# PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE **RESIDENTIAL BUILDING CODE.**









# LOT 77 THOMAS FARM **TBD BILL SHAW RD** SPRING LAKE, NC

### **GENERAL NOTES**

- 1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
- 2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.



TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE** 



C:\Users\Wade\Documents\Projects\Westan-Weaver\Hickory\Hickory\_8-25-20.dwg, 8/28/2020 3:18:50 PM



<u>SCALE NOTE:</u> 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE** 



## ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.

2.) VANITY LIGHTS TO BE SET @ 90" AFF. (TYP.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.

4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

## ELECTRICAL LEGEND

- ⊕ IIØ ∨ OUTLET
- 😑 110 V GFI OUTLET
- BB IIO V BASEBOARD OUTLET
- +++ 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GFI
- UEATHERPROOF
- € 22Ø ∨ OUTLET

- H SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- \_\_\_\_\_
- PENDANT LIGHT
- MINI CAN LIGHT
- FLUORESCENT LIGHT
- \$ SWITCH

- TELEPHONE AND DATA
- TV- TV CONNECTION

- 6P SPEAKER
- 110 V SMOKE/ CM DETECTOR
- 5D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM ALARM PANEL





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SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:
FIRST FLOOR ELECTRICAL PLAN
E-1

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE





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## ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.

2.) VANITY LIGHTS TO BE SET @ 90" AFF. (TYP.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.

4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

## ELECTRICAL LEGEND

- 😑 110 V GFI OUTLET
- BB IIO Y BASEBOARD OUTLET
- + 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 1107 GFI

- Ø 110 V DEDICATED CIRCUIT
- # 220 Y DEDICATED CIRCUIT
- ●H SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- -P- PENDANT LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT
- UNDERCABINET LIGHT

## 

- SWITCH \$
- \$<sub>D</sub> DIMMER SWITCH

- TELEPHONE AND DATA
- TY- TY CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- 6P SPEAKER
- Z 110 Y SMOKE/ CO DETECTOR
- 6D 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL

## ALARM PANEL

CEILING FAN W/ LIGHT



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LECTION S WEAVER HOME CAROLINA COLI HICKORY

DATE: AUGUST	25, 2020



SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE** 







- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2
- (UNO.) 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 4 (UNO).
- INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
- WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD DECULIDEMENTS
- REQUIREMENTS.SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
- FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

### **BRACE WALL PANEL NOTES:**

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**EXTERIOR WALLS:** ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

**REQUIRED LENGTH OF BRACING:** REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

**GYPSUM:** ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

**HD:** 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1







DATE: AUGUST 25, 2020
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

SECOND FLOOR FRAMING PLAN

S-2

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE





### TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEFT)	MAXIMUM STUD SPACING (INCHES) (PER TABLE R602.3(5)				
( ,	16	24			
UP TO 3'	1	1			
4'	2	1			
8'	3	2			
12'	5	3			
16'	6	4			
1					

## STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET

> SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE**



REV.:

S-3

SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY: **REVIEWED BY:** 

ATTIC FLOOR FRAMING PLAN



## ATTIC VENT CALCULATION:

1040 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 6.9 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

### STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE #2 SPF (UNO). 2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- 3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- 4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- 5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.









DATE:	AUGU	JST 25,	2020

EV.:
CALE: 1/4" = 1'-0"
DRAWN BY: WG
NGINEERED BY:
EVIEWED BY:

ROOF PLAN

S-4

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

## ATTIC VENT CALCULATION:

1116 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 7.4 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).





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	8.0.			retained reaction Signatu LO: Nu/ Signatu Signatu Nu/ Signatu Sig	I to design that exc are AD CH. (BASEI WSER OF C/ WSER	ART F( Dav ART F( ART F( AR	Dirt syste           Off.           Off.           OR JAC           DR JAC           State           St	In for all Indry Idry IK STU I 3 (5) I 8 E4 E40 I 34( 102 136 170	
A1GE	32'6"	45' 6"	Dimension Notes         1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise         2. All interior wall dimensions are to face of frame wall unless noted otherwise         3. all exterior wall to truss dimensions are to face of frame wall unless noted otherwise         3. All scherior wall dimensions are to face of frame wall unless noted otherwise         3. All exterior wall dimensions are to face of frame wall unless noted otherwise         0. All Koalls Shown Are Considered Load Bearing         Market Other Store         Roof Area = 1468.1 sq.ft.         Ridge Line = 52.07 ft.         Hip Line = 0 ft.         Horiz, OH = 985.57 ft.         Raked OH = 159.04 ft.         Decking = 50 sheets	Sanford / Harnett	Bill Shaw Road	Roof	08/3/21	David Landry	
			Drop BeamConnector InformationSymProductManufQtySupported MemberHeaderTrussHUS26USP4NA16d/3-1/2"16d/3-1/2"	CITY / CO.	ADDRESS	WODEL	DATE REV.	DRAWN BY	-
	21.0"		ProductsPlotIDLengthProductPliesNet QtyFab TypeBM112' 0"1-3/4"x 16" LVL Kerto-S22FFBM215' 0"1-3/4"x 16" LVL Kerto-S22FFBM312' 0"2x10 SPF No.222FFGDH12' 0"2x12 SPF No.222FFTruss Placement Plan $\int Truss Placement PlanScale: 1/4"=1'$	X Weaver Development Co. Inc.	ME lot 77 Thomas Farm	Hickory "A"	νте	#	
1				BUILDER	JOB NAA	PLAN	SEAL DA	QUOTE #	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards



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A5GE 1-4	32'6"	Dimension Notes         1. All exterior wall to wall cimensions are to face of frame wall unless noted therwise         2. All interior wall dimensions are to face of frame wall unless noted therwise         3. All exterior values noted to therwise         2. Considered Load Bearing         Tray Celling         Tray Celling         Drop Bear         Connector Information       Nail Information         Supported       Header       Truss         HUS26       USP       4       NA <th>CITY / CO. Spring Lake</th> <th>ADDRESS Bill Shaw Road</th> <th>MoDEL Reof 10500 10200 12750 15300 10200 15300</th> <th></th> <th>DRAWN BY David Landry David Landry David Landry</th> <th>SALES REP. Lenny Norris GALES REP. Lenny Norris GALES Rep. Lenny Norris</th>	CITY / CO. Spring Lake	ADDRESS Bill Shaw Road	MoDEL Reof 10500 10200 12750 15300 10200 15300		DRAWN BY David Landry David Landry David Landry	SALES REP. Lenny Norris GALES REP. Lenny Norris GALES Rep. Lenny Norris
M1GE 10	5:0"	Products           PlotID         Length         Product         Plies         Net Qty         Fab Type           BM1         12' 0"         1-3/4"x 16" LVL Kerto-S         2         2         FF           BM3         12' 0"         2x10 SPF No.2         2         2         FF           GDH         12' 0"         2x12 SPF No.2         2         2         FF           GDH         12' 0"         2x12 SPF No.2         2         2         FF	a BUILDER Weaver Development Co. Inc.	The sessal JOB NAME Lot 77 Thomas Farm	Hickory "A"	SEAL DATE	# 3LODO GRAM ON Vidual bu	JOB # JOP 10721-4569

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Trenco RE: J0721-4569 818 Soundside Rd Lot 77 Thomas Farm Edenton, NC 27932 Site Information: Customer: Weaver Development Inc. Project Name: J0721-4569 Lot/Block: 77 Model: Hickory Address: Subdivision: City: State: General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.3 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 25 individual, dated Truss Design Drawings and 0 Additional Drawings.

