

NOTES:

- GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN, BUILDER SHALL VERIET'S AND COORDINATE PER ACTUAL SITE CONDITIONS. NINDOM HEAD HEIGHTS.
 IST FLOOR = 6°-5° UNO, ON ELEVATIONS, 20° FLOOR = 7°-0° UNO, ON ELEVATIONS, ROOFINS, PITCHED SHINGLES PER DEVELOPER.

- WINDOWS: MANUFACTURER PER DEVELOPER, DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS
- ENTRY DOOR: AS SELECTED BY DEVELOPER. GARAGE DOORS: AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN.
- ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- PROTECTION AGAINST DECAY:
 (ALL PORTIONS OF A PORCH, SCREEN PORCH OR DECK FROM THE BOTTOM OF
 THE HEADER DOWN, INCLUDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE.)

- INSTALLAND COMMINICATION OF THE READER COMMINICATION OF TH

KEY NOTES:

MASONRY:

- ADHERED STONE VENEER AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- MASONRY FULL BRICK AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- MASONRY FULL STONE AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- 4 8" SOLDIER COURSE.
- 5 ROWLOCK COURSE
- (ii) NA
 TYPICALS.
 CORROSION RESISTANT SCREEN LOWERED VENTS, SIZE AS NOTED.
- 8 CODE APPROVED TERMINATION CHIMNEY CAP.
- 4 CORROSION RESISTANT ROOF TO WALL FLASHING, CODE COMPLIANT FLASHING PER NCRC R4052,8,3
- O STANDING SEAM METAL ROOF, INSTALL PER MANUFCATURER'S WRITTEN INSTRUCTIONS. DECORATIVE WROUGHT IRON, SEE DETAILS.
- SIDING.

 IZ VIRTL SHAKE SIDING FER DEVELOPER WITH VINYL CORNER TRIM FER DEVELOPER.
 (AT SPECIFIED LOCATIONS.
 FIBER CEMENT SHAKE SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.)
- 3 VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER.
- (AT SPECIFIED LOCATIONS: FIBER CEMENT LAP SIDING PER DEVELOPER W/ IX4 CORNER TRIM BOARD.)
- | VINYL WAYY SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER.
 (AT SPECIFIED LOCATIONS:
 FIBER CEMENT WAYY SIDING PER DEVELOPER W/ IX4 CORNER TRIM BOARD.)
- FIERE CEPTENT MAN'T SUMBLE PER VEYELLOPER WITH CONNER TRIM BOARD.)

 [5] VINNL BOADD AND BATT SUMNS PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER.

 (AT SPECIFIED LOCATIONS.

 [6] VINNL TRIM SUZE AS HOTED.

 (AT SPECIFIC LOCATIONS.

 IX FIBER CEPTENT TRIM OR EQUAL, UNLO. SUZE AS NOTED.

- PYPON SHUTTERS, TYPE AS SHOWN. SIZE AS NOTED. (AT SPECIFIC LOCATIONS: FALSE VINYL SHUTTERS, TYPE AS SHOWN. SIZE AS NOTED.)
- ALL MINDOWS MADSE OPENING IS LESS THAN 24" ABOVE THE FINISH FLOOR AND MADSE OPENING IS GREATER THAN 12" ABOVE THE OUTSIDE MALKING SURFACE MAST HAVE MINDOW OPENING LIMITING DEVOLES COMPLYING WITH THE NCRC SECTION R312.21 AND R312.22.



NOTES AT OPT 9'-1" PLT:

- WDW HT SET AT 7'-6"
- INTERIOR SOFFITS AT 8'-0"
- EXTERIOR SOFFITS AT 8'-0"



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PROJECT NO: GMD17049

SHEET TITLE: -RGPY GNN¬

GZVGTKOT GNGXÇVKQPU# -6GRHÛD¬

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ATTIC VENT CALCULATION FOR PLAN 'PENWELL': 1:150 RATIO.

THE NET FREE VENTILATING AREA SHALL NOT BE LESS THAN I/ISO OF THE AREA OF THE SPACE VENTILATED, PROVIDED THAT AT LEAST SO PERCENT AND NOT MORE THAN 80 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATOR'S LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED AT LEAST S TEET ABOVE THE EAVE OR CORNICE VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS.

EXCEPTIONS: 1. EXCLOSED ATTIC/RAFTER SPACES REQUIRING LESS THAN 1. SQ FT OF VENTILATION MAY BE VENTED WITH CONTINUOUS SOFFIT VENTILATION ONLY.

2. ENCLOSED ATTIC/RAFTER SPACES OVER UNCONDITIONED SPACE MAY BE VENTED WITH CONTINUOUS SOFFIT VENT ONLY

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILLATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY MITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMA CALLILATED VENTS REQUIRED. THE REQUIRED VENTILLATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED BY THE BUILDING POFFICIAL.

ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF

OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING IGA ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CEC REQUIREMENTS.
FER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-DUTS, AND ANY DOUBLE FRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2° CORROSION RESISTANT SOFFIT VENT AT INDERSIDE OF FRAMED ELEMENT.

I SQUARE INCH VENT FOR EVERY 150 SQUARE INCHES OF CEILING *144 SQ. IN. = 1 SQ. FT.

BLDG. (EILING (SF) X 144 = BLDG (SQ. IN.) BLDG. (SQ. IN.) / 150 = SQ. IN. OF VENT REQUIRED

ROOF AREA I:= |331 SF |331 SQ. FT. X 144 = 142528 SQ. IN. |42528 SQ. IN. / 150 = 1283.52 SQ. IN. OF VENT REQ'D

ROOF AREA 2:= 80 SF 80 SQ, FT, X I44 = 11520 SQ, IN, 11520 SQ, IN, / I50 = 76.80 SQ, IN, 0F VENT REQ'D

ALL ROOF DRAINAGE SHALL BE PIPED TO STREET OR APPROVED DRAINAGE FACILITY.

DASHED LINES INDICATE WALL BELOW.

LOCATE GUTTER AND DOWNSPOUTS PER BUILDER. PITCHED ROOFS AS NOTED.

- TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCS AND SHOP DRAWINGS TO THE BUILDER'S GENERAL CONTRACTOR AND BUILDING DEPARTMENT

FOR REVIEW PRIOR TO FABRICATIONS.

-ALL PLIMBING VENTS SHALL BE COMBINED INTO A MINIMUM AMOUNT OF ROOF PENETRATIONS, ALL ROOF PENETRATIONS SHALL OCCUR TO THE REAR OF THE MAIN RIDGE.

ATTIC VENT CALCULATION FOR PLAN 'PENWELL': 1:300 RATIO.

AS AN ALTERNATE TO THE I/I50 RATIO LISTED ABOVE, THE NET FREE CROSS-VENTILATION AREA MAY BE REDUCED TO I/300 HENA A CLASS I OR II VAPOR RETARDER IS INSTALLED ON THE WARM - IN - WINTER SIDE OF THE CEILING.

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMUM CALCULATED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED.
PROVIDE INSULATION STOP SUCH THAT INSULATION
DOES NOT OBSTRUCT REEE AIR MOVEMENT AS REQUIRED
BY THE BUILDING OFFICIAL.

BY THE BUILDING OFFICIAL.

ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING (AS ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION BETWEEN THE TWO OR 1901.ATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE PRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2° CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEMENT.

(PER SECTION R806.2)

I SQUARE INCH VENT FOR EVERY 300 SQUARE INCHES OF CEILING *144 SQ. IN. = 1 SQ. FT. BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) BLDG. (SQ. IN.) / 300 = SQ. IN. OF VENT REQUIRED SQ. IN. OF VENT REQUIRED / 2 = 50% AT HIGH & 50% AT LOW.

 ROOF AREA 2: =
 80 SF

 BO
 50, FT. X 144 =

 1520
 50, FT. X 144 =

 1520
 50, FT. X 144 =

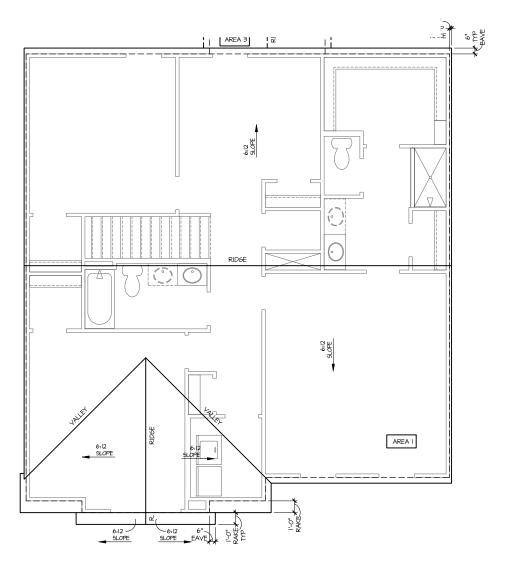
 38,40
 50, IN. /2 =

 19,20 SQ, IN. /2 =
 19,20

 50, IN. /2 =
 50, IN. /0 VENT AT LOW REQUIRED.

BUILDER TO PROVIDE (2) LAYERS OF UNDERLAYMENT AT ANY ROOF W/ A SLOPE FROM 2:12 TO LESS THAN 4:12

AT SINGLE FAMILY DETACHED PLANS: PREFINISHED VENTED SOFFIT AT EAVE PER MANUFACTURER. (VERIFY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NCRC SECTION R302.1.1 AND TABLE R302.1)



SCALE: I/4"=I'-0" AT 22"X34" LAYOUT I/8"=I'-0" AT II"XIT" LAYOUT

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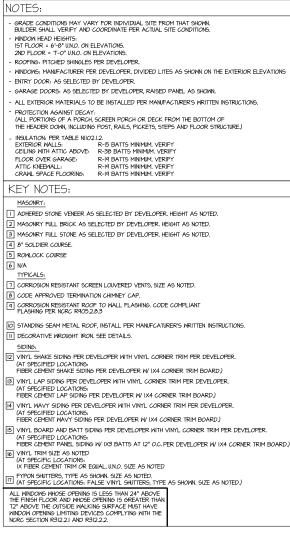
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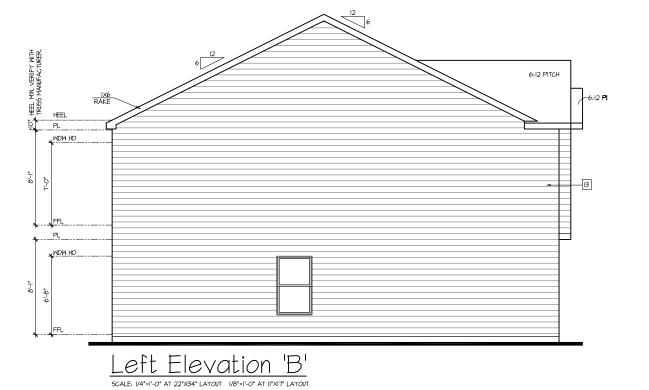
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AVAILABLE WITH OPTIONAL 9'-1" FIRST FLOOR PLATE

NOTES AT OPT 9'-1" PLT:

- WDW HT SET AT 7'-6"
- INTERIOR SOFFITS AT 8'-0"
- EXTERIOR SOFFITS AT 8'-0"







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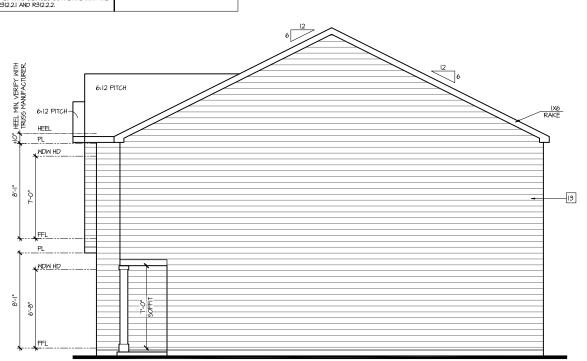
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Right Elevation 'B'

SCALE: 174"=1'-0" AT 22"X34" LAYOUT 1/8"=1'-0" AT 11"X17" LAYOUT

NOTES FOR NORTH CAROLINA:

IRRIGATION SYSTEM SHALL BE DESIGNED TO PREVENT THE SATURATION OF SOIL ADJACENT TO BUILDING.

