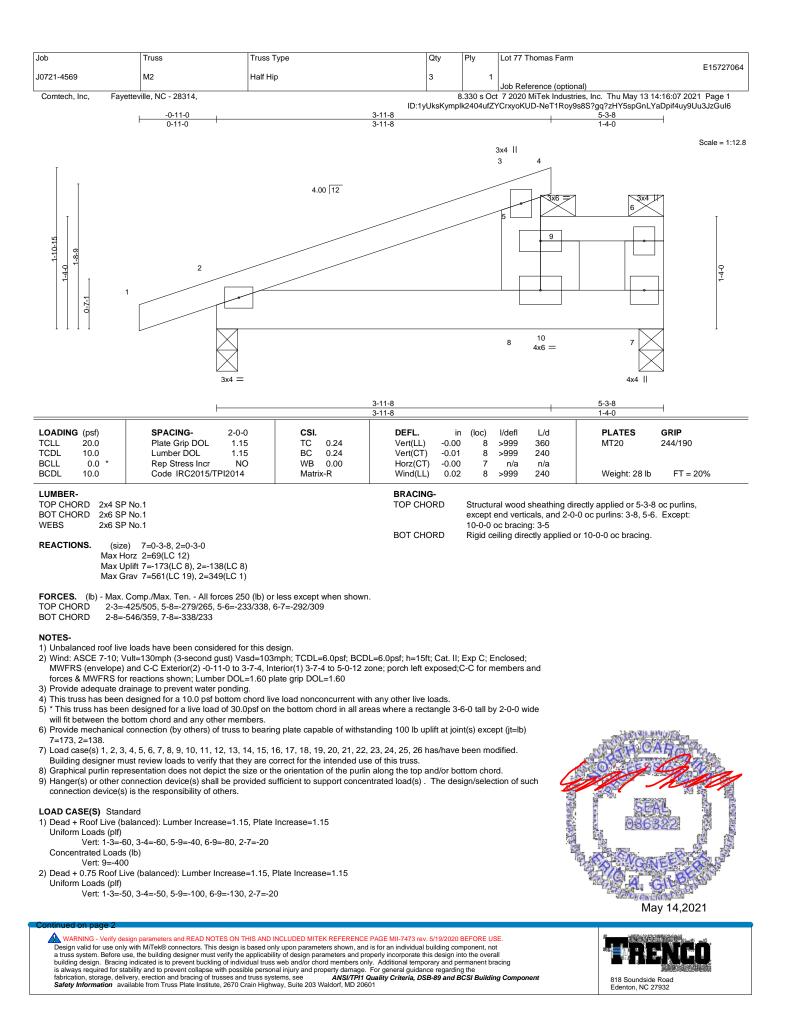
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147.6=115.



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1	Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
						E15727064
	J0721-4569	M2	Half Hip	3	1	
						Job Reference (optional)
	Comtech, Inc, Fayettevi	lle, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:07 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 14:16:07 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-NeT1Roy9s8S?gq?zHY5spGnLYaDpif4uy9Uu3JzGul6

LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 9=-350 3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (pff)
Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40
Concentrated Loads (Ib)
Vert: 9=-300
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=12, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
Concentrated Loads (lb) Vert: 9=548
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (pif)
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
Concentrated Loads (lb)
Vert: 9=566
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9 Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420 8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10
Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
Concentrated Loads (lb)
Vert: 9=154
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb)
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb)
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb)
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb)
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 5-6=-11, 2-7=-12
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=-6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-46, 2-3=-23, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=-21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-1, 3-4=-6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 1-2=-24, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-334 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-24, 2-3=9, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 1-2=-34
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=-6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-4, 2-3=-3, 3-4=-2, 5-6=-1, 2-7=-12 Horz: 1-2=-26, 2-3=, 3, -4=-2, 5-6=1, 2-7=-12 Horz: 1-2=-2, 3-9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-2, 3-9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-2, 3-9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-2, 2-3=, 9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-2, 1, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb)
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=-6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-19, 3-4=-6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-33 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=-33 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 9=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Hor: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Hor:: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-18, 2-3=-11, 3-4=-14, 5-6=-11, 2-7=-12 Hor:: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-33 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-4, 2-3=21, 3-4=-14, 5-6=-11, 2-7=-12 Hor:: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Hor:: 1-2=-2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Hor:: 1-2=-4, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 5-6=-11, 2-7=-12
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (b) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (b) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-44, 2-3=21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-62, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-44, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=-43, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-44, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-43, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-4, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-43, 3-4=-2, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=-4, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-4, 2-3=-21, 3-4=-2, 3-5=-39 Concentrated Load
$ \begin{array}{l} \mbox{Horz: } 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 \\ \mbox{Concentrated Loads (b) } \\ \mbox{Vert: } 9=43 \\ \mbox{10} \end{tabular} \end{tabular} \\ \mbox{10} \end{tabular} \end{tabular} \end{tabular} \\ \mbox{11} \end{tabular} \end{tabular} \end{tabular} \\ \mbox{11} \end{tabular} \end{tabular} \end{tabular} \end{tabular} \\ \mbox{11} \end{tabular} \end{tabular} \end{tabular} \end{tabular} \end{tabular} \\ \mbox{11} \end{tabular} \$
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (b) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (b) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-44, 2-3=21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-62, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-44, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=-43, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-44, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-43, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-4, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-43, 3-4=-2, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=-4, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-4, 2-3=-21, 3-4=-2, 3-5=-39 Concentrated Load

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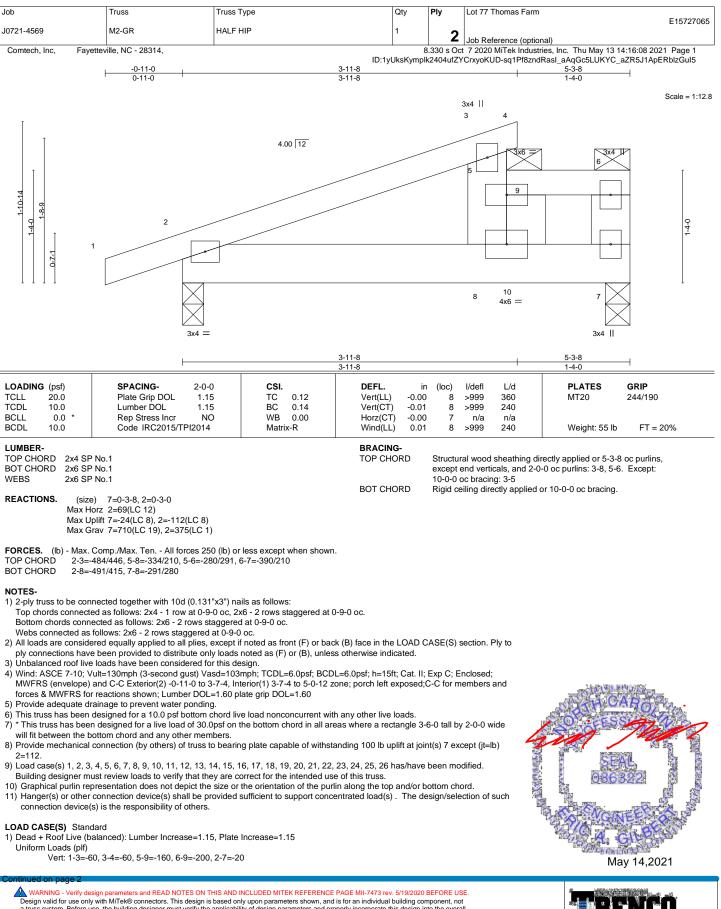
Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
					E15727064
J0721-4569	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayettevi	lle, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:07 2021 Page 3

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 14:16:07 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-NeT1Roy9s8S?gq?zHY5spGnLYaDpif4uy9Uu3JzGul6