	0	<b>T</b> N	B. (		0 1//
INO.	Seal#	Truss Name	Date	INO.	Seal#
1	E15727048	A1	8/3/2021	21	E15727068
2	E15727049	A1GE	8/3/2021	22	E15727069
3	E15727050	A2	8/3/2021	23	E15727070
4	E15727051	A3	8/3/2021	24	E15727071
5	E15727052	A4	8/3/2021	25	E15727072
6	E15727053	A5	8/3/2021		
7	E15727054	A5GE	8/3/2021		
8	E15727055	B1	8/3/2021		
9	E15727056	B1GE	8/3/2021		
10	E15727057	B2	8/3/2021		
11	E15727058	C1	8/3/2021		
12	E15727059	C1GE	8/3/2021		
13	E15727060	D1-GR	8/3/2021		
14	E15727061	D1GE	8/3/2021		
15	E15727062	M1	8/3/2021		
16	E15727063	M1GE	8/3/2021		
17	E15727064	M2	8/3/2021		
18	E15727065	M2-GR	8/3/2021		
19	E15727066	V1GE	8/3/2021		
20	F15727067	V2	8/3/2021		

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



V3 V4

V5

V6

V7

Truss Name Date 8/3/2021 8/3/2021 8/3/2021 8/3/2021 8/3/2021

Gilbert, Eric

August 03, 2021



		10-2-8		12-0-0		10-2-8
LOADING	i (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.34 10-13	>999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.64	Vert(CT) -0.47 10-13	>824 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.27	Horz(CT) 0.05 8	n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05 2-13	>999 240	Weight: 208 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(LC 13)

Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at joint 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932



10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



May 14,2021

MARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTek® connectors. This skip included only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual Truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and provery damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





F	10-2-8 10-2-8	<u> </u>	19-2-8 21	<u>-1-8</u> <u>24-2-8</u> 11-0 <u>3-1-0</u>	30-1-	-8 32-5-0	4		
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0-4	-0,Edge], [12:0-2-0,0-1-4], [	14:0-4-8,0-1-4], [15:0	0-3-8,0-4-12]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.75 BC 0.67 WB 0.70 Matrix-S	<b>DEFL.</b> Vert(LL) -0.2 Vert(CT) -0.3 Horz(CT) 0.0 Wind(LL) 0.1	in (loc) l/defl 21 17 >999 38 17 >999 39 10 n/a 17 2-17 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 247 lb	<b>GRIP</b> 244/190 FT = 20%		
LUMBER- TOP CHORD     BRACING- TOP CHORD       BOT CHORD     2x6 SP No.1       BOT CHORD     2x10 SP No.1*Except* 10-15: 2x6 SP 2400F 2.0E       WEBS     2x4 SP No.2									
REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-90(LC 12), 10=-90(LC 13) Max Grav 2=1393(LC 2), 10=1353(LC 2)									
FORCES.       (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-4=-2217/403, 4-5=-1870/483, 7-9=-2258/519, 9-10=-2889/551         BOT CHORD       2-17=-193/1848, 15-17=-195/1860, 13-15=-371/2525, 10-13=-380/2525         WEBS       4-17=-29/402, 7-15=-114/967, 9-15=-1075/232, 9-13=0/616, 5-7=-1955/459									
NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10: Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;									

MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8, Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8, Ext

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



BIS Soundside Road Edenton, NC 27932



L	9-11-8	18-11-8	20-10-8	23-11-8	29-10-8	32-2-0				
	9-11-8	9-0-0	<u> </u>	<u>3-1-0</u>	5-11-0	2-3-8				
Plate Offsets (X, Y)	[1:0-9-6,0-1-2], [5:0-3-0,Edge], [9:0-4-0]	Eagej, [11:0-2-0,0-1-4], [1	3:0-4-4,0-1-4], [14:0-3-8,	,0-4-12]						
LOADING(psf)TCLL20.0TCDL10.0BCLL0.0BCDL10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.76 BC 0.66 WB 0.70 Matrix-S	DEFL.         in           Vert(LL)         -0.21           Vert(CT)         -0.36           Horz(CT)         0.09           Wind(LL)         0.16	(loc) l/defl 16 >999 16 >999 9 n/a 1-16 >999	L/d 360 240 n/a 240	<b>PLATES</b> MT20 Weight: 243 lb	<b>GRIP</b> 244/190 FT = 20%			
LUMBER- TOP CHORD     2x6 SP No.1       BOT CHORD     2x10 SP No.1 *Except* 9-14: 2x6 SP 2400F 2.0E       WEBS     2x4 SP No.2										
REACTIONS. (siz Max H Max U Max C	REACTIONS.         (size)         1=Mechanical, 9=0-3-8           Max Horz         1=-111(LC 8)           Max Uplift         1=-76(LC 12), 9=-90(LC 13)           Max Grav         1=1345(LC 2), 9=1347(LC 2)									
FORCES.       (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       1-3=-2189/401, 3-4=-1853/491, 6-8=-2237/517, 8-9=-2874/549         BOT CHORD       1-16=-198/1827, 14-16=-200/1839, 12-14=-375/2511, 9-12=-384/2511         WEBS       6-14=-117/966, 3-16=-53/392, 4-6=-1931/474, 8-14=-1081/228, 8-12=0/620										
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads have been considered for this design.</li> <li>2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide</li> </ul>										

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss evaluat and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ANSTPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







		9-11-8	1	12-0-0	1		10-2-8	
Plate Offs	sets (X,Y)	[1:0-1-14,0-1-8]						
LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (lo	oc)	l/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.34 9-	-12	>999 360	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.64	Vert(CT) -0.47 9-	-12	>822 240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.27	Horz(CT) 0.05	7	n/a n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05	12	>999 240	Weight: 204 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 7=0-3-8 Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13) Max Grav 1=1278(LC 1), 7=1331(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021

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<b>⊢</b>	<u> </u>	22-2-8		+	32-5-0 10-2-8	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           *         Rep Stress Incr         YES           Code         IRC2015/TPI2014	CSI.         DEFL.           TC         0.29         Vert(LL)           BC         0.65         Vert(CT)           WB         0.27         Horz(CT)           Matrix-S         Wind(LL)	in (loc) -0.34 9-12 -0.47 9-12 0.05 7 0.05 12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	PLATES         GRIP           MT20         244/15           Weight: 206 lb         FT =	90 - 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 WEBS

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Trus Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Max Horz 1=-175(LC 17) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except 37=-101(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22,

21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.



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BOT CHORD 1-7=0/307, 5-7=0/307

WEBS 3-7=0/277

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



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Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 12 except 15=-181(LC 12), 11=-169(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- Unbalanced roof live loads have been considered for this design.
   Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 12 except (it=lb) 15=181, 11=169.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.



May 14,2021

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818 Soundside Road Edenton, NC 27932

Design valid for use only with MTerk® connectors. This design is back into ALD DED will the REFERENCE FAGE MIF/473 few. 519/2020 beFVRE USE. Design valid for use only with MTerk® connectors. This design is backed only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Compon Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
					E15727057
J0721-4569	B2	ROOF SPECIAL	1	2	
				<b>_</b>	Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,		. 8	3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:02 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 14:16:02 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-1gg8O5u01bqia370U?Vh6D4VyZWi1Mz9oun7O6zGuIB

