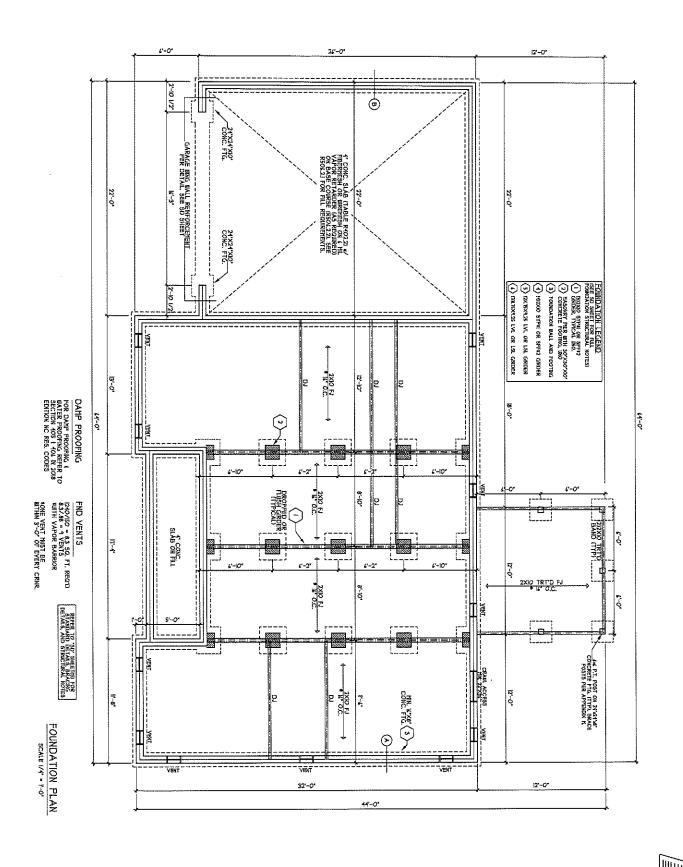
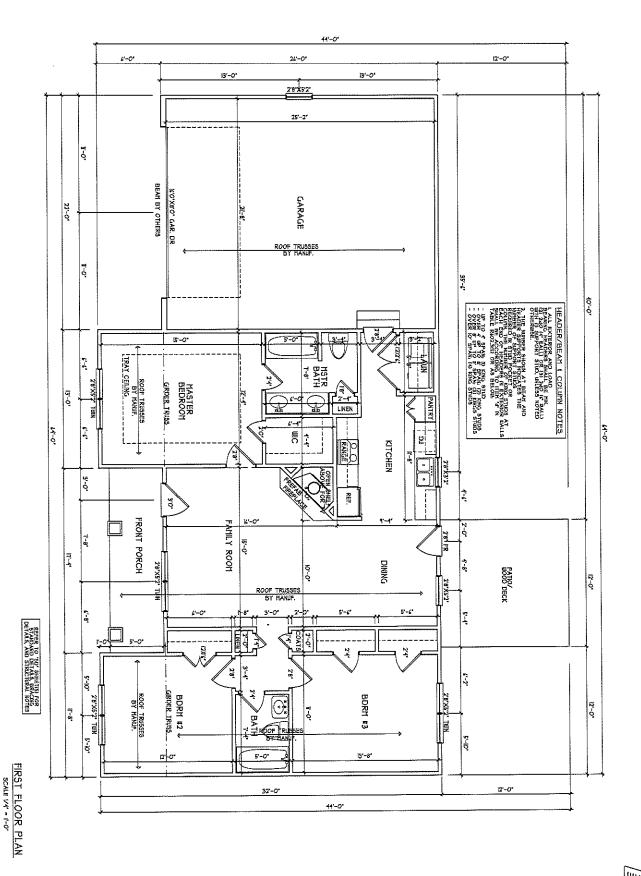


OZO320	H SQUARED HOME DESIGN, INC.	HEATHER HALL 185 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403	FIRST FLOOR FRONT PORCH WOOD DECK GARAGE	= 1240 = 86 = 144 = 572	#1240	"THE DAKOTA II" (LEFT HAND GARAGE) JRT MANG. PROP.	-
S BALDNG CODES 200 EDITOR	DESIGN, INC.	(919) 207-1403	GARAGE	= 572		JILI MANG. 1101.	



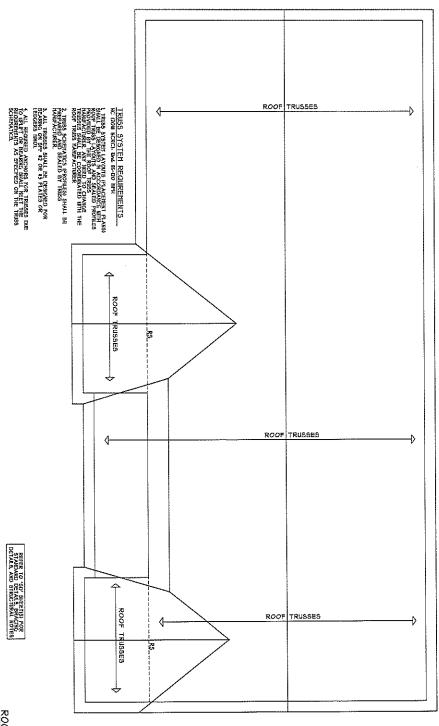
ANY OFFICATION OF THE	H SQUARED HOME	HEATHER HALL 165 HEATHERSTONE CT BENSON NC 27504	FRONT PORCH =	1240 88 144 #1240	"THE DAKOTA II"	
N ACCORDAGE SITH SOUTH A SECURITION SEALING COURS YOU SUTTON	DESIGN, INC.	(919) 207~1403	GARAGE =	572	JRT MANG. PROP.	



HEATHER HALL
165 HEATHER STONE CT
165 DESIGN, INC.

HEATHER HALL
165 HEATHER STONE CT
165 DESIGN, INC.

HEATHER HALL
165 HEATHER STONE CT
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165



ROOF PLAN

ANT CONTINUE OF THE DAKOTA II"

OF THE DAKOTA II"

HEATHER HALL ISS HEATHERSTONE CT BENSON NC 27504 SUNCE COTTON OF THE SHARE COTTON OF THE SHARE

STRUCTURAL NOTES

BALL BALONG., BRACED BALL PARELS SHALL BE CONSTRUCTED ACCORDIG TO SECTION RODZUG.

THE ARISMA HAN LOCATION OF BALONG SHALL CORPLY INTH HANE: SUDJOL, THE MISMAT HAN LOCATION OF BALONG SHALL BE DETENDED BY SECTION RODZUG.

THE LINE BALONG, SHALL BE SATISHEN PER BETHOD 3 BY CONTRIDUCELY SHALL BE ALISHEN PER BETHOD 3 BY CONTRIDUCELY SHALL BE ALISHEN PER BETHOD 3 BY CONTRIDUCELY SHALL BHANG GALUS BITH STRUCTURES, DESCRIBED, SHALL BE NOTALLED AS SPECIFIED.

MOTE THAT ANY SPECIFIC BRACED WALL DEFAIL SHALL BE NOTALLED AS SPECIFIED.

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SALLI SAL

IN ALL MOORIN BEARS AND PEACERS SHALL HAVE THE FOLLOWING BID SHPPORTS.

IN AND COMBINE FOR FOR THE SERVICE SHAME, 12 245 SINDE FOR BEATH

SPAN CREATER THAN 1-0" INVO.

SPAN CREATER THAN 1-0" INVO.

SPAN CREATER THAN 1-0" INVO.

ST. SHALL BE LANGATED NEERER URBER TH-200 PM 1-7-35 FM, E-1-400 FM

1-1. SHALL BE LANGATED NEERER URBER TH-200 PM 1-7-30 FM, E-1-2-2-0

LET SHALL BE LANGATED STANDAL URBER TH-200 PM 1-7-30 FM, E-1-2-2-0

LET SHALL BE LANGATED STANDAL URBER TH-200 PM 1-7-30 FM, E-1-2-2-0

NOTALL ALL CONRECTIONS PER SHAME/CINEERS NOT RECTIONS.

ALL ROOF TRUSS AND 1-JOST LAYOUTS SHALL DE PREFARED IN ACCORDANCE UTIH. ANY SEALED STRUCTURAL DRAWGIGS, TRUSSES AND 1-JOSTS SHALL BE INSTALLED ACCORDANG TO THE HUMBACTURES SPECENCIONS, ANY CHAKSE IN TRUSS OR 1-JOST LAYOHT SHALL BE COORDINATED WITH DESIGNER OR ENGINEER.

ID) FUTCH BEAMS SHALL BE BOLTED TOGETHER USING (2) ROCK OF 1/2" DAMETER BOLTS LASTIN (ASOT) LITH INCRESS PLACES WORK THE THEROUSE PHO OF SOLT. BOLTS SHALL BE SPACED AT 3"4" OC. (HAVA), AND STACKERED AT THE TOP AND SOTTON OF BEAM (2" EDGE DISTANCE), WITH 3 BOLTS LOCATED AT 1" FROM EACH END. 9 REBAR SHALL BE DEFORMED STEEL, ASTRIS, GRADE 10.

