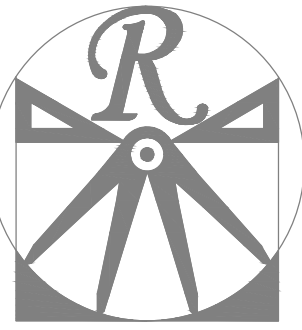


PLANS DESIGNED TO THE
2018 NORTH CAROLINA STATE
RESIDENTIAL BUILDING CODE.

LOT 75
THOMAS FARM
TBD OVERHILLS RD
SPRING LAKE, NC

GENERAL NOTES

1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
3. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED.
4. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL SITE REQUIREMENTS.



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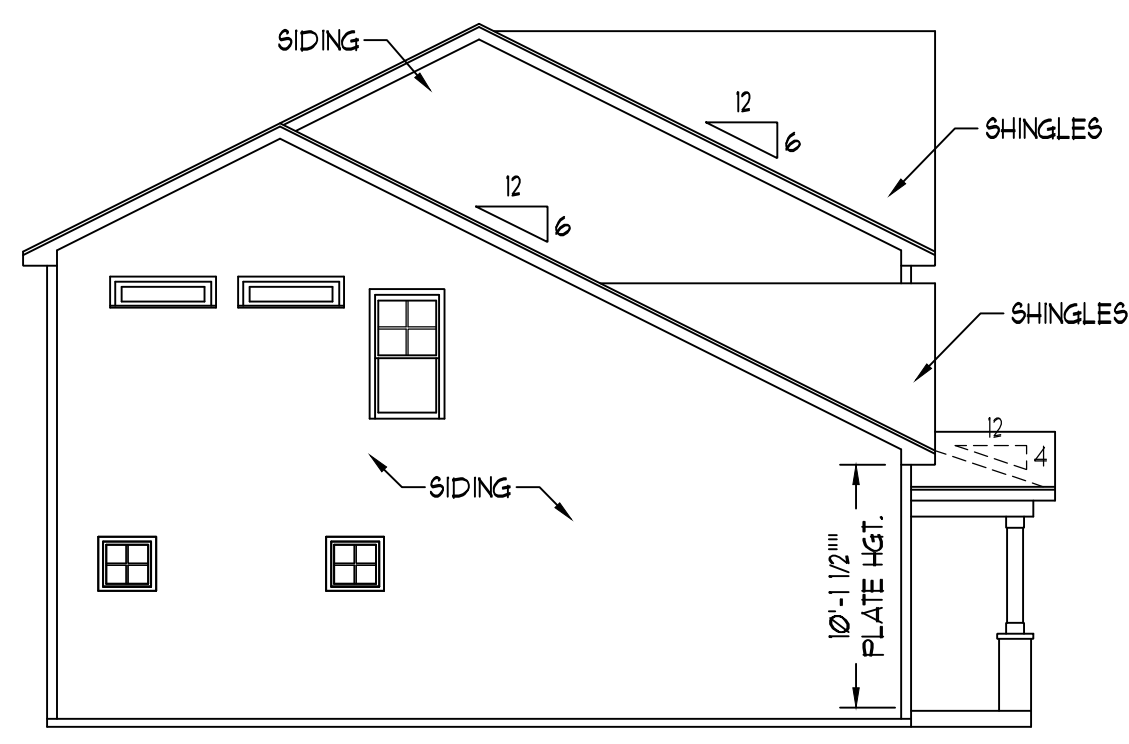


FRONT ELEVATION-C
SCALE: 1/4" = 1'-0"

NOTICE TO CONTRACTOR
All construction must comply with current NC Building Codes and is subject to field inspection and verification.

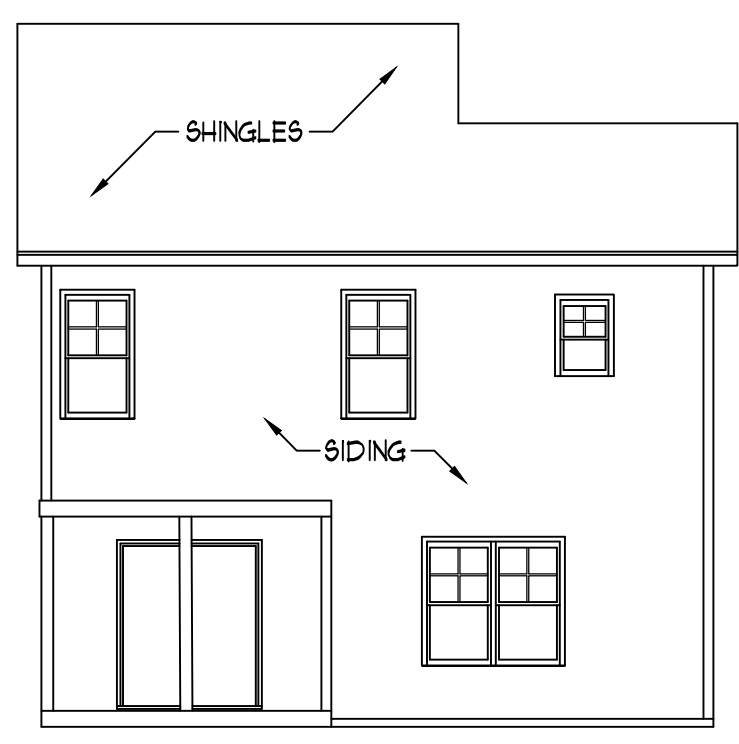
APPROVED
Limited building only review
Permit holder responsible for full compliance with the code

08/27/2021

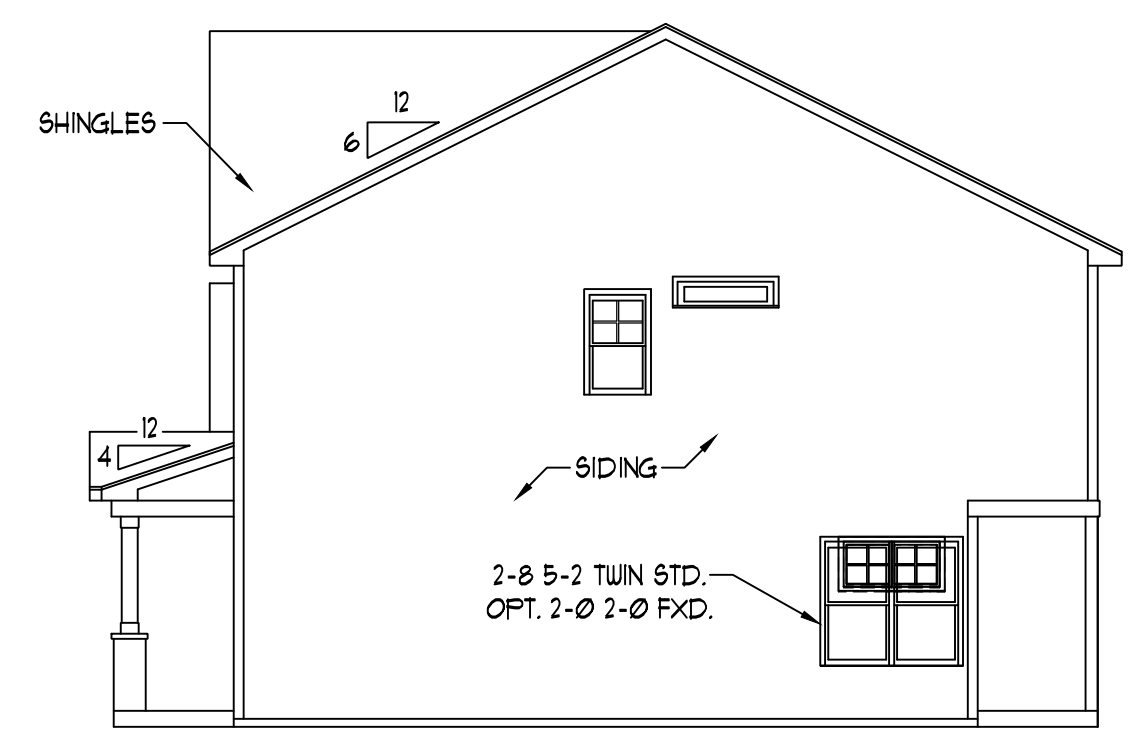


HVAC: MAINSTREAM
ELECTRICAL: DOUBLE J
PLUMBING: PIONEER

LEFT ELEVATION
SCALE: 1/8" = 1'-0"



REAR ELEVATION
SCALE: 1/8" = 1'-0"



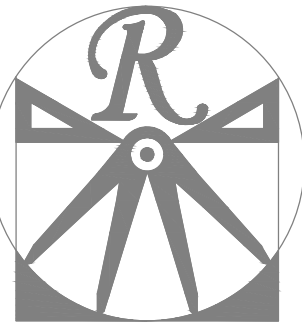
RIGHT ELEVATION
SCALE: 1/8" = 1'-0"

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.
11x17 PRINTS ARE NOT TO SCALE

WEAVER HOMES
CAROLINA COLLECTION
POPLAR

DATE: JULY 22, 2020
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

C - ELEVATIONS
A-3



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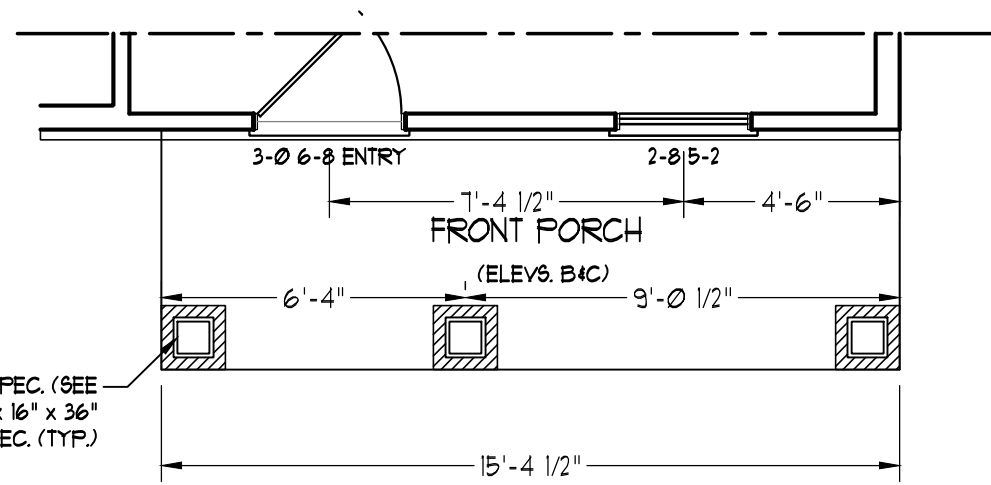
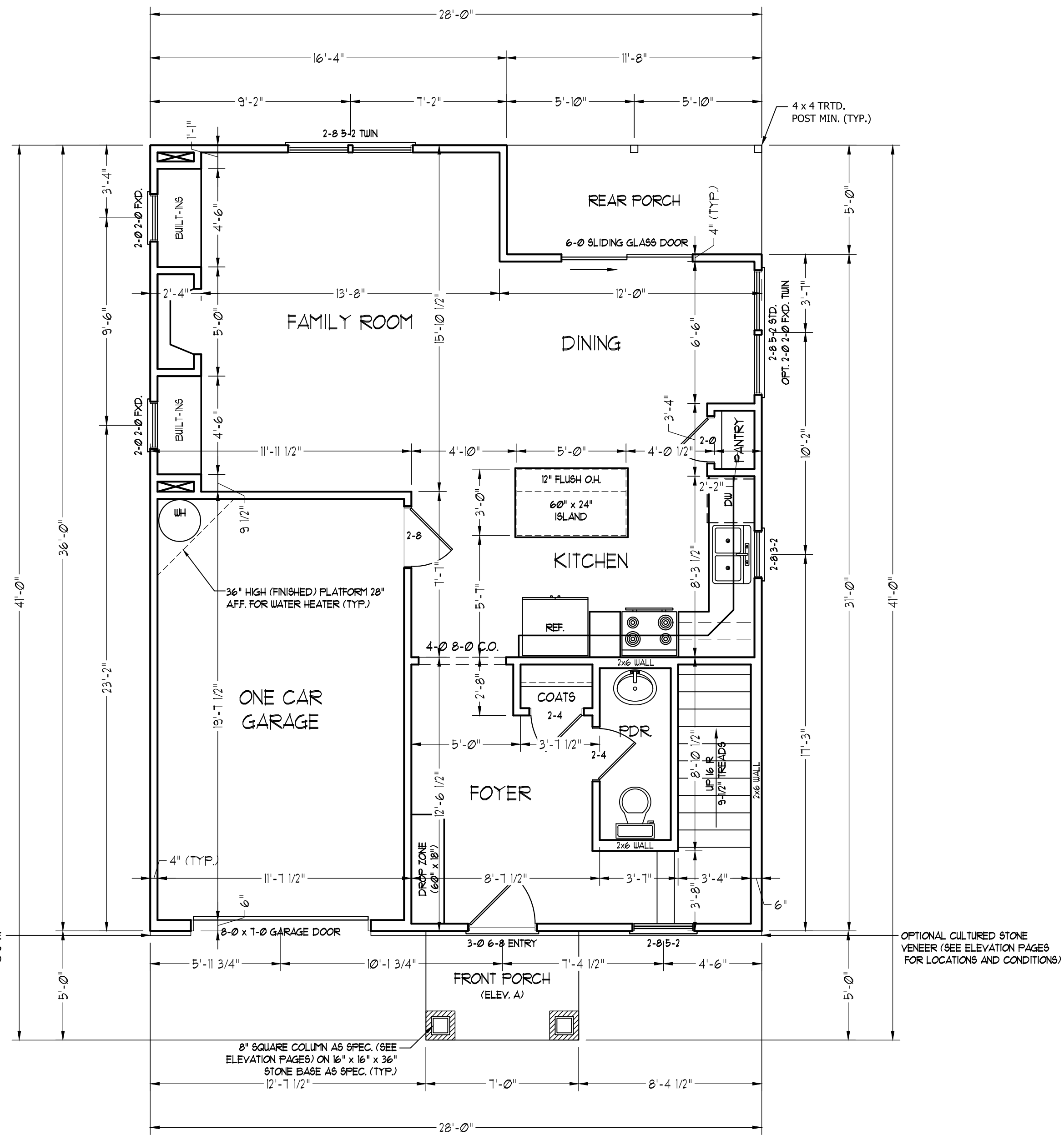
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FIRST FLOOR PLAN

A-4

SQUARE FOOTAGE (I.F.S.)	
1st FLOOR:	6711 SQ. FT.
2nd FLOOR:	800 SQ. FT.
TOTAL:	14711 SQ. FT.
GARAGE:	218 SQ. FT.
FRONT PORCH (ELEV.-A):	35 SQ. FT.
FRONT PORCH (ELEV.-B&C):	71 SQ. FT.
REAR PORCH:	58 SQ. FT.