- THIS PERIMETER DIMENSION PLAN IS FOR DIMENSIONAL INFORMATION ONLY.
- SLOPE ALL STOOPS AND HARDSCAPE MATERIAL AWAY FROM BUILDING TYPICAL.
- SLOPE GARAGE FLOOR I/8" PER FOOT TO GARAGE DOOR OPENING.
- VERIFY CURB CUT BLOCKOUT WITH GARAGE DOOR MANUFACTURER. REFER TO CIVIL DRAWINGS FOR FINISH SURFACE ELEVATIONS.
- FINISH GRADE SHALL PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING, REFER TO SOILS REPORT FOR ANY SPECIFIC REQUIREMENTS.
- REFER TO STRUCTURAL DRAWINGS FOR HOLDDOWNS, FOOTING DETAILS, CURB THICKNESS, AND INFORMATION NOT SHOWN ON THIS PLAN.
- PLUMBING FIXTURES, VENT LOCATIONS, ETC. ARE APPROXIMATE. CONTRACTOR TO VERIFY COUNT AND LOCATION. VERIFY THE SUPPLY FOR SEPARATE CONDUITS TO ANY ISLAND FOR GAS, WATER OR ELECTRIC.
- VERIFY ALL DOOR THRESHOLD HEIGHTS TO HARD SURFACES, δ I/4" MAX AT INSMING DOORS, (PER NORG SECTION R311,3.1,)
- TYP STOOP AT INSWING/SLIDER DOORS: 36" DEEP BY THE WIDTH OF THE DOOR SERVED, MINIMUM. (PER NORG SECTION R311.3.) PROVIDE A SLIP-RESISTANT FINISH.
- FOR THE USE OF EXPOSED GAS MATER HEATERS IN THE GARAGE, PROTECT THE WATER HEATER WITH 3" DIA CONCRETE FILLED STEEL PIPE EMBEDDED INTO CONCRETE FOOTING.

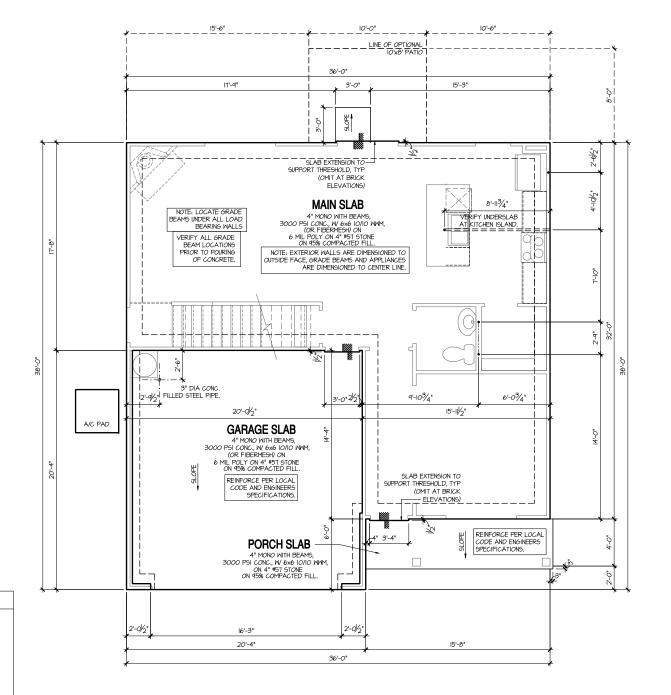
- S DIA CONCRETE FILLED STEEL FIFE EMBEDDED INTO CONCRETE FOOTING.

 SOLLS TREATMENT:

 BORACARE TERMITE TO BE APPLIED TO FRAMING PER PRODUCT SPECIFICATIONS.

 (PROVIDE CHEMICAL TREATMENT FOR PROTECTION FROM TERMITE INVESTATION ACCORDING TO THE STANDARDS OF THE NC DEPT OF AGRICULTURE).

 HOOD CONTACTING CONCRETE OR MASONRY OR LESS THAN CODE REQUIRED SEPARATION TO GRADE SHALL BE PRESEQUE TREATED OR FOUNDATION GRADE REDWOOD. SET ALL EXTERIOR WALL SILLS IN MASTIC.



Monolithic Slab Plan 'B'

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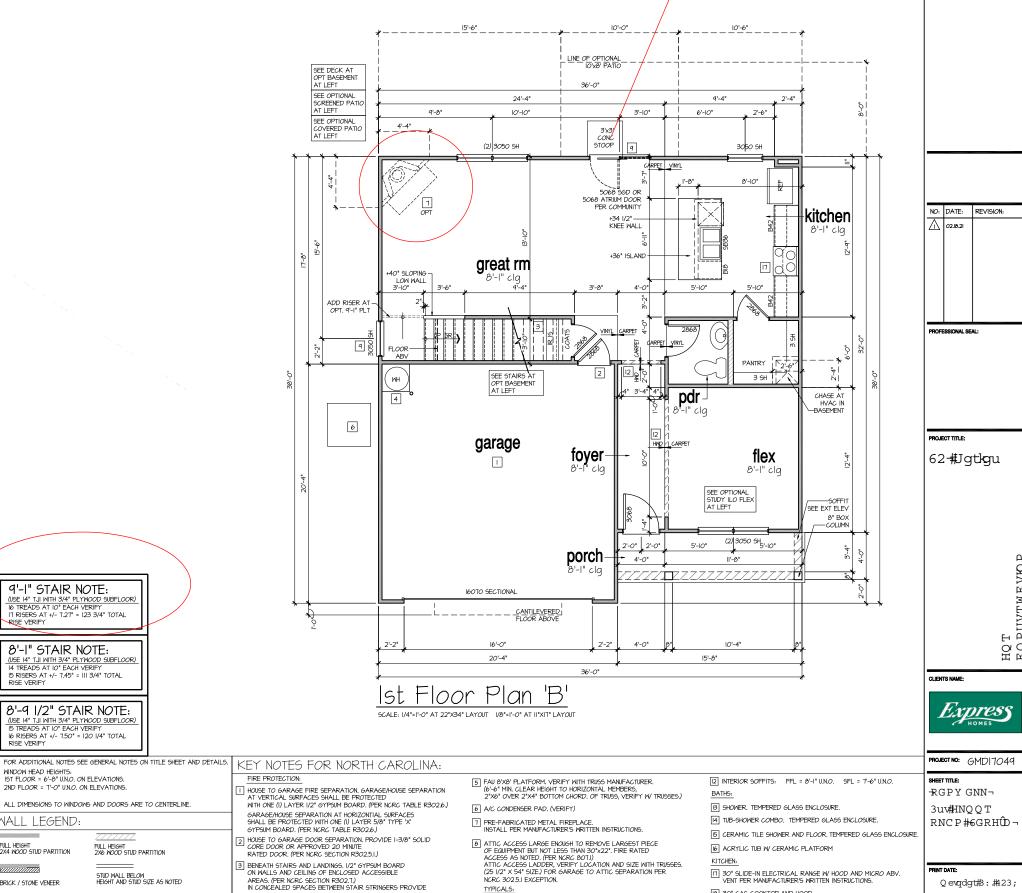
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10X12 PATIO

study PER ELEVATION SCALE: 1/4"=1'-0" AT 22"X34" LAYOUT 1/8"=1'-0" AT 11"X17" LAYOUT 9'-I" STAIR NOTE: (USE 14" T.JI WITH 3/4" PLYMOOD SUBFLOOR)
16 TREADS AT 10" EACH VERIFY
17 RISERS AT +/- T.27" = 123 3/4" TOTAL

8'-1" STAIR NOTE: (USE 14" T.JI WITH 3/4" PLYWOOD SUBFLOOR) 14 TREADS AT 10" EACH VERIFY 15 RISERS AT 4'- 7.45" = 111 3/4" TOTAL RISE VERIFY

RISE VERIFY

8'-9 1/2" STAIR NOTE: (USE 14" TJI WITH 3/4" PLYWOOD SUBFLOOR; 15 TREADS AT 10" EACH VERIFY

16 RISERS AT +/- 7.50" = 120 1/4" TOTAL RISE VERIFY

WINDOW HEAD HEIGHTS: IST FLOOR = 6'-6" U.N.O. ON ELEVATIONS. 2ND FLOOR = 7'-0" U.N.O. ON ELEVATIONS.

ALL DIMENSIONS TO WINDOWS AND DOORS ARE TO CENTERLINE.

IWALL LEGEND:

FULL HEIGHT 2X4 WOOD STUD PARTITION VIIIIIIIIII

BRICK / STONE VENEER

LOW GYPSUM BOARD WALL HEIGHT AND STUD SIZE AS NOTED

FULL HEIGHT 2X6 WOOD STUD PARTITION

STUD WALL BELOW HEIGHT AND STUD SIZE AS NOTED

DRYWALL OPENING. HEIGHT AS NOTED ON PLAN.

GARAGE/HOUSE SEPARATION AT HORIZONTIAL SURFACES SHALL BE PROTECTED WITH ONE (I) LAYER 5/8" TYPE 'X' GYPSUM BOARD. (PER NORC TABLE R302.6.)

3 BENEATH STAIRS AND LANDINGS, I/2" GYPSIM BOARD ON WALLS AND CEILING OF ENCLOSED ACCESSIBLE AREAS, (FER NORC SECTION R302.1) IN CONCEALED SPACES BETWEEN STAIR STRINGERS PROVIDE FIREBLOCKING PER R302.II

GAS WATER HEATER ON 18" HIGH PLATFORM. (PER CHAPTER 5 NCRC-PLUMBING)

TEMPERED SAFETY GLASS. (PER NCRC SECTION 308.4)

PLYWOOD SHELF ABOVE WITH DRYWALL FINISH OVER. HEIGHT AS NOTED.

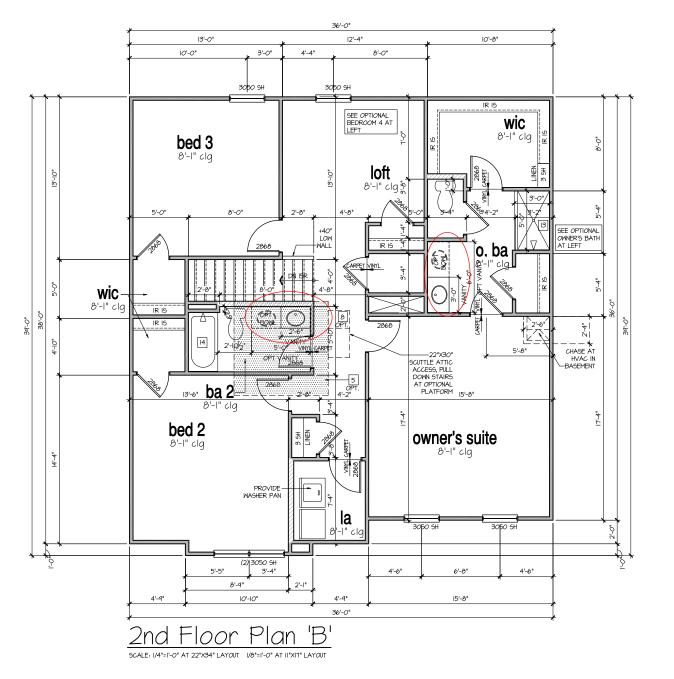
HALF WALL, HEIGHT AS NOTED.