THE POSITIVE AND HEGATIVE DESIGN PRESSURE FOR DOORS AND UNDOUS FOR A HEAR ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF.

I THE POSITIVE AND NEGATIVE DESIGN PRESSURES REQUIRED FOR ANY ROOF OR BUILL CLADDIG APPLICATION HOT SPECIFICALLY ADDRESSED IN THE NORTH CAROLINA STATE RESCRINTAL CODE - 20% EDITION SHALL BE AS FOLLOZS.

BRICK INTELS SHALL BE 3 1/2"56 1/2"4/4" STEEL ANGLE FOR UP TO 1"-0" SPAN AND 5"4"5/4" STEEL ANGLE WITH 1" LEG YERTICAL FOR SPANS UP TO 4"-0" ANO.

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-1. CONCRETE

TATE STUDS

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2 THE NUMBER SHOWN AT BEAM AND HEADER SHORE WINDERS OF SUPPORE STUDGE THE NUMBER OF SUPPORE STUDGES OF SHORE SHOWN THE NUMBER OF KING STUDG AT SHORE SHOWN THE SHORE SHOWN BEACHER OF SHORE SHOWN SHOW UP TO 4' SPAN, (1) KING STUD OVER 8' UP TO 8' SPAN, (2) KING STUDS OVER 8' UP TO 11' SPAN, (3) KINGS STUDS OVER 8' SPAN, (4) KING STUDS

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SNANOG-DYCH HILN BRYKE TYLEDE MEG DONLER

GARAGE 'MING MALL' REINFORCING FER ING F GURE RADZIOAS

REVATER

igoplus 2Jo syp 12 or spf12 grder, typical uno. FOUNDATION STRUCTURAL NOTES: NO 1200 NCRC); und 15-120 NPH

(a) CONCERTE BLOCK PERS SUE SHALL BE.

1 A LE TO SE HIGH.

1 A LE

FINISH GRADE

8' SOLIO CAP

CHAPCEY BLOCK

BAND HATERIAL SUBFLOOR

-EXPANSION JOINT - 4" CONC. SLAB BITH FIBERTESH OR WREKESH ON 4" CRUSHED STONE BARRIER ON 4" CRUSHED STONE

OF PLATE ENDS (EXEED TO

-2x4 \$11005

. 2

F-8XM. COHC FIG

(3) BALL FOOTING AS FOLLOWS:
DEPTH: 8' - UP TO 2-1/2 STORY
10" - 3 STORY SDNG - (OR EQUAL)

- 20' - UP TO 2-1/2 STORY

- 30' - 3 STORY

- W - 1 STORY

- W - 2 STORY

- 30' - 2 STORY

- 24' - 3 STORY

- 24' - 3 STORY

FOR FORBANION BULL HECHT AND BACKEL RECORDERUITS, BETHE TO MORTH CARDINA, RESCRETHAL CODE TABLE ROULI IT 1990 IN 1071E, ASUREIA SON BEARRA CAPACITY - 2000 PSF. CONTRACTION (MSI VESTO SITE CONSTITUTE AND CONTRACT SONS EMCAREM ET MARIANAL OR MST JAME SONS AND RECOMPLIENCE.

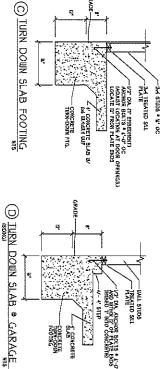
(1) (4) 240 SYP#2 OR SPF#2 GEDER.

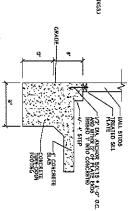
💽 (3) LT5X4.25 LYL OR LSL GRDER (3) (15X1) 25 LYE OR USE GIRDER

T. "B" DESCNATES A SIGNECANT PONT LOAD TO HAVE SOLD BLOCKING TO FER. SOLD BLOCK ALL BEAN BEARING PONTS NOTED TO HAVE THREE OR HORE STIEDS TO FID, TYPICAL. "TJ" = TRIPLE JOIST
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"TJ" = SAGLE JOIST

> TREATED SILL / V2" DIA AKCHOR BOLTS - 1"-0" O.C. AND UTHAN 0" OF PLATE EKDS (ERBED T) A CRAWL SECTION FLOOR JOIST

-34 TREATED SIL 24 STUDS * 12 OC L4 CONCRETE SLAB S/ CONCRETE TURN-DOWN FTG.





(E) THICKENED SLAB @ GARAGE - IRBATED SILL
- IRBA 1/- 4' STEP CONVERT ROSTRE DEEPLAND (F) TYPICAL THICKENED SLAB PARTY NAME OF STREET TREATED SAT PLATE
ATTACH O' V7' AL ANCHOR
BOLTS * 1'-O' JEHBED TO
OR APPROVED EQUAL



H SQUARED HOME DESIGN, INC.

HEATHER HALL 165 HEATHERSTONE CT BENSON NC 27504 (9)9) 207-1403

-STUDS AS SPECIFIED

*PLEASE NOTE THAT NOT ALL DETAILS APPLY TO EVERY PLAN.

BASIC BUILDING DETAIL SHEET

B) GARAGE SLAB

SE FOUNDATION BALL





Double 1-3/4" x 9-1/4" VERSA-LAM® 2.0 3100 SP

PASSED

Roof\Dropped Beams\FB1() (Dropped Beam)

BC CALC® Member Report Dry | 3 spans | No cant.

May 20, 2020 13:51:46

Build 7493

Job name:

Dry | 3 spans | No ca

Address: City, State, Zip:

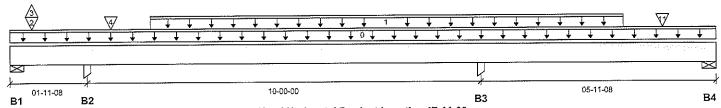
Customer:
Code reports: ESR-1040

File name: 2000367A.mmdl

Description: Roof\Dropped Beams\FB1()

Specifier: Designer:

Company:



Total Horizontal Product Length = 17-11-00

Reaction Summary (Down / Uplift) (lbs)

Bearing Live Dead Snow Wind Roof Live	
B1, 3-1/2" 0 / 242 290 / 77 71 / 380	
B2, 5-1/2" 697 / 0 190 / 702 811 / 54	
B3, 5-1/2" 602 / 0 161 / 594 653 / 0	
B4, 3-1/2" 93 / 0 21 / 74 163 / 67	

Loa	ad Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-11-00	Top		9				00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	03-06-12	15-06-12	Top		57			72	n\a
2	M1(c1)	Conc. Pt. (lbs)	L	00-06-12	00-06-12	Top		64			50	n\a
3	M1(c1)	Conc. Pt. (lbs)	L	00-06-12	00-06-12	Top					- 21	n\a
4	M1(c1)	Conc. Pt. (lbs)	L	02-06-12	02-06-12	Top		126			168	n\a
11	M1(c1)	Conc. Pt. (lbs)	L	16-06-12	16-06-12	Top		102			130	n\a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	779 ft-lbs	4.7%	125%	4	06-06-12
Neg. Moment	-1155 ft-lbs	7.1%	125%	5	01-11-08
End Shear	576 lbs	7.5%	125%	4	01-00-12
Cont. Shear	689 lbs	9.0%	125%	5	00-11-08
Total Load Deflection	L/999 (0.022")	n\a	n\a	4	07-00-12
Live Load Deflection	L/999 (0.012")	n\a	n\a	312	07-02-04
Total Neg. Defl.	L/999 (-0.003")	n\a	n\a	4	13-10-15
Max Defl.	0.022"	n\a	n\a	4	07-00-12
Span / Depth	13.0				

Bearin	g Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	3-1/2" x 3-1/2"	29 lbs	0.6%	0.3%	Spruce-Pine-Fir
B1	Uplift		622 lbs			
B2	Column	5-1/2" x 3-1/2"	1508 lbs	10.8%	10.4%	Unspecified
В3	Column	5-1/2" x 3-1/2"	1255 lbs	9.0%	8.7%	Unspecified
B4	Wall/Plate	3-1/2" x 3-1/2"	256 lbs	4.9%	2.8%	Spruce-Pine-Fir

Cautions

Uplift of -622 lbs found at bearing B1.



BC CALC® Member Report



Double 1-3/4" x 9-1/4" VERSA-LAM® 2.0 3100 SP

PASSED

Roof\Dropped Beams\FB1() (Dropped Beam)

Dry | 3 spans | No cant.

May 20, 2020 13:51:46

Build 7493

Job name:

Address: City, State, Zip:

Customer: Code reports:

ESR-1040

File name: Description:

2000367A.mmdl

Roof\Dropped Beams\FB1()

Specifier:

Designer:

Company:

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

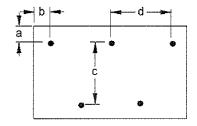
Calculations assume unbraced length of Top: 01-10-08, Bottom: 01-10-08.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Connection Diagram: Full Length of Member





a minimum = 2"

c = 5-1/4"

b minimum = 3"

d = 24"

Connectors are: 3-1/4 in. Pneumatic Gun Nails

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FioorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

May 20, 2020 13:51:46

Roof\Dropped Beams\GDH(i19) (Dropped Beam)

BC CALC® Member Report

Build 7493

Job name:

Address: City, State, Zip:

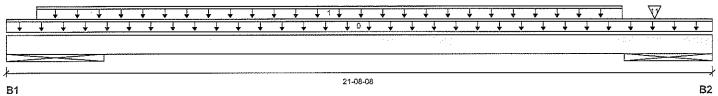
Customer: Code reports: ESR-1040 Dry | 1 span | No cant.