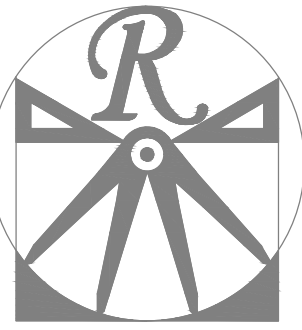
FRONT PORCH ELEVATIONS B&C

OPTIONAL CULTURED STONE VENEER (SEE ELEVATION PAGES FOR LOCATIONS AND CONDITIONS)

OPTIONAL CULTURED STONE VENEER (SEE ELEVATION PAGES FOR LOCATIONS AND CONDITIONS)

8" SQUARE COLUMN AS SPEC. (SEE ELEVATION PAGES) ON 16" x 16" x 36" STONE BASE AS SPEC. (TYP.)

8" SQUARE COLUMN AS SPEC. (SEE ELEVATION PAGES) ON 16" x 16" x 36" STONE BASE AS SPEC. (TYP.)



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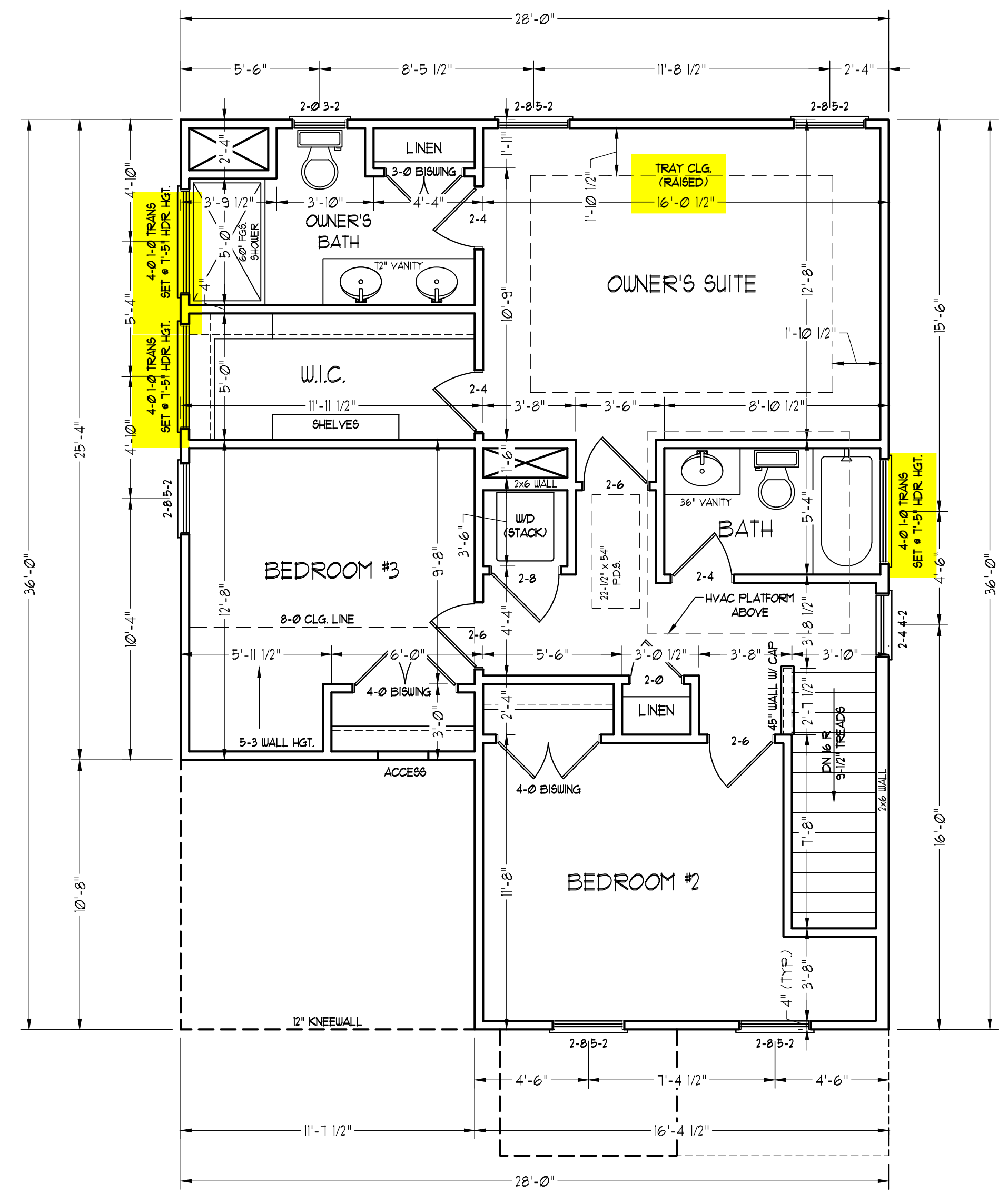
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REVIEWED BY:

SECOND FLOOR
PLAN
A-5

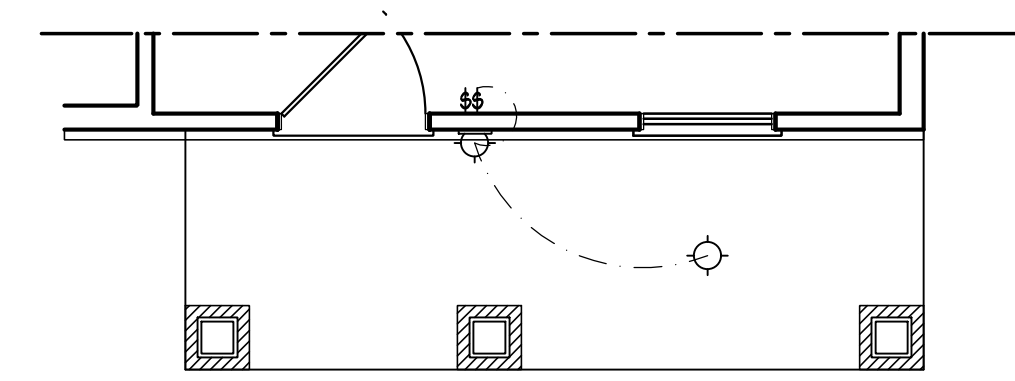
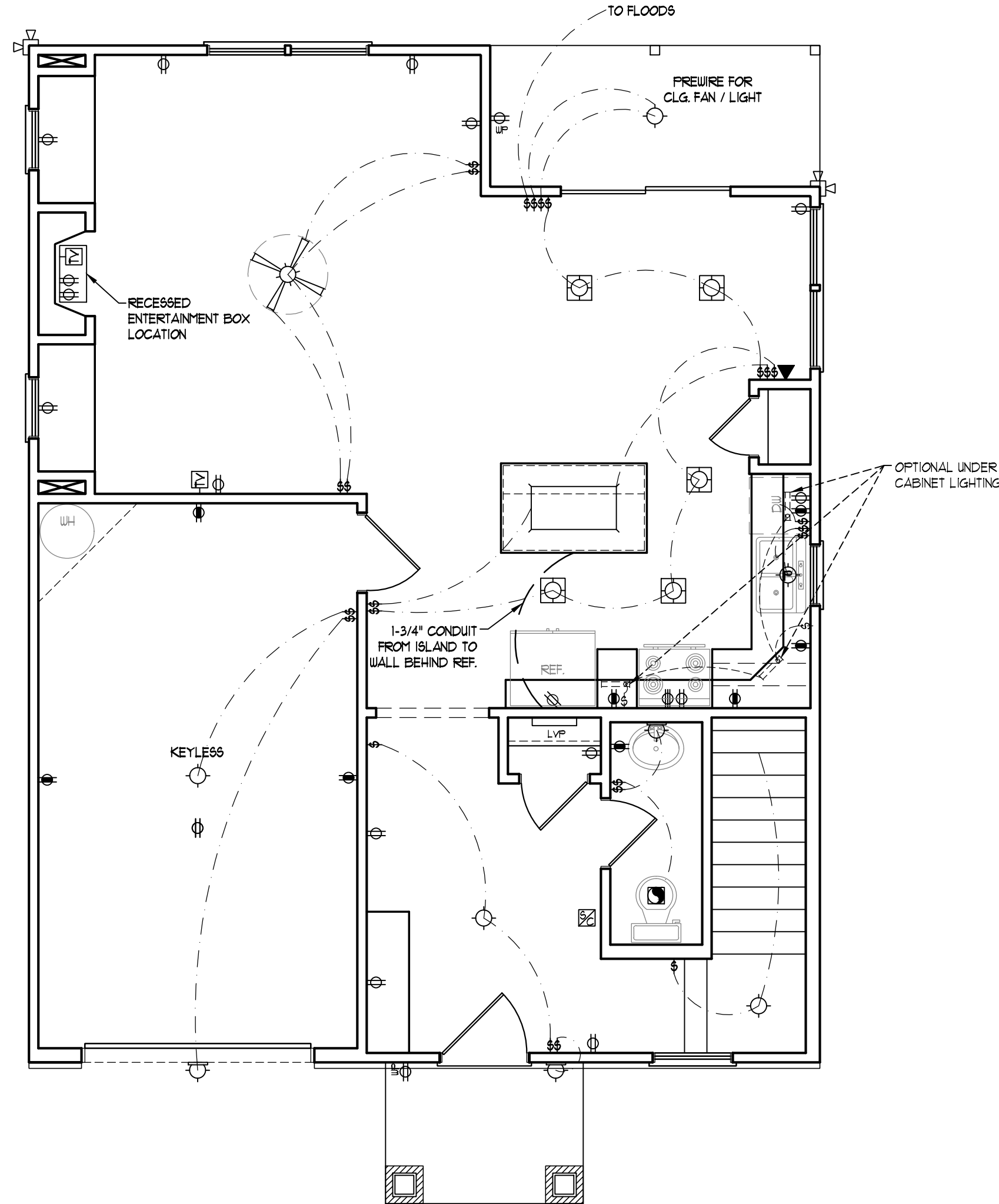


ELECTRICAL LAYOUT NOTES:

- 1.) BLOCK AND WIRE FOR ALL CEILING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET @ 30" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- 110 V OUTLET
- 110 V GFI OUTLET
- 110 V SWITCHED OUTLET
- 110 V BASEBOARD OUTLET
- 4-FLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GFI
- WEATHERPROOF
- 220 V OUTLET
- 110 V DEDICATED CIRCUIT
- 220 V DEDICATED CIRCUIT
- SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- PENDANT LIGHT
- RECESSED CAN LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT
- UNDERCABINET LIGHT
- FLOOD LIGHT
- SWITCH
- DIMMER SWITCH
- TELEPHONE
- DATA
- TELEPHONE AND DATA
- TV CONNECTION
- TV/ DATA
- CONDUIT FOR COMPONENT WIRING
- SPEAKER
- 110 V SMOKE/ CH DETECTOR
- 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM PANEL
- CEILING FAN
- CEILING FAN W/ LIGHT



FRONT PORCH ELEVATIONS B+C



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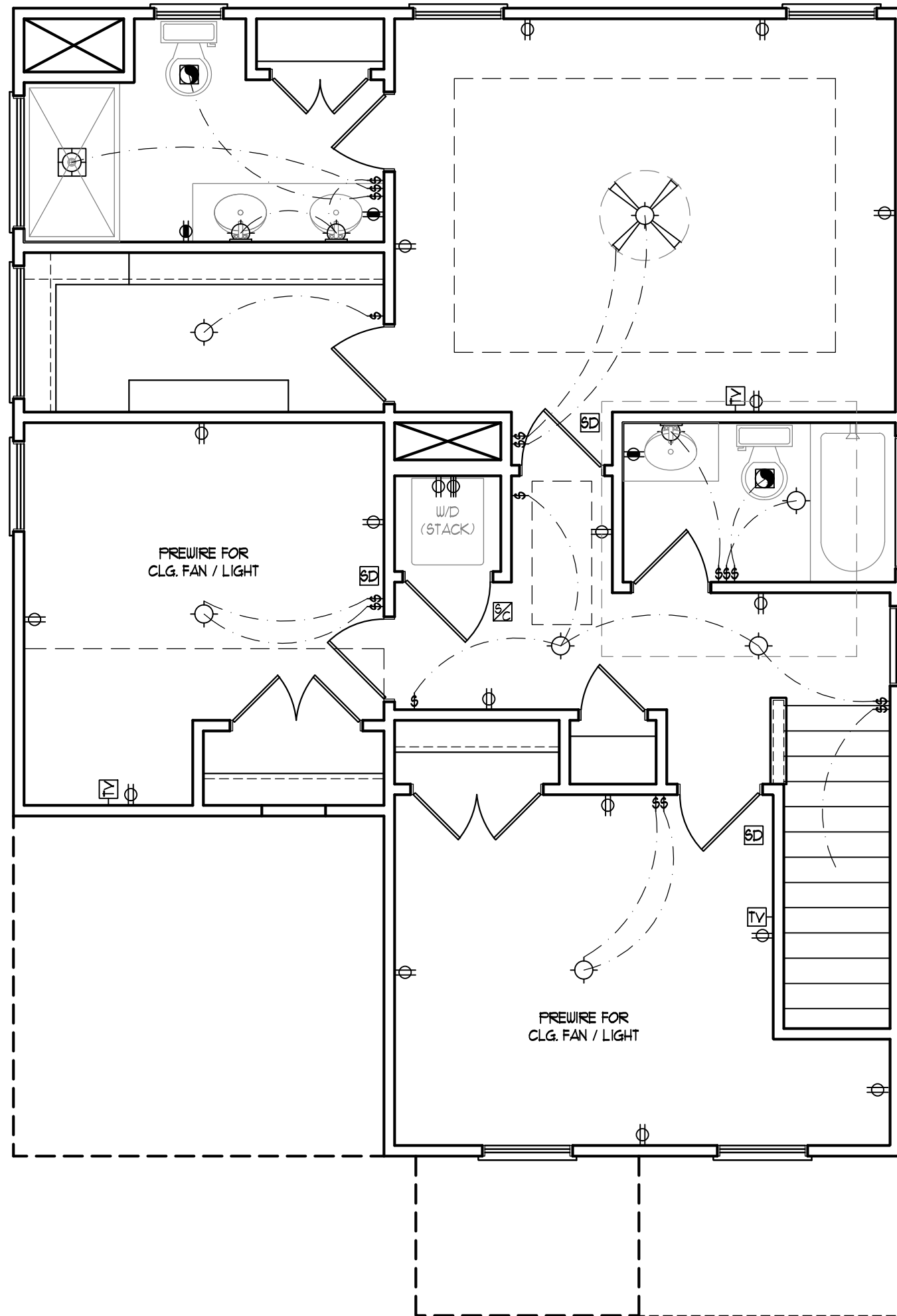
FIRST FLOOR
ELECTRICAL
PLAN
E-1

