

Trenco 818 Soundside Rd Edenton, NC 27932

Re: B0120-0229 Southport A-B-D

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E14048169 thru E14048190

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



February 7,2020

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





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members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=105, 8=105.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-60, 2-8=-20, 13-14=-60





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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-0 to 3-7-13, Interior(1) 3-7-13 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.



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Design Valid for use only with Mil eK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	Southport A-B-D		
B0120-0229	B2	COMMON	6	1			E14048174
Comtech. Inc. Fay	vetteville. NC 28309			8.130 s N	Job Reference (optiona Mar 11 2018 MiTek Indust	l) ries. Inc. Fri Feb 7 11	:31:07 2020 Page 1
	-0-10-8 7-9-4	15-6-0	ID:nl3Qa?g25pD	AIPQix6eo	V?zvBz3-I9WU6MTUJ?JI 31-0-0	MI7F24MYcO9dHHuYL	.hi_WzgbvK6znY6Y
	0-10-8 7-9-4	7-8-12	7-8-	12	7-9-4	0-10-8	
			5x5 =				Scale = 1:72.4
			5				
	I						
		9.00 12					
		14		15			
			// \\ `	$\langle \rangle$			
	4x6 -	2x4 \\			2x4 //		
-7-8					4x6 ×		
~							
	13			. /		16	
			\	$\langle / $		\searrow	
	2			$\mathbb{N}//\mathbb{N}$		8	
		¥	[•			ıېا د <mark>او</mark>	4-10
	ら 図 4x8	12	11	10		⊠ 4x8	6
		3x4 =	5x8 =	3x4 =			
	10-4	-3	20-7-13 10-3-11		<u>31-0-0</u> 10-4-3		
Plate Offsets (X,Y)	[2:0-0-3,0-0-4], [2:0-0-6,0-5-6],	8:0-0-3,0-0-4], [8:0-0-6,0-5-6]					
LOADING (psf)	SPACING- 2-0-		DEFL. i	n (loc)	l/defl L/d	PLATES	GRIP
TCDL 10.0	Lumber DOL 1.1	BC 0.54	Vert(CT) -0.2	1 10-12	>999 360	WI 20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YE Code IRC2015/TPI2014	S WB 0.57 Matrix-S	Horz(CT) 0.03 Wind(LL) 0.03	38 2-12	n/a n/a >999 240	Weight: 229 lb	FT = 20%
			BRACING-	-			
TOP CHORD 2x6 SP	P No.1		TOP CHORD	Structu	ral wood sheathing dired	ctly applied or 5-8-6 o	oc purlins.
WEBS 2x4 SP	9 No.1 9 No.2		BOI CHORD	Rigid ce	eiling directly applied or	10-0-0 oc bracing.	
WEDGE Left: 2x6 SP No.1. Right	nt: 2x6 SP No.1						
REACTIONS (Ib/size	a) 2-1280/0-5-8 8-1280/0-5-	2					
Max H	lorz 2=-292(LC 10)						
Max U Max G	lplift 2=-70(LC 12), 8=-70(LC 13 6rav 2=1411(LC 19), 8=1411(LC	20)					
FORCES. (lb) - Max.	Comp./Max. Ten All forces 25	0 (lb) or less except when shown	L.				
TOP CHORD 2-4=-	-1828/350, 4-5=-1683/480, 5-6=	1684/480, 6-8=-1828/350					
WEBS 5-10=	=-188/897, 6-10=-476/323, 5-12	=-120/1335 =-188/896, 4-12=-476/323					
NOTES-							
1) Unbalanced roof live	e loads have been considered fo	r this design. sd=103mph; TCDI =6 0psf; BCDI	I –6 Opsf: h–15ft: Cat II	Evn C: e	nclosed:		
MWFRS (envelope)	and C-C Exterior(2) -0-9-0 to 3-	7-13, Interior(1) 3-7-13 to 15-6-0,	Exterior(2) 15-6-0 to 19	-10-13 zo	ne;C-C for		
3) This truss has been	designed for a 10.0 psf bottom	hord live load nonconcurrent with	h any other live loads.				
 4) * This truss has been between the bottom 	n designed for a live load of 30. chord and any other members.	psf on the bottom chord in all are with BCDL = 10.0psf.	eas with a clearance gre	ater than	6-0-0	annun (CAD
5) Provide mechanical	connection (by others) of truss	o bearing plate capable of withsta	anding 100 lb uplift at joi	int(s) 2, 8.		""RTH	