LOAD CASE(S) Standard
2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15. Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-2=-250, 3-4=-50, 4-7=-50, 6-10=-20
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
Unitorini Lodas (pii) Vert: 1-2220 3-420 4-720 6-1040
4) Dead + 0.6 C-C Wind (Pos, Internal) Case 1: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-156, 3-4=27, 4-12=35, 6-12=27, 6-7=20, 6-10=-12
Horz: 3-439, 4-12=47, 6-12=39, 6-7=32
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (of)
Vert: 1-2=-170. 3-4=35. 4-11=27. 6-11=35. 6-7=58. 6-10=-12
Horz: 3-4=-47, 4-11=39, 6-11=47, 6-7=70
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=-51, 6-10=-20
7) Dead + 0.6 C-C Wind (Neq. Internal) Case 2: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=11, 6-10=-20
Horz: 3-4=38, 4-6=-38, 6-7=31
<li>bead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (of)</li>
Vert: 1-2=-179. 3-4=-13. 4-6=11. 6-7=4. 6-10=-12
Horz: 3-4=1, 4-6=23, 6-7=16
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-191, 3-4=11, 4-b=-13, b-/=2, b-10=-12
10) Dead + 0.6 MWFRS Wind (Neo, Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-201, 3-4=-35, 4-6=-11, 6-7=-4, 6-10=-20
Horz: 3-4=15, 4-6=9, 6-7=16
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-213. 3-4=-11. 4-6=-35. 6-7=-28. 6-10=-20
Horz: 3-4=-9, 4-6=-15, 6-7=-8
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-1/9, 3-4=21, 4-b=9, 6-7=2, 6-10=-12
13) Dead + 0.6 MWFRS Wind (Pos, Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12
Horz: 3-421, 4-6-33, 6-7-26
14) Dead + 0.6 MWFRS Wind (Pos. Internal) and Parallel: Lumber increase=1.60, Plate increase=1.60
Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12
Horz: 3-4=-33, 4-6=21, 6-7=14
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert. $1-2=191$ , $3-4=34$ , $4-6=21$ , $6-7=14$ , $6-10=12$ Hore: $3-4=-21$ $4-6=33$ $6-7=26$
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-201, 3-4=-1, 4-6=-13, 6-7=-6, 6-10=-20
Horz: 3-419, 4-6=7, 6-7=14
17) Dead + 0.0 MWFRS Wild (Neg. Internal) 2nd Paraleli. Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (off)
Vert: 1-2=-213, 3-4=-13, 4-6=-1, 6-7=6, 6-10=-20
Horz: 3-4=-7, 4-6=19, 6-7=26
18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
Uniform Loads (pit)
ven. i-z=-zzu, o-4=-zu, d-7=-zu, o-10=-zu 19) Dead + 0.75 Roof Live (bal) + 0.75(0.6 MWERS Wind (Neg. Int) Left): Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-236, 3-4=-61, 4-6=-43, 6-7=-38, 6-10=-20
Horz: 3-4=11, 4-6=7, 6-7=12
20) Dead + 0.75 KOOT LIVE (bal.) + 0.75(0.6 MWFKS WIND (Neg. Int) Kight): Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=-245, 3-4=-43, 4-6=-61, 6-7=-56, 6-10=-20
Horz: 3-4=-7, 4-6=-11, 6-7=-6

21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

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Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
					E15727057
J0721-4569	B2	ROOF SPECIAL	1	2	
				<b>_</b>	Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,			3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:02 2021 Page 3

ID:1yUksKymplk2404ufZYCrxyoKUD-1gg8O5u01bqia370U?Vh6D4VyZWi1Mz9oun7O6zGulB

#### LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-36, 4-6=-45, 6-7=-40, 6-10=-20 Horz: 3-4=-14, 4-6=5, 6-7=10

22) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

- Uniform Loads (plf)
  - Vert: 1-2=-245, 3-4=-45, 4-6=-36, 6-7=-31, 6-10=-20 Horz: 3-4=-5, 4-6=14, 6-7=19
- 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
  - Uniform Loads (plf) Vert: 1-2=-260, 3-4=-60, 4-7=-20, 6-10=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
  - Vert: 1-2=-220, 3-4=-20, 4-7=-60, 6-10=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
  - Vert: 1-2=-250, 3-4=-50, 4-7=-20, 6-10=-20
- 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-50, 6-10=-20

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			000				
			8-3-8				
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]						
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ir	n (loc) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.84	Vert(LL) -0.05	2-5 >999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.24	Vert(CT) -0.10	2-5 >969	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00	) 5 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.10	2-5 >886	240	Weight: 37 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x4 SF	P No.1		TOP CHORD	Structural wood	I sheathing dir	ectly applied or 5-3-4	oc purlins,
BOT CHORD 2x6 SF	P No.1			except end vert	icals.		
WEBS 2x4 SF	P No.2		BOT CHORD	Rigid ceiling dir	ectly applied o	or 10-0-0 oc bracing.	
OTHERS 2x6 SF	P No.1						

8-3-8

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- NOTES 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			8-3-8		
			8-3-8		
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [12:0-2-0,0-1-8]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	DEFL.         iii           Vert(LL)         0.09           Vert(CT)         -0.08           Horz(CT)         -0.00	n (loc) I/defl L/d 9 10-11 >999 240 8 10-11 >999 240 0 8 n/a n/a	PLATES         GRIP           MT20         244/190           M18SHS         244/190           Weight: 41 lb         FT = 20%
LUMBER- TOP CHORD 2x4 Sf BOT CHORD 2x6 Sf WEBS 2x4 Sf OTHERS 2x4 Sf 8-12: 2	<ul> <li>No.1</li> <li>No.1</li> <li>No.2</li> <li>No.2 *Except*</li> <li>2x6 SP No.1</li> </ul>		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8) Max Uplift 2=-216(LC 8), 8=-188(LC 8) Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.



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818 Soundside Road Edenton, NC 27932





				3-11	-8	1	3	-11-8		1		
Plate Off	sets (X,Y)	[1:0-0-4,0-0-5], [1:0-0-8,0-3-	6], [3:0-0-4,	0-0-5], [3:0-0	-8,0-3-6]							
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.02	3-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.57	Vert(CT)	-0.04	3-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.39	Horz(CT)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2	014	Matrix	-P	Wind(LL)	0.01	3-4	>999	240	Weight: 100 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No 2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz	1=91(LC 24)
Max Uplift	1=-191(LC 8), 3=-180(LC 9)
Max Grav	1=2919(LC 1), 3=2779(LC 2)

FORCES. (lb) -	Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.
TOP CHORD	1-2=-2418/177, 2-3=-2418/177
BOT CHORD	1-4=-100/1678, 3-4=-100/1678
WEBS	2-4=-154/3142

#### NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
- Top chords connected as follows: 2x6 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 1-3=-20

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1-0-3

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021



Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm	
10704 4500	24.02				E15727060	
J0721-4569	D1-GR	Common Girder	1	2		
				-	Job Reference (optional)	
Comtech, Inc, Fay	etteville, NC - 28314,			3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:05 2021 Page 2	
		ID:1yUksKymplk2404ufZYCrxyoKUD-RFLG16xuKWCHRWsb982OkrizsnSDEfZbUr?n_RzGul8				

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)

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BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No 2

WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. All bearings 7-11-0.

(lb) - Max Horz 2=-118(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152.8=148.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021

MARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTE k® connectors. This sket on LCULCUL MIT IN EXPECTED EXECT PAGE MIT-F473 (eV. 319/2/20 BEFORE USE. Design valid for use only with MTE k® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (nsf) SPACING- 2-0-0 CSI DEFL in (loc) l/defl L/d PLATES GRIP			5-0-0	
TCLL         20.0         Plate Grip DOL         1.15         TC         0.28         Vert(LL)         -0.01         2-4         >999         360         MT20         244/190           TCDL         10.0         Lumber DOL         1.15         BC         0.08         Vert(CT)         -0.01         2-4         >999         360         MT20         244/190           BCLL         0.0*         Rep Stress Incr         YES         WB         0.00         Horz(CT)         0.00         n/a         n/a           BCDL         10.0         Code IRC2015/TPI2014         Matrix-P         Wind(LL)         0.01         2-4         >999         240         Weight: 24 lb         FT = 20%	LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	NG (psf)         SPACING- 20.0         2-0-0         CSI.           20.0         Plate Grip DOL         1.15         TC         0.28           10.0         Lumber DOL         1.15         BC         0.08           0.0         *         Rep Stress Incr         YES         WB         0.00           10.0         Code IRC2015/TPI2014         Matrix-P	DEFL.         in         (loc)         l/defl         I           Vert(LL)         -0.01         2-4         >999         3           Vert(CT)         -0.01         2-4         >999         2           Horz(CT)         0.00         n/a         n           Wind(LL)         0.01         2-4         >999         2	L/d <b>PLATES GRIP</b> 660 MT20 244/190 7/a 140 Weight: 24 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

**REACTIONS.** (size) 2=0-3-0, 4=0-1-8

Max Horz 2=63(LC 8)

Max Uplift 2=-102(LC 8), 4=-79(LC 8) Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			· · · · · · · · · · · · · · · · · · ·		5-0-0			
LOADING (psf) TCLL         SPACING-         2-0-0 Plate Grip DOL         CSI.         DEFL.         in         (loc)         //defl         L/d         PLATES         GRIP           TCLL         20.0         Plate Grip DOL         1.15         TC         0.09         Vert(LL)         0.01         8         >999         240         MT20         244/190           TCDL         10.0         Lumber DOL         1.15         BC         0.09         Vert(CT)         -0.01         8         >999         240         MT20         244/190           BCLL         0.0 *         Rep Stress Incr         YES         WB         0.02         Horz(CT)         -0.00         6         n/a         n/a           BCDL         10.0         Code IRC2015/TPI2014         Matrix-S         WE         Verticet in the second in	DADING         (psf)           CLL         20.0           CDL         10.0           CLL         0.0 *           CDL         10.0	LOADING (P TCLL 20 TCDL 10 BCLL 0 BCDL 10	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.09 BC 0.09 WB 0.02 Matrix-S	DEFL.         in         (loc)         I           Vert(LL)         0.01         8         >           Vert(CT)         -0.01         8         >           Horz(CT)         -0.00         6	l/defl L/d >999 240 >999 240 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 27 lb         FT = 20%	

#### LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8)

Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147.6=115.