ELECTRICAL LAYOUT NOTES:

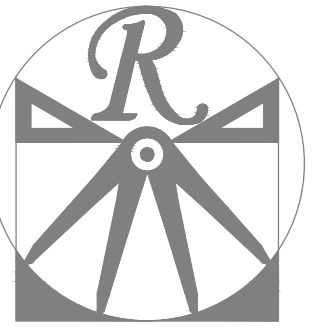
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- FLOOD LIGHT
- SWITCH
- DIMMER SWITCH
- TELEPHONE
- DATA
- TELEPHONE AND DATA
- TV CONNECTION
- TV/ DATA
- CONDUIT FOR COMPONENT WIRING
- SPEAKER
- 110 V SMOKE/ CO DETECTOR
- 110 V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- ALARM PANEL



- CEILING FAN
- CEILING FAN W/ LIGHT



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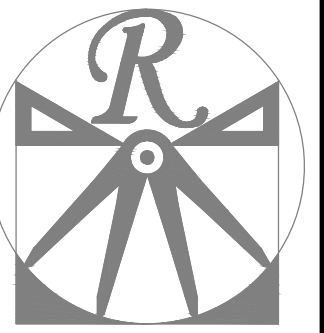
REVIEWED BY:

SECOND FLOOR
 ELECTRICAL
 PLAN

E-2

ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:

1. STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
2. FOR 120 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 6'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
3. FOR 130 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 4'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
4. MEAN ROOF HEIGHT IS LESS THAN 30 FEET.
5. EXTERIOR WALLS DESIGNED FOR 120 OR 130 MPH WINDS.
6. INSTALL 7/16" OSB SHEATHING ON ALL EXTERIOR WALLS OF ALL STORIES IN ACCORDANCE WITH SECTION R602.10.3 OF THE NCRC, 2018 EDITION.
7. ENERGY EFFICIENCY COMPLIANCE AND INSULATION VALUES OF THE BUILDING TO BE IN ACCORDANCE WITH CHAPTER 11 OF THE NCRC, 2018 EDITION.



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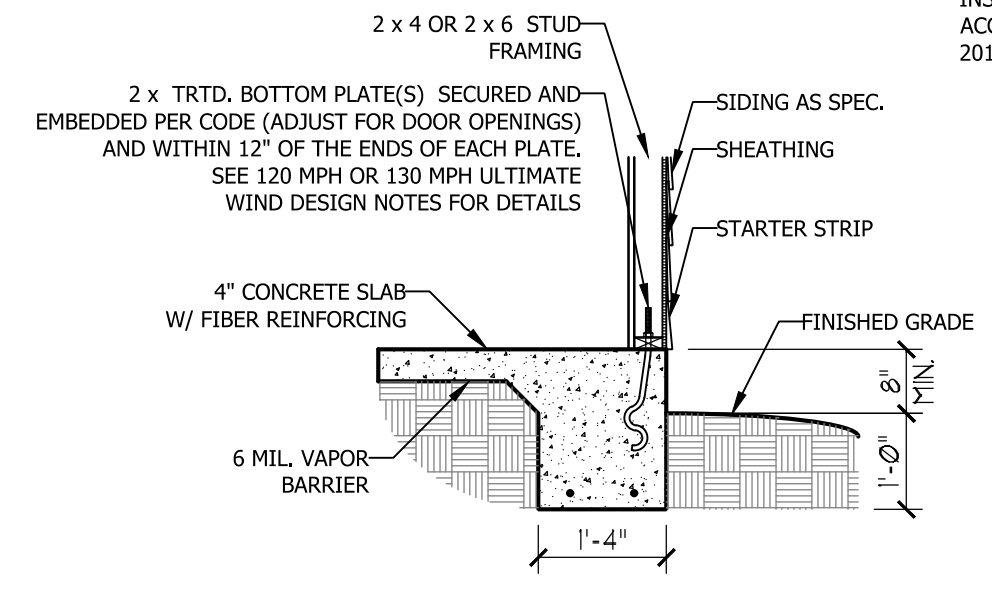
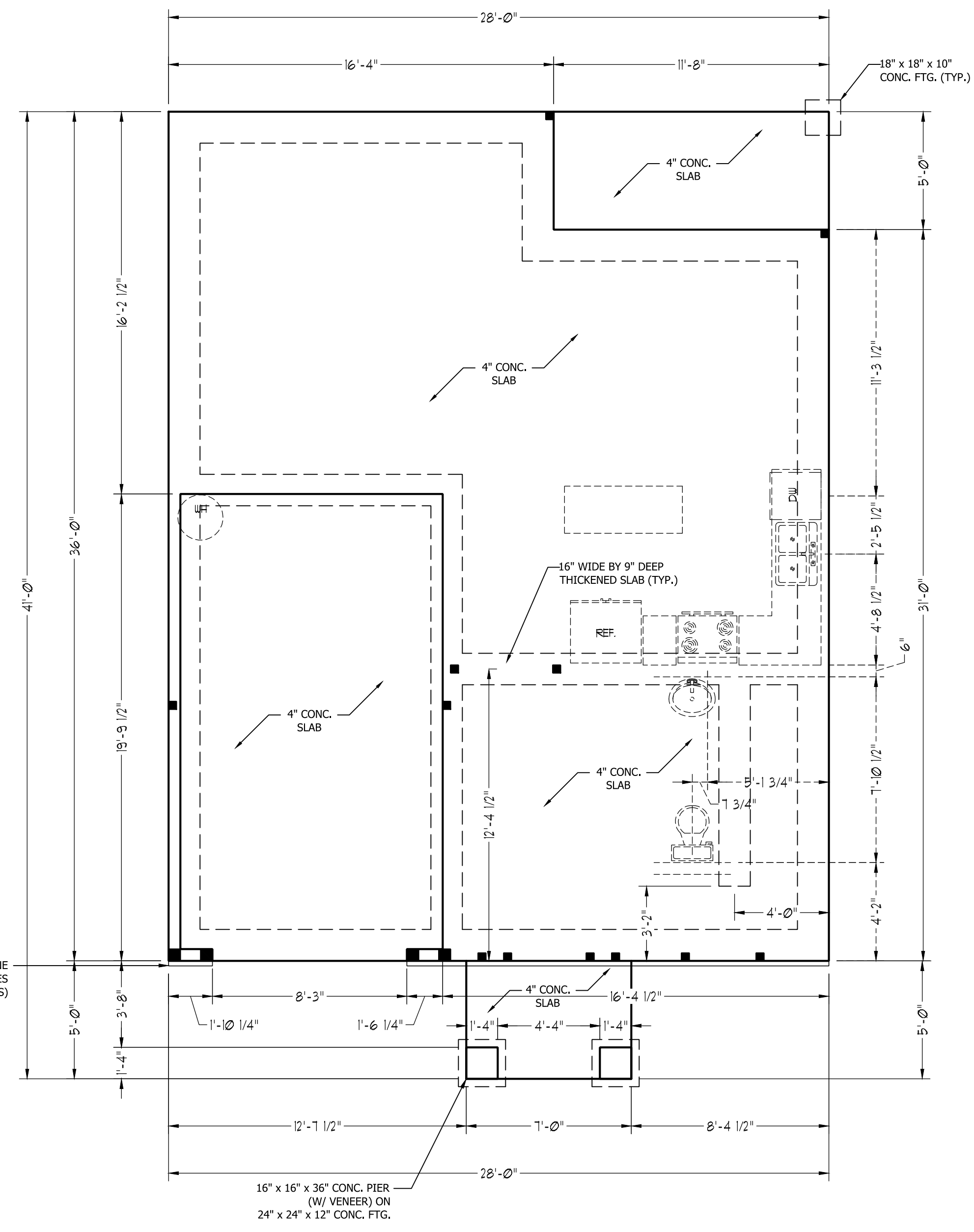
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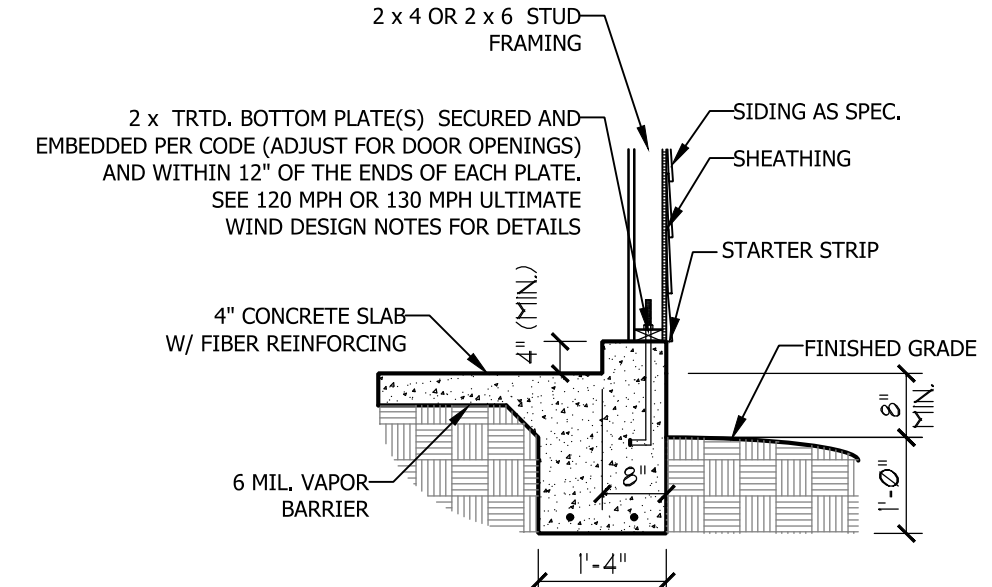
DATE: JULY 22, 2020
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SCALE: 1/4" = 1'-0"
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MONO SLAB
FOUNDATION
PLAN

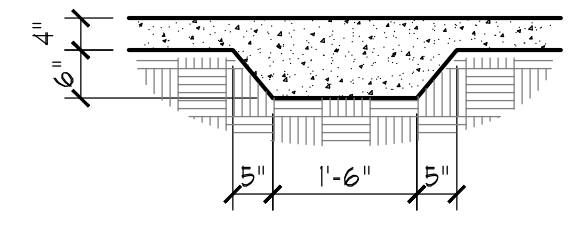
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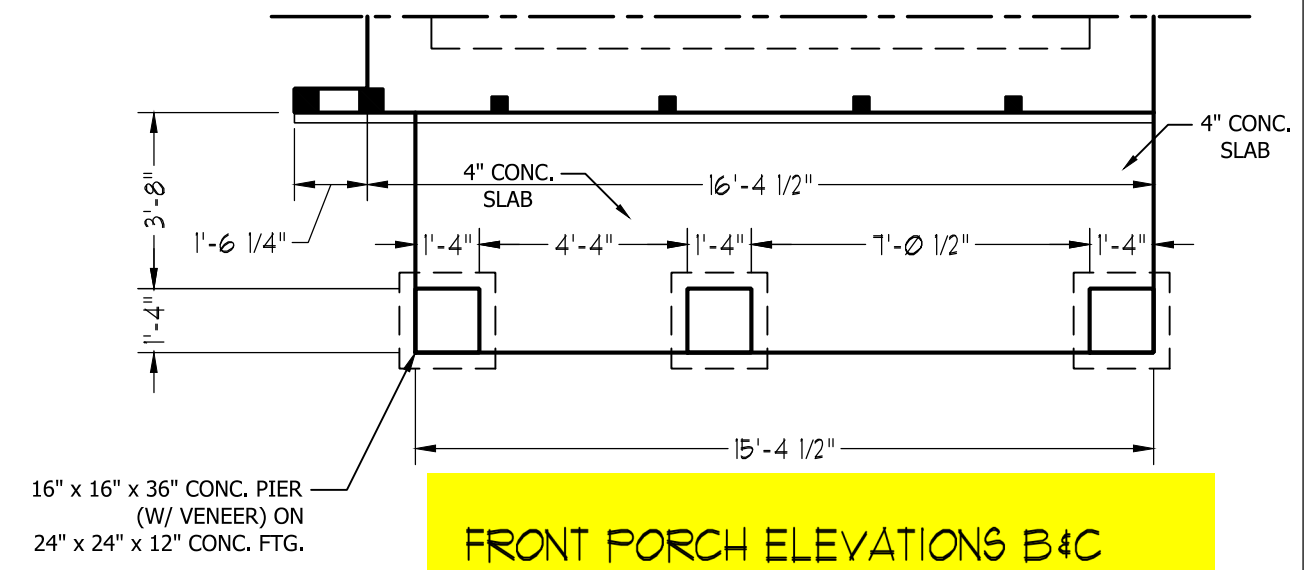
TYPICAL SLAB DETAIL



GARAGE CURB DETAIL



THICKENED SLAB DETAIL

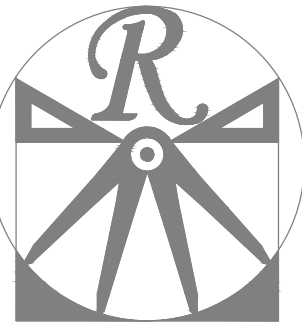


FRONT PORCH ELEVATIONS B+C

OPTIONAL CULTURED STONE VENEER (SEE ELEVATION PAGES FOR LOCATIONS AND CONDITIONS)

16" x 16" x 36" CONC. PIER (W/ VENEER) ON 24" x 24" x 12" CONC. FTG.