80" GAS COOKTOP AND HOOD.
VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

4 ELECTRIC OVEN WITH MICROWAVE OVEN.

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9'-1" STAIR NOTE: (USE 14" T.J WITH 3/4" PLYWOOD SUBFLOOR) 16 TREADS AT 10" EACH VERIFY 17 RISERS AT 4/- 7.27" = 123 3/4" TOTAL RISE VERIFY

8'-1" STAIR NOTE: (USE 14" T.JI MITH 3/4" PLYMOOD SUBFLOOR) 14 TREADS AT 10" EACH YERIPY 15 RISERS AT 1/- 7.4/5" = 111 3/4" TOTAL RISE YERIPY

FOR ADDITIONAL NOTES SEE GENERAL NOTES ON TITLE SHEET AND DETAILS. KEY NOTES FOR NORTH CAROLINA: WINDOW HEAD HEIGHTS: IST FLOOR = 6'-8" U.N.O. ON ELEVATIONS.

2ND FLOOR = 7'-O" U.N.O. ON ELEVATIONS

ALL DIMENSIONS TO WINDOWS AND DOORS ARE TO CENTERLINE.

NALL LEGEND:

FULL HEIGHT 2X4 WOOD STUD PARTITION

BRICK / STONE VENEER

LOW GYPSUM BOARD WALL HEIGHT AND STUD SIZE AS NOTED

FULL HEIGHT 2X6 WOOD STUD PARTITION

STUD WALL BELOW HEIGHT AND STUD SIZE AS NOTED

DRYWALL OPENING. HEIGHT AS NOTED ON PLAN. MEP'S

FIRE PROTECTION:

HOUSE TO GARAGE FIRE SEPARATION, GARAGE/HOUSE SEPARATION AT VERTICAL SURFACES SHALL BE PROTECTED WITH ONE (I) LAYER I/2" GYPSUM BOARD, (PER NCRC TABLE R302.6.) GARAGE/HOUSE SEPARATION AT HORIZONTIAL SURFACES SHALL BE PROTECTED WITH ONE (I) LAYER 5/8" TYPE 'X' GYPSUM BOARD. (PER NCRC TABLE R302.6.)

HOUSE TO GARAGE DOOR SEPARATION, PROVIDE I-3/8" SOLID CORE DOOR OR APPROVED 20 MINUTE

RATED DOOR. (PER NORG SECTION R302.5.I.) BENEATH STAIRS AND LANDINGS. I/2" GYPSUM BOARD ON WALLS AND CEILING OF ENCLOSED ACCESSIBLE AREAS. (PER NORC SECTION R302.7.)
IN CONCEALED SPACES BETWEEN STAIR STRINGERS PROVIDE FIREBLOCKING PER R302.II

FAU 8'X8' PLATFORM, VERIFY WITH TRUSS MANUFACTURER.
(6'-6" MIN. CLEAR HEIGHT TO HORIZONTAL MEMBERS,
2"X6" OVER 2"X4" BOTTOM CHORD. OF TRUSS, VERIFY W TRUSSES.)

6 A/C CONDENSER PAD. (VERIFY)

PRE-FABRICATED METAL FIREPLACE.
INSTALL PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

ATTIC ACCESS LARGE ENOUGH TO REMOVE LARGEST PIECE
OF EQUIPMENT BUT NOT LESS THAN 30"x22". FIRE RATED
ACCESS AS NOTED, (FER NCR: 80"1).
ATTIC ACCESS LADDER, VERIFY LOCATION AND SIZE WITH TRUSSES.
(25 1/2" X 54" SIZE). FOR GARAGE TO ATTIC SEPARATION PER
NCRC 30:25.] EXCEPTION.

TOTAL ACCESS LARGE ENOUGH TO ATTIC SEPARATION PER
NCRC 30:25.] EXCEPTION.

TOTAL ACCESS LARGE ENOUGH TO ATTIC SEPARATION PER
NCRC 30:25.] EXCEPTION.

TYPICALS:

TEMPERED SAFETY GLASS. (PER NCRC SECTION 308.4)

PLYWOOD SHELF ABOVE WITH DRYWALL FINISH OVER, HEIGHT AS NOTED. II HALF WALL, HEIGHT AS NOTED.

12 INTERIOR SOFFITS: FFL = 8'-I" U.N.O. SFL = 7'-6" U.N.O.

BATHS: 3 SHOWER, TEMPERED GLASS ENCLOSURE.

14 TUB-SHOWER COMBO. TEMPERED GLASS ENCLOSURE.

RNCP#6GRHÛD¬ 15 CERAMIC TILE SHOWER AND FLOOR, TEMPERED GLASS ENCLOSURE.

6 ACRYLIC TUB W CERAMIC PLATFORM

KITCHEN: TI 30" SLIDE-IN ELECTRICAL RANGE W HOOD AND MICRO ABV. VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

B 30" GAS COOKTOP AND HOOD.

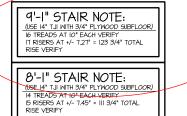
VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS. 19 ELECTRIC OVEN WITH MICROWAVE OVEN.

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- REFER TO FLOOR PLAN NOTES FOR TYPICAL FIRE PROTECTION NOTES AND LOCATIONS.
- THESE BUILDING SECTIONS MAY VARY AT ALTERNATE ELEVATION STYLES AND AT "PLAN OPTION" CONDITIONS, REFER TO MAIN FLOOR PLAN AND ALTERNATE FLOOR PLANS FOR INFORMATION NOT SHOWN HERE.
- BUILDING SECTIONS SHOWN HERE DEPICT VOLUMN SPACES WITHIN THE STRUCTURE, REFER TO STRUCTURAL DRAWINGS, TRUSS DRAWINGS, STRUCTURAL DETAILS AND CALCULATIONS BY OTHER FOR ALL STRUCTURAL INFO. ROOFING: PITCHED SHINGLE ROOF. REFER TO ROOF PLAN FOR TYPICALS.

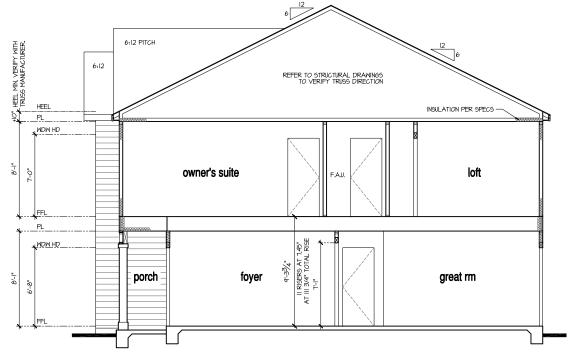
PER STATE RESIDENTIAL CODE COMPLIANCE METHOD TO BE DETERMINED BY BUILDER.

- WOOD FLOORS: FLOOR SHEATHING OVER FLOOR JOIST. REFER TO STRUCTURAL AND TRUSS DRAWINGS BY OTHERS
- VERIFY STAIRS MINIMUM AND MAXIMUM REQUIREMENTS FOR CONSTRUCTION CLEARANCES WITH LOCAL CODES.
- INSULATION:
- EXTERIOR WALLS ZONE 3: R-I3 BATTS MINIMUM, VERIFY EXTERIOR WALLS ZONE 4: R-I5 BATTS MINIMUM, VERIFY CEILING WITH ATTIC ABOVE COMPRESSED INSULATION:

CEILING WITH ATTIC ABOVE UNCOMPRESSED INSULATION (HEELS IN TRUSSES):
R-30 BATTS MINIMUM, VERIFY
FLOOR OVER GARAGE: R-14 BATTS MINIMUM, VERIFY

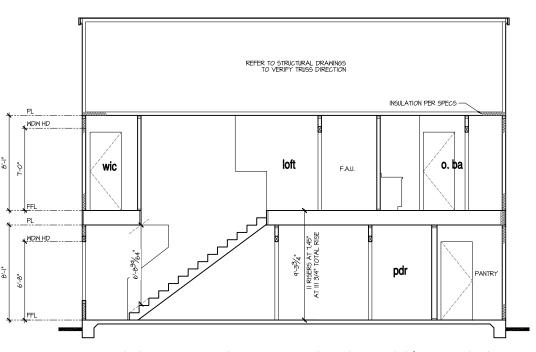
FLOOR OVER GARAGE: ATTIC KNEEWALL: CRAWL SPACE FLOORING:

WINDOW GLAZING "U" FACTOR: 0.35



Building Section Lat Monolithic Slab

REFER TO STRUCTURAL DRAWINGS TO VERIFY TRUSS DIRECTION INSULATION PER SPECS -MDW HD wic o. ba F.A.U. MDM HD pdr Building Section 2 at Optional 9' Plt scale: 1/4*=1-0" at 22754* LAYOUT 1/8*=1-0" at 11*XIT* LAYOUT



Building Section 2 at Monolithic Slab

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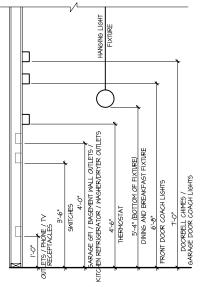
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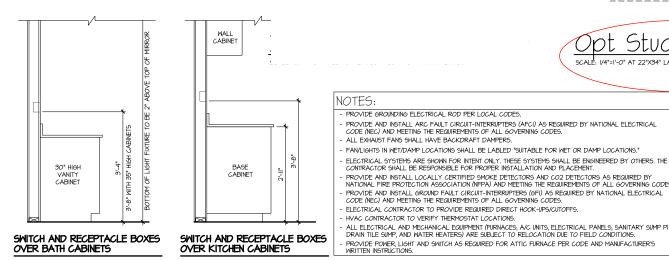
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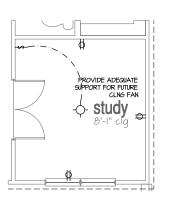
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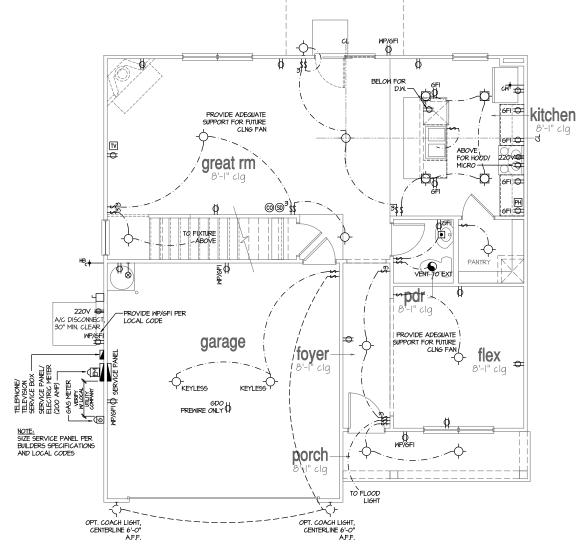


STANDARD ELECTRICAL BOX HEIGHTS









Ist Floor Plan

SCALE: I/4"=I'-0" AT 22"X34" LAYOUT I/8"=I'-0" AT II"XI7" LAYOUT

| | LEGI | END: | | | | | | |
|---------|---------------|---------------------------------------------------|------------|------------------------------------------------------------------|------------|-----------------------------------------|---------------|-------------------------------------------|
| | ф | DUPLEX OUTLET | ф | FLUSH-MOUNT LED CEILING FIXTURE | CH CH | CHIMES | | |
| | Фир/6FI | WEATHERPROOF GFI DUPLEX OUTLET | -ф- | HANGING FIXTURE | 9 | PUSHBUTTON SWITCH | ^ ^ | |
| THE | ∯ 6FI | GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET | CFP CFF | FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT) | <u>(90</u> | IIOV SMOKE DETECTOR W BATTERY BACKUP | \times | CEILING FAN (PROVIDE ADEQUATE SUPPORT) |
| | ф | HALF-SMITCHED DUPLEX OUTLET | - | | @ | CO2 DETECTOR | | |
| DDES. | ₽ 220∨ | 220 VOLT OUTLET | -\$ | 2-LIGHT VANITY FIXTURE | T | THERMOSTAT | ⊢⊗ | GAS SUPPLY WITH VALVE |
| ¥L. | 0 | REINFORCED JUNCTION BOX | -\$ | 3-LIGHT VANITY FIXTURE | PH | TELEPHONE | — ∔ HB | HOSE BIBB |
| | \$ | MALL SMITCH | - ∲ | 4-LIGHT VANITY FIXTURE | TV | TELEVISION | | 1/// 1/14/20 00/20 00/20 |
| P PITS, | \$ 3 | THREE-WAY SWITCH | -0 | WALL MOUNT FIXTURE | | ELECTRIC METER | TCM | I/4" WATER STUB OUT |
| | \$ 4 | FOUR-WAY SWITCH | • | EXHAUST FAN (VENT TO EXTERIOR) | - | ELECTRIC PANEL DISCONNECT SWITCH | ⋠ | WALL SCONCE |