		ID:19UKsKympik24040f21CfxyoK0D-ine11R0y9s85/gd
	104	AD CASE(S) Standard
		Uniform Loads (pif)
		Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12
		Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
		Concentrated Loads (lb)
		Vert: 9=43
	16)	Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
		Uniform Loads (plf)
		Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-7=-20
		Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
		Concentrated Loads (lb)
	47)	Vert: 9=-234
		Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
		Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20
		Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
		Concentrated Loads (lb)
		Vert: 9=-234
	18)	Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
		Uniform Loads (plf)
		Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20
		Concentrated Loads (lb)
		Vert: 9=-200
		Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
		Uniform Loads (plf)
		Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
		noi2. 172-19, 253-14, 354-19, 353-20 Concentrated Loads (Ib)
		Vert: 9=-454
:	20)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
		Uniform Loads (plf)
		Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20
		Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
		Concentrated Loads (lb)
	~	
		Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
		Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20
		Hor: 1-2-19, 2-3-14, 3-4-19, 3-5-9
		Concentrated Loads (Ib)
		Vert: 9=-375
	22)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
		Uniform Loads (plf)
		Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20
		Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
		Concentrated Loads (lb)
	<u> </u>	Vert: 9=-375
		1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
		Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20
		Concentrated Loads (lb)
		Vert: 9=-400
	24)	2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
		Uniform Loads (plf)
		Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20
		Concentrated Loads (lb)
		Vert: 9=-400
		3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
		Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-6=-100, 2-7=-20
		Vent 1:5-30, 54-3-0, 50-100, 2-1=20 Concentrated Loads (Ib)
		Vert: 9–350
	26)	4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
		Uniform Loads (plf)
		Vert: 1-3–-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20
		Concentrated Loads (lb)
		Vert: 9=-350

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818 Soundside Road Edenton, NC 27932

[Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
						E15727065
	J0721-4569	M2-GR	HALF HIP	1	2	
					_	Job Reference (optional)
	Comtech, Inc, Fayettevi	lle, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:08 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 14:16:08 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-sq1Pf8zndRasI_aAqGc5LUKYC_aZR5J1ApERblzGul5

Uniform Loads (plf)	
Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-5	€=-250, 2-7=-20
Concentrated Loads (lb) Vert: 9=-350	
3) Dead + Uninhabitable Attic Without Storage:	Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)	,
Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7	7=-40
Concentrated Loads (lb)	
Vert: 9=-300 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1:	Lumber Ingrasso-1.60. Plate Ingrasso-1.60
Uniform Loads (plf)	Lumber Increase=1.00, Flate Increase=1.00
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=	-108, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-	
Concentrated Loads (lb)	
Vert: 9=548	
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Uniform Loads (plf)	Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-7	78, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5	
Concentrated Loads (lb)	
Vert: 9=566	
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Uniform Loads (plf)	Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-	178 2-8=-9 8-10=2 7-10=-9
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=	
Concentrated Loads (lb)	
Vert: 9=-420	
 Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Uniform Loads (plf) 	Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6	=-178, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=5	
Concentrated Loads (lb)	
Vert: 9=-420	
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left Uniform Loads (plf)	. Lumber increase=1.60, Plate increase=1.60
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-1	131, 2-8=10, 8-10=33, 7-10=10
Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5	=7
Concentrated Loads (lb)	
Vert: 9=154 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Rig	ht: Lumber Increase-1.60. Plate Increase-1.60
Uniform Loads (plf)	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-1	19, 2-7=-12
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5	=-27
Concentrated Loads (lb) Vert: 9=43	
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Le	eft: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-15	
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-	·5=34
Concentrated Loads (lb) Vert: 9=-339	
	ight: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-1	
Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3- Concentrated Loads (lb)	·5=-0
Vert: 9=-234	
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1s	st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=	
Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3- Concentrated Loads (lb)	0=-39
Vert: 9=43	
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2r	nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-	
Concentrated Loads (lb)	<u></u> 2,
Vert: 9=43	
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3r	d Parallel: Lumber Increase=1.60, Plate Increase=1.60

2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-400

Uniform Loads (plf)

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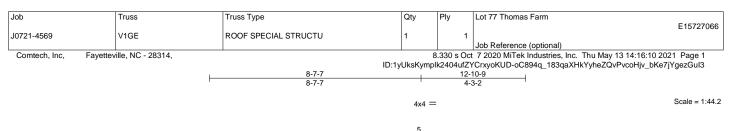
[dof	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
						E15727065
	J0721-4569	M2-GR	HALF HIP	1	2	
					_	Job Reference (optional)
	Comtech, Inc, Fayettevi	lle, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:08 2021 Page 3

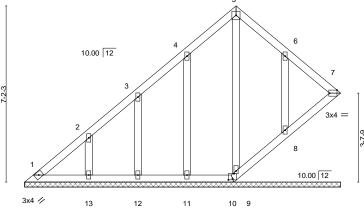
8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 14:16:08 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-sq1Pf8zndRasI_aAqGc5LUKYC_aZR5J1ApERblzGul5

LO	AD CASE(S) Standard
	Uniform Loads (plf)
	Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
	Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
	Concentrated Loads (Ib)
	Vert: 9=43
15)	Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
	Concentrated Loads (lb)
16)	Vert: 9=43 Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
10)	
	Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
	Concentrated Loads (lb)
	Vert: 9=-234
17)	Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20
	Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
	Concentrated Loads (lb)
	Vert: 9=-234
18)	Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20
	Concentrated Loads (lb) Vert: 9=-200
19)	Ver
10)	Uniform Loads (pf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3
	Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
	Concentrated Loads (lb)
	Vert: 9=-454
20)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20
	Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
	Concentrated Loads (lb)
21)	Vert: 9=-375 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
21)	Uniform Loads (plf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20
	Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
	Concentrated Loads (lb)
	Vert: 9=-375
22)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20
	Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
	Concentrated Loads (lb)
00)	Vert: 9=-375
23)	1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20
	Vert. 1-5=-60, 5-6=-160, 2-7=-20 Concentrated Loads (lb)
	Vert: 9=-400
24)	2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
,	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20
	Concentrated Loads (lb)
	Vert: 9=-400
25)	3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20
	Concentrated Loads (lb)
201	Vert: 9=-350 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
20)	4th Dead + 0.75 Root Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
	Viniti Loads (pn) Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20
	Concentrated Loads (Ib)
	Vert: 9=-350

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses sand truss systems, see **AVSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601











3x4 =

					0-0-0			4				
Plate Offs	ets (X,Y)	[7:0-3-11,Edge], [9:0-1-6	,0-1-0], [10:0-2	2-0,0-0-10]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.15	тс	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.00	7	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 75 lb	FT = 20%
				1								
LUMBER-	•					BRACING-						

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No 2 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12), 8=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

Н

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-295/189

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

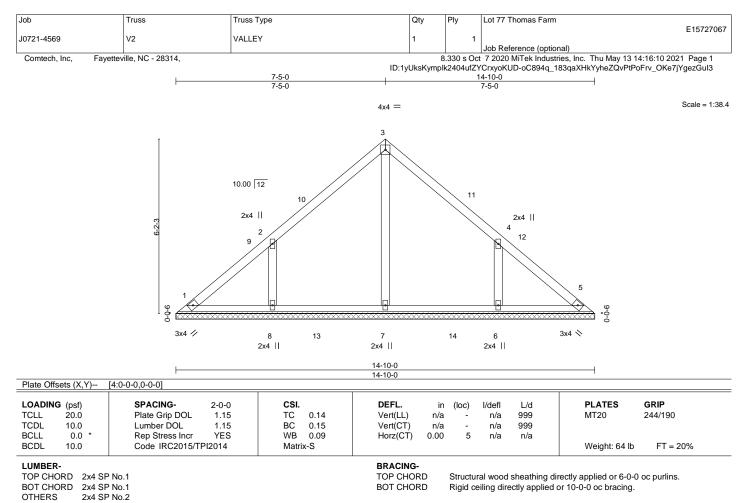
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.



May 14,2021

MARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design values for use only with MTek® connectors. This should be used only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracking indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Compon Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





REACTIONS. All bearings 14-10-0.

(lb) - Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) $1.5 \exp(2-150(LC + 2), 0 = 150(LC + 3))$ Max Grav All reactions 250 lb or less at joint(s) $1, 5 \exp(7=400(LC + 9), 8=393(LC + 9))$

WEBS 2-8=-338/247, 4-6=-338/247

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb)

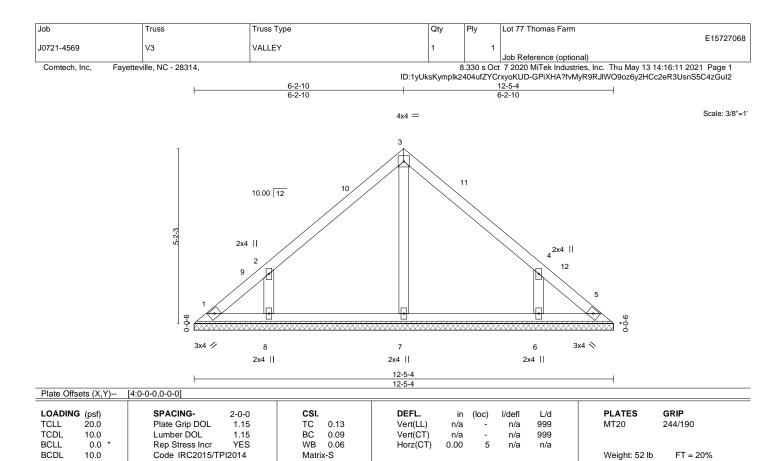
8=135, 6=135.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the appliciability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Componen Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 12-5-4.