16" x 16" x 36" CONC. PIER (W/ VENEER) ON 24" x 24" x 12" CONC. FTG.



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DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

SECOND FLOOR
FRAMING PLAN

S-2

STRUCTURAL NOTES:

1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
3. INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
4. WINDOW AND DOOR HEADERS TO BE SUPPORTED W/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
7. FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

BRACE WALL PANEL NOTES:

EXTERIOR WALLS: ALL EXTERIOR WALLS TO BE SHEATHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THEIR ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

GYPSON: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSON INSTALLED. WHEN NOT USING METHOD GB GYPSON TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

HD: 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1

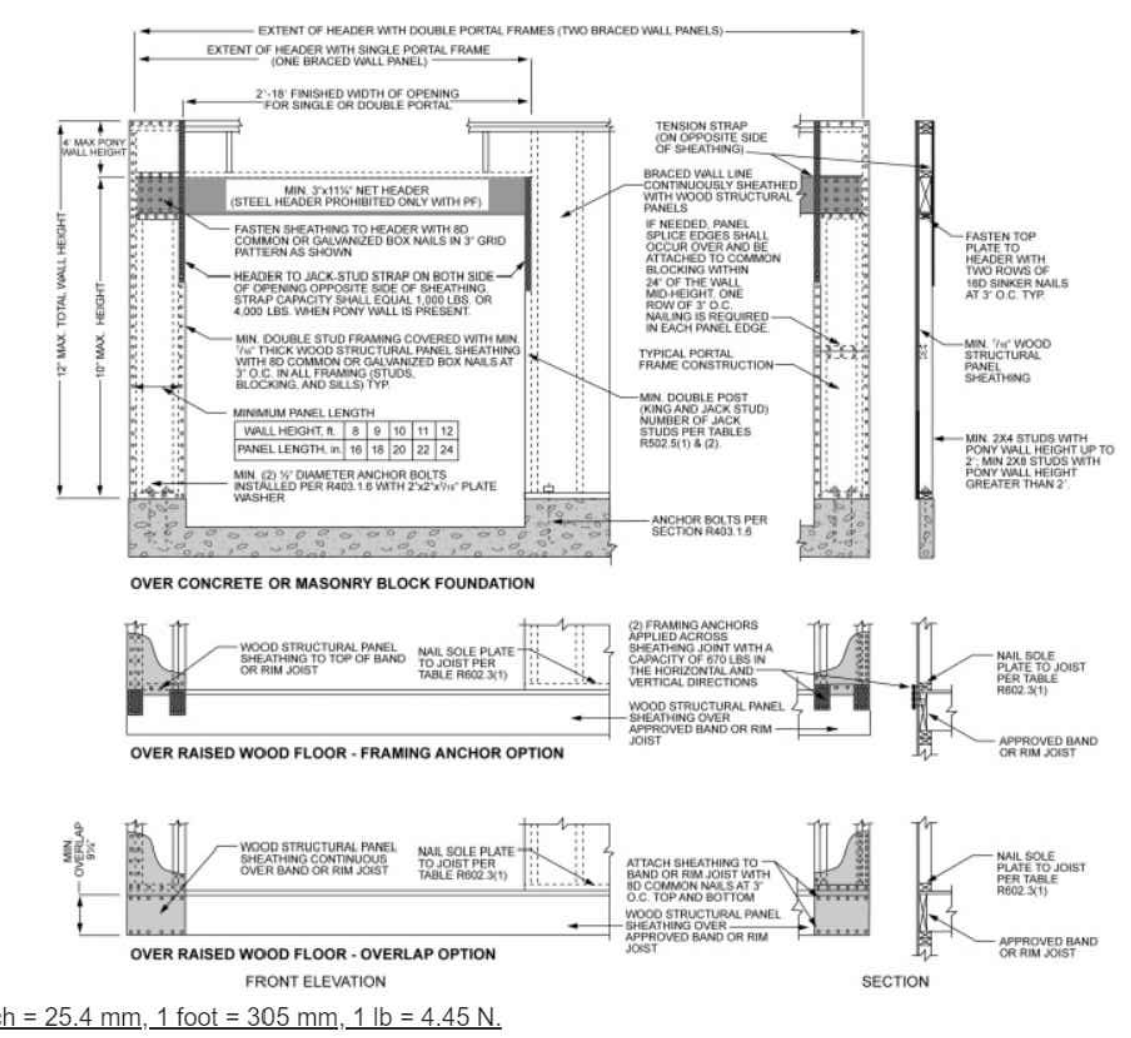
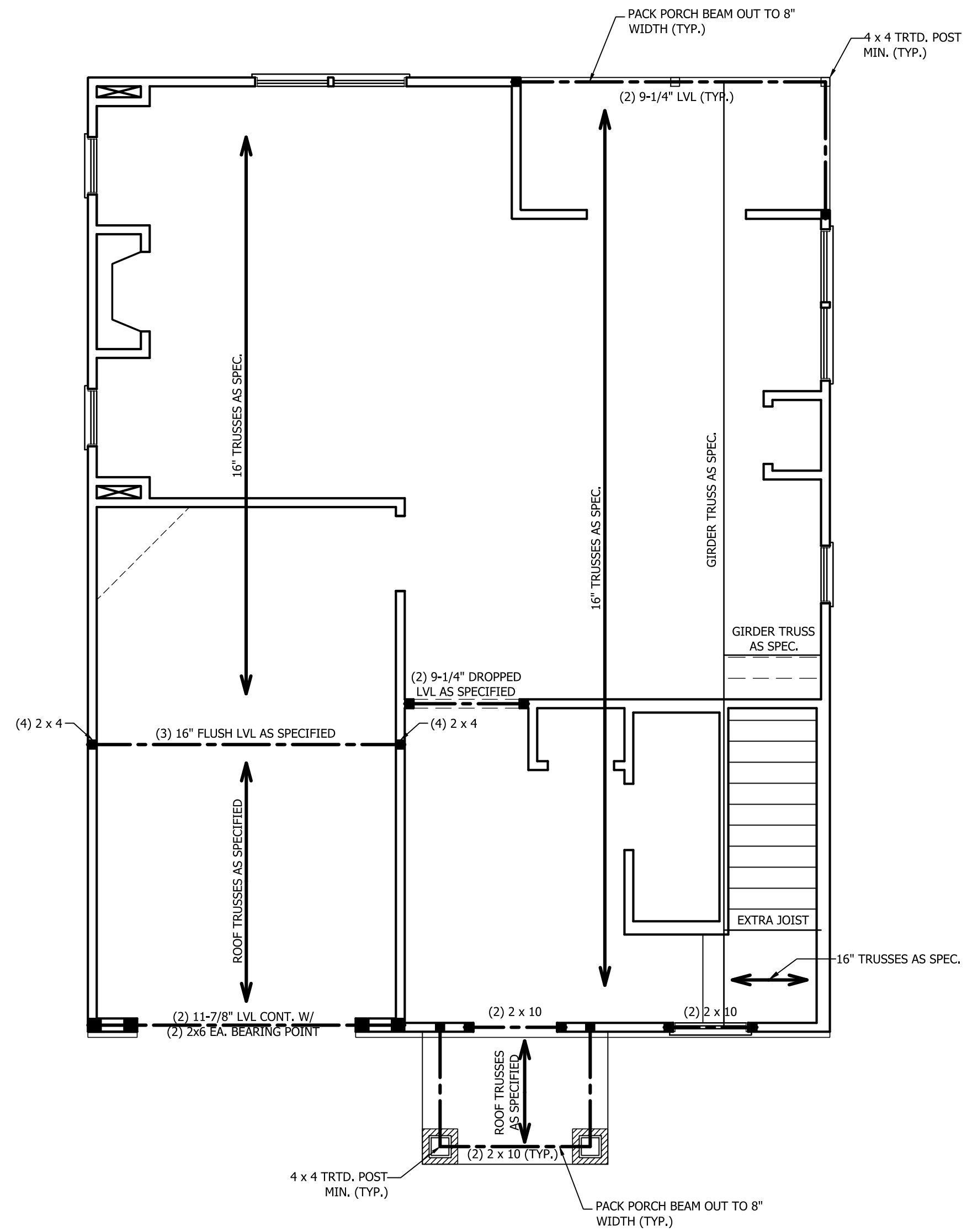
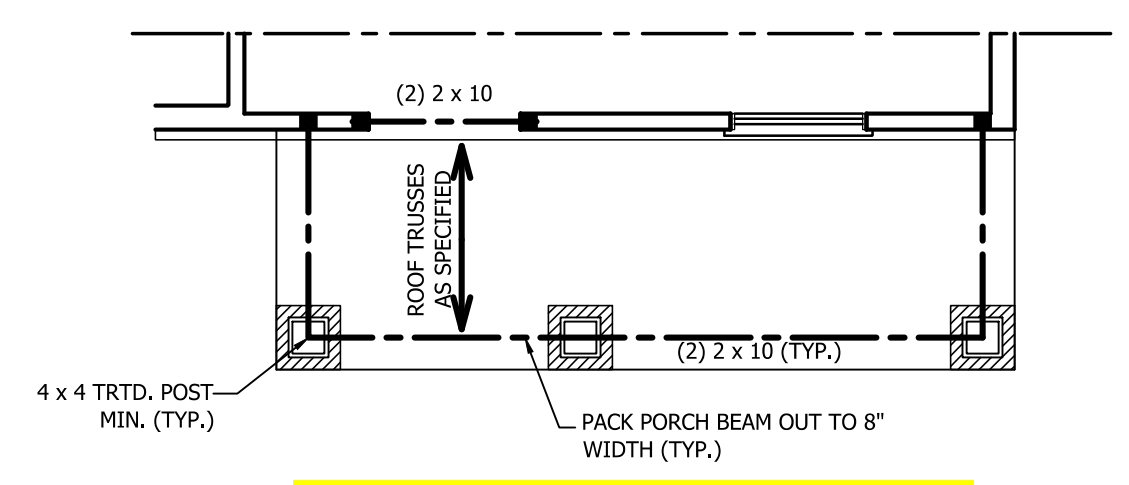
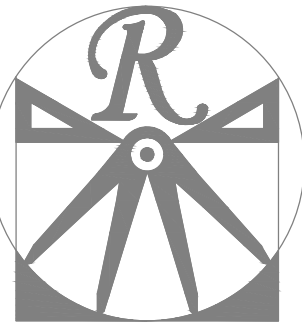


FIGURE R602.10.1
METHOD PF - PORTAL FRAME CONSTRUCTION



FRONT PORCH ELEVATIONS B & C



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WEAVER HOMES
CAROLINA COLLECTION
POPLAR

DATE: JULY 22, 2020

REV.:

SCALE: 1/4" = 1'-0"

DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:

ATTIC FLOOR
FRAMING PLAN

S-3

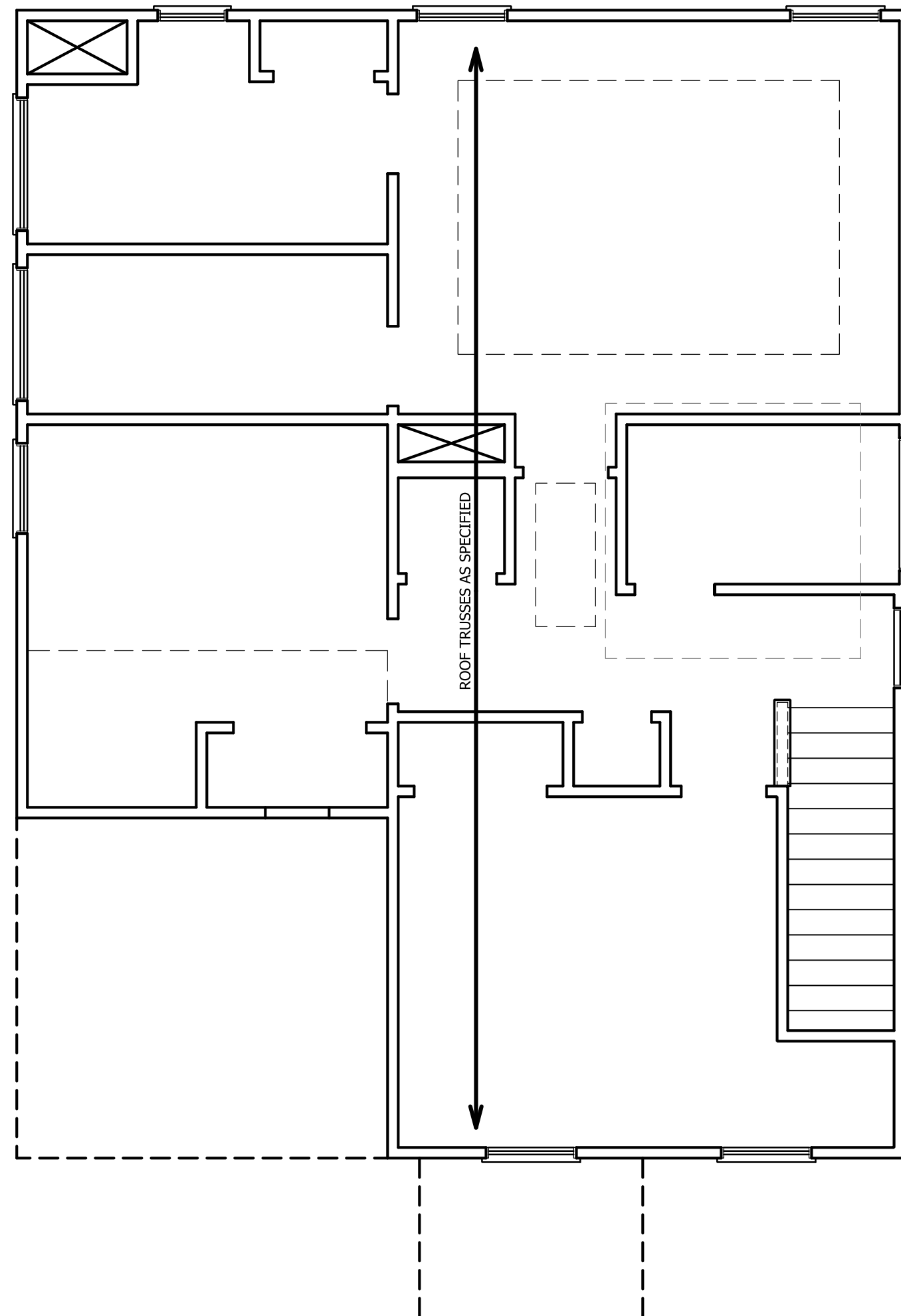
TABLE R602.7.5
MINIMUM NUMBER OF FULL HEIGHT STUDS
AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHES) (PER TABLE R602.3(5))	
	16	24
UP TO 3'	1	1
4'	2	1
8'	3	2
12'	5	3
16'	6	4

STRUCTURAL NOTES:

1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO).
2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.).