NO: DATE: REVISION:

O2.18-21

PROFESSIONAL SEAL:

PROJECT TITLE:

62#Jgtlgu

HQT EQPUVTWEVIQP

CLIENTS NAM



PROJECT NO: GMD17049

SHEFT TITLE:
RGPYGNN¬

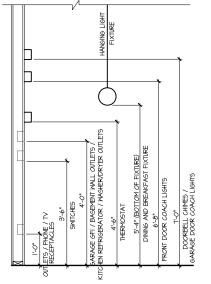
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PRINT DATE:

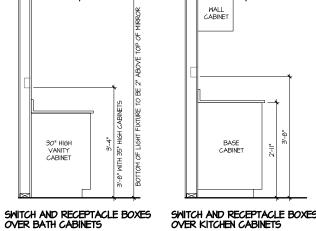
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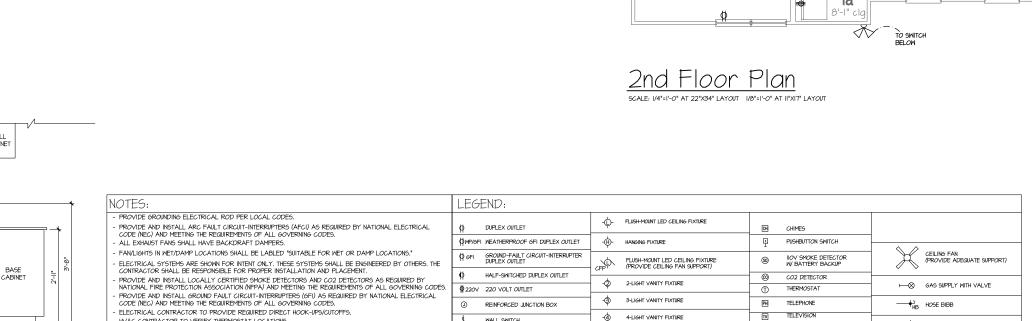
9



STANDARD ELECTRICAL BOX HEIGHTS



SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS



REINFORCED JUNCTION BOX

WALL SWITCH

THREE-WAY SWITCH

FOUR-WAY SWITCH

\$3

- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOUR-OPS/CUTOPPS.

- MAC CONTRACTOR TO VERIEY THERWOODSTAT LOCATIONS.

- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PI DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.

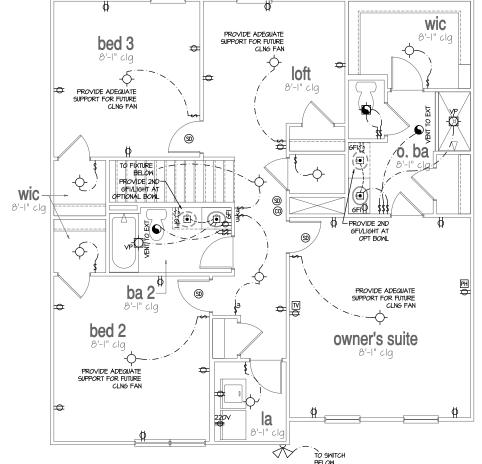
- PROVIDE POWER, LIGHT AND SMITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

3-LIGHT VANITY FIXTURE

4-LIGHT VANITY FIXTURE

WALL MOUNT FIXTURE

EXHAUST FAN (VENT TO EXTERIOR)



TELEPHONE

TELEVISION

ELECTRIC METER

ELECTRIC PANEL

DISCONNECT SWITCH

──| HDSE BIBB

MALL SCONCE

-+CM 1/4" WATER STUB OUT



UVTWEVKOP

HOT EQPt

NO: DATE: REVISION:

02.18.21

PROFESSIONAL SEAL:

PROJECT TITLE:

62#Jgtlgu

PROJECT NO: GMD17049

WVKNKV[#RNCP

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Q evqdgt#3: #423;

DESIGN SPECIFICATIONS:

Construction Type: Commerical □ Residential ⊠

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments

• ASCE 1-10: Minimum Design Loads for Buildings and Other Structures

| ign Le | oads: | | |
|--------|-------|-------------------|--------|
| | | Live Loads | |
| | 1.1. | Conventional 2x | 20 PSF |
| | 1.2. | Truss | 20 PSF |
| | | 12.1. Attic Truss | 60 PSF |
| 2. | Roof | Dead Loads | |
| | 2.1. | Conventional 2x | 10 PSF |
| | 22. | Truss | 20 PSF |
| 3. | Snow | | 15 PSF |
| | 3.1. | Importance Factor | lØ |
| 4. | Floor | Live Loads | |
| | 4.1. | Typ. Dwelling | 40 PSF |
| | 4.2. | Sieeping Areas | 3Ø PSF |
| | | | |

43. Decks ______ 4.4. Passenger Garage _____ 5. Floor Dead Loads 5.1. Conventional 2x ... 5.2. I-Joist IO PSE 5.3. Floor Truss ... Ultimate Design Wind Speed (3 sec. gust)
 Exposure
 Importance Factor 13Ø MPI-

63. Wind Base Shear 6.3.l. ∨x = 6.32.∨y =

Component and Cladding (in PSF)

| | ~ | | | |
|------------------|--------------------|--------------------|------------|--------------------|
| MEAN ROOF HT. | UP TO 30' | 30"1"-35" | 35'1"-40' | 40'1"-45' |
| ZONE I | 16.7,-18.0 | 17.5,-18.9 | 18.2,-19.6 | 18.7,-20.2 |
| ZONE 2 | 16.7,-21.0 | 17.5,-22.1 | 18.2,-22.9 | 18 .7,-23.5 |
| ZONE 3 | 16.7,-21.0 | 17.5,-22.1 | 18.2,-22.9 | 18 .7,-23.5 |
| ZONE 4 | 182,-19.0 | 19.2,-20 .0 | 19.9,-20.7 | 20.4,-21.3 |
| ZONE 5 | 18 2 ,-24.0 | 19.2,-25.2 | 19.9,-26.1 | 20.4,-26.9 |

| 3. | Seismi | c | |
|----|--------|--------------------------------|---|
| | 8.1. | Site Class | ₽ |
| | 8.2. | Design Category | C |
| | 8.3. | Importance Factor | Ø |
| | 8.4. | Seismic Use Group | 1 |
| | 8.5. | Spectral Response Acceleration | |
| | | 8 .5.1. Sms = %a | |

8.52.5ml = %g 8.6. Seismic Base Shea

8.6.2.Vy = 81. Basic Structural Sustem (check one) □ Bearing Wall
 □ Building Frame
 □ Moment Frame

□ Dual w/ Special Moment Frame
□ Dual w/ Intermediate R/C or Special Steel ☐ inverted Pendulum

GENERAL STRUCTURAL NOTES: ERAL BINUCIURAL NOTES: The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT

shall be considered the same entity.

The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.

The SER is not responsible for construction sequences, methods or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents should any non-conformities occur.

Any structural elements or details not fully developed on the

any structural elements or details not using developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to 9th milit for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions,

is not the responsibility of the SER or SUMMIT. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before

construction begins.

The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically

noted on the structural drawings.

This structure and all construction shall conform to all

applicable sections of the international residential code. This structure and all construction shall conform to all applicable sections of local building codes. All structural assemblies are to meet or except to requirements

of the current local building code

FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Varification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be



the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.

Any fill shall be placed under the direction or recommendation of a licensed professional engineer.

The resulting soil shall be compacted to a minimum of 95%

maximum dry density.

Excavations of footings shall be lined temporarily with a 6 mil polyethylene nembrane if placement of concrete does not occur within 24 hours of excavation.

No concrete shall be placed against any subgrade containing water, ice, frost, or loose material

STRUCTURAL STEEL:

1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.

Structural steel shall receive one coat of shop applied

rust-inhibitive paint.

All steel shall have a minimum yield stress (F_n) of 36 ksi unless

welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS DIL. Electrodes for shop and field welding shall be class ETOXX. All welding shall be performed by a certified welder per the above

NUMBELE:
Concrete shall have a normal weight aggregate and a minimum compressive strength (Fb) at 28 days of 3000 psi, unless otherwise noted on the plan.
Concrete shall be proportioned, mixed, and placed in

accordance with the latest editions of ACI 318: "Building Cod Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".

Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to 42% of

target values as follows: 3.1. Footings: 5% 3.2. Exterior **3**labs: 5%

No admixtures shall be added to any structural concrete without written permission of the SER.



STRUCTURAL PLANS PREPARED FOR:

PENWELL (BFK)

PROJECT ADDRESS:

OWNER: DR Horton, Inc. 8001 Arrowridge Blvd. Charlotte, NC 28273

DESIGNER: GMD Design Group 102 Fountain Brook Circle **C**ary, NC 27511

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUPMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

PLAN ABBREVIATIONS

Concrete slabs-on-grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab

subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab

cracking or other future defects resulting from unreported

cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 19-09 O.C. and in exterior slabs-on-grade at a maximum of 19-09 "unless otherwise noticed. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint. O. All welded wire fabric (WWF) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF, shall be securely supported during the concrete pour.

**Tibrous cancrect eninforcement, or filbermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered

abrasion resistance, and residual strength.
Fibermesh rehiforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.

Application of fibermesh per cubic yard of concrete shall equal a minimum of 0% by volume (15 pounds per cubic yard). Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry.

Steel reinforcing bars shall be new billet steel conforming to

ASIM A615, grade 600.

Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of the control of the

Standard Practice for Detailing Concrete Structures*
Horizontal footing and wall reinforcement shall be continuous
and shall have 90° bends, or corner bars with the same
size/spacing as the horizontal reinforcement with a class B

Lap reinforcement as required, a minimum of 40 bar diameters

For tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

The concrete slab-on-grade has been designed using a

Construction".

CONCRETE REINFORCEMENT:

ASTM A615, grade 60.

| AB | ANCHOR BOLT | PŤ | PRESSURE TREATED |
|-----|------------------------|-----|------------------------|
| ΔĦ | ABOVE FINISHED FLOOR | RS | ROOF SUPPORT |
| CJ | CEILING JOIST | SC | STUD COLUMN |
| CLR | CLEAR | 91 | SINGLE JOIST |
| DJ | DOUBLE JOIST | 5PF | SPRUCE PINE FIR |
| D6P | DOUBLE STUD POCKET | SST | SIMPSON STRONG-TIE |
| EE | EACH END | SYP | SOUTHERN YELLOW PINE |
| EW | EACH WAY | TJ | TRIPLE JOIST |
| NTS | NOT TO SCALE | TSP | TRIPLE STUD POCKET |
| ОС | ON CENTER | TYP | TYPICAL |
| PSF | POUNDS PER SQUARE FOOT | UNO | UNLESS NOTED OTHERWISE |
| PSI | POUNDS PER SQUARE INCH | WWF | WELDED WIRE FABRIC |

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the laugusts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

9. Where reinforcing dowels are required , they shall be equivalent

in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters

into the footing.

Where reinforcing steel is required vertically, dowels shall be

Design Specification for Wood Construction" (NDS). Unless

otherwise noted, all wood framing members are designed to be southern-Yellow-Pine (SYP) 2 or Southm-Spruce Pine (SYP) 2

LVL or PSL engineered wood shall have the following minimum

Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2.

winn wurth standard C-2 Mails shall be common wire nails unless otherwise nated. Lag screws shall conform to ANSI/ASME standard B1821-1981. Lead holes for lag screws shall be in accordance with NDS

specifications.