(lb) - Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-312/241. 4-6=-312/241

WEBS 2-8=-312/241, 4-6=-312/241

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.

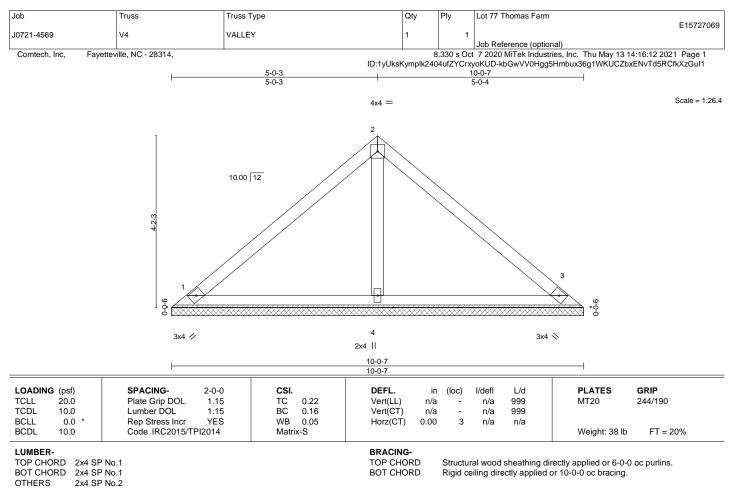


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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REACTIONS. (size) 1=10-0-7, 3=10-0-7, 4=10-0-7

Max Horz 1=-92(LC 8)

Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

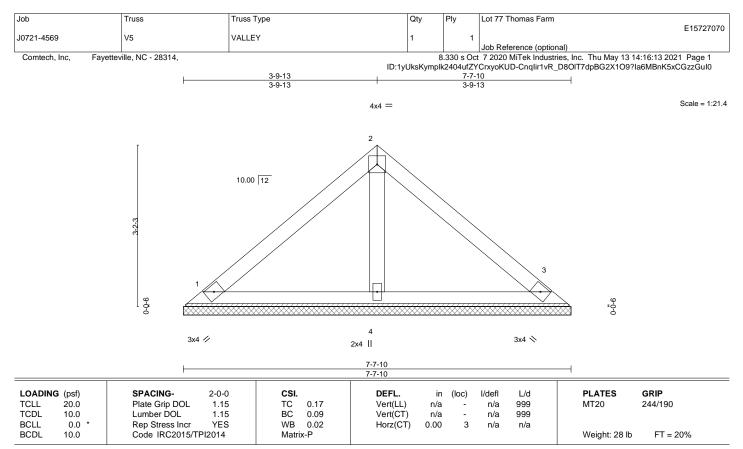
will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10

Max Horz 1=68(LC 9)

Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

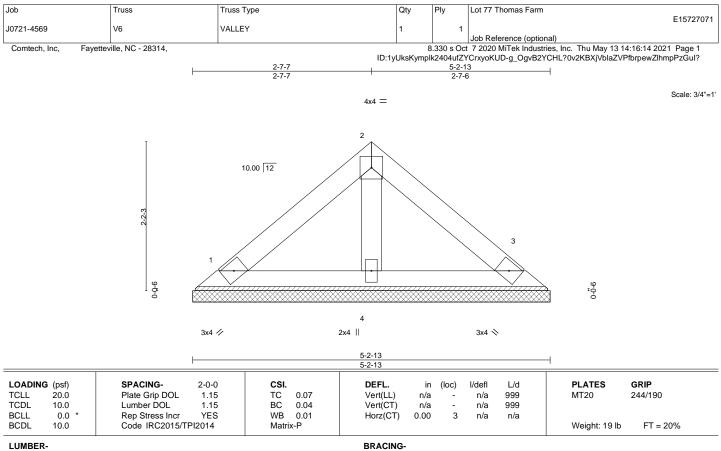


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13

Max Horz 1=-44(LC 8)

Max Uplift 1=-15(LC 13), 3=-19(LC 13)

Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



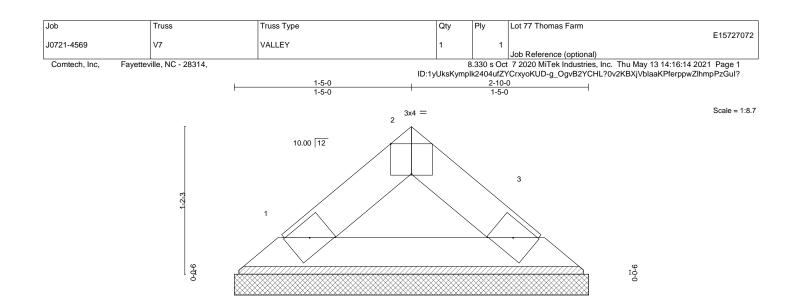
Structural wood sheathing directly applied or 5-2-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021

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3x4 🥢

3x4 📎

Plate Offsets (X,Y) [2:0-2-0,Edge]											
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15		0.01	Vert(LL)	n/a	(.00)	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC (0.03	Vert(CT)	n/a	-	n/a	999	-	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matrix-F	C						Weight: 8 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