DSP - DOUBLE STUD POCKET
TSP - TRIPLE STUD POCKET



ATTIC VENT CALCULATION:

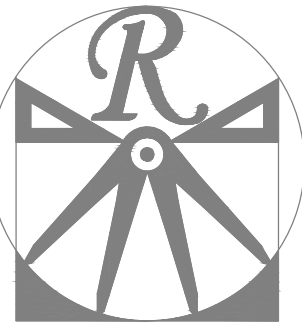
1180 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 7.9 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

ATTIC VENT CALCULATION:

1218 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 8.1 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

1. ALL FRAMING LUMBER TO BE #2 SPF (UNO).
2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



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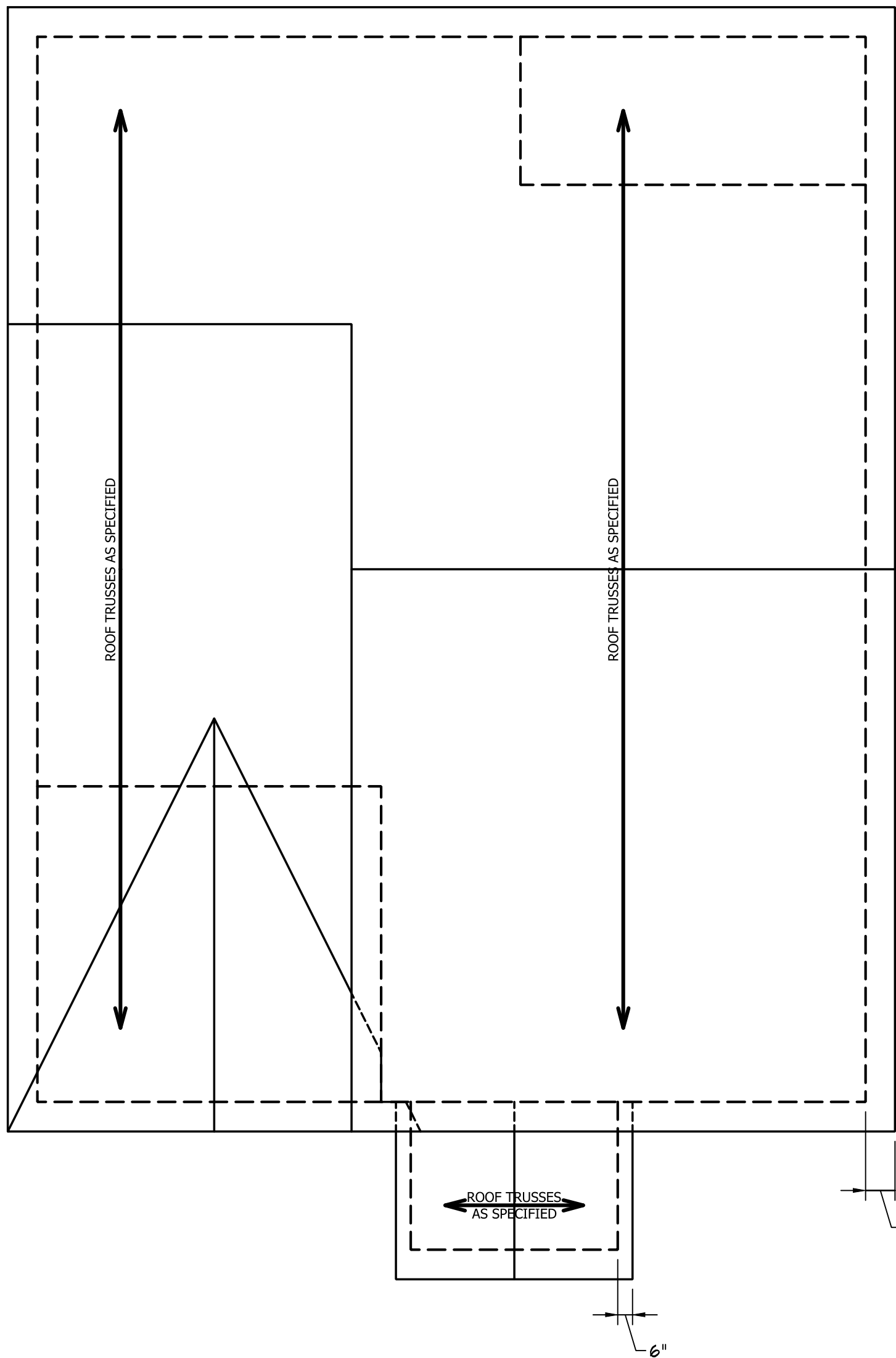
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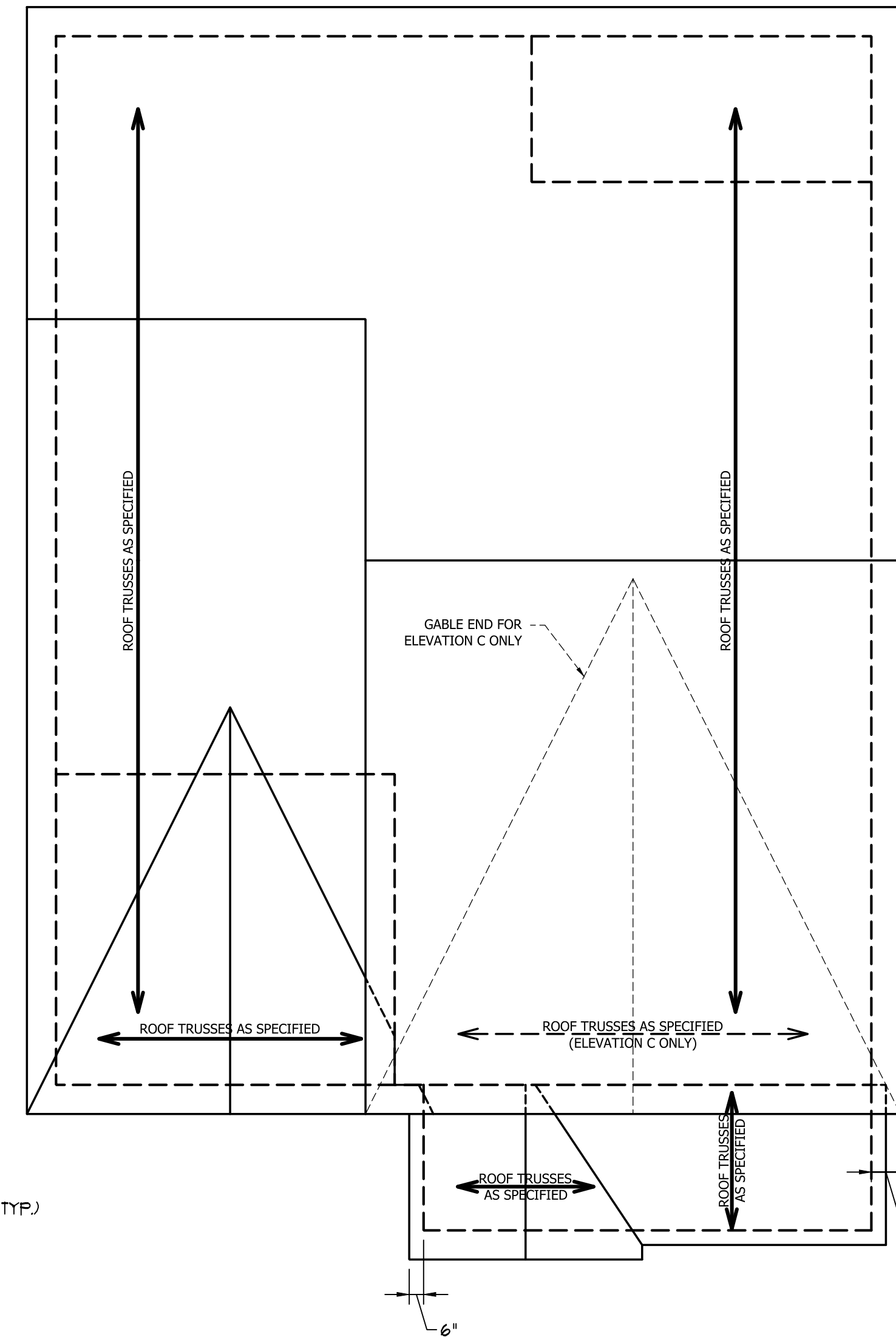
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ELEVATION A