All beams shall have full bearing on supporting framing members

unless otherwise noted.

Exterior and load bearing stud walls are to be 2x4 SYP 12 = 16"

O.C. unless atherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be

discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header.

King studs shall be continuous. individual studs forming a column shall be attached with one loof nail e 6° O.5. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly

Multi-ply beams shall have each ply attached with (3) 10d nails @

Four and five ply beams shall be boited together with (2) rows of 1/2" diameter through bolts staggered • 16" O.C. unless noted otherwise.

blocked at all floor levels to ensure proper load transfer

WOOD FRAMING:

1. Solid saum wood framing members shall conform to the specifications listed in the latest edition of the "National"

provided unless otherwise noted.

design values: 2.1. E = 1,900,000 psi

22. Fb = 2600 psi

2.3.Fv = 285 psi

SHEET LIST:

| Sheet No. | Des c ription |
|-----------|----------------------------------------|
| CS1 | Cover Sheet, Specifications, Revisions |
| 51.Øm | Monolithic Slab Foundation |
| S1.Øs | Stem Wall Foundation |
| 51.Øc | Crawl Space Foundation |
| 51.Øb | Basement Foundation |
| 52.Ø | Basement Plan |
| 53.Ø | First Floor Plan |
| 54.0 | Second Floor Plan |
| 55.Ø | Roof Framing Plan |

DR HORTON PROJECT SIGN-OFF:

| Mana g er | Signatur e |
|--------------------------------|-------------------|
| Operations | |
| Operations System | |
| Operations Product Development | |

REVISION LIST

| evision No. | Date | Project No. | Descri p tion |
|----------------|---------|----------------|---------------------------------------|
| 1 | 3.16.21 | TØØ82 | Added OX-15 Bracing Plan |
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| No. | | No. | Descri p tion |
|-----|---------|-------|--------------------------|
| 1 | 3.16.21 | TØØ82 | Added OX-15 Bracing Plan |
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ILLOOD TRUSSES: The wood trues manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review the review to overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood the review.

the wood trusses.

The wood trusses shall be designed for all required loadings. as specified in the local building code, the ASCE Standard.
"Minimum Design Loads for Buildings and Other Structures."
(ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to

The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."

The trues marifacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Commected Wood Trusses" (HIB-9). This bracing, both temporary and permanent, shall be shown on the shop drawings.

Also, the shop drawings shall show the required attachments for the trusses.

Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

TERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

ULOOD STRUCTURAL PANELS.

I. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA

All structurally required wood sheathing shall bear the mark of

Wood wall sheathing shall comply with the requirements of local Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to fraining, unless noted otherwise. Roof sheathing shall be APA rated sheathing exposure I or 2.

Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nall at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use

have a span rating consistent with the framing spacing, use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.

Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (1)-8d CC ingeherk nail at 6 "Or at panel edges and at 12"or in panel field unless otherwise noted on the plans. Sheathing shall have a span rating consistent with the framing sheathing shall have a span rating consistent with the framing spacing. Use suitable edge rating consistent with the framing spacing. Use suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code

Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

TRUCTURAL FIBERBOARD PANELS:

Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
All structurally required fiberboard sheathing shall bear the mark of the AFA

mark of the AFA.

Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more.

Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

summi





STRUCTURAL MEMBERS ONLY

SCALE 2564 MF-F-6F COMMIN BY JOSE

CHECKED BY: BCP OPENAL INCOPPATION
PROJECT * DATE
34000 10400

REFER TO COVER SHEET FOR A

FOUNDATION NOTES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- AMENDMENTS.

 STRICTURAL CONCRETE TO BE F. = 3000 PSI, PREPARED AND PLACED IN ACCORDANCE WITH ACI STANDARD 318.

 FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE ENFORCEMENT OFFICIAL.

 FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF
- 2000 PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
- 5. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF MASONRY.

 6. MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS
- SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE.
 PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.

- PROVIDE FOUNDATION WATERPROVING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.

 PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE.
- 10. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK
- CRAIL SPACE TO BE GRADED LEVEL AND CLEARED OF ALL DEBRIS
- CRAIL SPACE ID BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED FER THE 200 NORTH CAROLINA RESIDENTIAL CODE SECTION R40316. MINIMUM I/2" DIA. BOLTS SPACED AT 6'-0" ON CENTER WITH A "I" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION, MINIMUM (2) ANCHOR BOLTS FER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- - DJ = DOUBLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS
 DR = DOUBLE RAFTER
 TR = TRIPLE RAFTER GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TJ = TRIPLE JOIST CL = CENTER LINE OC = ON CENTER PL = POINT LOAD
- ALL PIERS TO BE 16"x16" MA\$ONRY AND ALL PILASTERS TO BE 8"x16" MA\$ONRY, TYPICAL. (UNO)
 WALL FOOTINGS TO BE CONTINUOUS CONCRETE, \$1ZES PER STRUCTURAL PLAN.
- 12. A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER OR HIS QUALIFIED
 REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT
- ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS, ADDITIONAL INFORMATION PER SECTION R602.103 AND FIGURES R602.1065, R602.10.1 R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON
WELL-PRAINED OR SAND-GRAVEL MIXTURE SOILS
CLASSIFIED AS GROUP I PER TABLE R405.

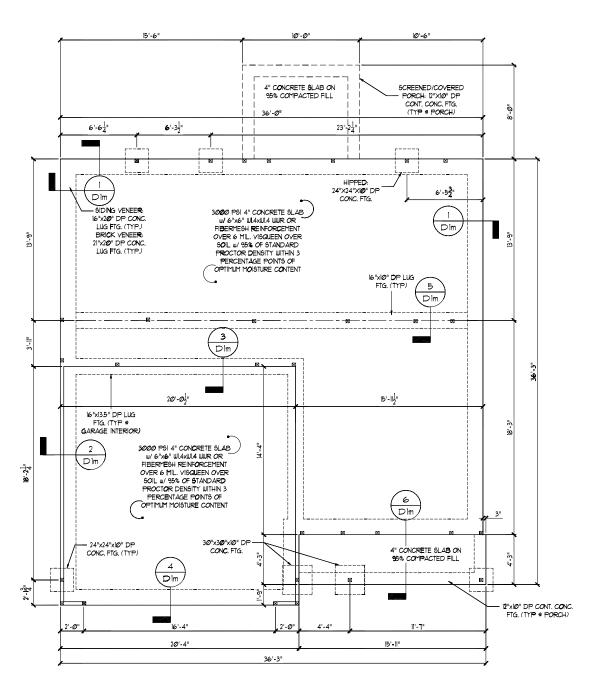
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u>
COMPLETED/REVISED ON <u>02/18/20</u>, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL ENGINEERING SEAL AFFELES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

MONOLITHIC SLAB FOUNDATION PLAN



ELEVATION B.F.K





Foundation Slab **Project:**Persol Lithic Proposition

SEAL 046048 C PHILBROW STRUCTURAL MEMBERS ONLY

CALL SECTION IN THE

DOMEN BY JOSE GEORD BY: BOP

OFFICIAL RECORDATION
PROJECT * DATE
2000 1000 PERFECTO COVER SHEET FOR A

S1.0m

| | REQUIRED BRACED WALL PANEL CONNECTIONS | | | | | | |
|----------------------------------|----------------------------------------|----------------|--------------------------------|--------------------------------|--|--|--|
| | | MIN. THICKNESS | REQUIRED CONNECTION | | | | |
| METHOD | MATERIAL | | PANEL EDGES | INTERMEDIATE SUPPORTS | | | |
| ८\$ -₩6₽ | WOOD STRUCTURAL PANEL | 3/8" | 6d COMMON NAILS 6 ° O.C. | 6d COMMON NAILS # 12" O.C. | | | |
| GB | GYPSUM BOARD | 1/2" | 5d COOLER NAILS** © 7" O.C. | 5d COOLER NAILS** # 7" O.C. | | | |
| WSP | WOOD STRUCTURAL PANEL | 3/8" | 6d COMMON NAILS | 6d COMMON NAILS # 12" O.C. | | | |
| PF | WOOD STRUCTURAL PANEL | 7/16" | PER FIGURE R602.10.6.4 | PER FIGURE R602.10.6.4 | | | |
| "OR EQUIVALENT PER TABLE RT@2.35 | | | | | | | |

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING
- CODE WITH ALL LOCAL AMENDMENTS.

 CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONTRACTOR SHALL COMPLY WITH
 THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
 PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:

- MICROLLAM (LVL): F₀ = 1600 P61, F_V = 125 P61, E = 19x00 P61
 PARALLAM (F6)L. F₀ = 12000 P61, F_V = 225 P61, E = 19x00 P61
 PARALLAM (F6)L. F₀ = 12000 P61, F_V = 230 P61, E = 125x10 P61
 ALL WOOD MEMBERS SHALL BE 70 SYP/9 SFF (UNC).

 COLUMNS AND JOISTS SHALL BE 70 SYP/9 SFF (UNC).
- 6. ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 12 SYP/12 SPF STUD COLUMN AT
- EACH END UNLESS NOTED OTHERWISE. ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM AGIS AND SHALL HAVE A MINIMUM COVER OF 3".
- FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION R40316 MINIMUM 1/2" DIA BOLTS SPACED AT 6'-0" ON CENTER WITH A 7" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE, ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION, MININUM (2) ANCHOR BOLTS FER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CELLING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- 10. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH I/3" DIA THRU BOLTS SPACED AT 24" OC. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL I/D3", MIN, EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMIM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 STP (1)25PF 12, DROPPED, FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, 6HALL BE (2) FLAT 2x4 SYP *2/SPF *2, DROPPED. (UNLESS NOTED OTHERWISE)

DJ = DOUBLE JOIST SJ = SINGLE JOIST FT = FLOOR TRUSS GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END DR = DOUBLE RAFTER TR = TRIPLE RAFTER OC = ON CENTER TJ = TRIPLE JOIST CL = CENTER LINE PI - POINT LOAD

NOTE:

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL

JOIST & BEAM SIZES SHOWN ARE MINIMMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602.108 AND FIGURES R602.1065, R602.10.1, R602.10.8(1) AND R602.108(2) OF THE 2015 IRC

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON

COMPLETED REVISED ON <u>02/18/20</u>, IT IS THE RESPONSIBILITY OF
THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING,
LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY
OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

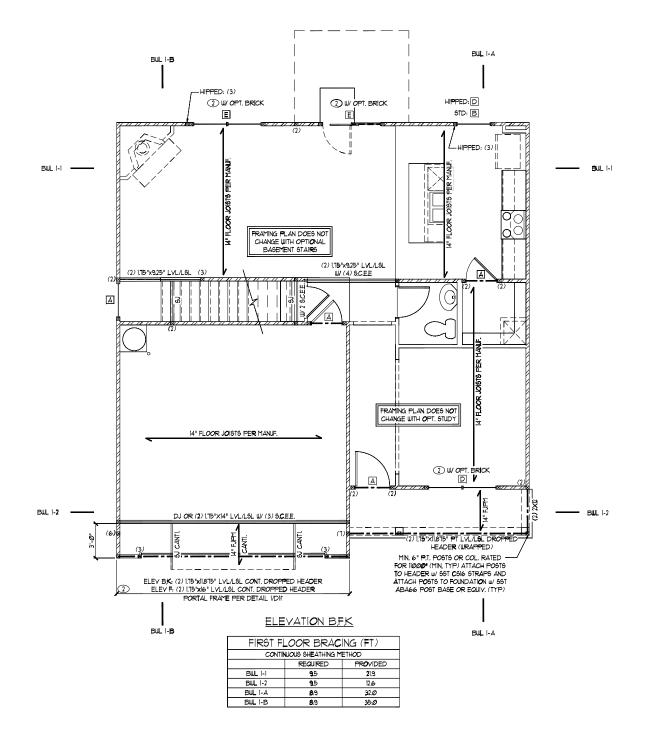
STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS, ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN

9CALE: 1/4"=1"-0" ON 22"x34" OR 1/8"=1"-0" ON 11"x1T



| | HEADER SCHEDULE | | | | | | |
|---|-----------------|--------------------|------------------|--|--|--|--|
| ſ | TAG | SIZE | JACKS (EACH END. | | | | |
| Ī | A | (2) 2x6 | (1) | | | | |
| | 3 | (2) 2x8 | (2) | | | | |
| | C | (2) 2xlØ | (2) | | | | |
| | D | (2) 2xl2 | (2) | | | | |
| ſ | E | (2) 9-1/4" L5L/LVL | (3) | | | | |
| ſ | F | (3) 2x6 | (1) | | | | |
| | G | (3) 2x8 | (2) | | | | |
| | H | (3) 2x10 | (2) | | | | |
| ſ | ĺ | (3) 2x12 | (2) | | | | |

HEADER SIZES SHOUN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE

| LINTEL SCHEDULE | | | | | | |
|-----------------|-------------------------------------|--------------------------|--|--|--|--|
| TAG | SIZE | OPPENING SIZE | | | | |
| Θ | L3x3x1/4" | LESS THAN 6'-0" | | | | |
| 2 | L5x3x1/4" | 6'- 0 " TO 10'-0" | | | | |
| 3 | L5x3-1/2"x5/16" | GREATER THAN 10'-0" | | | | |
| 4 | L5x3-1/2"x5/16" ROLLED OR EQUIV. | ALL ARCHED OPENINGS | | | | |

SECURE LINTEL TO HEADER W/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED ® 16" Q.C. (TYP FOR 3)

ALL HEADERS WHERE BRICK IS USED, TO BE: ()(UNO)

WALL STUD SCHEDULE

<u>15T & 2ND FLOOR LOAD BEARING STUDS:</u>
2x4 STUDS ● 16 * O.C. OR 2x6 STUDS ● 24 * O.C. 1ST FLOOR LOAD BEARING STUDS W/ WALK-UP ATTIC: 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BASEMENT LOAD BEARING STUDS: 2x4 STUDS # 12" OC OR 2x6 STUDS # 16" OC NON-LOAD BEARING STUDS (ALL FLOORS): 2x4 STUDS @ 24" O.C. TWO STORY WALL**S:**2x4 STUDS ● 12" O.C. OR 2x6 STUDS ● 16" O.C. BALLOON

FRAMED W/ CROSS BRACING @ 6'-0" O.C. VERTICALLY

| KING STUD REQUIREMENTS | | |
|------------------------|------------------|--|
| OPENING WIDTH | KINGS (EACH END) | |
| LESS THAN 3'-0" | (1) | |
| 3'-Ø TO 4'-Ø" | (2) | |
| 4'-0" TO 8'-0" | (3) | |
| 8'-0" TO 12'-0" | (5) | |
| 12'-0" TO 16'-0" | (6) | |

BRACED WALL NOTES

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION R60210 OF THE 2018 NC RESIDENTIAL CODE.
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND SPEEDS UP TO 130 MPH.
 REFER TO ARCHITECTURAL PLAN FOR DOORWINDOW OPENING

- SIZES.

 BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH IRC TABLE REGOLIDA.

 ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED IO FEET FOR ISOLATED PANEL METHOD AND IZ FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERS CALCULATIONS.

 MINIMUM PANEL LENGTH SHALL BE FER TABLE REGOLIDES.

 THE NITERIDE SHOTE OF STREEDICH WILL A AND, BROTH ADDITIONS.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM I/2" GYPSUM BOARD (UNO).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SUFFACES INCLIDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND
- THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

 A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF
- EACH END OF A BRACED WALL LINE.
- D. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 20 FEET.

 II. MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN
- ACCORDANCE WITH FIGURE R602.1099 OF THE 2015 IRC.
 BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.8 13. BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.82 AND
- FIGURES R602.10.8(1)4(2)4(3). CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE
- DESIGNED IN ACCORDANCE WITH SECTION R602.10.11
 PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.106.4 (UNO)
- 16 ON SCHEMATIC SHADED WALLS INDICATE BRACED WALL PANELS
 - GB = GYP9UM BOARD USP = WOOD STRUCTURAL PANEL

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| REQUIRED BRACED WALL PANEL CONNECTIONS | | | | | |
|----------------------------------------|--------------------------|----------------|-----------------------------------------------|---------------------------------|--|
| | | MIN. THICKNESS | REQUIRED CONNECTION | | |
| METHOD | MATERIAL | | 9 PANEL EDGES | # INTERMEDIATE SUPPORTS | |
| C \$ -W6P | WOOD STRUCTURAL PANEL | 3/8" | 6d COMM O N NAILS @ 6" O .C. | 6d COMMON NAILS © 12" O.C. | |
| GB | GYP9UM BOARD | 1/2" | 5d COOLER NAILS** # 7" O.C. | 5d COOLER NAIL\$** ⊕ 7" O.C. | |
| WSP | WOOD STRUCTURAL PANEL | 3/8" | 6d COMM O N NAILS @ 6" O .C. | 6d COMMON NAILS # 12" O.C. | |
| PF | WOOD STRUCTURAL PANEL | 7/16" | PER FIGURE R602.10.6.4 | PER FIGURE R602.106.4 | |
| "OR EQUIVALENT PER TABLE RT@235 | | | | | |

GENERAL STRUCTURAL NOTES

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS.
- CONTRACTOR SHALL VERIEY ALL DIMENSIONS, CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED

- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
 PROPERTIES USED IN THE DESIGN ARE AS FOLLOUIS.
 MICROLLAM (LV.L.): F₆ = 2600 PSI, F₇ = 285 PSI, E = 1.9x10° PSI
 PARALLAM (PSI.): F₆ = 27000 PSI, F₇ = 2700 PSI, E = 1.25x10° PSI
 ALL WOOD MEMBERS SHALL BET 9 SYP7° SPF UNLESS NOTED ON PLAN ALL STUD
 COLUMNS AND JOISTS SHALL BET 9.0 PTP 9.5 PF (UNL)
 ALL BEAMS SHALL BET SUPPORTED WITH A (2) 2x4 °2 SYP7°2 SPF UND.
- EACH END UNLESS NOTED OTHERWISE. 2. ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615
 AND SHALL HAVE A MINIMUM COVER OF 3'.

 2. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH
- CAROLINA RESIDENTIAL CODE SECTION RADIJA, MINIMUM IC!" DIA BOLITS SPACED AT 6'-0" ON CENTER WITH A T" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. ANCHOR BOLITS SHALL BE 12" FROM THE END OF EACH PLATE SECTION, MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL
- BE LOCATED IN THE CENTER THIRD OF THE PLATE.

 9. CONTRACTOR TO PROVIDED LOCKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- FERTENDICULAR TO RAFIEROS.

 FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX.) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D37. MIN EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 12/SPF 12. DROPPED. FOR NON-LOAD BEARING HEADERS EXCEPTING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP 12/SPF 12, DROPPED, (UNLESS NOTED OTHERWISE)

DJ = DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END

SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER

TJ = TRIPLE JOIST OC = ON CENTER CL = CENTER LINE PL = POINT LOAD

NOTE:

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST & BEAM SIZE**S** SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602.10.8 AND FIGURES R602.10.6.5, R602.10.1, R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>02/18/20</u>, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING LABORATORY & TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

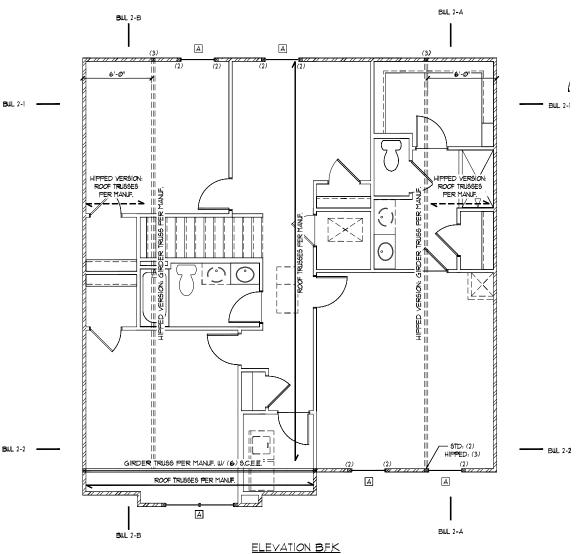
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN

SCALE: 1/4"=1"-0" ON 22"x34" OR 1/8"=1"-0" ON 11"x17"



| SECOND FLOOR BRACING (FT) | | | |
|-----------------------------|-------------------|---------------|--|
| CONTINUOUS SHEATHING METHOD | | | |
| | REQUI R ED | PROVIDED | |
| BWL 2-1 | 4.1 | 3ØØ | |
| BWL 2- 2 | 4.1 | 24 .0 | |
| BWL 2-A | 3.7 | 36 <i>.</i> Ø | |
| BWL 2-B | 3. T | 38 .Ø | |

| H | HEADER SCHEDULE | | | |
|-----|--------------------|------------------|--|--|
| TAG | SIZE | JACKS (EACH END) | | |
| Α | (2) 2x6 | (1) | | |
| В | (2) 2x8 | (2) | | |
| С | (2) 2x10 | (2) | | |
| D | (2) 2×12 | (2) | | |
| E | (2) 9-1/4" L5L/LVL | (3) | | |
| F | (3) 2x6 | (1) | | |
| G | (3) 2x8 | (2) | | |
| Н | (3) 2xlØ | (2) | | |
| 1 | (3) 2xl2 | (2) | | |

HEADER SITES SHOUN ON PLANS ARE MINIMUMS GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.
ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE. SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE.

| LINTEL SCHEDULE | | | |
|-----------------|-------------------------------------|------------------------|--|
| TAG | SIZE | OPENING SIZE | |
| Θ | L3x3x1/4" | LESS THAN 6'-0" | |
| 2 | L5x3xl/4" | 6'-0" TO 10'-0" | |
| 3 | L5x3-1/2"x5/l6" | GREATER THAN 10'-0" | |
| 4 | L5x3-1/2"x5/16" ROLLED OR EQUIV. | ALL ARCHED OPENINGS | |

SECURE LINTEL TO HEADER W/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR 3)

ALL HEADERS WHERE BRICK IS USED, TO BE: (UNO)

WALL STUD SCHEDULE

19T & 2ND FLOOR LOAD BEARING STUDS: 2x4 STUDS @ 16" O.C. OR 2x6 STUDS @ 24" O.C. 15T H.OOR LOAD BEARING STUDS W WALK-UP ATTIC: 2x4 STUDS = 12" OC. OR 2x6 STUDS = 16" OC. 2x4 STUDS = 12" OC. OR 2x6 STUDS: 2x4 STUDS = 12" OC. OR 2x6 STUDS: NON-LOAD BEARING STUDS (ALL FLOORS): 2x4 STUDS @ 24" O.C. TWO STORY WALLS:

2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON

| OPENING WIDTH | EQUIREMENTS T KINGS (EACH END) |
|----------------------------------|-----------------------------------|
| LESS THAN 3'-0" | (1) |
| 3'-0 TO 4'-0" | (2) |
| 4'- 0 " †0 8'- 0 " | (3) |
| 8'-0" TO 12'-0" | (5) |
| 12'-0" TO 16'-0" | (6) |

BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.00 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED
- PER SECTION R602.10 OF THE 2018 NC RESIDENTIAL CODE.

 WALLS ARE DESIGNED FOR SEISHIC ZONES A-C AND ULTIMATE WIND

 SPEEDS UP TO 130 MPH.

 REFER TO ARCHITECTURAL PLAN FOR DOORWINDOW OPENING

- SIZES.

 BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN
 ACCORDANCE WITH IRC TABLE R602104.

 ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND
 SHALL NOT EXCEED IO FEET FOR ISOLATED PANEL METHOD AND 12
 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMIP PANEL LENGTH SHALL BE PER TABLE R602/05,
 THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF
 MTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMIM
 1/2" GTPSUM BOARD (UNO).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS AND ON GABLE END WALLS
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- 9. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF EACH END OF A BRACED WALL LINE.
 10. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS
- SHALL NOT EXCEED 20 FEET.
- MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48' OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602109 OF THE 2015 IRC. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE
- BRACED WALL PANEL CONNECTIONS OF TROOPCELING SHALL CONSTRUCTED IN ACCORDANCE WITH SECTION R602.103
 BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.1032 AND FIGURES R602 10 8(1)4(2)4(3)
- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10.11
- 15. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE 16. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
 17. ABBREVIATIONS:

GB = GYPSUM BOARD WSP = WOOD STRUCTURAL PANEL
C6-XXX = CONT. SHEATHED ENG = ENGINEERED SOLUTION FF = PORTAL FRAME PF-ENG = ENG, PORTAL FRAME





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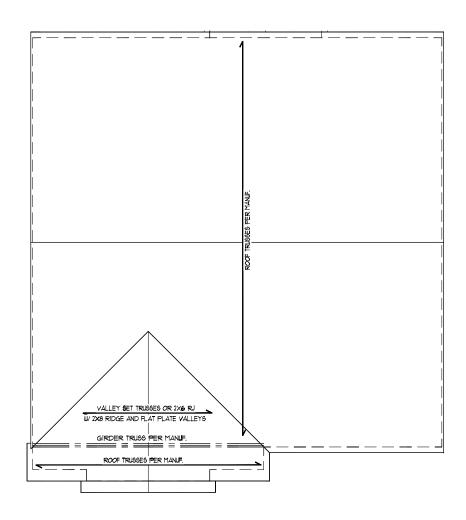


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THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR. HORTON</u> COMPLETED/REVISED ON <u>02/18/20</u>, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENSINEERING, LABORATORY 4 TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. CANOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACE TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN

SCALE: 1/4"=1"-0" ON 22"x34" OR 1/8"=1"-0" ON 11"x1

ELEVATION B.F.K.





2001 Arounidge Blvd. Charlotte, NC 26213

PROJECT:
PROMILES
First Floor Framing Plan



DRABNS
DATE GRADAGES
SCALE 2004 MAT-SP
BIT MAT-SP
FROMET N NO-THORS
DRAB BY AUE
CARCIED BY BCP

OPERAL RECIPIATION
PROJECT * DATE
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PEPER TO COVER SHEET FOR A CONFLICT LIST OF REVISIONS

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Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments

• ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

| 9" - | ougos. | | |
|------|--------|-------------------|--------|
| ٦. | Roof | Live Loads | |
| | 1.1. | Conventional 2x | 2Ø PSF |
| | 1.2. | Trus 8 | 2Ø PSF |
| | | 12.1. Attic Truss | 60 PSF |
| 2. | Roof | Dead Loads | |
| | 2.1. | Conventional 2x | 10 PSF |
| | 2.2. | Truse | 2Ø PSF |
| 3. | Snow | | 15 PSF |
| | 3.1. | Importance Factor | lø |
| 4. | Floor | Live Loads | |
| | 4.1. | Typ. Dwelling | 40 PSF |
| | | Sleeping Areas | |
| | | Decks | |
| | 4.4. | Passenger Garage | 50 PSF |
| | | | |

5. Floor Dead Loads
5.I. Conventional 2x ... 52 I-Joist

6.l. Exposure 62. Importance Factor... 63. Wind Base Shear

6.3.l. Vx =

632. Vy = T. Component and Cladding (in PSF)

| MEAN ROOF HT. | UP T Ø 3Ø' | 3 Ø'I"-35' | 35'1"-40' | 40'1"-45' |
|------------------|-------------------|-------------------|---------------------|------------|
| ZONE 1 | 16.7,-18.0 | 17.5,-18.9 | 18.2,-19.6 | 18.7,-20.2 |
| ZONE 2 | 16.7,-21.0 | 17.5,-22.1 | 18.2,-22.9 | 18.7,-23.5 |
| ZONE 3 | 16.7,-21.0 | 17.5,-22.1 | 18.2,-22.9 | 18.7,-23.5 |
| ZONE 4 | 18.2,-19.0 | 19.2,-20.0 | 19.9,-2 Ø .7 | 20.4,-21.3 |
| ZONE 5 | 18.2,-24.0 | 19.2,-25.2 | 19.9,-26.1 | 20.4,-26.9 |

Seismic Use Group ..

8.5. Spectral Response Acceleration 85.1. Sms = %g 85.2. Sml = %g 8.6. Seismic Base Shear

861.Vx = 862.Vy = 8.1. Basic Structural System (check one)

⊠ Bearing Wall ☐ Building Frame
☐ Moment Frame ☐ Dual w/ Special Moment, Frame

□ Dual w/ Intermediate R/C or Special Steel
□ Inverted Pendulum

8.8. Arch/Mech Components Anchored 8.9. Lateral Design Control: Seismic

9. Assumed Soil Bearing Capacity llind 🖂

STRUCTURAL PLANS PREPARED FOR

STANDARD DETAILS

PROJECT ADDRESS:

OUNER: DR Horton Carolinas Division

ARCHITECT/DESIGNER

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

PLAN ABBREVIATIONS:

| AB | ANCHOR BOLT | PT | PRESSURE TREATED |
|--------------|------------------------|-----|------------------------|
| AFF | ABOVE FINISHED FLOOR | RS | ROOF SUPPORT |
| CJ | CEILING JOIST | 5C | STUD COLUMN |
| CLR | CLEAR | SJ | SINGLE JOIST |
| DJ | DOUBLE JOIST | SPF | SPRUCE PINE FIR |
| DSP | DOUBLE STUD POCKET | SST | SIMPSON STRONG-TIE |
| EE | EACH END | SYP | SOUTHERN YELLOW PINE |
| EW | EACH WAY | TJ | TRIPLE JOIST |
| NTS | NOT TO SCALE | TSP | TRIPLE STUD POCKET |
| ОC | ON CENTER | TYP | TYPICAL |
| P S F | POUNDS PER SQUARE FOOT | UNO | UNLESS NOTED OTHERWISE |
| ₽91 | POUNDS PER SQUARE INCH | WWF | WELDED WIRE FABRIC |

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory 4 Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton, inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify **SUPHIT** immediately.

SHEET LIST:

| Sheet Na. | Description |
|-----------|----------------------------------------|
| CSI | Cover Sheet, Specifications, Revisions |
| Dlm | Monolithic Slab Foundation Details |
| Dls | Stem Wall Foundation Details |
| Dlc | Crawl Space Foundation Details |
| Dlb | Basement Foundation Details |
| DIf | Framing Details |

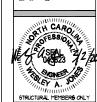
DR I

| HORTON PROJECT SIGN-OFF: | | |
|------------------------------------------|-----------|--|
| Manager | Signature | |
| Operations | | |
| perations System | | |
| erations Product Develo p ment | | |

TH CAROUS SUMMIT SUMMIT Engineering, Laboratory & Testing, P.C.

SÜMMIT

PROJECT:
Standard Details
Coversheet



- All structurally required fiberboard sheathing shall bear the mark of the AFA.
- local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

REVISION LIST:

| R e vision No. | Date | Project No. | Description |
|--------------------------|----------|----------------|---------------------------------------------------------------------------------------------------------------------------------|
| 1 | 5.11.17 | | Added box bay detail (2/D2f). Added deck options with basement, Revised deck options with stem wall and crawl space foundations |
| 2 | T.12.1T | | Revised stem wall insulation note. |
| 3 | 2.15.18 | | Revised garage door detail, NC only |
| 4 | 2.28.18 | | Added high-wind foundation details |
| 5 | 12.19.18 | | Revised per 2018 NCRC |
| 6 | 2.19.19 | | Revised per Mecklenburg County Comments |
| 1 | 3.1.19 | | Revised stem wall deck attachment and roof sheathing on wall sections. |
| 8 | 3.6.19 | | Corrected dimensions at perimeter footings |
| 9 | 32.20 | | Added tall turndown detail |
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NERAL STRUCTURAL NOTES:

The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, after, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the surposes of these construction documents the SER and SUMMIT. purposes of these construction documents the SER and SUMMIT

GENERAL STRUCTURAL NOTES:

- shall be considered the same entity.

 The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction
- to stabilize the structure.

 The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents should any non-conformities occur.

 Any structural elements or details not fully developed on the
- construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions,
- the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or 91/mill. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to 91/mill before construction begins.

 The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted to the structural drawings.
- noted on the structural drawings.

 This structure and all construction shall conform to all applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of local building codes.
 All structural assemblies are to meet or exceed to requirements.
- of the current local building code.

FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be

- 2. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- of a licensed professional engineer.
 The resulting earl shall be compacted to a minimum of 95%
- maximum dry density.

 5. Excavations of footings shall be lined temporarily with a 6 mill polyetylene memorane if placement of concrete does not occur within 24 hours of excavation.
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

- STRUCTURAL STEEL:

 1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design latest editions.
 Structural steel shall receive one coat of shop applied
- rust-inhibitive paint.

 3. All steel shall have a minimum yield stress (F_u) of 36 kg unless
- otherwise noted
- Welding shall conform to the latest edition of the American welding shall comform to the latest edition of the American Welding Society's Structural Welding Code AUSD III. Electrodes for shop and field welding shall be class ETØXX. All welding shall be performed by a certified welder per the above

- Number IE.

 Concrete shall have a normal weight aggregate and a minimum compressive strength (fe/ at 28 days of 3000 ps), unless otherwise noted on the plan.

 Concrete shall be proportioned, mixed, and placed in
- accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
 - 3.1. Footings: 5% 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER.

- Concrete slabs-on-grade shall be constructed in accordance Construction"
- The concrete slab-on-grade has been designed using a Any fill shall be placed under the direction or recomme subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab
 - cracking or other future defects resulting from urreported conditions not in accordance with the above assumptions. Control or solu cut joints shall be spaced in interior slabs-on-grade at a maximum of 15-01 O.C. and in exterior slabs-on-grade at a maximum of $|\mathcal{O}|$ unless otherwise noted. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished

 - process within 4 to 12 hours after the state has been has been intered.

 9. Reinforcing steel may extend through a control joint.

 Reinforcing steel may extend through a saw cut joint.

 10. All welded wire fabric (www.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWW. shall be securely supported during the concrete pour.

- CONCRETE REINFORCEMENT:

 I. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0,1% by volume (15 pounds per cubic yard) Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry
- standard.
 Steel reinforcing bars shall be new billet steel conforming to
- of the inferior of the state of size/spacing as the horizontal reinforcement with a class B
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- 9. Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters
- into the Footing.

 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise nated. WOOD FRAMING: Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National
- otherwise noted, all wood framing members are designed to be Spruce-Yellow-Pise (SYP) 2.

 LVL or PSL engineered wood shall have the following minimum

Design Specification for Wood Construction" (NDS), Unless

- sign values: 2.1. E = 1,900,000 psi
- 2.2. F_b = 2600 psi 2.3. F_v = 285 psi 2.4.Fc = 100 psi
- 1.4.1°C incorption.

 Who in contract with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2.
- Nails shall be common wire nails unless otherwise noted.

 Lag screws shall confrom to ANSI/ASME standard Bi82.1-1981.

 Lead holes for lag screws shall be in accordance with NDS specification.
- specifications
- All beams shall have full bearing on supporting framing members
- unless otherwise noted. Exterior and load bearing stud walls are to be 2x4 SYP $^{\circ}$ 2 = 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header.
- of one king stud shall be placed at each end of the header. King stude shall be continuous, individual stude forming a column shall be attached with one lod nail e 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer. Multi-ply beams shall have each ply attached with (3) lod nails e
- 10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be bolted together with (2) rous of 1/2" diameter through boilts staggered # 16" O.C. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each

- LICOD TRUBSES:

 I. The use of truss manufacturer/fabricator is responsible for the design of the use of trusses. Submit sealed shop chaulings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the observations are view to the design documents. The SER shall assume no compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.