2-10-0

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=2-10-0, 3=2-10-0

Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

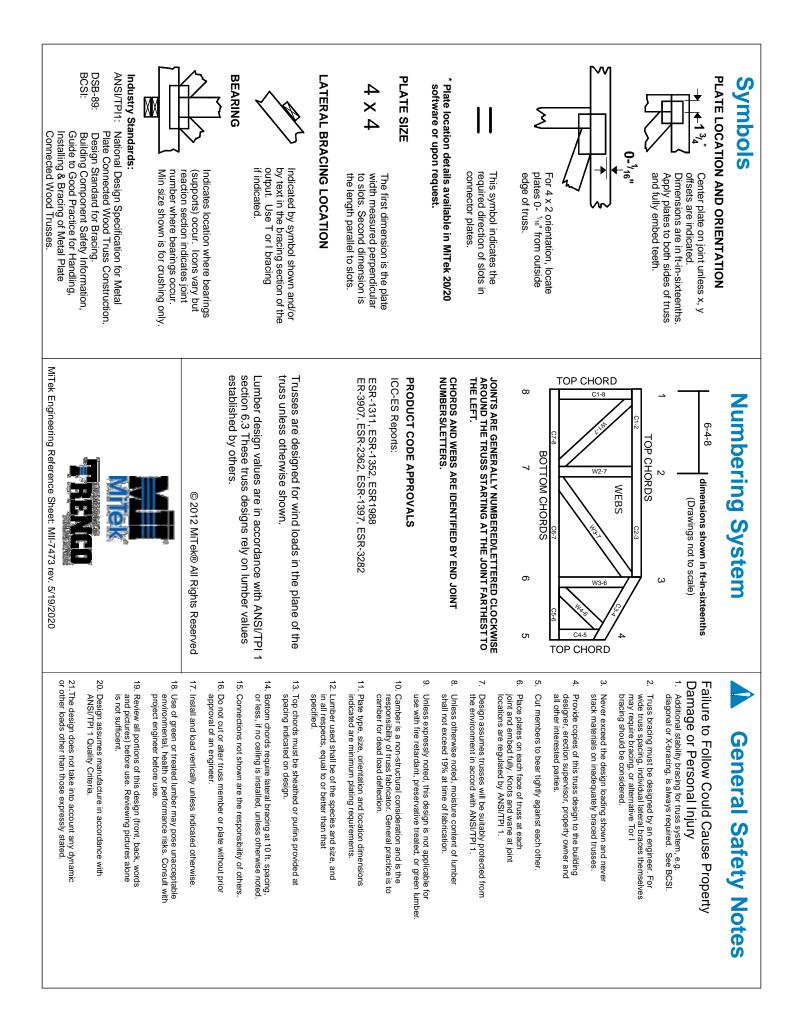


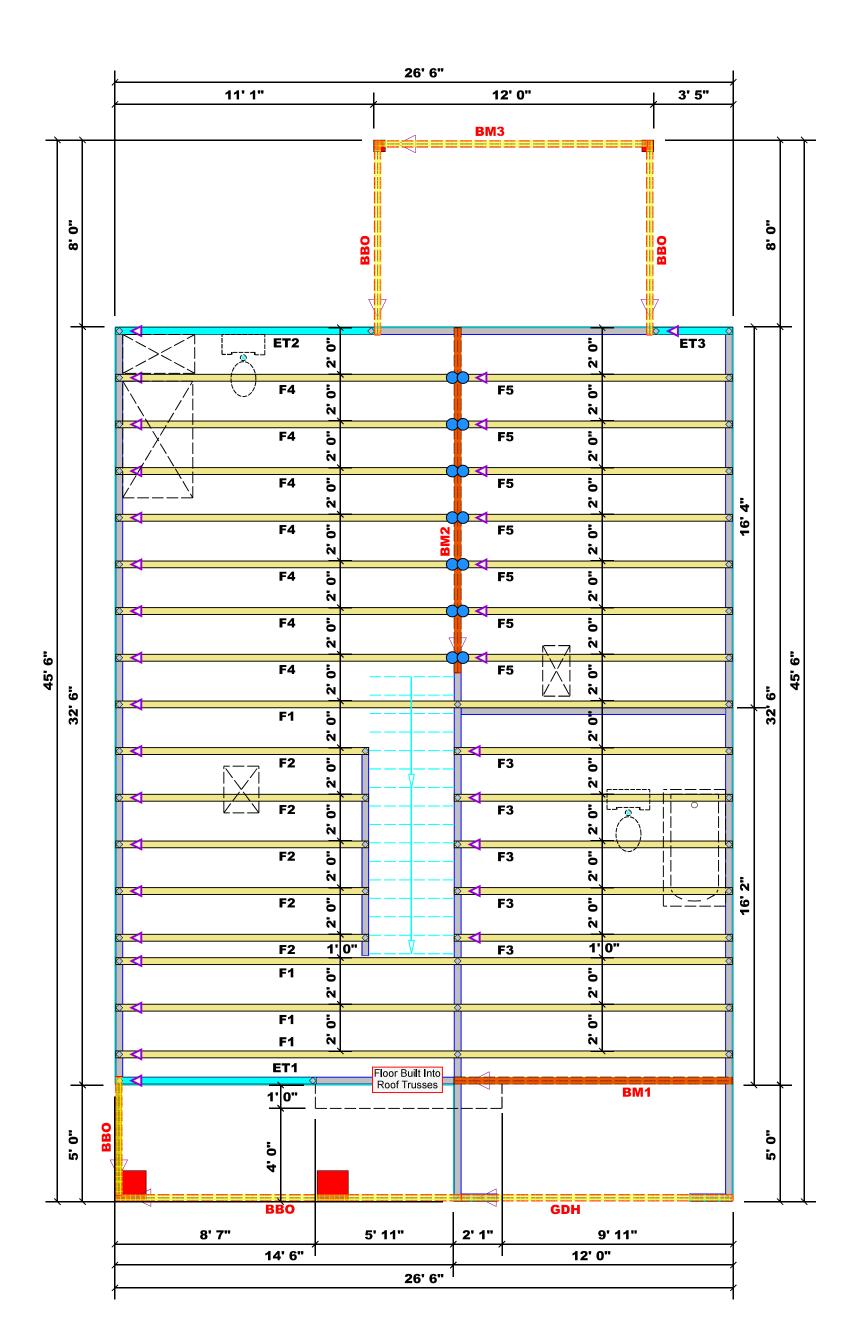
Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







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Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.

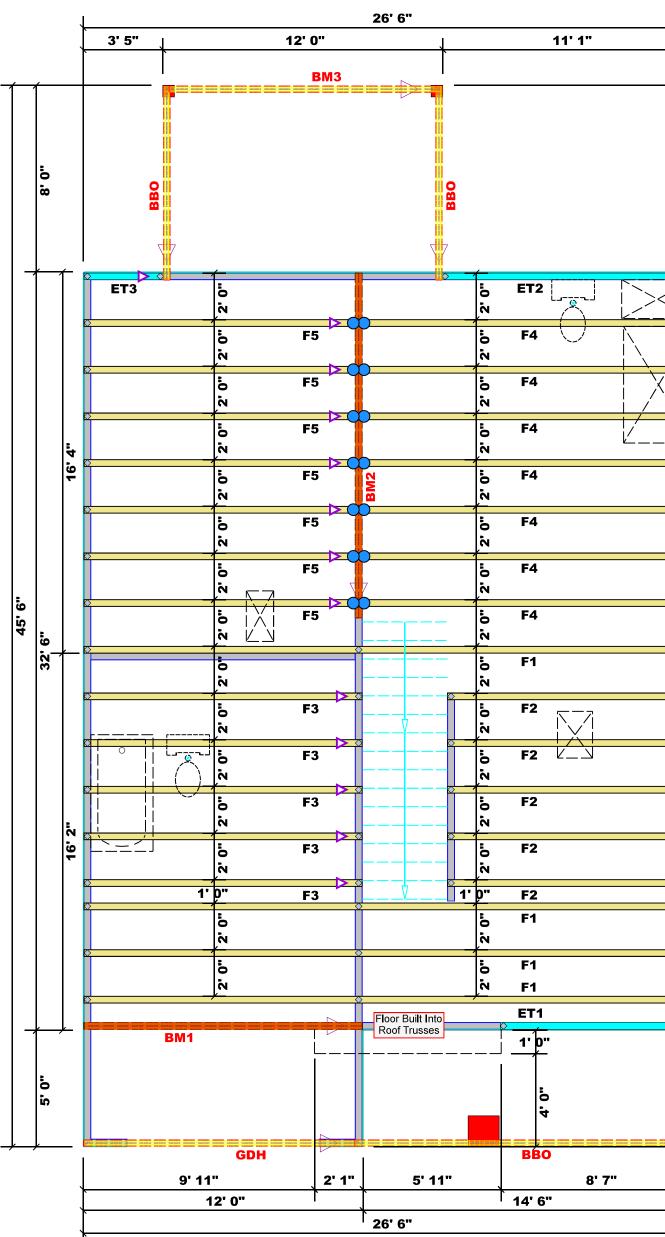
Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	ctor Info	rmat	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

Products							
PlotID	Length	Product	Plies	Net Qty	Fab Type		
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF		
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF		
BM3	12' 0"	2x10 SPF No.2	2	2	FF		
GDH	12' 0"	2x12 SPF No.2	2	2	FF		

Truss Placement Plan Scale: 1/4"=1'





		eilly R Fayet Phon	SES Road In teville ie: (910) : (910)	& B ndustr (N.C. 0) 864	ial Par 28309 -8787	∕IS ⁺k
	deemed requiren attachee requiren size and reaction Tables. retained reaction Signatu	to compl nents. The 1 Tables (nents) to 1 number 8 greater A register to design that excc A register to design that excc re	s less thar ly with the e contract derived fi determinu- of wood s than 3000 red design in the supp ceeds those red design ceed 1500 Davi Davi ART FC	e prescrip tor shall r from the p le the min studs req 0# but no n profess port syste e specifie n profess port syste 00#.	tive Code efer to the rescriptivi imum fou aired to si greater t ional shal m for any d in the a onal shal m for all	e e re Code indation upport than II be y tttached I be
	NGA	(BASEL NEER OF 7/ NEER	b on tABLE Set STUDS FEADER: FEADER: 2550 5100 7650 10200 12750	ES R5025() REQUIRED VSTRUER NOT SOLLS 0,074 1 2 3 0 4 0 5 0 4 0 5 0 4 0 5 0 4 0 5 0 1 0 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 1 0 0 1 0 1 0 0 1 0 0 1 0 0 0 1 0 0 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0) \$ (5) 8 54 545 NOLLSE 49 12 13 13 13 13 13 13 13 13 13 13	(ur to) S RCQ'D STUDS FOR (4) RLY HEADER
Dimension Notes I. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 3. All enterior wall to therwise 3. All exterior wall to the	Spring Lake	Bill Shaw Road	Floor	08/03/21	David Landry	Lenny Norris
face of frame wall unless noted otherwise All Walls Shown Are Considered Load Bearing Plumbing Drop Notes 1. Plumbing drop locations shown are NOT exact. 2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.	CITY / CO .	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
3. Adjust spacing as needed not to exceed 24"oc. Connector Information Nail Information Sym Product Manuf Qty Supported Header Truss HUS410 USP 14 NA 16d/3-1/2" 16d/3-1/2" HUS410 USP 14 NA 16d/3-1/2" Products Products PlotID Length Product Plies Net Qty Fab Type BM1 12' 0" 1-3/4"x 16" LVL Kerto-S 2 2 FF BM2 15' 0" 1-3/4"x 16" LVL Kerto-S 2 2 FF BM2 15' 0" 1-3/4"x 16" LVL Kerto-S 2 2 FF	Weaver Development Co. Inc.	Lot 77 Thomas Farm	Hickory "A"	Seal Date	Quote #	J0721-4570
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	BUILDER	JOB NAME	S PLACEM re designe	SEAL DATE	MODTE #	