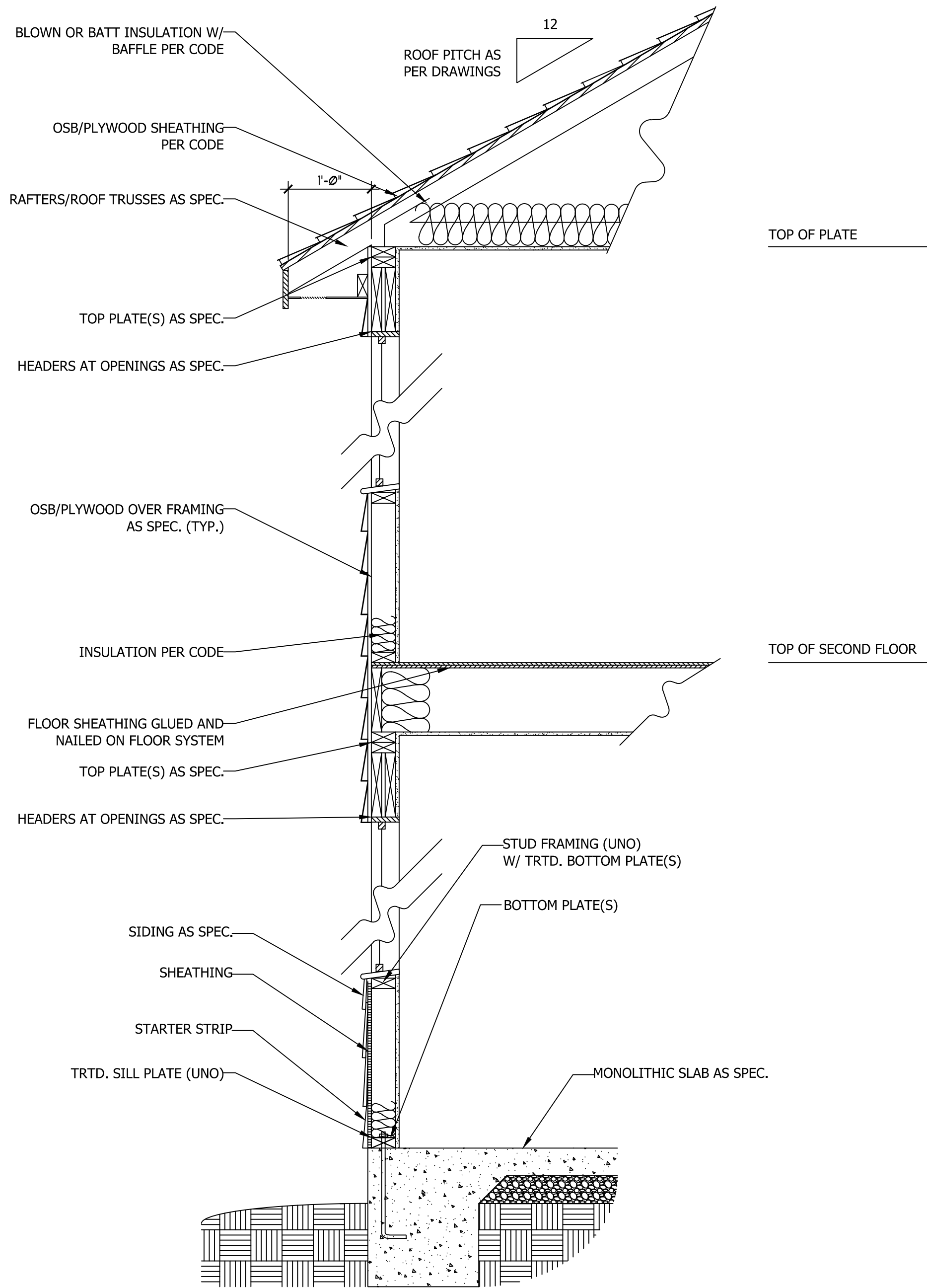


ELEVATION B&C

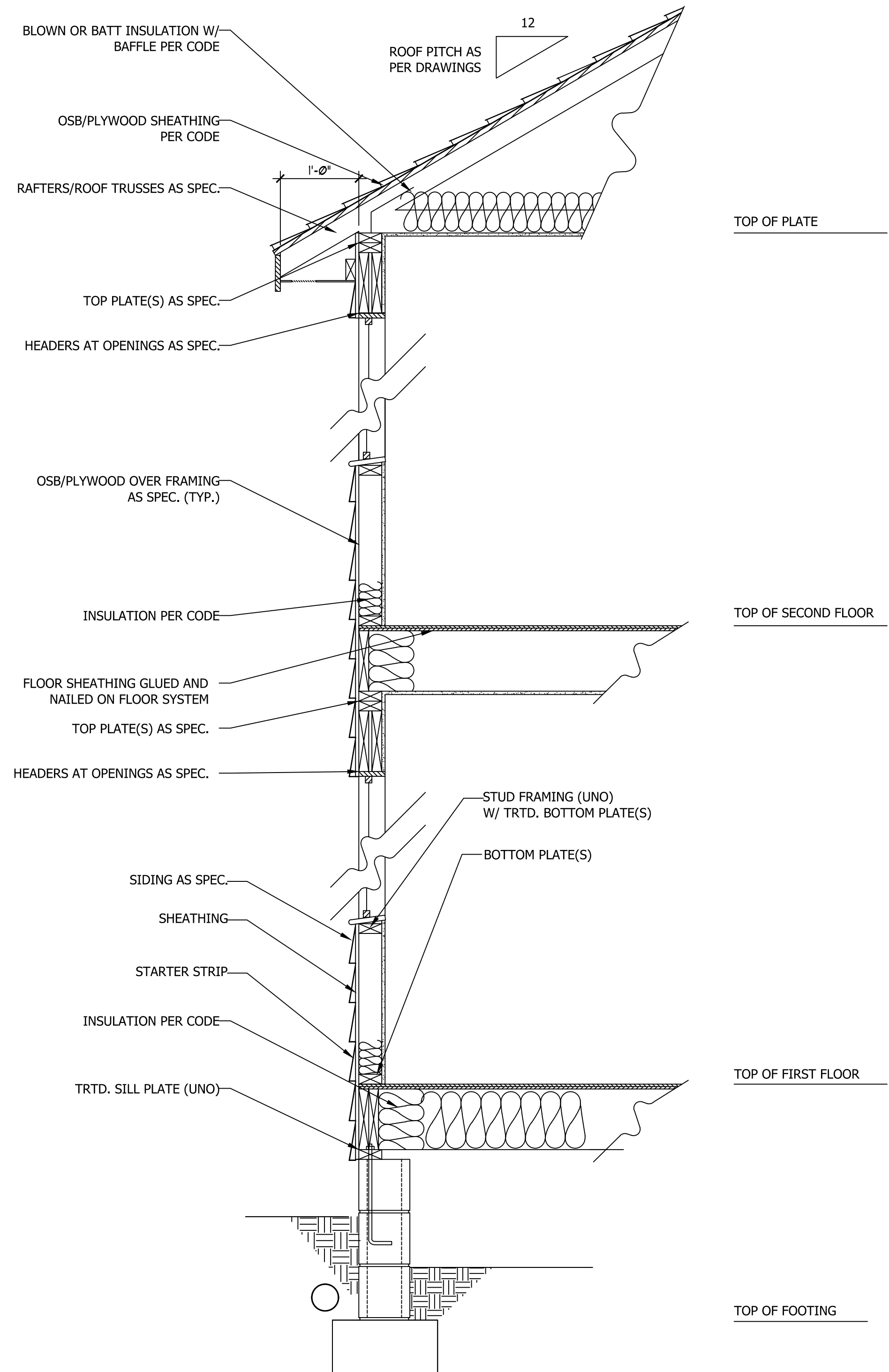
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CAROLINA COLLECTION
POPLAR

DATE: JULY 22, 2020
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

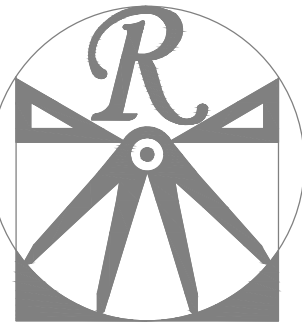
ROOF PLANS
S-4



**WALL SECTION W/ SLAB
W/ STD. SIDING SHOWN (NTS)**



**WALL SECTION W/ CRAWL SPACE
W/ STD. SIDING SHOWN (NTS)**



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**WEAVER HOMES
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DATE: JULY 22, 2020

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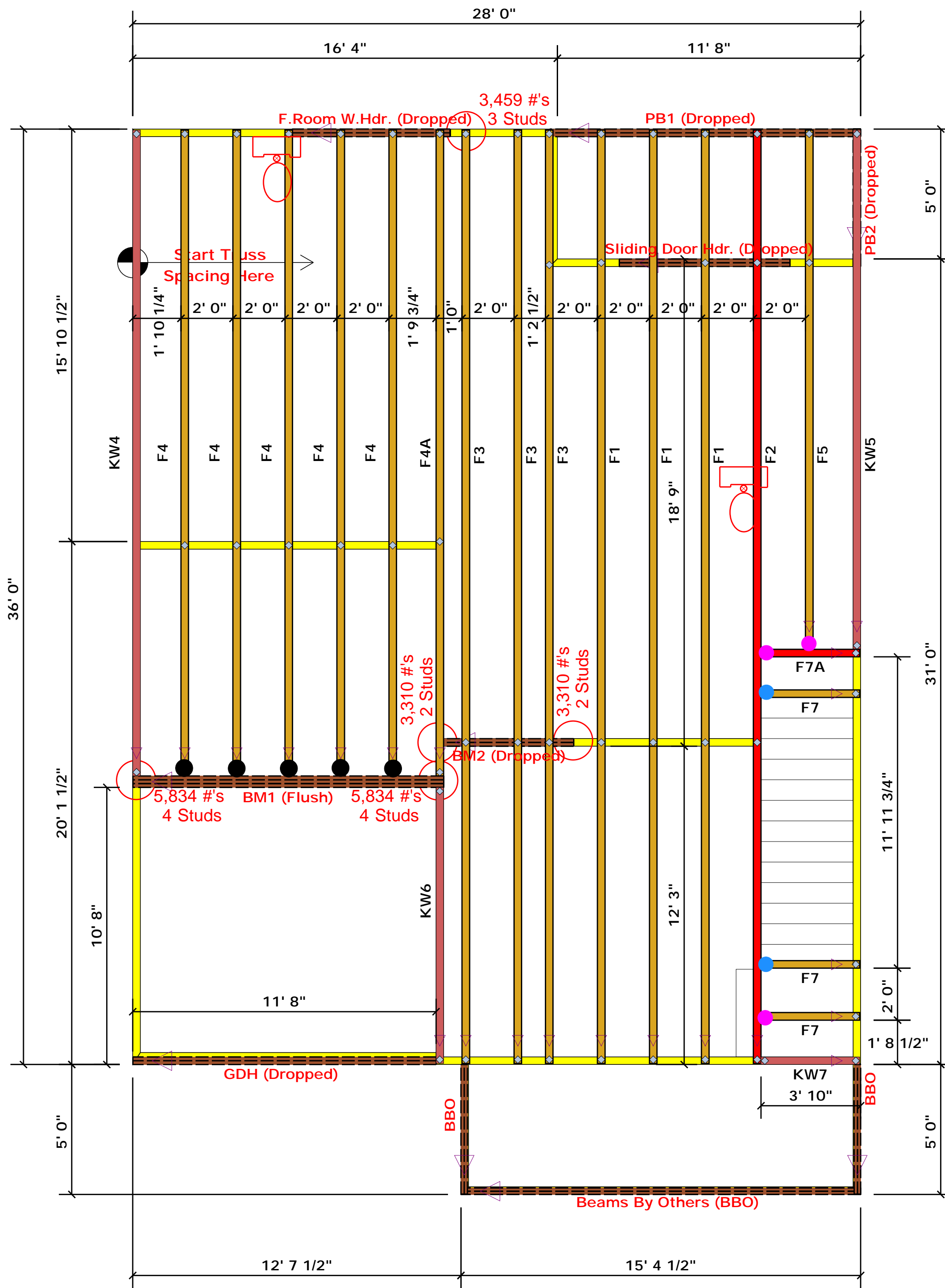
DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:

TYPICAL WALL SECTIONS

D-1



Truss Placement Plan SCALE: NTS

- = HUS410 (Qty. 5)
- = MSH422 (Qty. 3)
- = MSH422IF (Qty. 2)

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

PlotID	Length	Product	Plies	Net Qty	Fab Type
PB1 (Dropped)	12' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
F.Room W.Hdr. (Dropped)	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
Sliding Door Hdr. (Dropped)	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
BM2 (Dropped)	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
PB2 (Dropped)	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH (Dropped)	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
BM1 (Flush)	12' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

LOAD CHART FOR JACK STUDS

IRB ACTION (Up To)	IRB ACTION (Up To)	IRB ACTION (Up To)
1700 1	2550 1	3400 1
3400 2	5100 2	6500 2
5100 3	7650 3	10000 3
6800 4	13200 4	13600 4
8500 5	12750 5	17000 5
10200 6	15300 6	
11900 7		
13600 8		
15300 9		

BUILDER	Weaver Development	CITY / CO.	Spring Lake / Harnett
JOB NAME	Lot 75 Thomas Farm	ADDRESS	Overhills Rd.
PLAN	Poplar Elev. C	MODEL	Floor
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Christine Shivy
JOB #	J0721-4528	SALES REP.	Lenny Norris

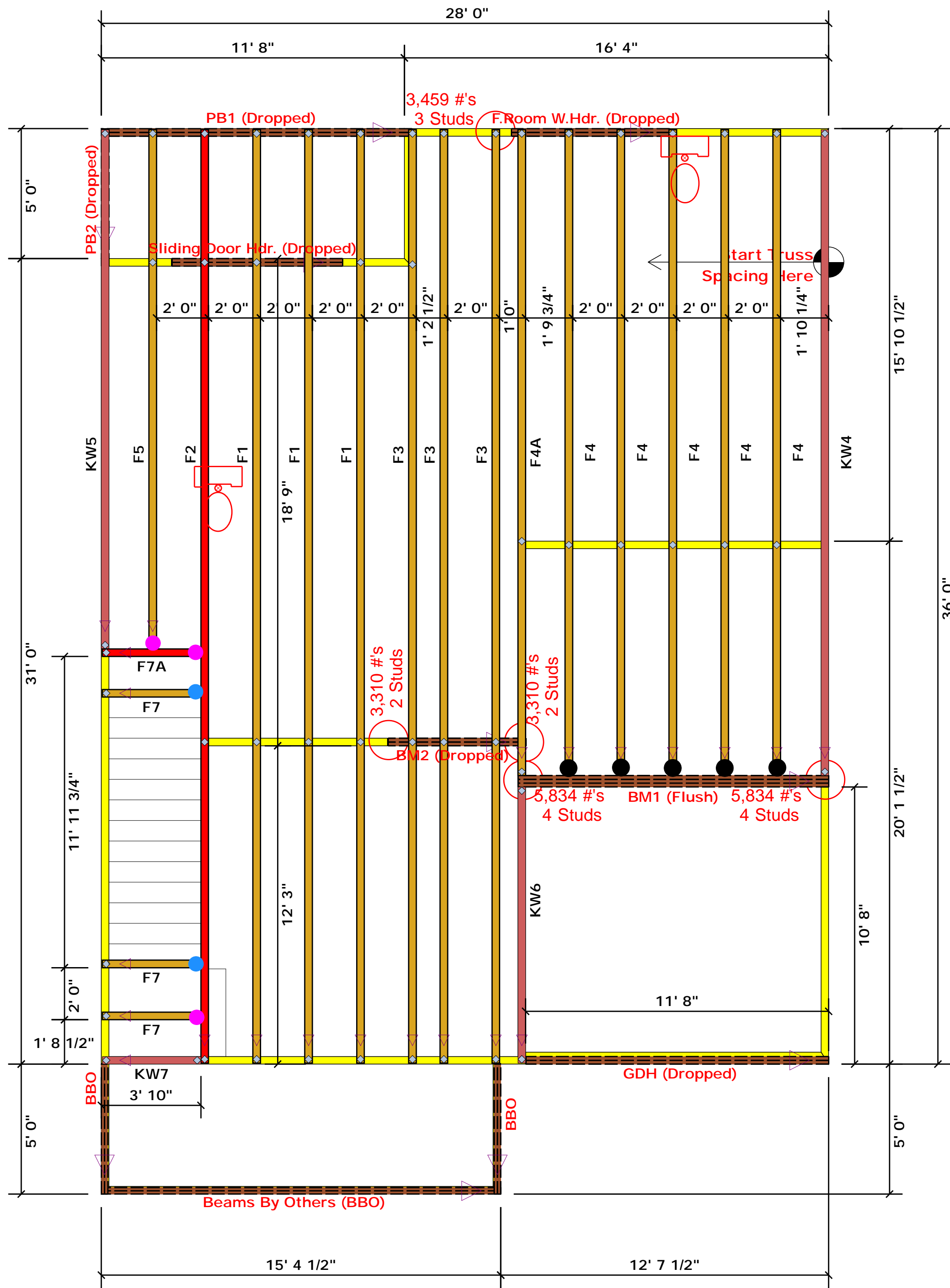
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Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: Christine Shivy
Christine Shivy

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444



Truss Placement Plan SCALE: NTS

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- = MSH422 (Qty. 3)
- = MSH422IF (Qty. 2)

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

PlotID	Length	Product	Plies	Net Qty	Fab Type
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BM2 (Dropped)	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
PB2 (Dropped)	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH (Dropped)	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
BM1 (Flush)	12' 0"	1-3/4"x 16" LVL Kerto-S	3	3	FF

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

LOAD CHART FOR JACK STUDS

MEMBER	SPACING	REACTION
1700	1	2550
3400	2	5100
5100	3	7650
6800	4	10200
8500	5	12750
10200	6	15300
11900	7	
13600	8	
15300	9	

BUILDER	Weaver Development	CITY / CO.	Spring Lake / Harnett
JOB NAME	Lot 75 Thomas Farm	ADDRESS	Overhills Rd.
PLAN	Poplar Elev. C	MODEL	Floor
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Christine Shivy
JOB #	J0721-4528	SALES REP.	Lenny Norris