2000367A.mmdI File name:

Roof\Dropped Beams\GDH(i19) Description:

Specifier: Designer:

Company:



Total Horizontal Product Length = 21-08-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 36"		2891 / 0		1374 / 1950	2804 / 0
B2, 32-1/2"		2878 / 0		1383 / 1999	2811 / 0

Loa	Load Summary					Live	Dead	Snow	Wind	Roof Live	Tributary	
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	21-08-08	Тор		18				00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-11-04	18-11-04	Тор		268			279	n∖a
11	A(c1)	Conc. Pt. (lbs)	L	19-11-04	19-11-04	Top		557			600	n\a

Value	% Allowable	Duration	Case	Location
17173 ft-lbs	44.1%	125%	1	11-11-04
3425 lbs	23.1%	125%	1	03-11-14
L/356 (0.544")	67.5%	n\a	1	10-11-04
L/718 (0.27")	50.2%	n\a	116	10-11-04
0.544"	54.4%	n\a	1	10-11-04
16.3				
1107 lbs	8.0%	100%		
1157 lbs	8.4%	100%		
	17173 ft-lbs 3425 lbs L/356 (0.544") L/718 (0.27") 0.544" 16.3 1107 lbs	17173 ft-lbs 44.1% 3425 lbs 23.1% L/356 (0.544") 67.5% L/718 (0.27") 50.2% 0.544" 54.4% 16.3 1107 lbs 8.0%	17173 ft-lbs 44.1% 125% 3425 lbs 23.1% 125% L/356 (0.544") 67.5% n\a L/718 (0.27") 50.2% n\a 0.544" 54.4% n\a 16.3 1107 lbs 8.0% 100%	17173 ft-lbs 44.1% 125% 1 3425 lbs 23.1% 125% 1 L/356 (0.544") 67.5% n\a 1 L/718 (0.27") 50.2% n\a 116 0.544" 54.4% n\a 1 16.3 1107 lbs 8.0% 100%

Bearin	g Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	36" x 5-1/4"	5695 lbs	4.2%	4.0%	Unspecified
B2	Wall/Plate	32-1/2" x 5-1/4"	5689 lbs	7.8%	4.4%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume unbraced length of Top: 01-10-08, Bottom: 01-10-08.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.



BC CALC® Member Report



Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

Roof\Dropped Beams\GDH(i19) (Dropped Beam)

Dry | 1 span | No cant.

May 20, 2020 13:51:46

Build 7493

Job name:

Address: City, State, Zip:

Customer: Code reports:

ESR-1040

2000367A.mmdl

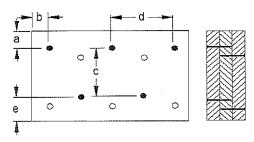
File name: Description:

Roof\Dropped Beams\GDH(i19)

Specifier: Designer:

Company:

Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3" c = 7-7/8" d = 24"

e minimum = 3"

Nailing applies to both sides of the member Connectors are: 3-1/4 in. Pneumatic Gun Nails

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



Trenco

818 Soundside Rd Edenton, NC 27932

Re: 2000367-2000367A 2307- JRT -DAKOTA II

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I41377982 thru I41378003

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



May 21,2020

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

	Truss	Truss Type	Qty	Ply	2307- JRT -DAKOTA II	141377982
-2000367A	A	ROOF TRUSS	10	1	Job Reference (optional)	
nponents (Dunn),	Dunn, NC ~ 28334,		ID:ceuvED	ckNGn_oUaH?	G0U8dzEcIS-SLjavwL7XTjun8	Ved May 20 13:01:53 2020 Page 1 8gGxhDR6I5kiJenG7ZIBzrNLzEbji
-0 <u>-10-</u> 0-10-	-8 6-7-12 8 6-7-12	11-0-0 4-4-4	13-0-0 15-0-0 2-0-0 2-0-0	19-4-4 4-4-4	26-0-0 6-7-12	26-10-8 0-10-8
4x4 1		8.00 12 4x4 4 17 18 15 19 1	\$1	20	8 9 9 13 21 22	Scale = 1:54.0 4x4 10 11 12 4x4 12 4x4 =
	800		18-0-0		. 26-0-0	

	8-0-0		10-0-0	 -0-0
Plate Offsets (X,Y)- (2:0 LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	D-2-0,0-1-12], [6:0-2-0,Edge], [10:0-2- SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.64 BC 0.76 WB 0.44 Matrix-S	DEFL. in Vert(LL) -0.39 1: Vert(CT) -0.56 1: Horz(CT) 0.03	 PLATES GRIP MT20 244/190 Weight: 164 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

Joh 2000367 84 Con

> TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.1 WEBS

2x4 SP No.3

(size) 16=0-3-8, 12=0-3-8

Max Horz 16=273(LC 11)

Max Uplift 16=-126(LC 12), 12=-126(LC 13) Max Grav 16=1165(LC 20), 12=1165(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-381/239, 4-5=-1323/338, 7-8=-1323/338, 8-10=-381/239, 2-16=-403/228, TOP CHORD

10-12=-402/228

15-16=-130/1191, 13-15=-11/927, 12-13=-82/1056 BOT CHORD

7-13=-152/617, 8-13=-265/281, 5-15=-151/617, 4-15=-265/281, 4-16=-1197/24, WEBS

8-12=-1196/24, 5-7=-791/286

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vuit=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 3x4 MT20 unless otherwise indicated.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16 and 12. This connection is for uplift only and does not consider lateral forces.
- 7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



Structural wood sheathing directly applied or 4-9-12 oc purlins,

4-16, 8-12

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of Individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



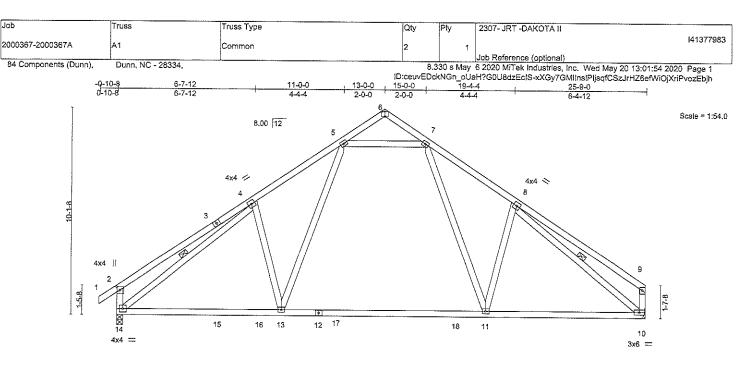


Plate Offsets (X,Y)—	8-0-0 8-0-0 2:0-2-0,0-1-12], [6:0-2-0,Edge]		18-0-0 10-0-0	25-9-0 7-9-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPt2014	CSI. TC 0.63 BC 0.77 WB 0.44 Matrix-MS	DEFL. in (loc) l/defl Vert(LL) -0.42 11-13 >728 Vert(CT) -0.59 11-13 >514 Horz(CT) 0.03 10 n/a	L/d PLATES 240 MT20 180 n/a Weight: 161 II	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

Job

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.1

2x4 SP No.3 WEBS

(size) 14=0-3-8, 10=Mechanical

Max Horz 14=270(LC 11)

Max Uplift 14=-125(LC 12), 10=-103(LC 13) Max Grav 14=1142(LC 19), 10=1049(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-369/232, 4-5=-1291/330, 7-8=-1237/329, 8-9=-255/166, 2-14=-392/223,

9-10=-267/148

BOT CHORD 13-14=-144/1158, 11-13=-43/883, 10-11=-123/975

WEBS 4-13=-262/277, 5-13=-144/626, 7-11=-140/526, 8-11=-236/271, 4-14=-1174/29,

8-10=-1218/107, 5-7=-762/287

NOTES-

1) Unbalanced roof live loads have been considered for this design.