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Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
					E15727064
J0721-4569	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:07 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 14:16:07 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-NeT1Roy9s8S?gq?zHY5spGnLYaDpif4uy9Uu3JzGul6

LOAD CASE(S) Standard
Concentrated Loads (lb) Vert: 9=-350
<ol> <li>Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)</li> </ol>
Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40 Concentrated Loads (lb)
Vert: 9=-300 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60 Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=12, 2-8=52, 8-10=115, 7-10=52 Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
Concentrated Loads (lb)
Vert: 9=548 5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52 Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
Concentrated Loads (lb)
Vert: 9=566 6) Dead + 0.6 C-C Wind (Nec. Internal) Case 1: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9 Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420 7) Dead + 0.6 C-C Wind (Nec. Internal) Case 2: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9 Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420 8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10 Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
Concentrated Loads (lb)
vert: 9=154 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
Concentrated Loads (lb)
vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Horz: 1-2=-0, 2-3=-1, 3-4=-0, 3-5=-33, 2-6=2, 8-10=23, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
Concentrated Loads (lb)
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
Concentrated Loads (lb)
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
Concentrated Loads (lb)
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
Concentrated Loads (lb)
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
Concentrated Loads (lb)
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60

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Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
					E15727064
J0721-4569	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:07 2021 Page 3

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 14:16:07 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-NeT1Roy9s8S?gq?zHY5spGnLYaDpif4uy9Uu3JzGul6

LOAD CASE(S) Standard
Uniform Loads (plf)
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
Concentrated Loads (b)
Vert. 9=43
bed + 0.0 WWFR3 wind (Neg. Internal) ist Falallet. Lumber inclease=1.00, Flate inclease=1.00
Unition Loads (pin)
Volt. $(-2-0, -2-0, -1, -3-0, -2, -3, -2, -2, -2, -2, -2, -2, -2, -2, -2, -2$
Vert 9=-234
17) Dead + 0.6 MWERS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (blf)
Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20
Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
Concentrated Loads (lb)
Vert: 9=-234
18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20
Concentrated Loads (lb)
19) Dead + 0.75 Root Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Lett): Lumber increase=1.60, Plate increase=1.60
Vell. 1-2=-31, 2-3=-30, 3-4=-31, 5-3==33, 6-3=-123, 2-6=-3, 6-10=13, 7-10=-3 Hore: 1.2=-10, 2.3=-14, 2.4=-10, 2.5=-26
Concentrated I oads (Ib)
Vert 9-454
20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20
Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
Concentrated Loads (lb)
Vert: 9=-375
21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20
Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
Vert: 9=-3/5 22) Devel: 9=-5/5 22) Devel: 9 - 5 - 5 - 5 - 5 - 5 - 5 - 5 - 5 - 5 -
22) Deal + 0.75 Noti Lite (bal.) + 0.75 Auto Floor + 0.75(0.5 WWFKS Wind (Neg. Int) 2nd Faralel). Lumber increase = 1.00, Flate increase = 1.00
Vert 1-2-40 2-345 3-440 5-986 6-9116 2-720
Horr: 1-210 2-35 3-410 3-50
Concentrated Loads (b)
Vert: 9=-375
23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20
Concentrated Loads (lb)
Vert: 9=-400
24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (pif)
Vert. 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20
Concentrated Loads (lb)
Vell. 9=-400 26) 2rd Dood + 0.75 Roof Live (upbelgaged): Lumber bergage-1.15 Root bergage-1.15
20) Sid Dead + 0.75 Kool Live (unbalanceu): Lumber increase=1.15, Frate increase=1.15
Vert 1:3-50 3-4=-50 5-6=-100 2-7=-20
Concentrated Loads (Ib)
Vert 9=-350
26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20
Concentrated Loads (lb)
Vert: 9=-350

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm	_
					E1572706	5
J0721-4569	M2-GR	HALF HIP	1	2		
				<b>_</b>	Job Reference (optional)	
Comtech, Inc, Fayettev	rille, NC - 28314,		. 8	3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:08 2021 Page 2	

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	Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20
	Concentrated Loads (lb)
3)	Vert: 9=-350 Dead + Uninhabitable Attic Without Storage: Lumber Increase-1 25, Plate Increase-1 25
3)	Uniform Loads (olf)
	Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40
	Concentrated Loads (lb)
	Vert: 9=-300
4)	Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pir)
	Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
	Concentrated Loads (lb)
	Vert: 9=548
5)	Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pir)
	Horz: 1-2=-63. 2-3=-70. 3-4=-63. 3-5=-55
	Concentrated Loads (lb)
	Vert: 9=566
6)	Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pir)
	Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
	Concentrated Loads (lb)
	Vert: 9=-420
7)	Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pir)
	Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
	Concentrated Loads (lb)
	Vert: 9=-420
8)	Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Vert 1-2=36 2-3=21 3-4=14 5-6=-131 2-8=10 8-10=33 7-10=10
	Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
	Concentrated Loads (lb)
2	Vert: 9=154
9)	Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12
	Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
	Concentrated Loads (lb)
40	Vert: 9=43
10	) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (olf)
	Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
	Concentrated Loads (lb)
11	Vert: 9=-339 ) Dead + 0.6 MW/ERS Wind (Neg. Internal) Right: Lumber Increase-1.60. Plate Increase-1.60.
	Uniform Loads (plf)
	Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20
	Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
	Concentrated Loads (lb)
12	Vell. 9=-234 ) Dead + 0.6 MW/ERS Wind (Pos. Internal) 1st Parallel: Lumber Increase-1.60. Plate Increase-1.60
12	Uniform Loads (plf)
	Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
	Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
	Concentrated Loads (lb)
13	) Dead + 0.6 MWERS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase=1.60
. 5	Uniform Loads (plf)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
	Vert: 9=43
14	) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60

2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-400

Uniform Loads (plf)

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Job	Truss	Truss Type	Qty	Ply	Lot 77 Thomas Farm
					E15727065
J0721-4569	M2-GR	HALF HIP	1	2	leb Deference (entionel)
					Job Reference (optional)
Comtech, Inc, Faye	tteville, NC - 28314,			3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 14:16:08 2021 Page 3