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lob	Truss	Truss Type	Otv	Plv	Southport A-B-D		
B0120-0229	B4		1	1			E14048176
Comtoch Inc. Equati			•	9 120 c M	Job Reference (option	nal) Intring Ing, Fri Fob, 7.11	·21:08 2020, Page 1
Connech, Inc., Payetti	eville, NC 20309		ID:nI3Qa?g25pD	AIPQix6eo	V?zvBz3-DL4sJiU64J	SDNHqEe43rxMAMJIuV	Q9AfCKKSsYznY6X
	-0-10-8 7-9-4	15-6-0	20-2-11	23-2 21-2-11 1-0-0 2-1	2-12 25-10-1	30-9-0	
	0-10-0 7-3-4	7-0-12	5x5 =	1-0-0 2-1	2-1-5	4-10-13	Scale = 1:73.0
			5	18			
		_		10			
		9.00 12		4x	6 =		
		17		5x5 :	=		
I			// \\ `) in the second	<		I
	4x6 -	2x4 \\			4x4 \		
		4			4x6 \		
Ņ		3			¥ 19	6x10 M18SHS 🚿	Ņ
8-11-	16			、 /			8-11
			\	\wedge //			
	2						Te1
		¥	[•]				1-6-0
-	4x8	15	14	13		12 11	
		3x4 =	5x8 =	3x4 =		4x8 =	
	10	4-3	20-2-11	20-7-13	30-9-0		
Plate Offsets (X,Y) [2:	0-0-6,0-5-6], [2:0-0-3,0-0-4], [7:0-3-8,Edge], [10:0-0-14,0-9-6]	9-10-8	0-3-2	10-1-3		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. i	n (loc)	l/defl L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	5 TC 0.67 5 BC 0.54	Vert(LL) -0.26 Vert(CT) -0.32	5 13-15 2 13-15	>999 360 >999 240	MT20 M18SHS	244/190 244/190
BCLL 0.0 *	Rep Stress Incr YES	WB 0.57	Horz(CT) 0.03	3 12	n/a n/a	Weight: 267 lb	ET - 20%
		Matrix-S		5 2-15	2355 240		F T = 20 /8
TOP CHORD 2x6 SP No	o.1 *Except*		TOP CHORD	Structura	al wood sheathing dir	rectly applied or 5-8-10	oc purlins,
6-7: 2x4 S BOT CHORD 2x6 SP No	5P No.1 5.1		BOT CHORD	except e Riaid cei	nd verticals, and 2-0	-0 oc purlins (6-0-0 ma or 10-0-0 oc bracing.	x.): 6-10, 6-7.
WEBS 2x4 SP No.	0.2 *Except*		WEBS	1 Row a	t midpt 8	3-12	
WEDGE	+ SF N0.3		301113	I DIACE	at 51(5). 0		
Left: 2x6 SP No.1							
REACTIONS. (lb/size) Max Horz	2=1270/0-5-8, 12=1214/Mee	chanical					
Max Uplif	t 2=-69(LC 12), 12=-53(LC 13)	3)					
Max Grav	/ Z=1400(LC 19), 1Z=1335(L0	5 20)					
FORCES. (lb) - Max. Co TOP CHORD 2-4=-18	mp./Max. Ten All forces 25 10/348, 4-5=-1663/476, 5-6=-	0 (lb) or less except when shown. 1436/418, 6-8=-1206/238, 6-7=-45	52/271,				
7-10=-58 BOT CHORD 2-15=-18	81/214, 10-12=-447/234 83/1480_13-15=0/989_12-13:	=-131/1228					
WEBS 4-15=-46	60/311, 5-15=-180/893, 8-12=	-1444/116, 8-13=-309/257, 5-13=-	-111/718				
NOTES-							
 Unbalanced roof live load Wind: ASCE 7-10; Vult= 	ads have been considered for =130mph (3-second gust) Va	[·] this design. sd=103mph; TCDL=6.0psf; BCDL=	=6.0psf; h=15ft; Cat. II;	Exp C; en	closed;		
MWFRS (envelope) and 19-10-13 to 20-3-8 zon	d C-C Exterior(2) -0-9-0 to 3-7	7-13, Interior(1) 3-7-13 to 15-6-0, E	Exterior(2) 20-3-8 to 21	-2-11, Inte	rior(1)	mini	CADIN
3) Provide adequate drain	age to prevent water ponding			.o grip 2 0 2		""RTH	9111
4) All plates are M120 pla5) This truss has been des	tes unless otherwise indicated signed for a 10.0 psf bottom c	a. hord live load nonconcurrent with a	any other live loads.			A PARTIE	Soldingin
6) * This truss has been do between the bottom cho	esigned for a live load of 30.0 ord and any other members.	psf on the bottom chord in all area with BCDL = 10.0psf.	is with a clearance gre	ater than 6	-0-0	ma ,	Ref
7) Refer to girder(s) for tru	iss to truss connections.						SEAL
9) Graphical purlin representation	entation does not depict the s	ize or the orientation of the purlin a	along the top and/or bo	ottom chore	ł.	Ξ 03	36322
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						CA	GILBE
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February 7,2020