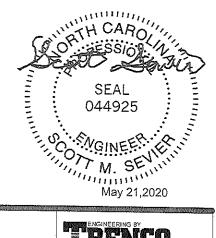
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 3x4 MT20 unless otherwise indicated.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Refer to girder(s) for truss to truss connections.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=103.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 4-8-2 oc purlins,

4-14, 8-10

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N, Lee Street, Suite 312, Alexandria, VA 22314.



b	Truss	Truss Type	Qty	P	'ly	2307- JRT	-DAKOTA II		141377984
00367-2000367A	A1E	Common Supported Gable	1		1	Job Refere	nce (optional)		
34 Components (Dunn),	Dunn, NC - 28334,		ID:ceuvEl	8.330 ockNGn	oUaH?	6 2020 MiT	ek Industries, In S-L6y5IIOdaiEK	c. Wed May 20 13: GIRRVnm9byTtJJq7	01:57 2020 Page 1 'j7k9Dpx3W7zEbje
	-0 <u>-10-8</u> 0-10-8	13-0-0 13-0-0	15.00			25-9-0 12-9-0			
	0+10-8	13-0-0	4x4 =						Scale = 1:65
			9						
		8.00 12	8	10					
		7			11				
	ļ.	6x6 = 6				12			
	10-1-8	5		X			13		
		4					14		
	3x4 3							15 16	
	1 2 1 2 1 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1		G E				<u> </u>	8.7-	
	17			XXXXXX	****	******			
	32 31 3x4	30 29 28 27 26 3x4	5 25 24 = .	23	22	21	20 19	18 17	
	 		25-9-0 25-9-0						
OADING (psf)	SPAÇING-	2-0-0 CSI.	DEFL.		(loc)	l/defl	L∕d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL.	1.15 TC 0.28 1.15 BC 0.15	Vert(LL) Vert(CT)	00.00- 00.00-	1 1	n/r n/r	120 90	MT20	244/190
CDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL Rep Stress Incr Code IRC2015/TF	YES WB 0.14	Horz(CT)	-0.00	17	n/a	n/a	Weight: 191 lb	FT = 20%
UMBER-			BRACING TOP CHOI		Structi	ıral wood st	neathing direct	ly applied or 6-0-0	oc purlins,

BOT CHORD

WEBS

except end verticals.

1 Row at midpt

10-0-0 oc bracing: 31-32,30-31,29-30.

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

9-24, 8-25, 10-23

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS 2x4 SP No.3 **OTHERS**

REACTIONS. All bearings 25-9-0.

(lb) - Max Horz 32=270(LC 9)

Max Uplift All uplift 100 b or less at joint(s) 25, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 32=-281(LC 8),

17=-302(LC 11), 31=-282(LC 9), 18=-268(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 25, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 32=335(LC 9),

17=306(LC 8), 24=270(LC 13), 31=332(LC 10), 18=349(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

7-8=-221/274, 8-9=-265/324, 9-10=-265/324, 10-11=-222/274

9-24=-291/175 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) One H2,5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 32, 17, 25, 27, 28, 29, 30, 31, 23, 22, 21, 20, 19, and 18. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WAKNING - verify design parameters and KEAD WOTES ON THIS AND INCLUDED MILES KETEKENCE PAGE MILES TRUCKED SETURE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the abrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply 2307- JRT -DAKOTA II 2000367-2000367A 141377985 A2 Roof Special Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:01:58 2020 Page 1 ID:ceuvEDckNGn_oUaH?G0U8dzEclS-pJWTyePFL?MBtv0e3UHO89?_6j?6SU3JSTgc2ZzEbjd 13-0-0 6-4-4 4x6 || Scale = 1:62.7 8.00 12 2x4 || 4x4 🥠 3x4 🛷 4x4 || 7872 1-5-8 10-0-ByB = 15 16 10 11 9 6x6 = 3x6 = 5x12 == 3x4 || 18-8-8 25-9-0

Plate Offsets (X,Y)	9-6-0 [2:0-2-0,0-1-12], [7:0-3-13,0-0-4], [7:0-0	-3,0-0-2], [7:Edge,0-3-2],	9-2-8 [8:0-5-8,0-4-0], [10:0-6-0,0-3-0]	7-0-8		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.54 BC 0.87 WB 0.52 Matrix-MS	DEFL. in (loc) l/defl Vert(LL) -0.17 10-11 >999 Vert(CT) -0.34 9-10 >917 Horz(CT) 0.03 7 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 161 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 *Except*

6-9: 2x4 SP No.3 2x4 SP No.3 WEBS

WEDGE

Right: 2x4 SP No,3

REACTIONS (size) 11=0-3-8, 7=Mechanical

Max Horz 11=-250(LC 10)

Max Uplift 11=-125(LC 12), 7=-104(LC 13) Max Grav 11=1087(LC 1), 7=1023(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-380/197, 4-5=-1106/324, 5-6=-1426/447, 6-7=-1459/270, 2-11=-403/196

BOT CHORD 10-11=-185/1016, 6-8=-422/297, 7-8=-119/1127 WEBS

4-10=-280/266, 5-10=-111/458, 8-10=-39/659, 5-8=-257/776, 4-11=-990/85

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 11. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 4-3-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component available from Truss Plate institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	2307- JRT -DAKOTA II	141377986
2000367-2000367A	А3	ROOF TRUSS	8	1	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		(DED-13)(8.330 s May	6 2020 MiTek Industries,	Inc. Wed May 20 13:02:00 2020 Page 1 v7DA0BvJsDa5JNXiFwPSbvn9j7RzEbjb
	0.40.0	7-12 , 11-0-0	13-0-0 15-0-0	19-4-4	26-0-0 6-7-12	
	-0-10-8 6 0-10-8 6	7-12 11-0-0 7-12 4-4-4	2-0-0 2-0-0	4-4-4	6-7-12	'
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		3x4 // 4	//	1	8	
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				//		4x4
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	8	\				
	14	15 16 13 171	2	18 11	1 19 20	10
		·-	4 =		· =	4x4 =
	4x4 =	3x4 = 3x	· ·			
		8-0-0	18-0-0		26-0-0	
		8-0-0	10-0-0		8-0-0	
Plate Offsets (X,Y)-	2:0-2-0,0-1-12], [6:0-2-0,E	dge]				
	SPACING-	2-0-0 CSI.	DEFL.	in (loc)	l/defl L/d	PLATES GRIP
LOADING (psf) TCLL 20.0	Plate Grip DOL	1.15 TC 0.61		.39 11-13	>798 240	MT20 244/190
TCDL 10.0	Lumber DOL	1.15 BC 0.76	(- ,	.56 11-13	>551 180	
BCLL 0.0 *	Rep Stress incr	YES WB 0.46	Horz(CT) 0	.03 10	n/a n/a	Weight: 162 lb FT = 20%
BCDL 10.0	Code IRC2015/TPI	2014 Matrix-MS				110,9111 102 10 2070

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.1 2x4 SP No.3

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

4-14, 8-10

BOT CHORD WEBS

1 Row at midpt

(size) 14=0-3-8, 10=0-3-8 REACTIONS.

Max Horz 14=268(LC 9)

Max Uplift 14=-126(LC 12), 10=-105(LC 13) Max Grav 14=1166(LC 20), 10=1106(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-371/232, 4-5=-1320/333, 7-8=-1329/336, 8-9=-291/175, 2-14=-394/223,

TOP CHORD 9-10=-292/156

13-14=-144/1184, 11-13=-37/920, 10-11=-123/1053 BOT CHORD

7-11=-148/619, 8-11=-269/279, 5-13=-145/607, 4-13=-257/277, 4-14=-1203/32, WEBS

8-10=-1250/97, 5-7=-795/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; and vertical left and right exposed; end vertical left exposed; end vertical le exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 10. This connection is for uplift only and does not consider lateral forces.

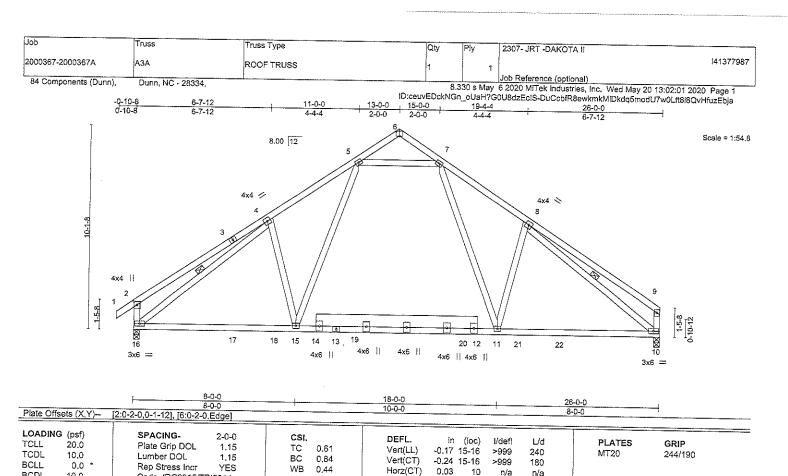
6) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE.

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BRACING-

TOP CHORD

BOT CHORD

WEBS

n/a

except end verticals.

1 Row at midpt

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 4-10-6 oc purlins,

4-16, 8-10

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2

10.0

BOT CHORD 2x4 SP No.2 *Except* 12-14: 2x8 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 16=0-3-8, 10=0-3-8 Max Horz 16=268(LC 9)

Max Uplift 16=-126(LC 12), 10=-105(LC 13) Max Grav 16=1159(LC 20), 10=1099(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

TOP CHORD 2-4=-384/226, 4-5=-1282/338, 7-8=-1291/341, 8-9=-319/169, 2-16=-412/219,

9-10=-307/152

BOT CHORD 15-16=-147/1155, 11-15-40/897, 10-11=-126/1025

WEBS 7-11=-149/596, 8-11=-271/278, 5-15=-146/583, 4-15=-260/275, 4-16=-1133/44,

8-10=-1179/109, 5-7=-772/291

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MS

3) All plates are 3x4 MT20 unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16 and 10. This connection is for uplift only and does not consider lateral forces.