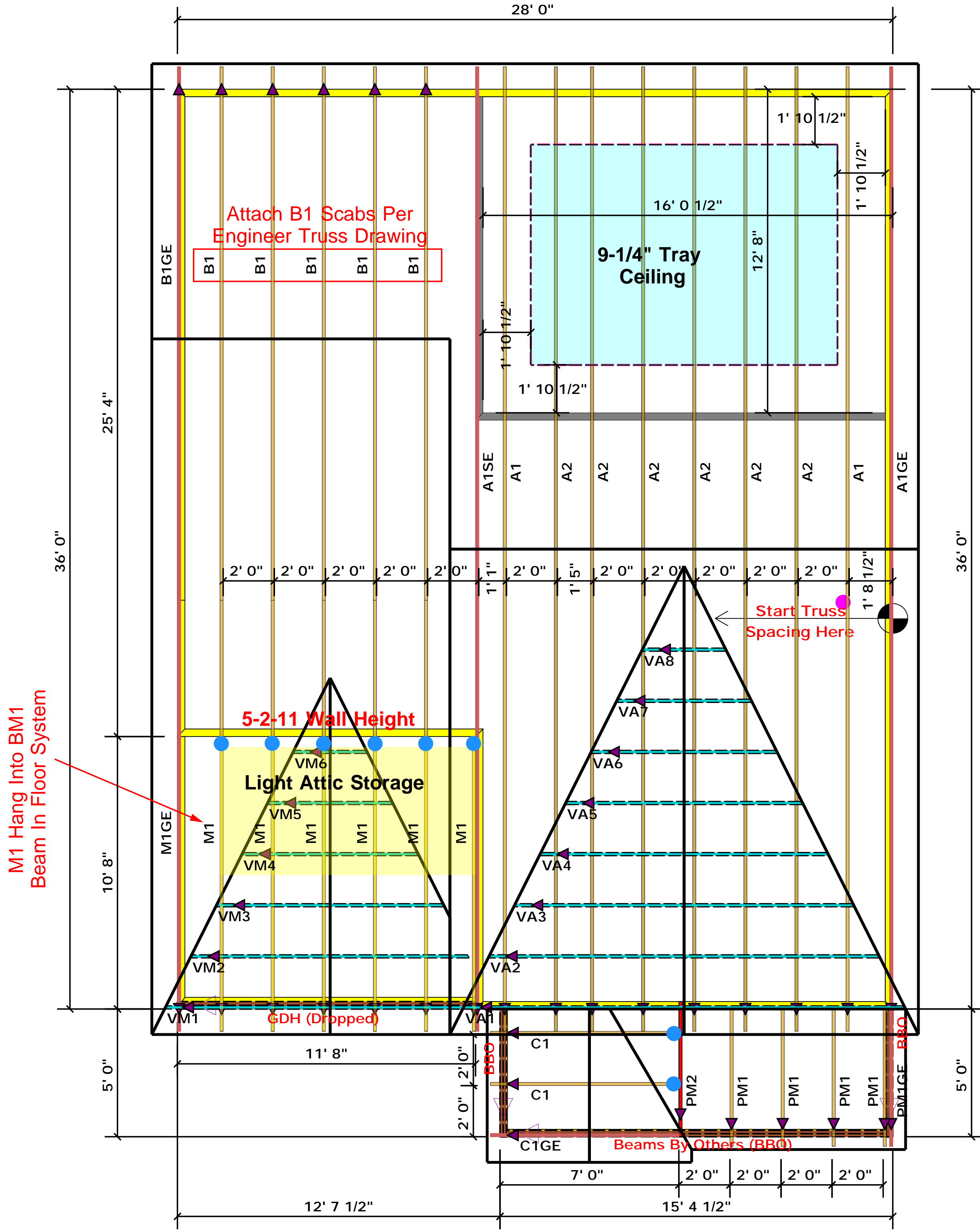
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ROOF & FLOOR TRUSSES & BEAMS

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Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444



Truss Placement Plan
SCALE: NTS

● = JUS24 (Qty. 8)

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

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○ -- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

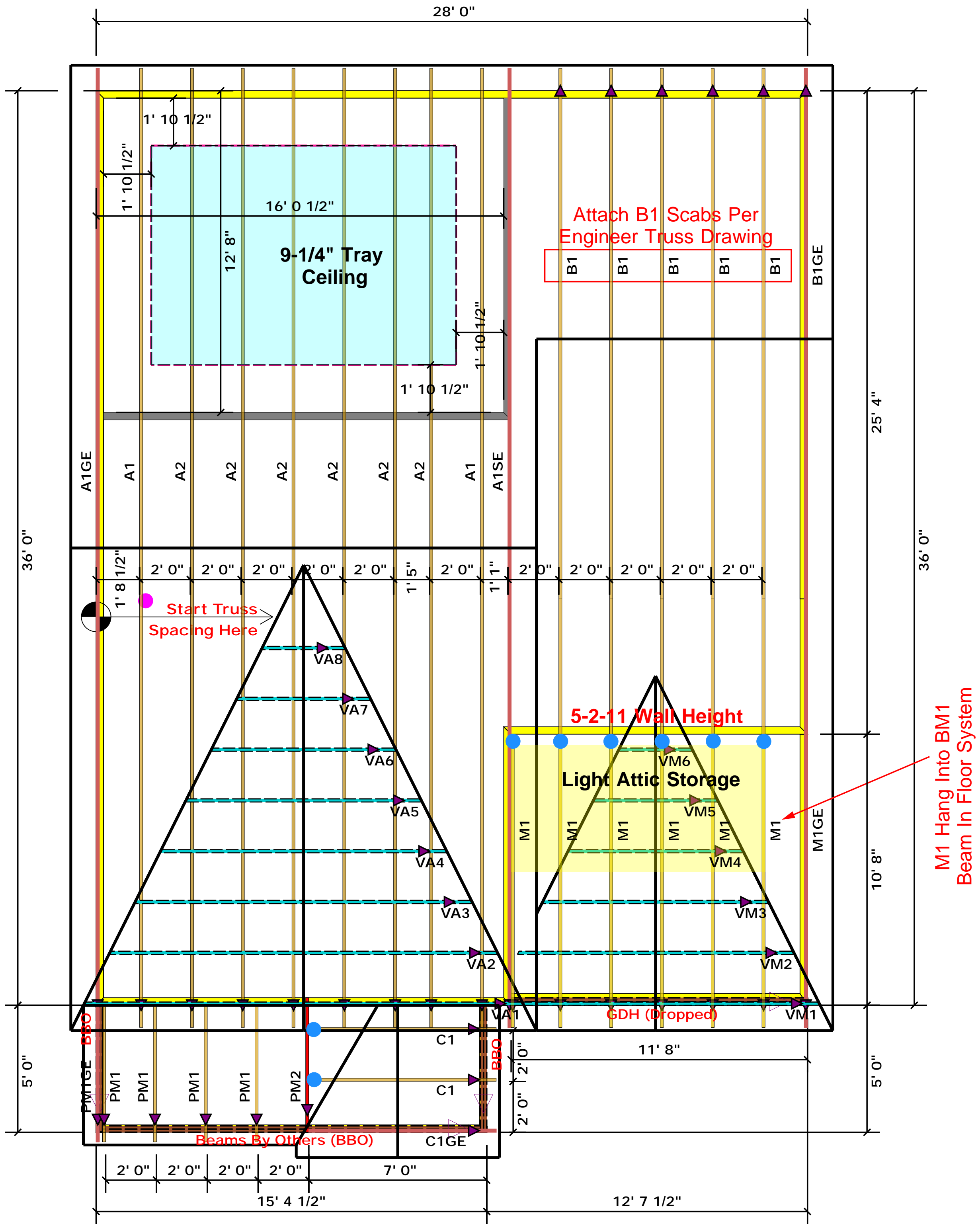
LOAD CHART FOR JACK STUDS

NO. JACKS	SPACING	LOAD	NO. JACKS	SPACING	LOAD
1700	1	2550	1	3400	
3400	2	5100	2	6800	
5100	3	7650	3	10200	
6800	4	10200	4	13600	
8500	5	12750	5	17000	
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

BUILDER	Weaver Development	CITY / CO.	Spring Lake / Harnett
JOB NAME	Lot 75 Thomas Farm	ADDRESS	Overhills Rd
PLAN	Poplar Elev. C	MODEL	Roof
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Christine Shivy
JOB #	J0721-4527	SALES REP.	Lenny Norris

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Truss Placement Plan
SCALE: NTS

● = JUS24 (Qty. 8)

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

LOAD CHART FOR JACK STUDS

LOAD REACTION (UP TO 10' SPACING)	MEMBER SIZE	LOAD REACTION (UP TO 10' SPACING)
1700	1	2560
3400	2	5100
5100	3	7650
6800	4	10200
8500	5	12750
10200	6	15300
11900	7	
13600	8	
15300	9	

BUILDER	Weaver Development
JOB NAME	Lot 75 Thomas Farm
PLAN	Poplar Elev. C
SEAL DATE	Seal Date
QUOTE #	Quote #
JOB #	J0721-4527

CITY / CO.	Spring Lake / Harnett
ADDRESS	Overhills Rd
MODEL	Roof
DATE REV.	/ /
DRAWN BY	Christine Shivy
SALES REP.	Lenny Norris

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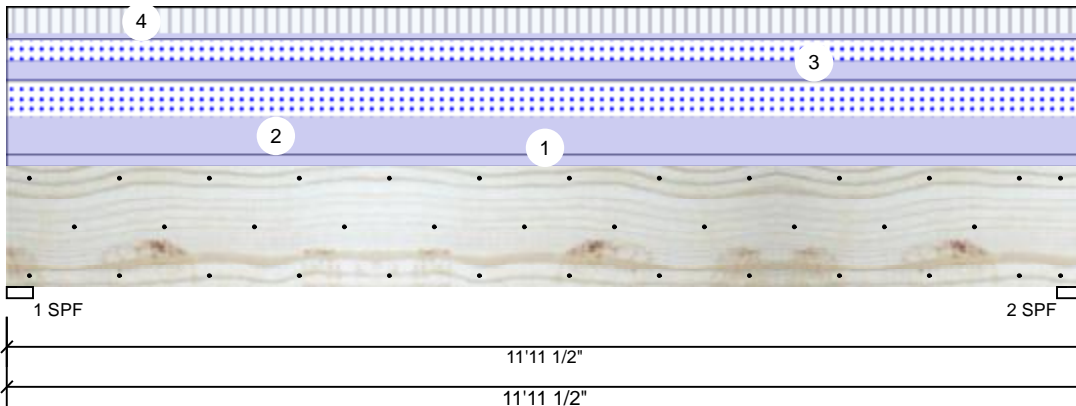


Client: Weaver Development
 Project: Poplar Elev. C
 Address: Poplar Elev. C

Date: 3/24/2021
 Input by: Christine Shivy
 Job Name: Poplar
 Project #:

BM1 Kerto-S LVL 1.750" X 16.000" 3-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	3	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	Yes
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	1046	3287	2350	0	0
2	1046	3287	2350	0	0

Bearings

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	75%	3287 / 2547	5834	L	D+0.75(L+S)
2 - SPF	3.500"	75%	3287 / 2547	5834	L	D+0.75(L+S)

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16187 ft-lb	5'11 3/4"	62010 ft-lb	0.261 (26%)	D+0.75(L+S)	L
Unbraced	16187 ft-lb	5'11 3/4"	16274 ft-lb	0.995 (99%)	D+0.75(L+S)	L
Shear	4898 lb	1'6 5/8"	20608 lb	0.238 (24%)	D+0.75(L+S)	L
LL Defl inch	0.057 (L/2434)	5'11 3/4"	0.288 (L/480)	0.200 (20%)	0.75(L+S)	L
TL Defl inch	0.130 (L/1063)	5'11 3/4"	0.384 (L/360)	0.340 (34%)	D+0.75(L+S)	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	80 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Load
2	Uniform			Top	253 PLF	0 PLF	253 PLF	0 PLF	0 PLF	B1
3	Uniform			Near Face	140 PLF	0 PLF	140 PLF	0 PLF	0 PLF	M1
4	Uniform			Far Face	58 PLF	175 PLF	0 PLF	0 PLF	0 PLF	F4
	Self Weight				19 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



This design is valid until 1/8/2023

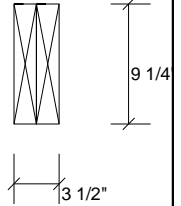
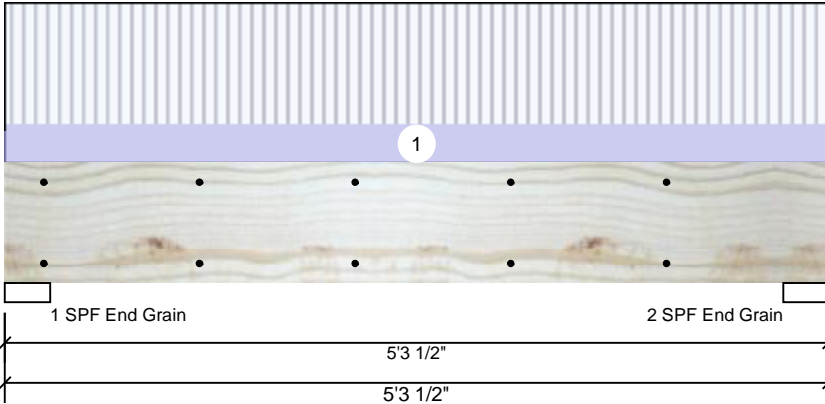


Client: Weaver Development
 Project: Poplar Elev. C
 Address: Poplar Elev. C

Date: 3/24/2021
 Input by: Christine Shivy
 Job Name: Poplar
 Project #:

BM2 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	2469	842	0	0	0
2	2469	842	0	0	0

Bearings

Bearing	Length	Cap.	React D/L	Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	31%	842 / 2469	3310	L	D+L	
2 - SPF End Grain	3.500"	31%	842 / 2469	3310	L	D+L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3654 ft-lb	2'7 3/4"	12542 ft-lb	0.291 (29%)	D+L	L
Unbraced	3654 ft-lb	2'7 3/4"	10922 ft-lb	0.335 (33%)	D+L	L
Shear	2059 lb	1'	6907 lb	0.298 (30%)	D+L	L
LL Defl inch	0.035 (L/1681)	2'7 3/4"	0.121 (L/480)	0.290 (29%)	L	L
TL Defl inch	0.046 (L/1253)	2'7 3/4"	0.161 (L/360)	0.290 (29%)	D+L	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	311 PLF	933 PLF	0 PLF	0 PLF	0 PLF	F3
	Self Weight				7 PLF					