RENCO

818 Soundside Road Edenton, NC 27932

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WEBS 4-14=-5/454, 8-12=-4/454, 5-7=-717/334

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-15 to 3-7-14, Interior(1) 3-7-14 to 10-0-0, Exterior(2) 10-0-0 to 14-1-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON MICLODED MITER REPERIENCE PAGE mit-14/3 at 900, 1002/015 BEPORE 052. Design valid for use only with MITeR works connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11 except (jt=lb) 16=335, 17=172, 18=264.



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- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=299, 7=328.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1279 lb down and 76 lb up at 1-10-12, 1279 lb down and 76 lb up at 3-10-12, 1267 lb down and 76 lb up at 5-10-12, 1232 lb down and 73 lb up at 7-10-12, 1232 Ib down and 73 lb up at 9-10-12, 1268 lb down and 68 lb up at 11-10-12, 1293 lb down and 68 lb up at 13-10-12, and 1293 lb down and 68 lb up at 15-10-12, and 1299 lb down and 62 lb up at 17-10-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2

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SEAL

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minum February 7,2020

[Job	Truss	Truss Type	Qty	Ply	Southport A-B-D		
						E14048181		
	B0120-0229	D2	Common Girder	1	2			
						Job Reference (optional)		
	Comtech, Inc., Fayet	eville, NC 28309	•		8.130 s M	ar 11 2018 MiTek Industries, Inc. Fri Feb 7 11:31:14 2020 Page 2		
	-		ID:nI3Qa?g25pDAIPQix6eoV?zvBz3-2VR8alYtf9CN5CIO_KAGAdQTkjwHqnoXaGnn4CznY6R					

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 9=-1194(B) 10=-1200(B) 8=-1194(B) 11=-1200(B) 12=-1200(B) 13=-1194(B) 14=-1194(B) 15=-1194(B) 16=-1200(B)

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 Max Uplift
 All uplift
 100 lb or less at joint(s)
 16, 10, 14, 12 except
 15=-261(LC 12), 11=-255(LC 13)