7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



FT = 20%

Weight: 187 lb

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



2307- JRT -DAKOTA II Qty Ply Truss Type 141377988 Job Truss ROOF TRUSS 2000367-2000367A Α4 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:02 2020 Page 1 Dunn, NC - 28334, ID:ceuvEDckNGn_oUaH?G0U8dzEcIS-i4I_o?SmPEscMWKPIKMKI?AfcKNJOKquM4eqBKzEbjZ 84 Components (Dunn), 0-0 | 15-0-0 0-0 | 2-0-0 19-4-4 11-0-0 -0-10-8 0-10-8 Scale = 1:63.1 3x4 = 6 3x4 🛷 3x4 ≥ 8.00 12 5 4x4 🗸 4x4 > 3x4 / 10-1-8 3x4 || 4x4 | 19 12 15 14 13 10 17 18 4x6 || 4x6 [] 3x6 = 4x6 |1 3x4 = 3x4 == 3x6 8x8 === 4x6 || 18-0-0 Plate Offsets (X,Y)--[2:0-2-0,0-1-12], [6:0-2-0,Edge] **PLATES** GRIP Цd 1/defl DEFL in (loc) CSI. 2-0-0 SPACING-LOADING (psf) 240 MT20 244/190 -0.17 15-16 >999 TÇ 0.63 Vert(LL) Plate Grip DOL 1.15 20.0 TCLL 180 >999 -0.26 15-16 1,15 ВĊ 0.79 Vert(CT) Lumber DOL TCDL 10,0 n/a WB 0.41 Horz(CT) 0.03 10 n/a 0.0 Rep Stress Incr YES BCLL FT = 20% Weight: 186 lb Matrix-MS Code IRC2015/TPI2014 BCDL 10.0 BRACING-LUMBER-Structural wood sheathing directly applied or 4-9-10 oc purlins, TOP CHORD TOP CHORD 2x4 SP No.2 except end verticals. 2x4 SP No.2 *Except* BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 12-14: 2x8 SP No.2 1 Row at midpt 4-16, 8-10 WEBS

WEBS 2y4 SP No.3

REACTIONS.

(size) 16=0-3-8, 10=Mechanical

Max Horz 16=270(LC 9)

Max Uplift 16=-125(LC 12), 10=-103(LC 13) Max Grav 16=1132(LC 20), 10=1040(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-4=-381/226, 4-5=-1247/335, 7-8=-1193/334, 8-9=-265/160, 2-16=-408/219,

TOP CHORD 9-10=-273/144

15-16=-147/1125, 11-15=-46/856, 10-11=-127/943 BOT CHORD

4-15=-264/275, 5-15=-145/599, 7-11=-142/499, 8-11=-239/269, 4-16=-1095/41,

8-10=-1140/119, 5-7=-757/290

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-Č Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=103.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16. This connection is for uplift only and does not consider lateral forces.
- 8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WAKNING - verify design parameters and READ NOTES ON THIS AND INCLUDED will BE REPREDUCE PAGE MID-13 Fet. TUDIS/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of frusses and truss systems, see

ANSITPM Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qly Ply 2307- JRT -DAKOTA II 141377989 2000367-2000367A A4E Common Supported Gable Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334, 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:03 2020 Page 1 ID:ceuvEDckNGn_oUaH?G0U8dzEciS-AGJM?LTOAY_T_gvbs2tZrDjuoktX7rD2bKONkmzEbjY 4x4 === Scale = 1:65.6 8.00 12 10 6x6 🗸 12 X 3x4 |] 15 16 -7-8 32 31 30 29 26 27 26 25 24 23 22 21 20 18 17 3x4 3x4 = LOADING (psf) SPACING. 2-0-0 DEFL. in l/def L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1,15 TC 0.28 Vert(LL) -0.00 n/r 120 MT20 TCDL 10.0 244/190 Lumber DOL 1.15 0.15 Vert(CT) -0.00n/r 90 BCLL 0.0 Rep Stress Incr YES WB 0.14 Horz(CT) -0.0017 Code IRC2015/TPI2014 BCDL 10.0 Matrix-R Weight: 191 lb FT = 20% LUMBER-BRACING-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

WEBS

except end verticals.

1 Row at midpt

10-0-0 oc bracing: 31-32,30-31,29-30.

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

9-24, 8-25, 10-23

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. All bearings 25-9-0.

(lb) - Max Horz 32=270(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 25, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 32=-281(LC 8),

17=-302(LC 11), 31=-282(LC 9), 18=-268(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 25, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 32=335(LC 9),

17=306(LC 8), 24=270(LC 13), 31=332(LC 10), 18=349(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 7-8=-221/274, 8-9=-265/324, 9-10=-265/324, 10-11=-222/274

TOP CHORD **WEBS** 9-24=-291/175

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITEMS connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly and prop



b	Truss	Truss Type	Qty	Ply	2307- JRT -DAKOTA I	1	141377992
00367-2000367A	B1	Roof Special	1	1	Job Reference (options	al)	
4 Components (Dunn),	Dunп, NC - 28334,		1Dream/EDekNG	8.330 s May	6 2020 MiTek Industrie	s, Inc. Wed May 20 13:02:00 cm4RnZfbTkYGQmOIR4o65	3 2020 Page 1 nl058P zEbiT
		2-3-8 6-6-	0 10-8	-8	13-0-0 13-10-8 2-3-8 0-10-8		,
		2-3-8 4-2-	8 4-2-	8	2-3-6 0-10-6		
			4x6				Scale = 1:44
	3x6	10.00 12 3x4 10.00 4x8 =	13 14	4x8 =	8 7	1.0.0	
	25	(4 3x6 =			3x6 = 2x4		
		, 2-3-8	10-8-8		13-0-0 2-3-8		
		2-3-8	8-5-0		2-3-8		
OADING (psf)		1-0-0 CSI.	DEFL. Vert(LL) -	in (loc) 0.20 9-10	l/defl L/d >762 240	PLATES GRI MT20 244	P /190
CLL 20.0 CDL 10.0		1.15 TC 0.23 1.15 BC 0.76	Vert(CT) →	0.42 9-10	>362 180	1111,20	
JUL 10.0 I	Rep Stress Incr	YES WB 0.17	Horz(CT)	D.11 7	n/a n/a	Weight: 85 lb	

TOP CHORD

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD

2x4 SP No.3 WEBS (size) 12=0-3-8, 7=0-3-8 REACTIONS.

Max Horz 12=-195(LC 8) Max Uplift 12=-42(LC 12), 7=-61(LC 13) Max Grav 12=506(LC 1), 7=572(LC 1)

FORCES. (lb) - Max, Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-2=-449/110, 2-3=-823/271, 3-4=-730/257, 4-5=-442/111, 1-12=-538/102, TOP CHORD

2-10=-375/218, 9-10=-31/338, 4-9=-357/202 BOT CHORD

WEBS 3-9=-153/441, 3-10=-172/523, 1-11=-79/358, 5-8=-48/353

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 7. This connection is for uplift only and does not consider lateral forces.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



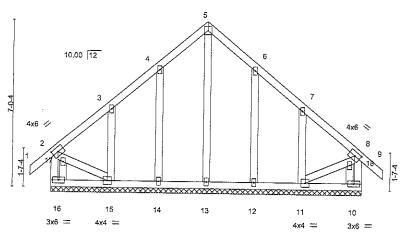
Job Truss Truss Type Oty Ply 2307- JRT -DAKOTA II 141377993 2000367-2000367A BE Common Supported Gable Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:09 2020 Page 1

ID:ceuvEDckNGn_oUaH?G0U8dzEcIS-_QgdGOY9mNkdibMlCI_z4UzzY9yXXaKw_grixQzEbjS

4x4 =

13-10-8 0-10-8

Scale = 1:46.3



13-0-0 13-0-0

Plate Offsets (X,Y)	[2:0-2-14,0-2-0]. [8:0-2-14,0-2-0]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.05 WB 0.10 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 9 -0.00 9 0.00 11	l/defi n/r n/r n/a	L/d 120 90 n/a	PLATES MT20	GRIP 244/190
		INIBUIX-6					Weight: 95 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 2x4 SP No.3 OTHERS

BRACING-

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals. BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 13-0-0.