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<ul> <li>Unitom Loads (pif)</li> <li>Vert: 1-2=-1, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb)</li> <li>Vert: 9=43</li> <li>15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60</li> <li>Uniform Loads (plf)</li> <li>Vert: 1-2=-2, 2-3=-9, 3-4=2, 5-6=-119, 2-7=-12</li> <li>Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb)</li> <li>Vert: 9=43</li> <li>16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60</li> <li>Uniform Loads (plf)</li> <li>Vert: 9=43</li> <li>16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60</li> <li>Uniform Loads (plf)</li> <li>Vert: 1-2=-6, 2-3=-19, 3-4=-26, 3-5=-12</li> <li>Concentrated Loads (lb)</li> <li>Vert: 9=-234</li> <li>17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60</li> <li>Uniform Loads (plf)</li> <li>Vert: 9=-234</li> <li>17) Dead + 0.6 MWFRS Vind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60</li> <li>Uniform Loads (plf)</li> </ul>
Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Ver: 9=43 15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-1, 3-4=-6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 9=-234
<ul> <li>Vert: 9=43</li> <li>15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)</li> <li>Vert: 1-2=, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb)</li> <li>Vert: 9=43</li> <li>16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60</li> <li>Uniform Loads (plf)</li> <li>Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12</li> <li>Concentrated Loads (lb)</li> <li>Vert: 9=-234</li> <li>17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60</li> <li>Uniform Loads (plf)</li> </ul>
<ul> <li>13) Dead + 0.6 MWFRS Wind (POS. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43</li> <li>16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234</li> <li>17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)</li> </ul>
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Concentrated Loads (lb) Vert: 9=43 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
<ul> <li>Vert: 9=43</li> <li>16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234</li> <li>17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)</li> </ul>
Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Concentrated Loads (b) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20
Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 Concentrated Loads (lb)
Vert: 9=-234
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20 Concentrated Loads (lb)
Vert: 9=-200 Yest:
Uniform Loads (plf)
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
Concentrated Loads (lb)
20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20
Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
Vert: 9=-375
21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (olf)
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20
Concentrated Loads (lb)
Vert: 9=-375 22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-40, z-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, z-7=-20 Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
Concentrated Loads (lb) Vert: 9=-375
23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20
Concentrated Loads (lb)
24) 2nd Dead H of Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20
Concentrated Loads (lb)
25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
Unitorm Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20
Concentrated Loads (lb)
26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20
Concentrated Loads (lb)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **AVSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601











3x4 =

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Plate Offsets (X,Y)	[7:0-3-11,Edge], [9:0-1-6,0-1-0], [10:0-2-	-0,0-0-10]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) n/a -	n/a 999	MT20 244/190
BCLL 0.0 *	Rep Stress Incr YES	WB 0.08 Matrix-S	Horz(CT) 0.00 7	n/a n/a	Weight: 75 lb FT - 20%
		Marix C	BRACING-		

#### LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No 2 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12), 8=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-295/189

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.



May 14,2021

MARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design values for use only with MTek® connectors. This should be used only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracking indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Compon Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 14-10-0.

(lb) - Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=-338/247, 4-6=-338/247
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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135, 6=135.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

**REACTIONS.** All bearings 12-5-4.

(lb) - Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-312/241. 4-6=-312/241

WEBS 2-8=-312/241, 4-6=-312/241

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in to the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Componer Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





**REACTIONS.** (size) 1=10-0-7, 3=10-0-7, 4=10-0-7

Max Horz 1=-92(LC 8)

Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

**REACTIONS.** (size) 1=7-7-10, 3=7-7-10, 4=7-7-10

Max Horz 1=68(LC 9)

Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13

Max Horz 1=-44(LC 8)

Max Uplift 1=-15(LC 13), 3=-19(LC 13)

Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 5-2-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 14,2021

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3x4 🥢

3x4 📎

Plate Offsets (X,Y)	2-10-0 Plate Offsets (X,Y) [2:0-2-0,Edge]							
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.01 BC 0.03 WB 0.00 Matrix-P	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) l/defl a - n/a a - n/a D 3 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 8 lb	<b>GRIP</b> 244/190 FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

2-10-0

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

**REACTIONS.** (size) 1=2-10-0, 3=2-10-0

Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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	Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Signature									
	LOr	AD CH	ART FC	DR JA	K STU	IDS				
	Not (pt an) NG (pt an) 1700 3400 5100 6800 8500 10200 11900 13600 15300	1945 1957 1967 1967 1967 1977 1 2 3 4 5 6 7 8 9	ده در ۲۵۵۵ ۱۹۹۸ - ۲۹۹۹ ۱۹۹۹ - ۲۹۹۹ ۱۹۹۹ ۱۹۹۹ - ۲۹۹۹ ۱۹۹۹ - ۲۹۹۹ ۱۹۹۹ ۱۹۹۹ ۱۹۹۹ ۱۹۹۹ ۱۹۹۹ - ۲۹۹۹ ۱۹۹۹ ۱۹۹۹ - ۲۹۹۹ ۱۹۹۹ ۱۹۹۹ ۱۹۹۹ ۱۹۹۹ ۱۹۹۹ ۱۹۹۹ ۱۹۹۹	- 1 - 2 - 3 - 4 - 5 - 6	(13 (6) ≥ 54 END S S S S S S S S S S S S	ог но записа (к) но 2 00 3 00 4 00 5				
	<ol> <li>Spring Lake</li> </ol>	Bill Shaw Road	Floor	V. 08/03/21	3y David Landry	EP. Lenny Norris				
	ITY / C(	DDRESS	NODEL	ATERE	RAWN E	ALES RE				
	BUILDER Weaver Development Co. Inc.	JOB NAME Lot 77 Thomas Farm	PLAN Hickory "A" M	SEAL DATE Seal Date D.	QUOTE # Quote # DI	<b>JOB #</b> J0721-4570 S.				
3)	THIS IS These to comport design See indi identified designed perman for the support and col designed consult truss d	A TRUSS russes an nents to b at the sp lividual de ded on the er is resp ent braci overall st t structum umns is f er. For ge BCSI-B1 elivery pa	S PLACEN re designe pe incorpt ecification essign she placemen placemen onsible fo ng of the ructure. T e includin the respoo neral guid and BCS ickage or	IENT DIA ed as indi orated int n of the b ets for ea ets for ea et	GRAM ON vidual buo o the buil uilding de ch truss g. The buo ary and floor syst n of the to s, beams, s, beams, f the buil arding br ided with sbcindus	ILY. ilding ding esigner. design ilding ilding acem and russ walls, ding acing, the stry.com				

Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

## All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.

Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Info	ormation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
$\bigcirc$	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

	Products									
PI	otID	Length	Product	Plies	Net Qty	Fab Type				
BI	<b>V</b> 1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF				
BI	<b>V</b> 12	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF				
BI	<b>V</b> I3	12' 0"	2x10 SPF No.2	2	2	FF				
G	DH	12' 0"	2x12 SPF No.2	2	2	FF				

Truss Placement Plan Scale: 1/4"=1'





-	I TR Re F	ROC RUS eilly Re Fayett Phone Fax:	OF & SES teville e: (910)	<b>C</b> FL( & FL( & B ndustr , N.C.: ) 864-4	OOF EAN 28309 -8787 444	₹ NS rk
	Bearing ra leemed to equireme ttached T equireme ize and r eactions 5000#. A tetained to eaction to ables. A etained to eactions	eactions o comply ents. The Tables ( ) ents ) to o number o greater t register o design that exce o design that exce	less thar y with the e contract derived f determin of wood s than 3000 red design the supp red stoss ed design the supp red stoss Davi Davi	n or equal p prescrip tor shall r rom the p e the min studs requires with but not n profess port syste e specifie off. d La DR JAC	to 3000# tive Code efer to the rescriptivi imum foou ired to s greater to ional shal m for an d in the a onal shal m for all Mode ndry	f are y ec Code indation iupport than ill be y attached Il be Y JD S
	NUM9 NUM9 NUM9 NUM9 NUM9 NUM9 NUM9 NUM9	(BASED) (BASED) SER OF CA: 2 2 3 4 5 6 7 8 9	2550 12750 15300	SR REQUIRE           SR	2011 201 20	20 20 20 20 20 20 20 20 20 20
is       is	Spring Lake	Bill Shaw Road	Floor	08/03/21	David Landry	Lenny Norris
All Walls Shown Are Considered Load Bearing	CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
Image: Decations prior to setting Floor Trusses.         3. Adjust spacing as needed not to exceed 24"oc.         Connector Information         Sym         Product       Manuf         Qty       Supported         Member       Header         Truss         HUS410       USP         Image: Product       Manuf         Official equation         Product       Manuf         Other product       Header         Truss         HUS410       USP         Image: Product product       Pleis         Net Qty       Fab Type         BM1       12'0"         HUX       16"/VL Kerto-S       2         PlotID       Length       Product         Plot       Nail       16"/VL Kerto-S       2         BM2       15'0"       1-3/4"x 16" LVL Kerto-S       2       FF         BM3       12'0"       2x10 SPF No.2       2       FF	Weaver Development Co. Inc.	Lot 77 Thomas Farm	Hickory "A"	Seal Date	Quote #	J0721-4570
GDH         12' 0"         2x12 SPF No.2         2         2         FF           -         1         Truss Placement Plan         -	BUILDER THIS IS 1 These tru	<b>JOB NAME</b>	<b>B</b> PLACEM re designed	SEAL DATE	# HOUTE #	# <b>BO</b> NLY. uilding

= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

1	/	C	lient: V roject:	Veaver De	velopment			Date Inpu	e: ut by:	8/3/2021 David La	l andry				Page 1 of 8
1	sDesign	A	ddress: E	Bi <b>ll</b> Shaw	Road			Job	Name:	Lot 77 T	homas F	arm			
BM1	Kerto-SI\	// 1.7	750" X	16.0	00" 2-	-Plv - P	AS	SFD	L	evel: Leve	570 I				
						- <b>- y</b> - ·									
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		2		1	)										_/_
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	- The	-		•	ation	•	•	•		Nin				ĂЙ	1'4"
•	• •		• 114 S	•	• ****•	•	1.1	•	•	•	•				
1 SPF										2 SI					
]				11'11										1 13	8 1/2"
1				11'1'	1''						1				
Member I	nformation						Rea	ctions	UNP	ATTERN	NED Ib	(Uplift	)		
Туре:	Girder		Applicatio	n:	Floor		Brg	Direc	tion	Live	•	Dead	Snow	Wind	Const
Plies: Moisture Co	2 ndition: Drv		Design M Buildina (	ethod: Code:	ASD IBC/IRC 201	5	1	Vertic	al	0		2869 2869	2079 2079	0	0
Deflection L	L: 480		Load Sha	ring:	No			ventica	a	U		2009	2079	0	U
Deflection T	L: 360		Deck:		Not Checked										
Temperature	Normal - II e: Temp <= 100	)°F	Celling:		Gypsum 1/2										
	·						Bea	rings							
							Be	aring L	ength	Dir. Vort	Cap.	React D/L	. lb Tota	Ld. Case	Ld. Comb.
							2-	SPF 3	8.500" 8.500"	Vert	95% 95%	2869 / 20	)79 4948 )79 4948	L L	D+S D+S
Analysis R	esults	Leastion A	llowed	Canaaita	( Camb		1								
Moment	13679 ft-lb	5'11 1/2" 3	9750 ft-lb	0.344 (34	y Comb. 1%) D+S	L									
Unbraced	13679 ft <b>-I</b> b	5'11 1/2" 13	3699 ft <b>-l</b> b	0.999	D+S	L									
Shear	3615 lb	1'7 1/2" 1:	3739 lb	0.263 (26	6%) D+S	L									
LL Defl inc	h 0.069 (L/2000)	5'11 1/2" 0.	.287 (L/480)	0.240 (24	4%) S	L									
TL Defl inc	h 0.164 (L/840)	5'11 1/2" 0.	.383 (L/360)	0.428 (43	3%) D+S	L	-								
Design No	otes	aral movement	and rotation	at the end	bearings Lat	eral support	4								
may also	be required at the int	erior bearings l	by the buildir	ng code.											
2 Fasten al to exceed	l plies using 3 rows o 1 6".	f 10d Box halls	(.128x3°) at	12" O.C. IV	iaximum end o	distance not									
3 Refer to 4 Girders a	ast page of calculatio re designed to be sur	ns for fasteners	s required fo bottom edae	r specified	loads.										
5 Top loads	must be supported e	equally by all pl	ies.	,-											
7 Lateral sl	enderness ratio base	d on single ply	width.	•											
ID	Load Type	Lo	ocation T	rib Width	Side	Dead 0.9		Live 1	Snov	v 1.15	Wind 1	.6 Cons	t. 1.25 Co	omments	
1	Uniform				Тор —	120 PLF		0 PLF		0 PLF	0 P	_F	0 PLF Wa	all	
2	Uniform				Гор	349 PLF		0 PLF	34	19 PLF	0 P	_F	0 PLF A2		
	Sell Weight														
Notes		chemicals	5		6. For 1	flat roofs provide p	roper dra	nage to pr	event	Manufactur	er Info		Comtect	n, Inc. Roilly Road, Suite :	#620
Calculated Structur structural adequac	ed Designs is responsible only y of this component based o	of the <b>Handling</b> In the 1, LVL beam	& Installation	) or dri <b>l</b> ed	pond	ing				Metsä Wood 301 Merritt 7	d 7 Bui <b>l</b> dina	. 2nd Floor	Fayettev	ille, NC	
design criteria a responsibility of th ensure the comp	and loadings shown. It is e customer and/or the contrac ponent suitability of the inte	the 2 Refer to tor to regarding ended fastening	manufacturer's installation n details beam etc	equirements, equirements,	formation multi-ply and code					Norwalk, CT (800) 622-5	06851 850		28314 910-864	TRUS	
application, and to <b>Lumber</b>	verify the dimensions and loads.	approvals 3. Damaged 4. Deciment	Beams must not b	e used	ined					www.metsav ICC-ES: ES	wood.com R-3633	ı/us			
<ol> <li>Dry service con</li> <li>LVL not to be to</li> </ol>	ditions, unless noted otherwise reated with fire retardant or con	5. Provide la rosive lateral dis	ateral support at placement and rota	bearing points ation	to avoid This	s design is va <b>l</b> id	until 5/	24/2024						comt	есн
Version 21.40.33	8 Powered by iStruct™ I	Dataset: embedd	ed						I_				CSD	1 DEAW	

CSD

TisDe	sign	Client: We Project: Address: Bi <b>l</b>	aver Developmer I Shaw Road	nt		Date: Input by: Job Name	8/3/2021 David Landry :: Lot 77 Thomas Farm	Page 2 of 8
BM1 Ke	rto-S LVL	1.750" X ′	16.000''	2-Ply -	PASSI	Project #:	J0721–4570 Level: Level	
						I		
	• •	· · ·	•	•	• •	•	<	1'4"
	<u> </u>		• 11'11"		<u> </u>	•		↓ ↓ ↓ 3 1/2"
1			11'11"				1	
Fasten all plies Capacity Load Yield Limit per Foot Yield Mode Edge Distance Min. End Distance Load Combination Duration Factor	ysis using 3 rows of 10 0.0 % 0.0 PL 245.6 NV 1 1/2" 3" 1.00	Od Box nails (.12 F PLF ).	8x3") at 12" c	o.c Maximu	um end dis	stance no	ot to exceed 6".	
Notes Calculated Structured Desig structural adequacy of this design criteria and loat responsibility of the custom ensure the component s application, and to verify the	the sis responsible only of the the component based on the 1 lings shown. It is the 2 rend/or the contractor to reintebility of the intended dimensions and loads.	chemicals ndling & Installation .VL beams must not be cut or o Refer to manufacturer's regarding installation requ astening details, beam streng approvals	fri∎ed product information irrements, mu <b>lti⊰ply</b> th values, and code	. For flat roofs prov ponding	ride proper drainag	e to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Comtech, Inc. 1001 S. Relly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
Lumber 1. Dry service conditions, u 2. LVL not to be treated with	3. [ 4. [ 4. [ 5. F h fire retardant or corrosive	Damaged Beams must not be u Design assumes top edge is lat Provide lateral support at bea ateral displacement and rotatio	ised erally restrained aring points to avoid n	This design is	valid until 5/24/2	2024	ICC-ES: ESR-3633	соттесн