 Max Grav
 All reactions
 250 lb or less at joint(s)
 16, 10, 14, 12, 11 except
 13=297(LC 22), 15=250(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-15 to 3-7-14, Exterior(2) 3-7-14 to 6-6-0, Corner(3) 6-6-0 to 10-10-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 12 except (jt=lb) 15=261, 11=255.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





20-0-0

Plate Offset	ts (X,Y)	[17:0-4-0,0-4-8]		1		1					T	
	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 2	20.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	-0.00	12	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	-0.00	12	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	12	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	912014	Matri	x-S						Weight: 112 lb	FT = 20%
LUMBER-						BRACING-						
TOP CHOR	DP CHORD 2x4 SP No.1					TOP CHOR	2D	Structu	ral wood	sheathing di	rectly applied or 6-0-0 o	oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.3

REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 2=-92(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 12, 2, 19, 20, 21, 22, 17, 16, 15, 14

Max Grav All reactions 250 lb or less at joint(s) 12, 2, 18, 19, 20, 21, 22, 17, 16, 15, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 10-0-0, Corner(3) 10-0-0 to 14-4-13 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 2, 19, 20, 21, 22, 17, 16, 15, 14.



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	10-0-0 10-0-0				20-0-0 10-0-0					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.57 BC 0.74 WB 0.10 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.16 4-7 -0.35 2-7 0.03 4 0.05 2-7	l/defl >999 >667 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 91 lb	GRIP 244/190 FT = 20%		

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x4 SP No.1WEBS2x4 SP No.2

REACTIONS. (lb/size) 4=840/0-5-8, 2=840/0-5-8 Max Horz 2=54(LC 16) Max Uplift 4=-62(LC 13), 2=-62(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1173/280, 3-4=-1173/280

BOT CHORD 2-7=-133/993, 4-7=-133/993

WEBS 3-7=0/453

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-14 to 3-7-14, Interior(1) 3-7-14 to 10-0-0, Exterior(2) 10-0-0 to 14-4-13 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

C Vermannen 111111111 SEAL 036322 GI minum February 7,2020



818 Soundside Road Edenton, NC 27932

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-8-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



				000					
				6-0-0					1
Plate Offsets (X,Y)	[4:Edge,0-2-0]								
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 MCDL 10.0	SPACING- Plate Grip DC Lumber DOL Rep Stress In Code IBC201	2-0-0 DL 1.15 1.15 cr YES 15/TPI2014	CSI. TC 0.52 BC 0.13 WB 0.00 Matrix-P	DEFL. Vert(LL) -0 Vert(CT) -0 Horz(CT) -0 Wind(L) 0	in (lo 01 2 03 2 00 00	nc) I/defl 2-4 >999 2-4 >999 4 n/a 2 ****	L/d 360 240 n/a 240	PLATES MT20 Weight: 28 lb	GRIP 244/190
							2.0	1101g111 2010	
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF	P No.1 P No.1			BRACING- TOP CHORD	Stru	uctural wood cept end verti	sheathing di	rectly applied or 6-0-0	oc purlins,
WEBS 2x4 SF	P No.2			BOT CHORD	Rig	id ceiling dire	ctly applied	or 10-0-0 oc bracing.	

OTHERS 2x4 SP No.3

REACTIONS. (Ib/size) 2=316/0-3-8, 4=218/0-1-8 Max Horz 2=84(LC 8) Max Uplift 2=-128(LC 8), 4=-72(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-4=-161/268

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-2-0 to 3-2-13, Exterior(2) 3-2-13 to 5-10-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=128.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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	I				6-0-)					1	
Plate Offsets (X,Y)	[4:Edge,0-2-0]											
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.43 0.13 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.03 0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190	
BCDL 10.0	Code IRC2015/T	PI2014	Matrix	κ-P	Wind(LL)	0.00	2	****	240	Weight: 27 lb	FT = 20%	
LUMBER- TOP CHORD 2x4 SP No.1					BRACING- TOP CHOF	2D	Structu	ral wood	sheathing di	rectly applied or 6-0-0	oc purlins,	