(lb) - Max Horz 16=-202(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 12 except 15=-169(LC 12), 11=-165(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

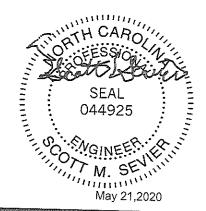
NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; Willia, Addit 1-10, Valid-Toothan (0-second gast) vasad-tooman, Toole-olopsi, 0-olopsi, 11-olot, Cat. II, EAP B, Encic MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right
- exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/ITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Ply 2307- JRT -DAKOTA II Truss Type Job Truss 141377994 2000367-2000367A BGR Roof Special Girder Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:12 2020 Page 1 Dunn, NC - 28334, 84 Components (Dunn), ID:ceuvEDckNGn_oUaH?G0U8dzEcIS-P?MmuQa22I7CZ35KuRXgi6bRNMmNkm5Mge3MYIzEbjF 10-8-8 13-0-0 Scale = 1:44.6 4x4 | 3 10,00 12 3x6 4 3x6 💉 3x6 🛇 3x6 4 1-7-4 1-7-4 9 15 16 17 5x12 14 12 LUS26 8x8 = LUS26 5x12 == 13 7 LUS26 2x4 || 4x4 = 2x4 il 4x4 = LUS26 LUS26 13-0-0 2-3-8 [9:0-4-0,0-4-8] Plate Offsets (X,Y)-PLATES GRIP DEFL. in Udefi 1/d CSI. (loc) 2-0-0 SPACING-LOADING (psf) 244/190 MT20 TC 0.29 Vert(LL) -0.04 9-10 >999 240 Plate Grip DOL 1 15 20.ó TCLL 180 вс 0.87 Vert(CT) -0.09 9-10 >999 TCDL Lumber DOL 1.15 10.0 Horz(CT) 0.06 6 n/a n/a WB 0.74 0.0 * Rep Stress Incr NO BCLL FT = 20% Weight: 197 lb Matrix-MS Code IRC2015/TPI2014 BCDL 10.0 BRACING-LUMBER-Structural wood sheathing directly applied or 6-0-0 oc purlins, TOP CHORD TOP CHORD 2x4 SP No.2 except end verticals. 2x6 SP No.2 *Except* BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2-11.4-7: 2x4 SP No.3 2x4 SP No.3 WEBS

REACTIONS.

(size) 12=0-3-8, 6=0-3-8 Max Horz 12=178(LC 11)

Max Uplift 12=-413(LC 12), 6=-362(LC 13) Max Grav 12=3735(LC 1), 6=3288(LC 1)

FORCES, (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

1-2=-4156/562, 2-3=-3092/450, 3-4=-3092/450, 4-5=-4254/553, 1-12=-3176/425,

5-6=-3255/423

10-11=-44/458, 2-10=-171/1009, 9-10=-468/3281, 8-9=-453/3338, 4-8=-154/1095 BOT CHORD 3-9=-449/3582, 4-9=-1141/267, 2-9=-1078/274, 1-10=-380/3096, 5-8=-414/3223 WEBS

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows:
 - Top chords connected as follows: 2x4 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc, 2x4 - 1 row at 0-7-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

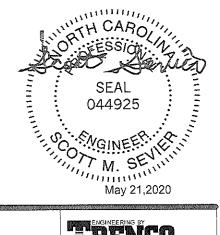
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 12, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 6. This
- connection is for uplift only and does not consider lateral forces. 9) Use Simpson Strong-Tie LUS26 (4-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-0-12 from the left

end to 9-0-12 to connect truss(es) to back face of bottom chord. 10) Fill all nail holes where hanger is in contact with lumber.

Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 997 ib down and 123 ib up at 10-10-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard Continued on page 2

> MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guildance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
>
> ANS/ITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	2307- JRT -DAKOTA II
2000367-2000367A	BGR	Roof Special Girder	1		I41377994
84 Components (Dunn)	Duon NC 28224				Job Reference (optional)

84 Components (Dunn),

Dunn, NC - 28334.

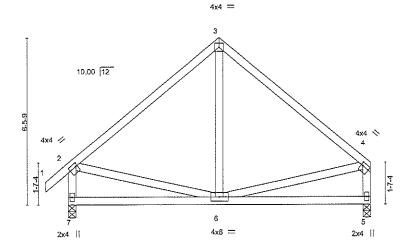
8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:12 2020 Page 2 ID:ceuvEDckNGn_oUaH?G0U8dzEcIS-P?MmuQa22I7CZ35KuRXgi6bRNMmNkm5Mge3MYIzEbjP

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Dead + Roor Live (paranceu): Lumber micrease - 1.10, Frate micrease - 1.10
Uniform Loads (pif)
Vert: 1-3=-60, 3-5=-60, 11-12=-20, 8-10=-20, 6-7=-20
Concentrated Loads (lb)
Vert: 8=-997(B) 13=-998(B) 14=-1003(B) 15=-1003(B) 16=-1003(B) 17=-1003(B)



Scale = 1:42.7



5-10-0 1 11-8-0 5-10-0 5-10-0

Plate Offsets (X,Y)	[2:0-0-12,0-1-8], [4:0-0-12,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.50 BC 0.28 WB 0.08 Matrix-MS		oc) I/defl L/d 5-6 >999 240 5-6 >999 180 5 n/a n/a	PLATES GRIP MT20 244/190 Weight: 71 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 7=0-3-8, 5=0-3-8

Max Horz 7=181(LC 9)

Max Uplift 7=-56(LC 12), 5=-37(LC 13) Max Grav 7=519(LC 1), 5=452(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-422/118, 3-4=-415/112, 2-7=-468/160, 4-5=-401/117

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This
 connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a russ system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see MSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty 2307- JRT -DAKOTA II 141377996 2000367-2000367A CE Common Supported Gable Job Reference (optional)

8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:14 2020 Page 1
ID:ceuvEDckNGn_cUaH?G0U8dzEcIS-LOUWJ6clawNwoMFI?rZ8nXgqHAfqCqzf7yYTcdzEbjN 84 Components (Dunn), Dunn, NC - 28334,

5-10-0

Scale = 1;42,7

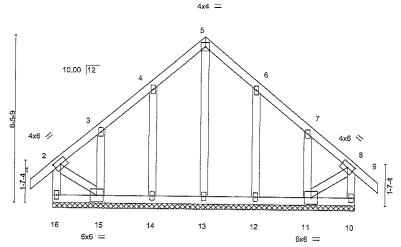


Plate Offsets (X,Y) [2:0-2-14,0-2-0], [8:0-2-14,0-2-0]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.04 WB 0.08 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 9 -0.00 9 0.00 10	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 82 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3

BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 11-8-0.

(lb) - Max Horz 16=188(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 12 except 15=-159(LC 12), 11=-154(LC 13)

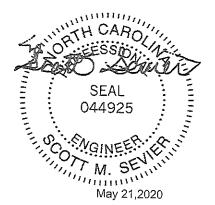
Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITeWo connectors. This design is based only upon parameters shown at for as individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracking indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312. Alexandria, VA 22314.



Qty Ply 2307- JRT -DAKOTA II Truss Type Job Truss 141377997 2000367-2000367A CGR Common Girder 2 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:16 2020 Page 1 Dunn, NC - 28334, 84 Components (Dunn), ID:ceuvEDckNGn_oUaH?G0U8dzEclS-HmcHkodY6Xdd2gO57Gccsyl?8z89gdkybG1ZhWzEbjL 11-8-0 5-10-0 Scale = 1:40.9 4x4 == 10.00 12 3x6 📏 3x6 4 3 10 8 5 111526 LUS26 6 Av4 == LUS26 LUS26 8x8 = 4x4 ==

Plate Offsets (X,Y)	5:0-4-0,0-4-12]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.82 BC 0.88 WB 0.57 Matrix-MS	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.05 5-6 >999 240 MT20 244/190 Vert(CT) -0.10 5-6 >999 180 Horz(CT) Horz(CT) 0.00 4 n/a n/a Weight: 156 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUS26

LUMBER-

2x4 SP No.2 TOP CHORD 2x6 SP No.2 BOT CHORD 2x4 SP No.3 WEBS

REACTIONS.

(size) 6=0-3-8, 4=0-3-8

Max Horz 6=-164(LC 31) Max Uplift 6=-315(LC 12), 4=-333(LC 13)

Max Grav 6=2867(LC 1), 4=3028(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-2517/350, 2-3=-2518/350, 1-6=-2052/302, 3-4=-2050/302 TOP CHORD

5-6-211/468, 4-5=-110/387 BOT CHORD

2-5=-265/2762, 1-5=-198/1516, 3-5=-200/1508 WEBS

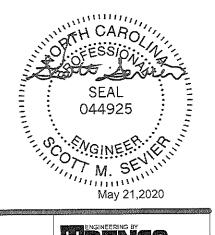
NOTES-

- 1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1,60 plate grip DOL=1,60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 6, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 4. This connection is for uplift only and does not consider lateral forces.
- 9) Use Simpson Strong-Tie LUS26 (4-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 10-0-12 to connect truss(es) to back face of bottom chord.
- 10) Fill all nail holes where hanger is in contact with lumber.

Continued on page 2

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall a truss system, Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



dob	Truss	Truss Type	Qty	Ply	2307- JRT -DAKOTA II
2000367-2000367A	CGR	Common Girder	1		l41377997
84 Components (Dunn)	Dunn NC 20224		·		Job Reference (optional)

84 Components (Dunn),

Dunn, NC - 28334.