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CSD

ŕ	is Design		Client: Project:	Weaver Developm	ent	Da Inp	ite: out by:	8/3/2021 David Landry	Page 4 of 8
- 4	Ispesign		Address:	Bill Shaw Road		Jol Pro	b Name oject #:	∴ Lot 77 Thomas Farm J0721-4570	
BM2	Kerto-S	LVL	1.750"	X 16.000"	2-Ply	- PASSED	<b>)</b>	_evel: Level	
	• •	•	•	• •	• •	•	•	• • •	
.		•			•		•		
	• •	•	•	• •	• •	٠	•	• • •	
1 SPF	End Grain							2 SPF End	I Grain
					14'7 1/2"				
1				1	4'7 1/2"				1
Multi-Ply	/ Analysis								
Fasten all	plies using 3	rows of 10	d Box nails	s (.128x3") at 12"	o.c Maxim	um end distar	nce no	ot to exceed 6".	
Capacity Load		79.8 % 196.0	; PLF						
Yield Limit pe	er Foot er Fastener	245.6 81.9 lb	PLF						
Yield Mode		IV							
Edge Distand	ce tanco	1 1/2"							
Load Combir	nation	D+L							
Duration Fac	tor	1.00							
Notes		c	hemica <b>l</b> s		6. For flat roofs pro	wide proper drainage to j	prevent	Manufacturer Info	Comtech, Inc. 1001 S, Reilly Road, Suite #639
Calculated Struct structural adequi design criteria	tured Designs is responsible acy of this component be and loadings shown.	le only of the <b>Hau</b> ased on the 1. L It is the 2. F	ndling & Install VL beams must not b Refer to manufac	ation e cut or dri∎ed turer's product information	ponding		Γ	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk. CT 06851	Fayetteville, NC USA 28314 010-864-TELIS
responsibility of ensure the con application, and t	the customer and/or the mponent suitability of the to verify the dimensions and	contractor to r he intended fi d loads. a	egarding installatio astening details, bea opprovals Damaged Beams must	en requirements, multi-ply im strength values, and code				(800) 622-5850 www.metsawood.com/us	510-004-11005
1. Dry service c 2. LVL not to be	onditions, unless noted other a treated with fire retardant	3. L 4. C erwise 5. F or corrosive L	Design assumes top e Provide lateral suppo ateral displacement a	n not be used dge is laterally restrained ort at bearing points to avoid nd rotation	This design is	valid until 5/24/2024		IUU-ES: ESR-3633	соттесн
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	1	Client:	Weaver Developm	ient		Date:	8/3/2021	Page 6 of 8
Í	icDesign	Project:				Input by:	David Landry	
- +	Ispesign	Address.		1		Project #:	J0721–4570	
BM3	S-P-F #2	2.000" X	10.000"	2-Plv -	PASSE	D L	evel: Level	
	••••			_ · · · <b>,</b>		-		
								Ξ. /
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.		•	• •	•	•	•		. <u> </u>
	F End Grain						2 SPE End G	
				10				
				12				3"
				12'				I
Multi-Ply	/ Analysis							
Fasten all	plies using 2 rov	vs of 10d Box nails	(.128x3") at 12"	o.c Maxin	num end dis	tance no	t to exceed 6".	
Load		0.0 PLF						
Yield Limit pe	er Foot	157.4 PLF						
Yield Linit pe	er Fasterier	10.7 lb. IV						
Edge Distan	се	1 1/2"						
Min. End Dis Load Combir	stance nation	3"						
Duration Fac	stor	1.00						
						.—		
						F	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
								Layetteville, NC USA 28314
								910-864-TRUS
				This design i	s valid until 5/24/2	2024		соттесн
				5				

Tis	Design		Client: Project: Address:	Weaver De Bill Shav	evelopment v Road			Date: Input by: Job Name	8/3/202 David La Lot 77 T	1 andry īhomas Farr	n			Page 7 of 8
GDH	S_P_F #2	2 0	00" X	12 00	0" 2_P	Div _ P	<u> </u>	Project #:	J0721–4 evel: Leve.	.570 I				
	0-1 -1 <i>#2</i>	2.0		12.00	0 2-1	ıy - ı /								
							3							
		2												
					1									- <del>/-</del> -
·	FILM	·	•				•	•		• •			M	
	- Thin				att the	ite			1.1.1				M	11 1/4
1 SPF E	nd Grain								2 SPF E	nd Grain				
Į.					8'10"						1			3"
1				:	8'10"						1			
Member Inf	formation						Reactio	ons UNF	ATTERI	NED Ib (I	Jplift)			
Туре:	Girder		Applica	tion:	Floor		Brg Di	rection	Live	e De	ad :	Snow	Wind	Cons
Plies: Moisture Cond	z dition: Dry		Building	g Code:	ASD IBC/IRC 2015		1 Ve	ertical	(	) -	751 751	88 88	0	(
Deflection LL:	480		Load S	haring:	No		_							
Deflection TL: Importance:	360 Normal - II		Deck: Ceilina		Not Checked Gypsum 1/2"									
Temperature:	Temp <= 100	)°F			- , ,									
							Bearing	gs						
							1 - SPF	g Length = 3.500"	Dir. Vert	Cap. Re 19%	act D/L Ib 751 / 88	10tai 839	Ld. Case	La. Comb D+S
Analysis Re	sults						End Grain							
Analysis	Actual	Location	Allowed	Capacit	y Comb.	Case	2 - SPF	3.500"	Vert	19%	751 / 88	839	L	D+S
Moment	1490 ft-lb	4'5"	4153 ft-Ib	0.359 (3	6%) D	Uniform	Grain							
Unbraced Shear	1490 ft-lb 542 lb	4'5" 1'2 3/4"	3539 ft-lb 2734 lb	0.421 (4)	2%) D 0%) D	Uniform								
LL Defl inch	0.004	4'5 1/16"	0.209 (L/48	0.100 (2 0) 0.021 (2	%)S	L								
TI Deflineb	(L/22622)	415 1/16"	0.270 (1./26	0) 0 151 (1	50/) D+S									
	0.042 (L/2301)	43 1/10	0.279 (1/30	0) 0.131 (1	576) D+3	L	1							
1 Provide sup	oport to prevent late	eral movemen	nt and rotation	on at the end	d bearings. Later	al support	1							
may also be 2 Fasten all p	e required at the int dies using 2 rows o	erior bearing f 10d Box na	s by the buil ils (.128x3")	ding code. at 12" o.c. N	/laximum end dis	stance not								
to exceed 6 3 Refer to las	5". It page of calculatio	ns for fasten	ers required	for specified	l loads.									
4 Girders are	designed to be su	oported on th	e bottom ed	ge only.										
6 Top must be	e laterally braced a	t end bearing	piles. Is.											
7 Lateral slen	Iderness ratio base	d on single p	ly width. Location	Trib Width	Side	Dead 0.9	Live	e 1 Snov	w 1.15	Wind 1 6	Const 1	25 Cor	nments	
1	Uniform		_0000000	would	Тор	60 PLF	0 P	LF	0 PLF	0 PLF	0 Pl	LF Wall		
2	Uniform				Тор	90 PLF	0 P	LF	0 PLF	0 PLF	0 P	LF B1G	Ε	
3	Tie-In	0-0-0	to 8-10-0	1-0-0	Тор	20 PSF	0 P	SF 2	20 PSF	0 PSF	0 P\$	SF Roo	f Load	
									Manufactu	rer Info		Comtech, I 1001 S. Re	inc. ei v Road. Suite #	639
												Fayetteville USA 28314	e, NC	
												910-864-T	RUS	
													omt	есні
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CISION 21.40.338	- owered by istruct <sup>im</sup>	vataset, empe	aueu									CSD	DESIGN	

Í	isDesign	Client: Project: Address:	Weaver Development Bill Shaw Road		Date: Input by: Job Name:	8/3/2021 David Landry Lot 77 Thomas Farm	Page 8 of 8
GDH	S_P_F #2	2 000" X	12 000" 2_6		Project #:	J0721–4570 evel: Level	
GDH	3-F-F #2	2.000 A	12.000 2-1	-iy - FA33L			
•	•	• •	•	• •	•	••	$\overline{\mathbf{n}}$ $\mathbf{t}$
•	٠	• •	•	• •	•	•••	
	F End Grain					2 SPF End Grain	<b>N</b>
			8'10"				<b>∕</b> −− <b>′</b> 3"
1			8'10"			1	
Multi-Ply	/ Analysis						
Fasten all	p <b>l</b> ies using 2 row	vs of 10d Box nails (	.128x3") at 12" o.c I	Maximum end dis	tance not	t to exceed 6".	
Capacity Load		0.0 % 0.0 PLF					
Yield Limit pe Yield Limit pe	er Foot er Fastener	157.4 PLF 78.7 lb.					
Yield Mode Edge Distand	ce	IV 1 1/2"					
Min. End Dist	tance	3"					
Load Combin Duration Fac	nation tor	1.00					
						Manufacturer Info	Comtech, Inc. 1001 S. Rei∎y Road, Suite #639
							Fayetteville, NC USA 28314 910-864_TRUS
			This	design is valid until 5/24/2	024		соттесн