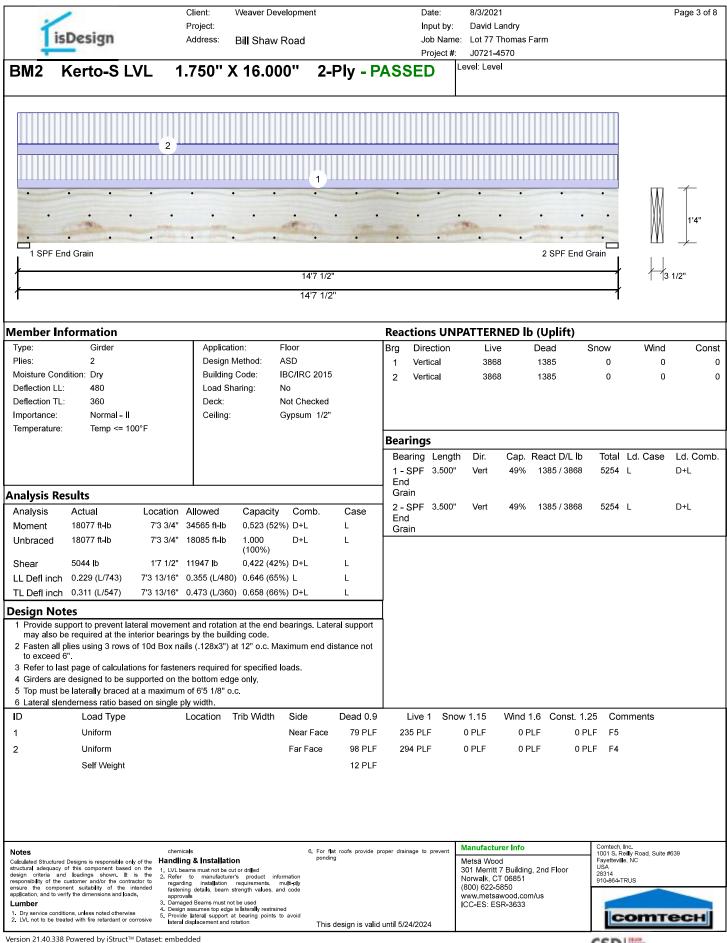
= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

	•		lient: Weaver Devel	opment		Date: nput by:	8/3/2021 David Landry				Page 1 of 8
is	Design	A	ddress: Bill Shaw R	oad			Lot 77 Thomas	Farm			
3M1 k	Kerto-S L	<u>// 1 '</u>	750'' X 16.000)" 2-Ply -		Project #:	J0721–4570 evel: Level				
			750 × 10.000	2=riy =	FASSE						
		2									
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					1					MM	
	17400			***********	-	17.1	With			M	1'4"
	•	•	•			•	2 SPF			Ш	<u> </u>
			4 4 1 4 4 11								4 /0!!
ļ			11'11"							3	1/2
I			11'11"				I				
/lember Inf	ormation				Reactio	ns UNP	ATTERNED	b (Uplift)			
Туре:	Girder		1	oor	Brg Dii	rection	Live	Dead	Snow	Wind	Con
Plies: Moisture Cond	2 lition [.] Drv		Design Method: AS Building Code: IB	SD C/IRC 2015		rtical	0	2869 2869	2079 2079	0 0	
Deflection LL:	480		Load Sharing: No		2 Ve	rtical	0	2869	2079	U	
Deflection TL:	360			ot Checked							
Importance: Temperature:	Normal - II Temp <= 10	0°E	Ceiling: Gy	/psum 1/2"							
remperature.	Temp <= To	01			Bearing	IS					
					-	Length	Dir. Cap.	React D/L lb		d. Case	Ld. Com
					1 - SPF 2 - SPF		Vert 95% Vert 95%				D+S D+S
Analysis Res	sults					3.300	Ven 93%	200972078	4940 L		D+3
Analysis	Actual	Location A		Comb. Case							
Moment Unbraced	13679 ft ⊣ b 13679 ft ⊣ b	5'11 1/2" 3 5'11 1/2" 1	9750 ft-lb 0.344 (34%) 3699 ft-lb 0.999	D+S L D+S L							
			(100%)								
Shear	3615 lb	1'7 1/2" 1	,								
	0.069 (L/2000) 0.164 (L/840)		.287 (L/480) 0.240 (24%) .383 (L/360) 0.428 (43%)								
Design Not											
1 Provide sup	port to prevent late		and rotation at the end be	arings. Lateral suppo	rt						
2 Fasten all p	lies using 3 rows o		by the building code. s (.128x3") at 12" o.c. Max	mum end distance no	ot						
to exceed 6 3 Refer to last		ons for fastener	s required for specified loa	ıds.							
4 Girders are	designed to be su	pported on the	bottom edge only								
	ust be supported a laterally braced a										
	derness ratio base	0 1 7		Sido Deeda	0 15	1 8	v 1 15 \\A&~ -1	16 Const	25 0		
ID 1	Load Type Uniform	L		Side Dead C Top 120 P				1.6 Const. ² PLF 0	I.25 Comm PLF Wall	ients	
2	Uniform			Top 120 P Top 349 P					PLF A2		
	Self Weight			12 P			·		-		
Notes		chemical	ŝ	6. For flat roofs provi	de proper drainage t	o prevent	Manufacturer Info		Comtech, Inc. 1001 S. Reilly	Road Suite #	639
Calculated Structured I structural adequacy o	Designs is responsible only f this component based	of the Handling	& Installation	ponding			Metsä Wood 301 Merritt 7 Bui l dir	ng, 2nd Floor	Fayetteville, N USA	IC	
responsibility of the consure the component	loadings shown. It i ustomer and/or the contra ant suitability of the in	s the 2.Refer t ctor to regarding tended fastening	o manufacturer's product inform i installation requirements, mu	ti-ply			Norwalk, CT 06851 (800) 622-5850		28314 910-864-TRU	S	
application, and to verif	fy the dimensions and loads	s. approvals 3. Damageo	Beams must not be used	and		,	www.metsawood.co	om/us			
1. Dry service condition	ons, unless noted otherwise ed with fire retardant or co	4. Design a 5. Provide	ssumes top edge is laterally restrained lateral support at bearing points to a splacement and rotation		alid until 5/24/20				CO	mt	есн
				i nis design is v	anu until 5/24/20	24					

TisDe	sign	Project:	aver Developmer I Shaw Road	nt		Date: Input by: Job Name	8/3/2021 David Landry :: Lot 77 Thomas Farm	Page 2 of 8
que	rto-S LVL	1.750" X ′		2-Ply -	PASSI	Project #:	J0721–4570 Level: Level	
						I		
	• •	· · ·	•	•	• •	•	<	1'4"
	<u> </u>		• 11'11"		<u> </u>	•		↓ ↓ ↓ 3 1/2"
1			11'11"				1	
Multi-Ply Anal Fasten all plies Capacity Load Yield Limit per Foot Yield Limit per Faste Yield Mode Edge Distance Load Combination Duration Factor	using 3 rows of 10 0.0 % 0.0 PL 245.6	F PLF	8x3") at 12" c	o.c Maximu	um end dis	stance no	ot to exceed 6".	
Notes Calculated Structured Desig structural adequacy of this design criteria and loat responsibility of the custom ensure the component s application, and to verify the	is is responsible only of the component based on the ings shown. It is the er and/or the contractor to uitability of the intended	chemicals ndling & Installation .VL beams must not be cut or a Refer to manufacturer's regarding installation requ astening details, beam streng approvals	lri∎ed product information irements, multi-ply	. For flat roofs prov ponding	ride proper drainag	e to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Comtech, Inc. 1001 S. Relly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
Lumber 1. Dry service conditions, u 2. LVL not to be treated wit	3. 4. [4.] 4. Second content of the second	Damaged Beams must not be u Design assumes top edge is lat Provide lateral support at bea ateral displacement and rotatio	erally restrained aring points to avoid	This design is	valid until 5/24/2	2024	ICC-ES: ESR-3633	соттесн