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation
 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
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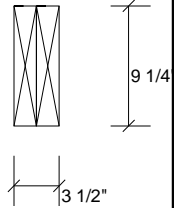
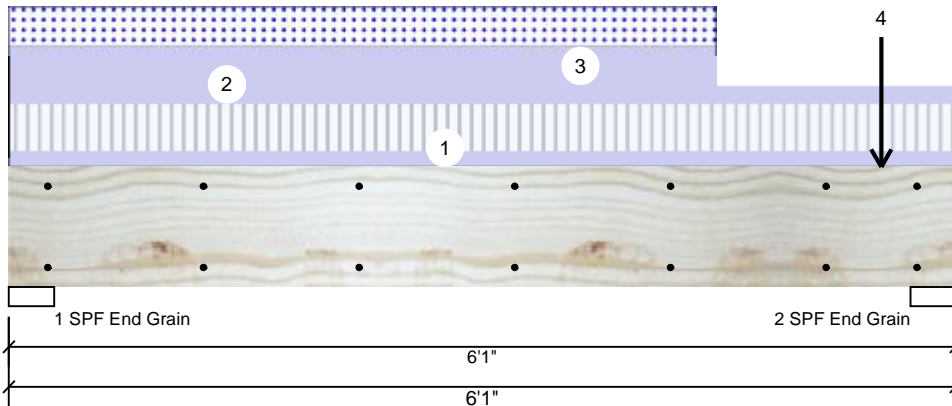


Client: Weaver Development
 Project: Poplar Elev. C
 Address: Poplar Elev. C

Date: 3/24/2021
 Input by: Christine Shivy
 Job Name: Poplar
 Project #:

F. Room W. Hdr. Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	973	1523	795	0	0
2	973	1871	1144	0	0

Bearings

Bearing	Length	Cap.	React D/L	Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	27%	1523 / 1327	2850	L	D+0.75(L+S)	
2 - SPF End Grain	3.500"	32%	1871 / 1588	3459	L	D+0.75(L+S)	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3235 ft-lb	3' 3/8"	12542 ft-lb	0.258 (26%)	D+L	L
Unbraced	3685 ft-lb	3' 5/16"	10944 ft-lb	0.337 (34%)	D+0.75(L+S)	L
Shear	2105 lb	5'1"	7943 lb	0.265 (26%)	D+0.75(L+S)	L
LL Defl inch	0.027 (L/2474)	3' 1/2"	0.141 (L/480)	0.190 (19%)	0.75(L+S)	L
TL Defl inch	0.059 (L/1152)	3' 1/2"	0.188 (L/360)	0.310 (31%)	D+0.75(L+S)	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	107 PLF	320 PLF	0 PLF	0 PLF	0 PLF	F4
2	Uniform			Top	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Load
3	Part. Uniform	0-0-0 to 4-6-8		Top	264 PLF	0 PLF	264 PLF	0 PLF	0 PLF	B1
4	Point	5-7-4		Top	740 lb	0 lb	740 lb	0 lb	0 lb	A1SE
	Self Weight				7 PLF					

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation
 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
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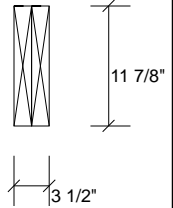
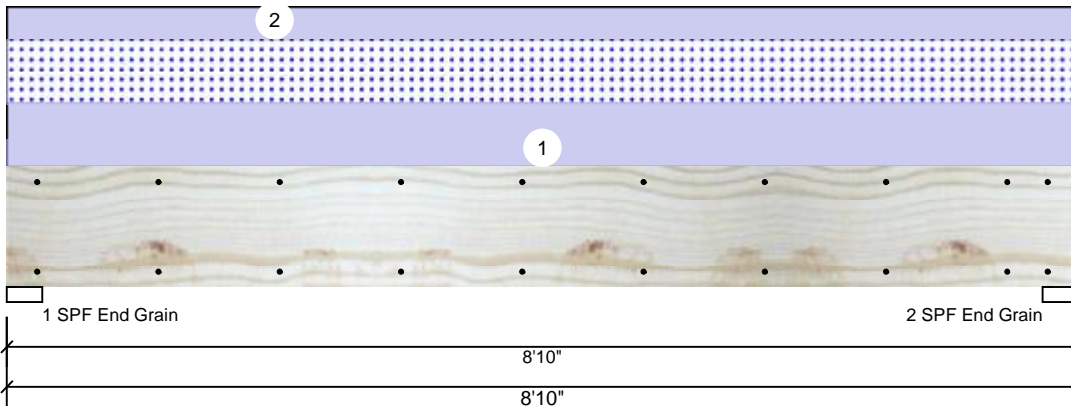


Client: Weaver Development
 Project: Poplar Elev. C
 Address: Poplar Elev. C

Date: 3/24/2021
 Input by: Christine Shivy
 Job Name: Poplar
 Project #:

GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	827	521	0	0
2	0	827	521	0	0

Bearings

Bearing	Length	Cap. React	D/L Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	13%	827 / 521	1348	L	D+S
2 - SPF End Grain	3.500"	13%	827 / 521	1348	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2676 ft-lb	4'5"	22897 ft-lb	0.117 (12%)	D+S	L
Unbraced	2676 ft-lb	4'5"	10756 ft-lb	0.249 (25%)	D+S	L
Shear	976 lb	1'2 5/8"	10197 lb	0.096 (10%)	D+S	L
LL Defl inch	0.016 (L/6189)	4'5 1/16"	0.209 (L/480)	0.080 (8%)	S	L
TL Defl inch	0.042 (L/2392)	4'5 1/16"	0.279 (L/360)	0.150 (15%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	118 PLF	0 PLF	118 PLF	0 PLF	0 PLF	M1
2	Uniform			Top	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Loads
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
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www.metsawood.com/us
 ICC-ES: ESR-3633

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 Fayetteville, NC
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 28314
 910-864-TRUS



This design is valid until 1/8/2023

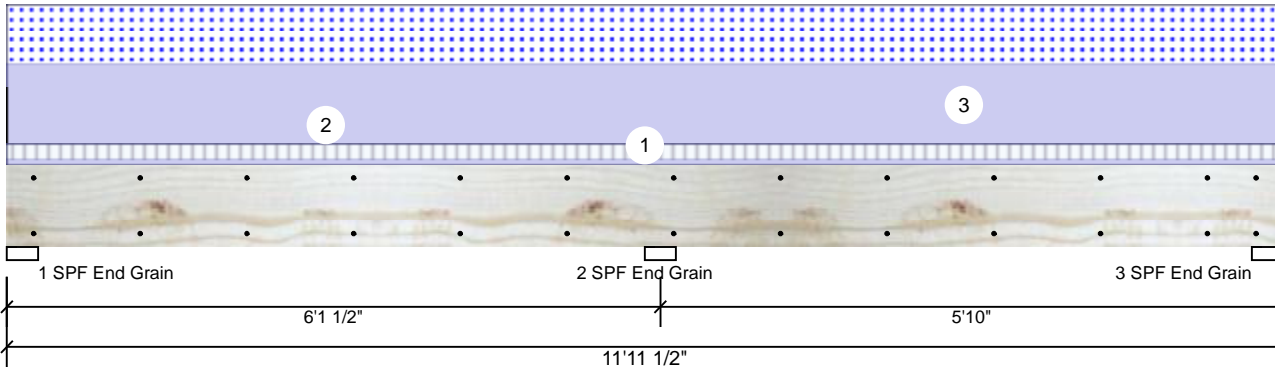


Client: Weaver Development
 Project: Poplar Elev. C
 Address: Poplar Elev. C

Date: 3/24/2021
 Input by: Christine Shivy
 Job Name: Poplar
 Project #:

PB1 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	238	1352	936	0	0
2	665	3780	2618	0	0
3	221	1256	870	0	0

Bearings

Bearing	Length	Cap.	React D/L	Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	22%	1322 / 976	2298	L_	D+S	
2 - SPF End Grain	3.500"	61%	3841 / 2661	6502	LL	D+S	
3 - SPF End Grain	3.500"	20%	1225 / 922	2146	_L	D+S	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-3744 ft-lb	6' 1 1/2"	14423 ft-lb	0.260 (26%)	D+S	LL
Unbraced	-3744 ft-lb	6' 1 1/2"	10676 ft-lb	0.351 (35%)	D+S	LL
Pos Moment	2417 ft-lb	2' 6 1/2"	14423 ft-lb	0.168 (17%)	D+S	L_
Unbraced	2417 ft-lb	2' 6 1/2"	10676 ft-lb	0.226 (23%)	D+S	L_
Shear	2604 lb	5' 4 1/4"	7943 lb	0.328 (33%)	D+S	LL
LL Defl inch	0.019 (L/3767)	2' 11 7/8"	0.147 (L/480)	0.130 (13%)	S	L_
TL Defl inch	0.042 (L/1677)	2' 11 5/16"	0.197 (L/360)	0.210 (21%)	D+S	L_

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	32 PLF	94 PLF	0 PLF	0 PLF	0 PLF	F1, F2 & F5
2	Uniform			Top	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Load
3	Uniform			Top	370 PLF	0 PLF	370 PLF	0 PLF	0 PLF	A2
	Self Weight				7 PLF					

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.
Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation
 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

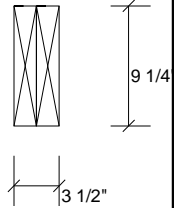
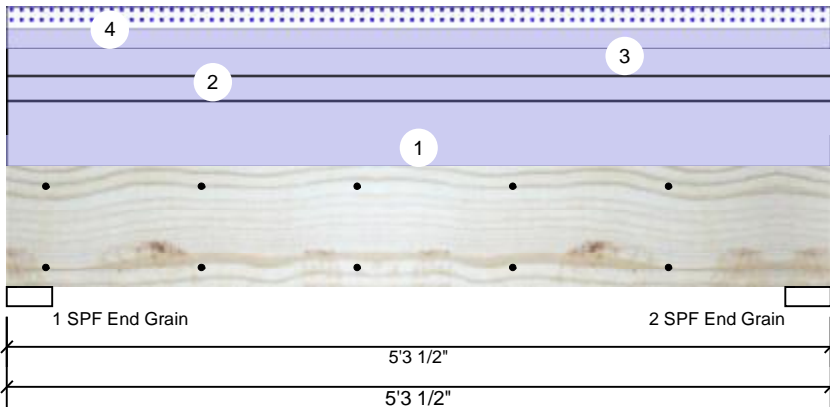
6. For flat roofs provide proper drainage to prevent ponding
 This design is valid until 1/8/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
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PB2 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	720	106	0	0
2	0	720	106	0	0

Bearings

Bearing	Length	Cap. React	D/L Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	8%	720 / 106	826	L	D+S
2 - SPF End Grain	3.500"	8%	720 / 106	826	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	795 ft-lb	2'7 3/4"	11288 ft-lb	0.070 (7%)	D	Uniform
Unbraced	795 ft-lb	2'7 3/4"	10138 ft-lb	0.078 (8%)	D	Uniform
Shear	448 lb	4'3 1/2"	6216 lb	0.072 (7%)	D	Uniform
LL Defl inch (L/39203)	0.001	2'7 3/4"	0.121 (L/480)	0.010 (1%)	S	L
TL Defl inch (L/5023)	0.012	2'7 3/4"	0.161 (L/360)	0.070 (7%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Load
2	Uniform			Top	50 PLF	0 PLF	0 PLF	0 PLF	0 PLF	A1GE
3	Uniform			Top	50 PLF	0 PLF	0 PLF	0 PLF	0 PLF	KW5
4	Uniform			Top	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	Roof Load
	Self Weight				7 PLF					

Notes
 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber
 1. Dry service conditions, unless noted otherwise
 2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation
 1. LVL beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally restrained
 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

Manufacturer Info
 Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
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 ICC-ES: ESR-3633

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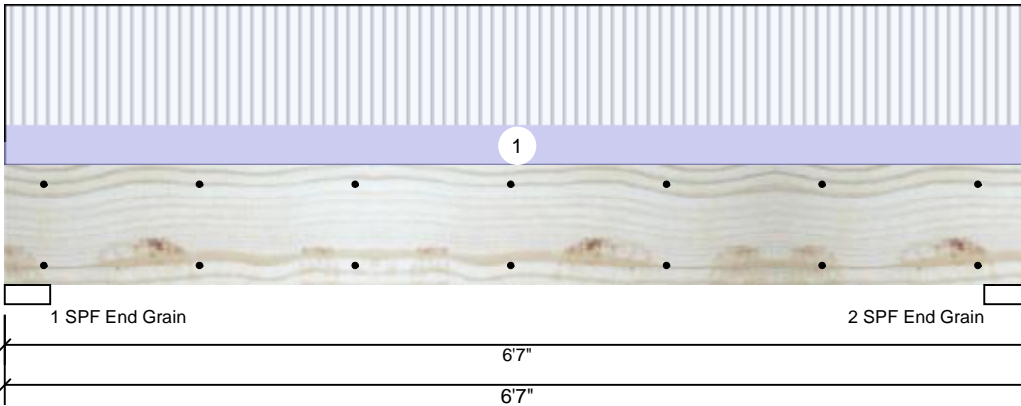


Client: Weaver Development
 Project: Poplar Elev. C
 Address: Poplar Elev. C

Date: 3/24/2021
 Input by: Christine Shivy
 Job Name: Poplar
 Project #:

Sliding Door Hdr. Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	2051	708	0	0	0
2	2051	708	0	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	26%	708 / 2051	2759	L	D+L
2 - SPF End Grain	3.500"	26%	708 / 2051	2759	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3931 ft-lb	3'3 1/2"	12542 ft-lb	0.313 (31%)	D+L	L
Unbraced	3931 ft-lb	3'3 1/2"	9934 ft-lb	0.396 (40%)	D+L	L
Shear	1921 lb	1'	6907 lb	0.278 (28%)	D+L	L
LL Defl inch	0.053 (L/1383)	3'3 1/2"	0.153 (L/480)	0.350 (35%)	L	L
TL Defl inch	0.071 (L/1028)	3'3 1/2"	0.204 (L/360)	0.350 (35%)	D+L	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	208 PLF	623 PLF	0 PLF	0 PLF	0 PLF	F1 & F2
	Self Weight				7 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

Manufacturer Info

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