8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:16 2020 Page 2 ID:ceuvEDckNGn_oUaH?G0U8dzEciS-HmcHkodY6Xdd2gO57Gccsyl?8z89gdkybG1ZhWzEbjL

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 4-6=-20

Concentrated Loads (lb)

Vert: 5=-997(B) 7=-997(B) 8=-997(B) 9=-997(B) 10=-997(B)



lob	Truss	Truss Type	Qty	Ply	2307- JRT -DAKOTA II	ī41378000
2000367-2000367A	V2	Valley	1	1	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		<u> </u>	8.330 s May	6 2020 MiTek Industries.	Inc. Wed May 20 13:02:18 2020 Page 1
84 Components (Donn),	Dani, 110 2000 11	3-4-8	ID:ceuvEDck	kNGn_oUaH?0 6-9-0	60U8dzEcIS-E9j19Tfoe8tL	H_YUEhe4yNqVsn0y8fjF2aWgmPzEbjJ
	 	3-4-8		3-4-8		
			4x4 ==			Scale = 1:20.
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			2			
		10.00 12				
		10,00 [12	// \			
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	2-9-12					
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			4		2x4 📎	
	2x-	1 //	2x4		2,4 1	
	0-0 <u>-5</u> 0-0-5		6-9-0 6-8-11			
	0-0-5					
LOADING (psf)	SPACING-	2-0-0 CSI.	DEFL.	in (loc)	l/defl L/d n/a 999	PLATES GRIP MT20 244/190
TCLL 20.0	Plate Grip DOL.	1.15 TC 0.16 1.15 BC 0.09		n/a - n/a -	п/а 999 n/a 999	W1120 244100
TCDL 10.0 BCLL 0.0	Lumber DOL Rep Stress Incr	1.15 BC 0.09 YES WB 0.03		0.00 3	n/a n/a	
BCDL 10.0	Code IRC2015/TPI					Weight: 25 lb FT = 20%
		<u>, </u>	BRACING-			
LUMBER- TOP CHORD 2x4 SP	No.2		TOP CHORE	D Structi	ral wood sheathing dire	ctly applied or 6-0-0 oc purlins.
DOT CHORD 2v4 SP			BOT CHORE	D Rigid o	eiling directly applied or	Tu-u-u oc bracing.

BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

(size) 1=6-8-6, 3=6-8-6, 4=6-8-6 REACTIONS.

Max Horz 1=-61(LC 8)

Max Uplift 1=-29(LC 13), 3=-36(LC 13) Max Grav 1=136(LC 1), 3=136(LC 1), 4=203(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WAKNING - verify design parameters and READ NUTES ON THIS AND INCLUDED MITER REFERENCE PAGE MINTATS rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty 2307- JRT -DAKOTA II 141378001 2000367-2000367A V3 Valley 1 Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334, 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:19 2020 Page 1 ID:ceuvEDckNGn_oUaH?G0U8dzEcIS-iLHPMpfRPS?Cv77goP9JUbNhiBLDt6POHEGDIrzEbjI 3-6-10 3x4 == Scale = 1:10,2 2 10.00 12 3 2-0-6 50.4 2x4 // 2x4 📎 3-6₀10 0-0-5

Plate Offsets (X,Y)-[2:0-2-0,Edge] LOADING (psf) SPACING-2-0-0 CSI, DEFL. in I/defI L/d (loc) PLATES GRIP TCLL 20.Ó Plate Grip DOL 1.15 TC BC 0.03 Vert(LL) n/a 999 n/a MT20 244/190 TCDL 10.0 Lumber DOL 1 15 0.09 Vert(CT) n/a n/a 999 **BCLL** 0.0 Rep Stress Incr YES WB 0.00 Horz(CŤ) 0.00 3 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-P Weight: 11 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

(size) 1=3-6-0, 3=3-6-0

Max Horz 1=-28(LC 8)

Max Uplift 1=-10(LC 12), 3=-10(LC 13) Max Grav 1=110(LC 1), 3=110(LC 1)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

ORTH CAR WI. SEVIEW May 21,2020

Structural wood sheathing directly applied or 3-6-10 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTE460 competors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component
Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



				Qty	Ply	2207_ 11	RT -DAKOTA II		
lob	Truss	Truss Type		City	r-iy	2307-31	CI -DANOTA II		141378002
2000367-2000367A	V4	Valley		1	1		(
				8.3	30 e Mar	Job Refe	rence (optional) iTek Industries, In	c. Wed May 20 13	:02:20 2020 Page 1
84 Components (Dunn),	Dunn, NC - 28334,		ID:ceuvEI	o.s. DckNGn_oU	aH?G0U	8dzEcIS-A	Xroa9g3Am73Wh	lisM6gY1owoGbg0	QcZzYVu?nqHzEbjH
	L	4-3-11			8-7 4-3-	-6			
	E	4-3-11			43-	77			
			4x4 =						Scale = 1:24.4
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	0-0-5		8-7-6						
	0-0 <u>-5</u> 0-0-5		8-7-2				3	,	
	SPACING-	2-0-0 CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
LOADING (psf) TCLL 20.0		1,15 TC 0.30				n/a	999	MT20	244/190
TCDL 10.0		1.15 BC 0.15	Vert(C	Ť) n/a		n/a	999		
BCLL 0.0 *		YES WB 0.04	Horz(C	O.00 (T	3	n/a	n/a	Weight: 32 lb	FT = 20%
BCDL 10.0	Code IRC2015/TPI2	014 Matrix-P					<u></u>	weight. 32 lb	11 - 2076
LUMBER-			BRAC	ING-					
TOP CHORD 2x4 SP I	No.2		TOP C	HORD				y applied or 6-0-0	oc purlins.
BOT CHORD 2V4 SP No 2				BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.					

BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

(size) 1=8-6-13, 3=8-6-13, 4=8-6-13 Max Horz 1=-80(LC 8)

Max Uplift 1=-37(LC 13), 3=-47(LC 13)

Max Grav 1=179(LC 1), 3=179(LC 1), 4=266(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/ITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



2307- JRT -DAKOTA II Truss Type Job Truss Qty 141378003 2000367-2000367A V5 Valley | Job Reference (optional) | 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:20 2020 Page 1 Dunn, NC - 28334, 84 Components (Dunn). ID:ceuvEDckNGn_oUaH?G0U8dzEcIS-AXroa9g3Am73WHisM6gY1owrRbi_cZLYVu?nqHzEbjH Scale = 1:17.1 4x4 == 2 10.00 12 5-0-0 0-0-4 2x4 |] 2x4 🛇 2x4 // LOADING (psf) CRIP SPACING-2-0-0 CSI. DEFI in (loc) l/defl Ľ∕d **PLATES** TCLL 20.0 Plate Grip DOL 1.15 TC 0.09 Vert(LL) n/a n/a 999 MT20 244/190 вс TCDL 10.0 Lumber DOL 1.15 0.05 Vert(CT) n/a n/a 999 WB BCLL 0.0 Rep Stress Incr YES 0.02 Horz(CT) 0.00 n/a п/а BCDI. 10.0 Code IRC2015/TPI2014 Matrix-P Weight: 19 lb FT = 20%**BRACING-**LUMBER-TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

(size) 1=5-4-6, 3=5-4-6, 4=5-4-6

Max Horz 1=-48(LC 8)

Max Uplift 1=-22(LC 13), 3=-28(LC 13)

Max Grav 1=106(LC 1), 3=106(LC 1), 4=157(LC 1)

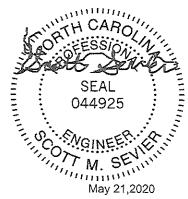
FORCES. (lb) - Max, Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, defivery, erection and bracing of trusses snath truss systems, see MSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N, Lee Street, Suite 312, Alexandria, VA 22314.

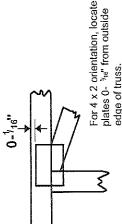


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



plates 0- 've" from outside edge of truss.

This symbol indicates the

required direction of slots in

connector plates

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

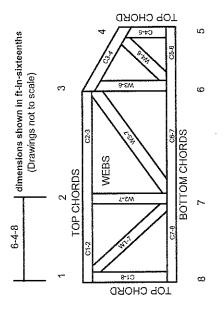
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. DSB-89: Design Standard for Bracing.

DSB-89: BCSI:

Beagn Component Safety Brains.
Guide to Good Practice for Handling, Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282 Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

Ceneral Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.