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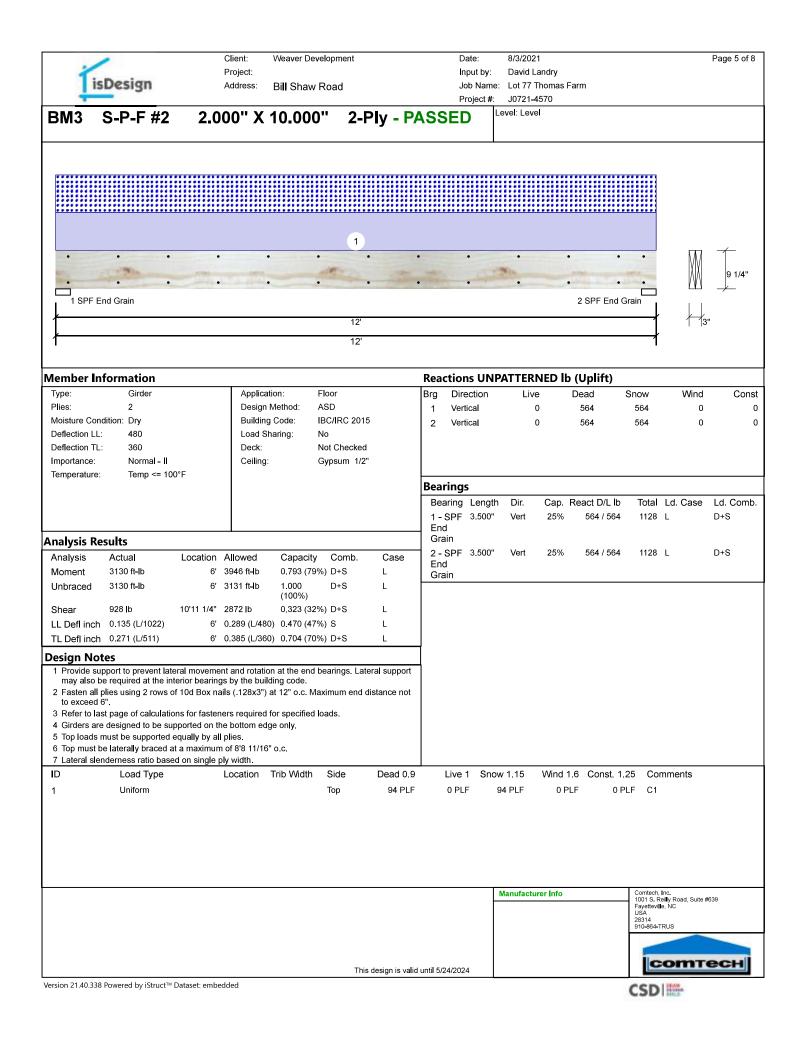


CSD

ŕ	isDesign		Client: Project: Address:	Weaver Developm Bill Shaw Road		In	ate: iput by:	8/3/2021 David Landry : Lot 77 Thomas Farm	Page 4 of 8
+						Р	roject #:	J0721-4570	
BM2	Kerto-S	LVL	1.750"	X 16.000"	2-Ply	- PASSEI	D l	Level: Level	
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	•	•	• •	• •	•	• •	•	• • • •	
	• •	•	•	• •	• •	•	•		
1 SPF	End Grain							2 SPF End	
					14'7 1/2"				3 1/2"
1				1	4'7 1/2"				1
Multi-Ply	/ Analysis								
Fasten all	-			s (.128x3") at 12"	o.c Maxim	um end dista	ince no	ot to exceed 6".	
Capacity Load		79.8 % 196.0							
Yield Limit pe Yield Limit pe		245.6 81.9 b							
Yield Mode	er Fasterier	IV							
Edge Distand		1 1/2"							
Min. End Dis Load Combir		3" D+L							
Duration Fac		1.00							
Notes		c	hemicals		6. For flat roofs pro	ovide proper drainage to	prevent	Manufacturer Info	Comtech, Inc. 1001 S. Relly Road, Suite #639
Calculated Struct structural adequi design criteria	tured Designs is responsible acy of this component ba and loadings shows the	e only of the Har ased on the 1. L It is the 2. F	ndling & Install VL beams must not b Refer to manufac	e cut or dri∎ed turer's product information	ponding	-	Ē	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	Fayetteville, NC USA 28314 910-864-TRUS
ensure the con application, and t	the customer and/or the o mponent suitability of th to verify the dimensions and	contractor to n ne intended fa loads. a	egarding installatio	on requirements, multi-ply im strength values, and code				(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service c 2. LVL not to be	onditions, unless noted othe e treated with fire retardant	erwise 5 F	Design assumes top e	dge is laterally restrained ort at bearing points to avoid	This design is	valid until 5/24/202		ICC-ES: ESR-3633	соттесн
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CSD



	1	Client:	Weaver Developm	ient		Date:	8/3/2021	Page 6 of 8
Í	isDesign	Project:				Input by:	David Landry Lot 77 Thomas Farm	
- +	Ispesign	Address:	Bill Shaw Road	1		Project #:	J0721-4570	
BM3	S-P-F #2	2.000" X	10.000"	2-Plv -	PASSE	D L	evel: Level	
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	F End Grain						2 SPF End G	
				10				
				12'				1 13"
				12'				I
	y Analysis							
Fasten all Capacity	plies using 2 rov	vs of 10d Box nails	(.128x3") at 12"	o.c Maxin	num end dis	tance no	t to exceed 6".	
Load		0.0 PLF						
Yield Limit pe Yield Limit pe		157.4 PLF 78.7 lb						
Yield Linit pe	er Fasterier	10.7 lb. IV						
Edge Distan		1 1/2"						
Min. End Dis Load Combir		3"						
Duration Fac		1.00						
						.—		
						F	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
								USA
								26314 910-864-TRUS
				This design i	s valid until 5/24/2	2024		соттесн
				5				

Ťi	sDesign	Client: Project Addres					e: 8/3/202 it by: David I Name: Lot 77	_andry	m			Page 7 of
-			Bill Onum				ect #: J0721-					
GDH	S-P-F #2	2.000"	X 12.00	0" 2-F	Ply - P/	ASSED	Level: Lev	el				
						3						
		2										
		2		1					_			
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	and the second			atter	-	-			-		ŇŇ	11 1
1 SPF	End Grain						2 SPF	End Grain				<u> </u>
			{	8'10"					┦		<i> </i> − 3	;"
ſ			٤	3'10"					イ			
lember li	nformation					Reactions	UNPATTER	NED Ib (Uplift)			
Type: Plies:	Girder 2		blication: sign Method:	Floor ASD		Brg Direct			ead S	now	Wind 0	Co
Moisture Co			Iding Code:	IBC/IRC 2015		1 Vertica 2 Vertica			751 751	88 88	0	
Deflection LI			ad Sharing:	No								
Deflection TI Importance:	L: 360 Normal – II	De		Not Checked Gypsum 1/2"								
Temperature			ling:	Gypsull 1/2								
i e i i p e i a i a i a	in internet					Bearings						
						Bearing L	-	Cap. Re	act D/L lb	Total L	.d. Case	Ld. Con
						1 - SPF 3 End	.500" Vert	19%	751 / 88	839 L		D+S
nalysis R	esults	•				Grain 2 - SPF 3	5.500" Vert	19%	751 / 88	839 L		D+S
Analysis		Location Allowe			Case	End	0.000 Ven	1970	751700	039 L		013
Moment Unbraced	1490 ft-lb 1490 ft-lb	4'5" 4153 ft 4'5" 3539 ft-	,		Uniform Uniform	Grain						
Shear	542 lb	1'2 3/4" 2734 lb	0.198 (20		Uniform							
LL Defl incl		4'5 1/16" 0.209 (,	L							
	(L/22622)		· · · · · · · · · · · · · · · · · · ·	, 								
	n 0.042 (L/2381)	4'5 1/16" 0.279 (_/360) 0.151 (15	5%) D+S	L	4						
esign No		-1	4-41			4						
	upport to prevent latera be required at the inte			i bearings. Late	ral support							
2 Fasten all to exceed	plies using 2 rows of	10d Box nails (.128	x3") at 12" o.c. N	laximum end di	istance not							
	ast page of calculation	s for fasteners requ	ired for specified	l loads.								
	re designed to be supp must be supported ec		n edge only.									
	be laterally braced at e											
7 Lateral sle	enderness ratio based			Side	Dood 0.0		Spow 1 15	Wind 1.6	Const 1.2	5 Comr	monto	
1D	Load Type Uniform	Locatio	n Trib Width	Side Top	Dead 0.9 60 PLF	Live 1 0 PLF	Snow 1.15 0 PLF	0 PLF	Const. 1.2 0 PL		nents	
2	Uniform			Тор	90 PLF	0 PLF	0 PLF	0 PLF	0 PL 0 PL			
2 3	Tie-In	0-0-0 to 8-10	-0 1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PS			
				P								
							Manufact	urer Info		Comtech, Inc 1001 S. Reilly Fayetteville, I	/ Road, Suite # NC	639
										USA 28314		
										910-864-TRL	15	
											отт	