### RE: J0721-4570

Lot 77 Thomas Farm

**Trenco** 818 Soundside Rd Edenton, NC 27932

### Site Information:

Customer: Lot/Block:	Weaver Development Inc. 77	Project Name: J0721-4570 Model: Hickory
Address:		Subdivision:
City:		State:

## General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.3 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 8 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E15727040	ET1	8/3/2021
2	E15727041	ET2	8/3/2021
3	E15727042	ET3	8/3/2021
4	E15727043	F1	8/3/2021
5	E15727044	F2	8/3/2021
6	E15727045	F3	8/3/2021
5	E15727044	F2	8/3/2021
6	E15727045	F3	
7	E15727046	F4	8/3/2021
8	E15727047	F5	8/3/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters and project specific up parameters and project use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

August 03, 2021



			8-7-0			
Plate Offsets (X,Y)	[5:0-1-8,Edge], [17:0-1-8,0-1-8], [18:0-1-	8,0-1-8]				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.00 WB 0.05 Matrix-P	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) l/defl L/d a - n/a 999 a - n/a 999 b 9 n/a n/a	<b>PLATES</b> MT20 Weight: 54 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	SP No.1(flat) SP No.1(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or	ectly applied or 6-0-0 10-0-0 oc bracing.	oc purlins,

8-7-0

REACTIONS. All bearings 8-7-0.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

OTHERS

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

#### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb) Vert: 3=-71 6=-71 19=-71 20=-77



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			11-1-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [14:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.07 BC 0.01 WB 0.04 Matrix-S	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 10 n/a n/a	<b>PLATES</b> MT20 Weight: 52 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S OTHERS 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,

REACTIONS.

DNS. All bearings 11-1-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



May 13,2021

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BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WEBS 2x4 SP No.3(flat) OTHERS

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.





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<b> </b>	14-7-12				26-5-0				
Plate Offsets ()	(,Y)	[4:0-1-8,Edge], [15:0-1-8,Edge], [16:0-1	-8,Edge], [21:0-1-8,Edge]				11-3-		
LOADING (psf TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	f) D D D D	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.55 BC 0.59 WB 0.53 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (lo -0.17 21-2 -0.25 21-2 0.03 1	c) l/defl 2 >999 2 >697 4 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 129 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD         2x4 SP No.1(flat)         BRACIN           BOT CHORD         2x4 SP No.1(flat)         TOP CHORD           BOT CHORD         2x4 SP No.1(flat)         BOT CHORD           WEBS         2x4 SP No.3(flat)         BOT CHORD           REACTIONS.         (size)         22=0-3-8, 17=0-3-8, 14=0-3-8           Max Gray, 22=7394         C 10)         17=16504         C 7)					2D Stru exc 2D Rigi 6-0-	ctural wood ept end ver d ceiling din 0 oc bracin	l sheathing dir icals. ectly applied c g: 17-19,16-17	rectly applied or 6-0-0 o or 10-0-0 oc bracing, 1 7.	oc purlins, Except:
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-1987/0, 3-4=-1987/0, 4-5=-1740/0, 5-6=-1740/0, 6-8=0/1282, 8-9=0/1282, 9-10=-1183/0, 10-11=-1183/0, 11-12=-1183/0         BOT CHORD       21-22=0/1314, 20-21=0/1987, 19-20=0/1987, 17-19=-191/818, 16-17=-366/574, 15-16=0/1183, 14-15=0/954         WEBS       8-17=-284/0, 2-22=-1440/0, 2-21=0/745, 3-21=-260/0, 6-17=-1781/0, 6-19=0/1122, 4-19=-646/0, 9-17=-1465/0, 9-16=0/917, 10-16=-329/0, 12-14=-1044/0, 12-15=-86/311									
NOTES-									

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.





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			10-10-0		
Plate Offsets (X,Y)	[8:0-1-8,Edge]				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.42 BC 0.35 WB 0.30 Matrix-S	DEFL.         ir           Vert(LL)         -0.08           Vert(CT)         -0.11           Horz(CT)         0.02	n (loc) I/defl L/d 3 9-10 >999 480 9-10 >999 360 2 7 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 56 lb         FT = 20%F, 11%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

(size) 10=0-3-8, 7=0-3-8 REACTIONS. Max Grav 10=576(LC 1), 7=576(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1234/0, 3-4=-1234/0, 4-5=-1234/0

BOT CHORD 9-10=0/981, 8-9=0/1234, 7-8=0/982 WEBS 2-10=-1073/0, 5-7=-1075/0, 5-8=0/485, 2-9=0/478, 3-9=-255/0, 4-8=-266/0

NOTES-

 Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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				11-11-0			
Plate O	ffsets (X,Y)	[8:0-1-8,Edge], [9:0-1-8,Edge]					
LOADII TCLL TCDL BCLL BCDL	NG (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.68 BC 0.56 WB 0.34 Matrix-S	DEFL.         in           Vert(LL)         -0.19           Vert(CT)         -0.29           Horz(CT)         0.02	(loc) l/defl L/d 9-10 >740 480 9-10 >490 360 7 n/a n/a	PLATES MT20 Weight: 59 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER-TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)				BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,

11-11-0

REACTIONS. (size) 10=0-3-8, 7=0-3-8 Max Grav 10=635(LC 1), 7=635(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1508/0, 3-4=-1508/0, 4-5=-1508/0

BOT CHORD 9-10=0/1112, 8-9=0/1508, 7-8=0/1121

WEBS 2-10=-1219/0, 2-9=0/558, 5-7=-1228/0, 5-8=0/655, 4-8=-353/0

#### NOTES-

 Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





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10

1.5x3 ||

9

3x6 =

			14-6-0			
1			14-6-0			1
Plate Offsets (X,Y)	[4:0-1-8,Edge], [11:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.61 BC 0.84 WB 0.46 Matrix-S	DEFL. in Vert(LL) -0.20 Vert(CT) -0.25 Horz(CT) 0.03	(loc) I/defi L/d 9-10 >838 480 9-10 >684 360 8 n/a n/a	PLATES MT20 Weight: 73 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.	) oc purlins,

#### REACTIONS. (size) 12=0-3-8, 8=Mechanical

Max Grav 12=778(LC 1), 8=784(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2243/0, 3-4=-2243/0, 4-5=-2186/0, 5-6=-2186/0

11-12=0/1424, 10-11=0/2243, 9-10=0/2243, 8-9=0/1429 BOT CHORD WEBS

2-12=-1561/0, 2-11=0/958, 3-11=-303/0, 6-8=-1573/0, 6-9=0/836, 5-9=-271/41, 4-9=-428/186

### NOTES-

2

3x6 =

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

11

3x4 =

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.





8

3x6 =

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11-7-8							
			11-7-8				
Plate Offsets (X,Y) [1:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8,Edge]							
LOADING (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.60 BC 0.51 WB 0.33 Matrix-S	DEFL.         in           Vert(LL)         -0.16           Vert(CT)         -0.25           Horz(CT)         0.02	(loc) l/defl L/d 9-10 >846 480 9-10 >540 360 7 n/a n/a	<b>PLATES</b> MT20 Weight: 58 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E	
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	2 No.1(flat) 2 No.1(flat) 2 No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,		
REACTIONS. (size Max G	e) 10=Mechanical, 7=0-3-8 irav 10=626(LC 1), 7=619(LC 1)						

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1441/0, 3-4=-1441/0, 4-5=-1441/0

BOT CHORD 9-10=0/1081, 8-9=0/1441, 7-8=0/1087

WEBS 2-10=-1190/0, 2-9=0/515, 5-7=-1190/0, 5-8=0/606, 4-8=-323/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



May 13,2021

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