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- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- . Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- i0. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T. I.							
dob	Truss	Truss Type	Qty	Ply	2307- JRT -DAKOTA II		
2000367-2000367A	V1	Valley	1	1		J413	377999
84 Components (Dunn),					Job Reference (optional)		
64 Components (Dunn),	Dunn, NC - 28334,		8,	330 s May	6 2020 MiTek Industries	Inc. Wed May 20 13:02:17 2020, Box	e 1
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	0-9 <u>-5</u> 0-0-5	9-11-	-6				
	0-0-5	9-11-	2			-	
LOADING (psf)	SPACING- 2-0-0	csi.	EFL. in	(loc)	I/defi L/d	PLATES GRIP	
TCLL 20.0	Plate Grip DOL 1,15	TC 0.30 Ve	ert(LL) n/a	` -	n/a 999	MT20 244/190	
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES		ert(CT) n/a		n/a 999		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	orz(CT) 0.00	3	n/a n/a	Weight: 38 lb FT = 20%	
						77 Signit 30 to 17 - 20%	
LUMBER- TOP CHORD 2x4 SP No	. 2		RACING- OP CHORD	Ot	dd.abd.s		
BOT CHORD 2x4 SP No			OT CHORD	Structura Rigid cei	il wood sheathing directly ling directly applied or 10	y applied or 6-0-0 oc purlins.	
OTHERS 2x4 SP No	.3		. 5110110	. aga oo	mig amoody applied Of To	7-0-0 00 bracing.	
	1						

REACTIONS. (s

(size) 1=9-10-13, 3=9-10-13, 4=9-10-13

Max Horz 1=-94(LC 8)

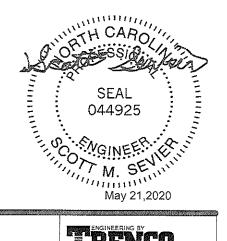
Max Uplift 1=-32(LC 13), 3=-43(LC 13), 4=-7(LC 12)

Max Grav 1=194(LC 1), 3=194(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and Is for an individual building component, not a russ system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see MSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



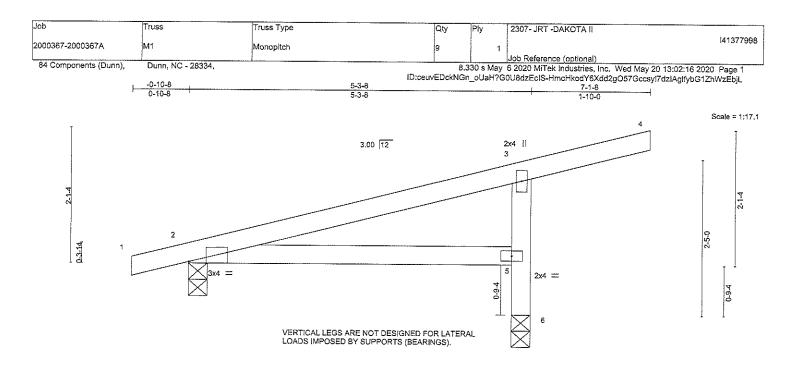


Plate Offsets (X,Y) [2:0-3-4,Edge]							
CADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.28 BC 0.24 WB 0.00 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	0.03	oc) [/de 5-9 >99 5-9 >99 6 n/	9 240 9 180	PLATES MT20 Weight: 23 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-3-8 oc purlins,

except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=92(LC 11)

Max Uplift 2=-58(LC 8), 6=-96(LC 12)

Max Grav 2=240(LC 1), 6=343(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 5-6=-343/231, 3-5=-283/242

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. MARKINIO - Verity design parameters and KEAD MOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MIN-14/3 FEV. DIVISIZED BEFORE USE.

Design valid for use only with MITek occupancy. The design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

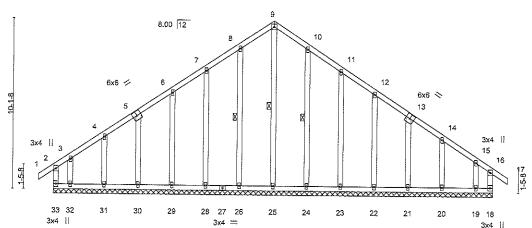
ANSI/TP11 Quality Criteria, DSB-99 and ECSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qtγ Plv 2307- JRT -DAKOTA II 141377990 2000367-2000367A ΑE GABLE Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:05 2020 Page 1 ID:ceuvEDckNGn_oUaH?G0U8dzEcIS-6fR7Q1Vei9EBD_2_zTv1weoEXYZ2bihL32tUofzEbjW 11-0-0 11-0-0 -0<u>-10-8</u> 0-10-8 13-0-0 2-0-0 15-0-0 2-0-0

4x4 =

Scale = 1:65.6



LOADING (psf) SPACING-2-0-0 1,15 CSI. DEFL in l/defl (loc) L/d PLATES GRIP TCLL 20.0 Plate Grip DOL TC 0.27 Vert(LL) -0.00 17 120 n/r MT20 244/190 TCDL 10.0 Lumber DOL BC 1.15 0.15 Vert(CT) -0.00 17 n/r 90 **BCLL** 0.0 Rep Stress Incr YES WR 0.14 Horz(CT) 0.01 18 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-R Weight: 194 (b FT = 20%

26-0-0

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

1 Row at midpt

BOT CHORD WEBS

Rigid ceiling directly applied or 6-0-0 oc bracing. 9-25, 8-26, 10-24

REACTIONS. All bearings 26-0-0.

(lb) - Max Horz 33=273(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 26, 28, 29, 30, 31, 24, 23, 22, 21, 20 except 33=-260(LC 8),

18=-209(LC 9), 32=-255(LC 9), 19=-216(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 18, 26, 28, 29, 30, 31, 24, 23, 22, 21, 20 except 33=303(LC 11), 25=269(LC 13), 32=320(LC 10), 19=279(LC 11)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 7-8=-218/276, 8-9=-262/327, 9-10=-262/327, 10-11=-218/276

WEBS 9-25=-294/172

NOTES-

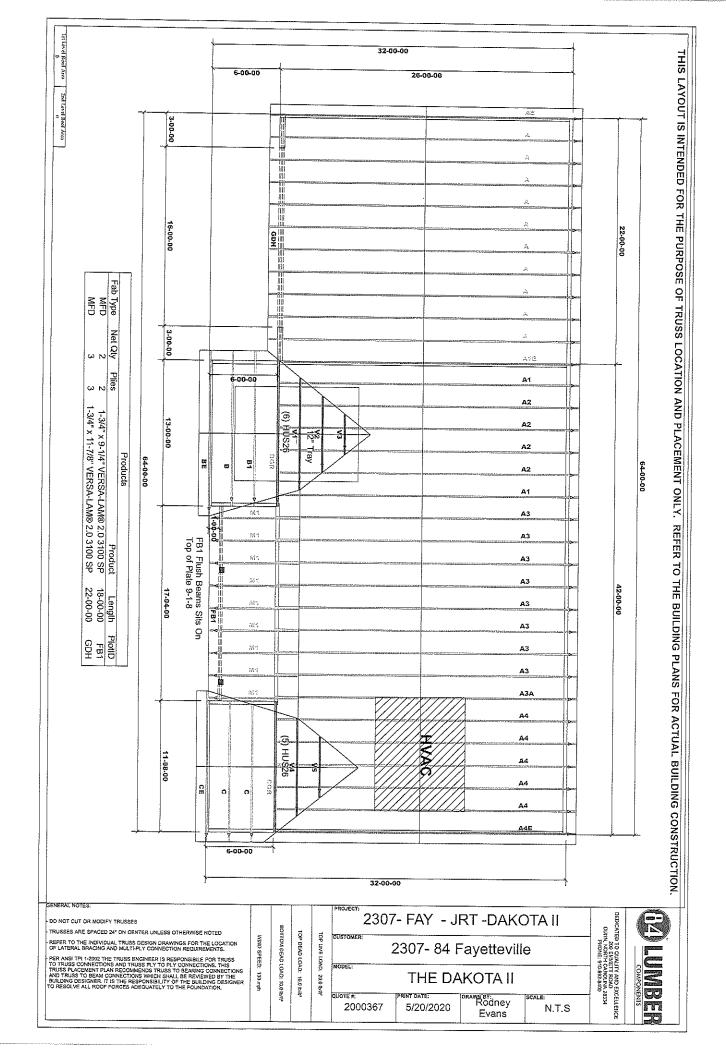
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) N/A
- 11) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Job Truss Truss Type Qty 2307- JRT -DAKOTA II 141377991 2000367-2000367A В Common 84 Components (Dunn), Dunn, NC - 28334 ID:ceuvEDckNGn_oUaH?G0U8dzEcIS-21ZtrjWvEmUvSHCM5txV?3tUsLBD3g4dWMMbtXzEbjU 13-10-8 0-10-8 13-0-0 4x4 === Scale = 1:44,6 10.00 12 6x6 4 6x6 📏 1-7-4 6 2x4 || 4x8 = 2x4 || Plate Offsets (X,Y)---[1:0-2-12,0-1-8], [3:0-2-12,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) I/defi 1// **PLATES** GRIP TOLL 20.0 Plate Grip DOL 1.15 TC 0.66 Vert(LL) -0.03 6-7 >999 240 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 ВС 0.36 Vert(CT) -0.07 6-7 >999 180 BCLL 0.0 Rep Stress Incr YES WB 0.09 Horz(CT) 0.00 5

BRACING-

TOP CHORD

BOT CHORD

n/a

except end verticals.

n/a

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 78 lb

FT = 20%

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

10.0

2x4 SP No.3 WEBS

REACTIONS. (size) 7=0-3-8, 5=0-3-8

Max Horz 7=-195(LC 10)

Max Uplift 7=-42(LC 12), 5=-61(LC 13) Max Grav 7=506(LC 1), 5=572(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code #RC2015/TPI2014

TOP CHORD 1-2=-473/120, 2-3=-480/127, 1-7=-449/126, 3-5=-515/169

BOT CHORD 6-7=-187/265

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Matrix-MS

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at Jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.



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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314,