Í	isDesign	Client: Project: Address:	Weaver Development Bill Shaw Road			8/3/2021 David Landry Lot 77 Thomas Farm	Page 8 of 8
GDH	S-P-F #2	2 000" X	12.000" 2-F		Project #:	J0721–4570 evel: Level	
GDH	3-F-F #2	2.000 A	12.000 2-1	-iy - FA33L			
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•	٠	• •	•	• •	•	•••	
	F End Grain					2 SPF End Grain	Λ ΄
			8'10"				∕ −− ′ 3"
1			8'10"			1	
Multi-Ply	/ Analysis						
	p l ies using 2 row		.128x3") at 12" o.c I	Maximum end dis	tance not	t to exceed 6".	
Capacity Load		0.0 % 0.0 PLF					
Yield Limit pe Yield Limit pe		157.4 PLF 78.7 lb.					
Yield Mode Edge Distand	ce	IV 1 1/2"					
Min. End Dist	tance	3"					
Load Combin Duration Fac		1.00					
						Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
							Fayetteville, NC USA 28314 910-864-TRUS
			This	design is valid until 5/24/2	024		соттесн



RE: J0721-4570

Lot 77 Thomas Farm

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Weaver Development Inc. Lot/Block: 77	Project Name: J0721-4570 Model: Hickory
Address:	Subdivision:
Citv:	State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.3 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 8 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7	Seal# E15727040 E15727041 E15727042 E15727043 E15727044 E15727045 E15727046	Truss Name ET1 ET2 ET3 F1 F2 F3 F4	Date 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021
7 8	E15727046 E15727047	F4 F5	8/3/2021 8/3/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters and project specific up parameters and project use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

August 03, 2021

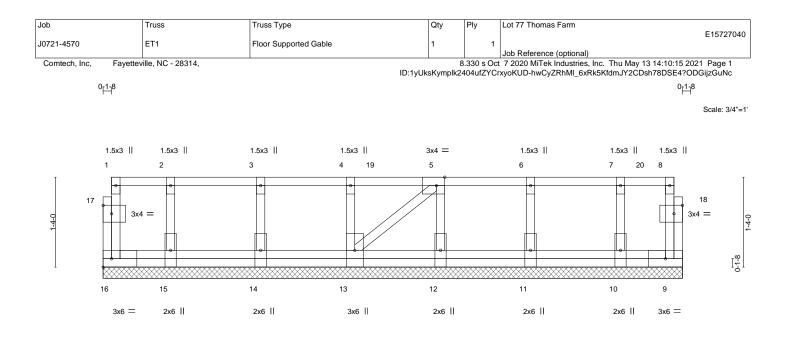


Plate Offsets (X,Y) [5:0-1-8,Edge], [17:0-1-8,0-1-8], [18:0-1	-8 0-1-81	8-7-0 8-7-0			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.00 WB 0.05 Matrix-P	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	- n/a 999 - n/a 999	PLATES MT20 Weight: 54 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	<i>y</i> 11	oc purlins,

REACTIONS. All bearings 8-7-0.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

OTHERS

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

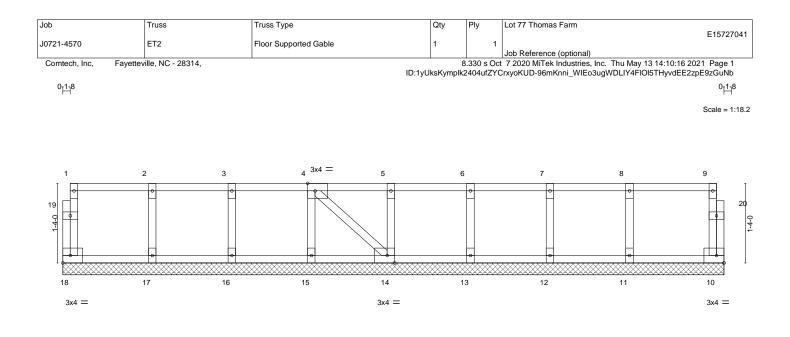
Uniform Loads (plf) Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb) Vert: 3=-71 6=-71 19=-71 20=-77





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			11-1-0 11-1-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [14:0-1-8,Edge]					
LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.01 WB 0.04 Matrix-S	DEFL. ii Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	a - n/a 999	PLATES MT20 Weight: 52 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or	, ,,	l oc purlins,	

REACTIONS.

DNS. All bearings 11-1-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

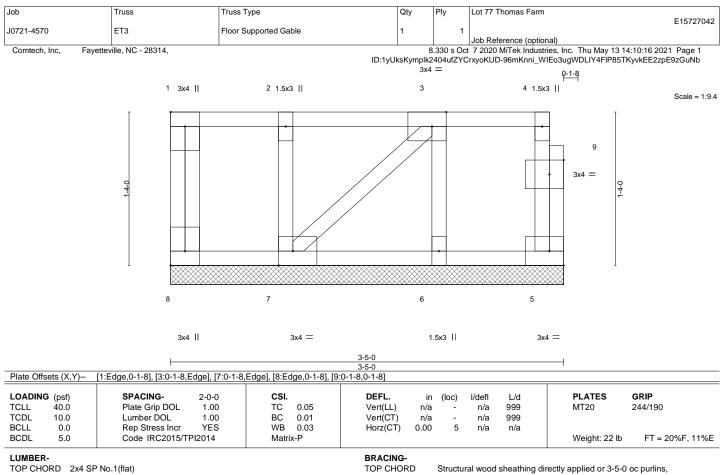
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



May 13,2021

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BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WEBS 2x4 SP No.3(flat) OTHERS

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

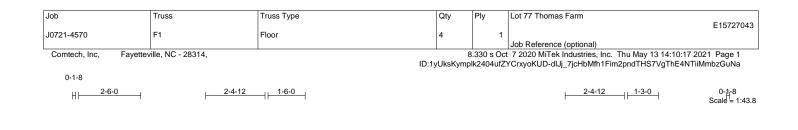
6) CAUTION, Do not erect truss backwards.