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (lb/size) 2=316/0-3-8, 4=218/0-1-8 Max Horz 2=59(LC 8)

Max Uplift 2=-70(LC 8), 4=-30(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-0 to 3-2-13, Interior(1) 3-2-13 to 5-10-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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			5-0-0 5-0-0					
Plate Offsets (X,Y)	[4:Edge,0-2-0]	· · · · · · · · · · · · · · · · · · ·						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.27 BC 0.08 WB 0.00	DEFL. in Vert(LL) -0.01 Vert(CT) -0.01 Horz(CT) 0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.00	2	****	240	Weight: 22 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF	2 No.1	BRACING- TOP CHORD	Structur	ral wood sl	heathing dire	ectly applied or 5-0-0	oc purlins,	

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=278/0-3-8, 4=176/0-1-8 Max Horz 2=50(LC 8)

2x4 SP No.2

Max Uplift 2=-67(LC 8), 4=-24(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

WEBS

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-0 to 3-2-13, Interior(1) 3-2-13 to 4-10-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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3x4 =

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.24	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.06	Vert(CT)	0.01	1	n/r	120		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 17 lb	FT = 20%

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LUMBER-
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TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=110/4-0-0, 2=240/4-0-0, 4=40/4-0-0

Max Horz 2=44(LC 8) Max Uplift 3=-45(LC 12), 2=-60(LC 8)

Max Grav 3=110(LC 1), 2=240(LC 1), 4=80(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Corner(3) -1-2-0 to 3-2-13, Exterior(2) 3-2-13 to 4-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing. 4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED WITER REFERENCE PAGE MIL-14's rev. 10/03/2013 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





	5-6-0				11-0-0		
1	5-6-0		1		5-6-0		I
Plate Offsets (X,Y)	[2:0-2-0,Edge], [4:0-2-0,Edge]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IpcrYES	CSI. TC 0.33 BC 0.25 WB 0.06	DEFL. in Vert(LL) -0.02 Vert(CT) -0.05 Horz(CT) 0.01	(loc) l/defl 2-6 >999 2-6 >999	L/d 360 240	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.03	4-6 >999	240	Weight: 44 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF OTHERS 2x4 SF REACTIONS. (lb/siz: Max H Max L	 No.1 No.1 No.2 No.3 e) 2=490/0-3-8, 4=490/0-3-8 torz 2=-54(LC 13) Jplift 2=-120(LC 12), 4=-120(LC 13) 		BRACING- TOP CHORD BOT CHORD	Structural woc Rigid ceiling d	d sheathing dir irectly applied c	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins.
FORCES.(lb) - Max.TOP CHORD2-3=BOT CHORD2-6=WEBS3-6=	Comp./Max. Ten All forces 250 (lb) or -666/448, 3-4=-666/448 -267/553, 4-6=-267/553 0/257	less except when shown.					
NOTES-							

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 5-6-0, Corner(3) 5-6-0 to 9-10-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

between the bottom chord and any other members. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=120, 4=120.



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F	5-6-0								
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.30 BC 0.25 WB 0.06 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.02 2-6 -0.05 2-6 0.01 4 0.02 4-6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 40 lb	GRIP 244/190 FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 WEBS

REACTIONS. (lb/size) 2=490/0-3-8, 4=490/0-3-8 Max Horz 2=-32(LC 17) Max Uplift 2=-42(LC 12), 4=-42(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-666/231, 3-4=-666/231

BOT CHORD 2-6=-114/553, 4-6=-114/553 3-6=0/257

WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 5-6-0, Exterior(2) 5-6-0 to 9-10-13 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED WITER REFERENCE PAGE MIL-14's rev. 10/03/2013 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