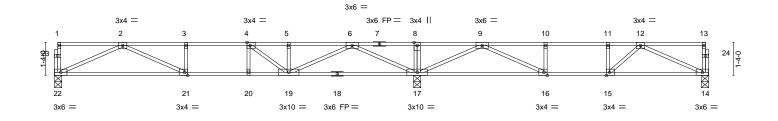




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H	14-7-12 14-7-12					<u>26-5-</u> 11-9-		
Plate Offsets (X,Y	[4:0-1-8,Edge], [15:0-1-8,Edge], [16:0-1	-8,Edge], [21:0-1-8,Edge]						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.55 BC 0.59 WB 0.53 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.17 21-22 -0.25 21-22 0.03 14	l/defl >999 >697 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 129 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2× WEBS 2× REACTIONS.	4 SP No.1(flat) 4 SP No.1(flat) 4 SP No.3(flat) (size) 22=0-3-8, 17=0-3-8, 14=0-3-8 ax Grav 22=728(LC 10), 17=1669(LC 1), 14	I=562(LC 7)	BRACING- TOP CHOR BOT CHOR	D Structu except D Rigid c	end vert	cals.	rectly applied or 6-0-0 d or 10-0-0 oc bracing, 1 7.	•
TOP CHORD BOT CHORD WEBS	Max. Comp /Max. Ten All forces 250 (lb) of 2-3=-1987/0, 3-4=-1987/0, 4-5=-1740/0, 5-6- 9-10=-1183/0, 10-11=-1183/0, 11-12=-1183/ 21-22=0/1314, 20-21=0/1987, 19-20=0/1987 15-16=0/1183, 14-15=0/954 3-17=-284/0, 2-22=-1440/0, 2-21=0/745, 3-2 1-19=-646/0, 9-17=-1465/0, 9-16=0/917, 10-	1740/0, 6-8=0/1282, 8-9= 0 ′, 17-19=-191/818, 16-17≕ 1=-260/0, 6-17=-1781/0, 6	=0/1282, -366/574, -19=0/1122,					
NOTES-								

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

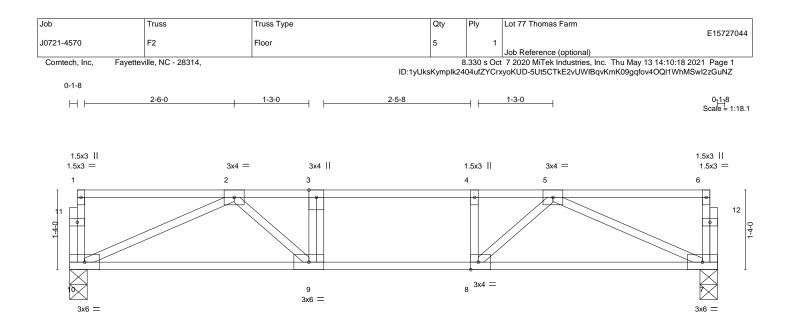
5) CAUTION, Do not erect truss backwards.





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<u> </u>			<u>10-10-0</u> 10-10-0			
Plate Offsets (X,Y)	[8:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.42 BC 0.35 WB 0.30 Matrix-S	DEFL. in Vert(LL) -0.08 Vert(CT) -0.11 Horz(CT) 0.02	3 9-10 >999 480 9-10 >999 360	PLATES MT20 Weight: 56 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied o	<i>y</i>) oc purlins,

(size) 10=0-3-8, 7=0-3-8 REACTIONS. Max Grav 10=576(LC 1), 7=576(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1234/0, 3-4=-1234/0, 4-5=-1234/0

BOT CHORD 9-10=0/981, 8-9=0/1234, 7-8=0/982 WEBS 2-10=-1073/0, 5-7=-1075/0, 5-8=0/485, 2-9=0/478, 3-9=-255/0, 4-8=-266/0

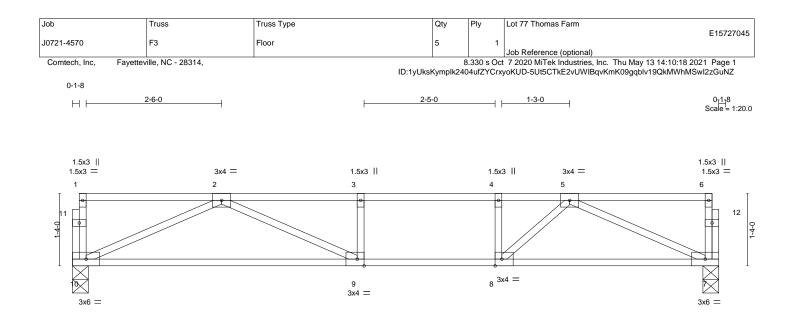
NOTES-

 Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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			11-11-0			
			11-11-0			
Plate Offsets (X,Y)	[8:0-1-8,Edge], [9:0-1-8,Edge]					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ii	n (loc) l/defl L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.68	Vert(LL) -0.19	9 9-10 >740 480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.56	Vert(CT) -0.29	9 9-10 >490 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.34	Horz(CT) 0.02	2 7 n/a n/a		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 59 lb	FT = 20%F, 11%E
LUMBER-			BRACING-			
TOP CHORD 2x4 SP	No.1(flat)		TOP CHORD	Structural wood sheathing dir	ectly applied or 6-0-0) oc purlins,
BOT CHORD 2x4 SP	No.1(flat)			except end verticals.		•
WEBS 2x4 SP	PNo.3(flat)		BOT CHORD	Rigid ceiling directly applied o	or 10-0-0 oc bracing.	
	-) 40.000 7.000		BOT CHOILD	Trigid centing directly applied o	i 10-0-0 oc bracing.	

11-11-0

REACTIONS. (size) 10=0-3-8, 7=0-3-8 Max Grav 10=635(LC 1), 7=635(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1508/0, 3-4=-1508/0, 4-5=-1508/0

BOT CHORD 9-10=0/1112, 8-9=0/1508, 7-8=0/1121

WEBS 2-10=-1219/0, 2-9=0/558, 5-7=-1228/0, 5-8=0/655, 4-8=-353/0

NOTES-

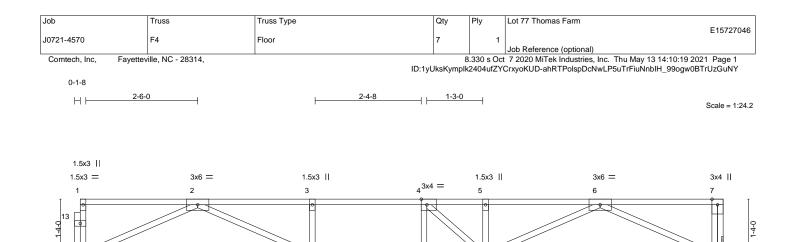
 Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





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10

1.5x3 ||

9

3x6 =

L			14-6-0			
			14-6-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [11:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.61 BC 0.84 WB 0.46 Matrix-S	DEFL. ir Vert(LL) -0.20 Vert(CT) -0.25 Horz(CT) 0.03	9-10 >838 480 9-10 >684 360	PLATES MT20 Weight: 73 lb	GRIP 244/190 FT = 20%F. 11%E
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP	2 No.1(flat) 2 No.1(flat) 2 No.3(flat)	Indux 0	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0	

REACTIONS. (size) 12=0-3-8, 8=Mechanical

Max Grav 12=778(LC 1), 8=784(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2243/0, 3-4=-2243/0, 4-5=-2186/0, 5-6=-2186/0

11-12=0/1424, 10-11=0/2243, 9-10=0/2243, 8-9=0/1429 BOT CHORD WEBS

2-12=-1561/0, 2-11=0/958, 3-11=-303/0, 6-8=-1573/0, 6-9=0/836, 5-9=-271/41, 4-9=-428/186

NOTES-

2

3x6 =

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

11

3x4 =

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



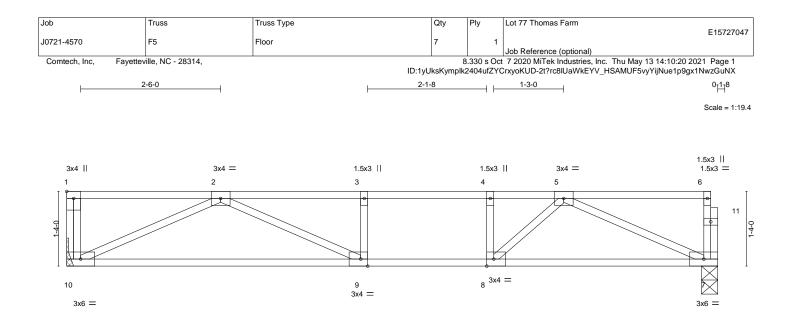


8

3x6 =

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			<u>11-7-8</u> 11-7-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8	,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.60 BC 0.51 WB 0.33 Matrix-S	DEFL. in Vert(LL) -0.16 Vert(CT) -0.25 Horz(CT) 0.02	9-10 >846 480 9-10 >540 360	PLATES MT20 Weight: 58 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	<i>y</i> 11) oc purlins,
REACTIONS. (size Max G	e) 10=Mechanical, 7=0-3-8 Grav 10=626(LC 1), 7=619(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1441/0, 3-4=-1441/0, 4-5=-1441/0

BOT CHORD 9-10=0/1081, 8-9=0/1441, 7-8=0/1087

WEBS 2-10=-1190/0, 2-9=0/515, 5-7=-1190/0, 5-8=0/606, 4-8=-323/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



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