 The wood trusses shall be designed for all required loadings
- Ine wood trusses shall be designed for all required loadings as specified in the local building code, the AGCE Standard "Minimum Design Loads for Buildings and Other Structures."

 (ASCE 1-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- the trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings.

 Also, the shop drawings shall show the required attachments for
- the trusses.

 Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer

EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

- WOOD STRUCTURAL PANELS:

 I. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- All structurally required wood sheathing shall bear the mark of

blacking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code. Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (I)-bd CC ringshark nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.

Wood wall sheathing shall comply with the requirements of local

building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more

information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.

Roof sheathing shall be APA rated sheathing exposure I or 2.

attached to its supporting roof framing with (I)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless

otherwise noted on the plans. Sheathing shall be applied with

the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use

suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur

Roof sheathing shall be continuous over two supports and

state Building Code.

Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

- STRUCTURAL FIBERBOARD PANELS:

 1. Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards
- 3. Fiberboard wall sheathing shall comply with the requirements of

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

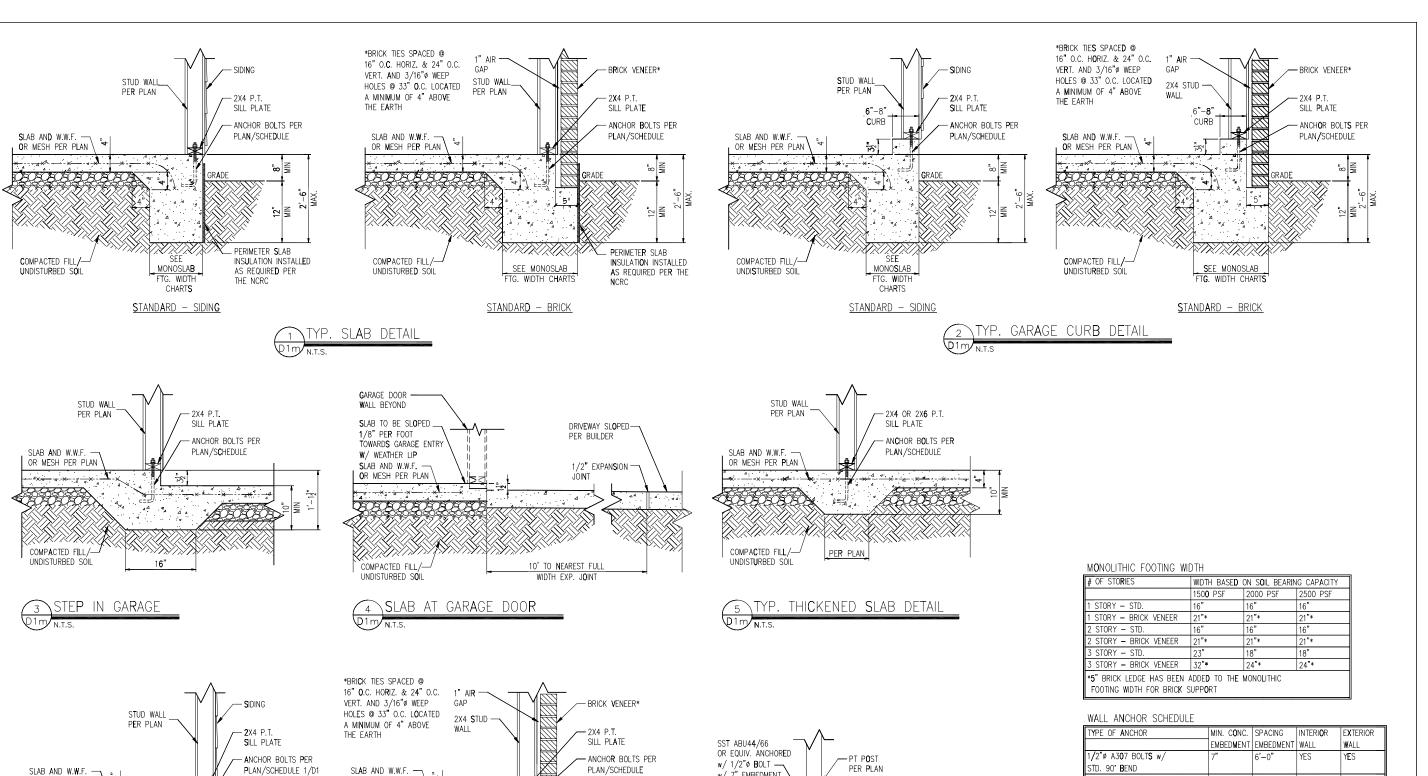
8CALE: 22x34 V4"+1"-8" lbt1 V8"+1"-8"

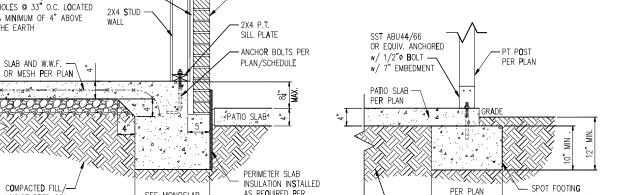
PROJECT 1 P-19Ø1-1Ø

DRAWN BY: LAG CHECKED BY: WAJ

DATE: 3/2/2

CSI





AS REQUIRED PER

THE NCRC

<u>STANDARD - BRICK</u>

SEE MONOSLAB

FTG. WIDTH CHARTS

PATIO SLAB DETAIL

UNDISTURBED SOIL

- PATIO SLAB⁴

SEE

MONOSI AF

FTG WIDTH

CHARTS

STANDARD - SIDING

- PERIMETER SLAB

THE NCRC

I**n**sulati**o**n inst**a**lled

AS REQUIRED PER

OR MESH PER PLAN

COMPACTED FILL/-

UNDISTURBED SOIL

6A COVERED PATIO DETAIL

- COMPACTED FILL/

UNDISTURBED SOIL

OR CONTINUOUS

LUG FOOTING PER PLAN

| _ | TALE ANOTHOR SOMEDULE | | | | | |
|---|--------------------------------------|--------------------|-----------|-------------------|----------|--|
| | TYPE OF ANCHOR | MIN. CONC. | SPACING | INTERI O R | EXTERIOR | |
| I | | EMBED M ENT | EMBEDMENT | WALL | WALL | |
| I | 1/2"ø A3 0 7 BOLT S w/ | 7" | 6'-0" | YES | YES | |
| | STD. 90° BEND | | | | | |
| ı | S\$T - MAS | 4" | 5'-0" | NO | YES | |
| ı | HILTI KWIK BOLT KBI 1/2-2-3/4 | 2-1/4" | 6'-0" | YES | NO | |
| ı | 1/2"ø HILTI THREADED ROD | 7" | 6'-0" | YES | YES | |
| | w/ HIT HY150 ADHESIVE | | | | | |

NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

- NOTES: 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
 SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC







Details Foundation Slab PROJECT:
Standard Details

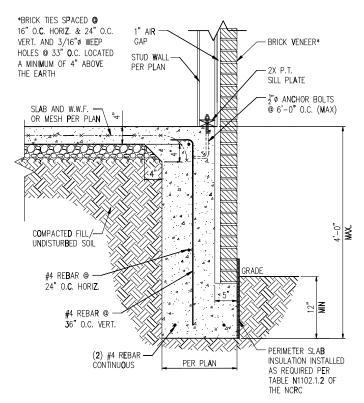
Monolithic \$



DATE: 3/2/2 8CALE: 27x34 1/4"+1"-**8"** 18x1 1/8":1"-**8"** PROJECT & P-19Ø1-1ØR DRAWN BY: LAG CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

Dlm



- NOTES:

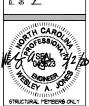
 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
 - PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
 SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
 - 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
 - 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
 - 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC





Details Foundation Slab PROJECT:
Standard Details

Monolithic (



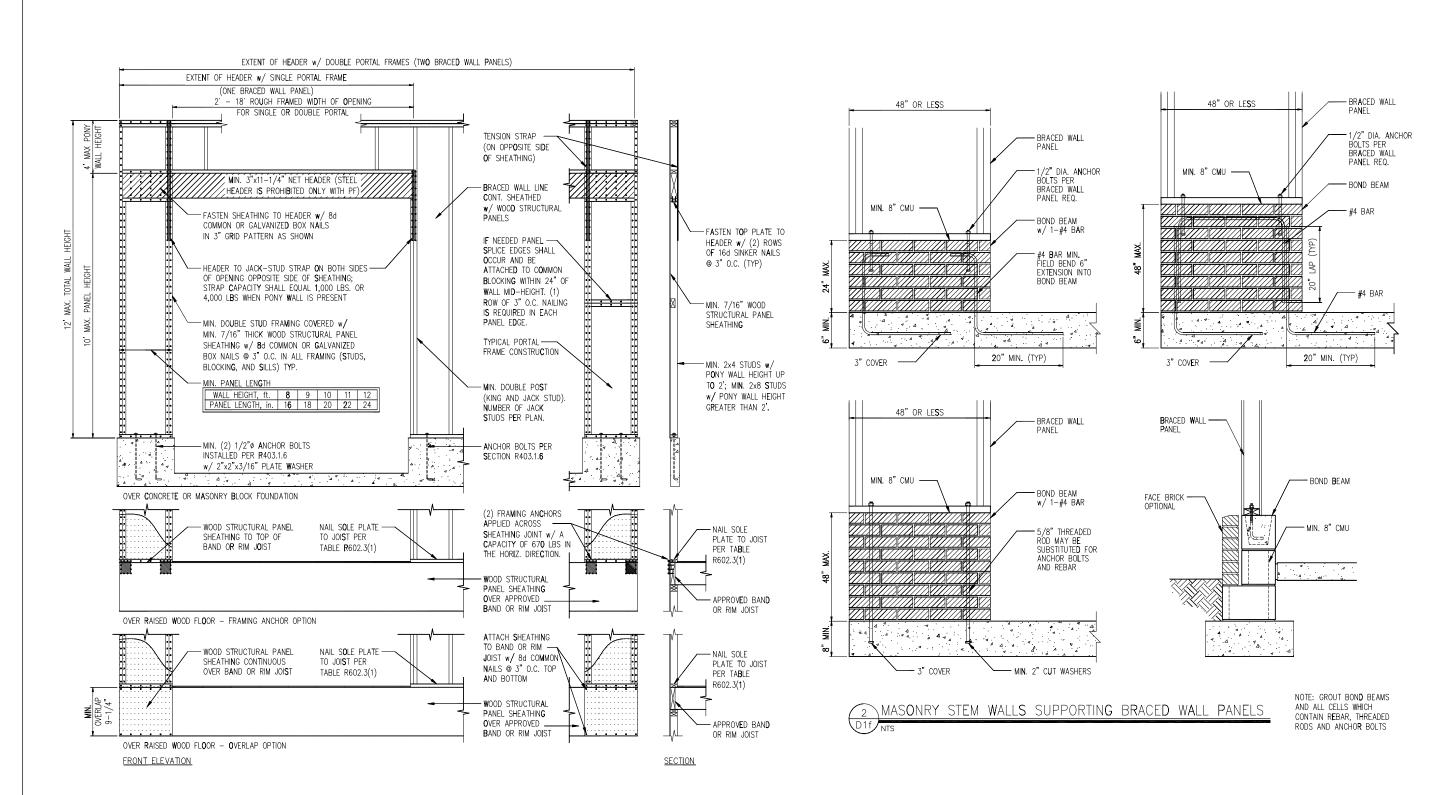
DATE: 3/2/28 8CALE: 22x34 1/4"+1-**6"** lbt1 1/8"+1-**6"** PROJECT 4 P-19Ø1-1Ø

CHECKED BY: WAJ

DRAWN BY: LAG

REFER TO GOVER SHEET FOR A COMPLETE LIST OF REVISIONS

D2m





SÜMMIT

SUMMIT Engineering, Laboratory & Testing, P.C.

CLIENT:
DR Horton Carolina Divi
8001 Arrowridge Blvd.
Charlotte, NC 20213

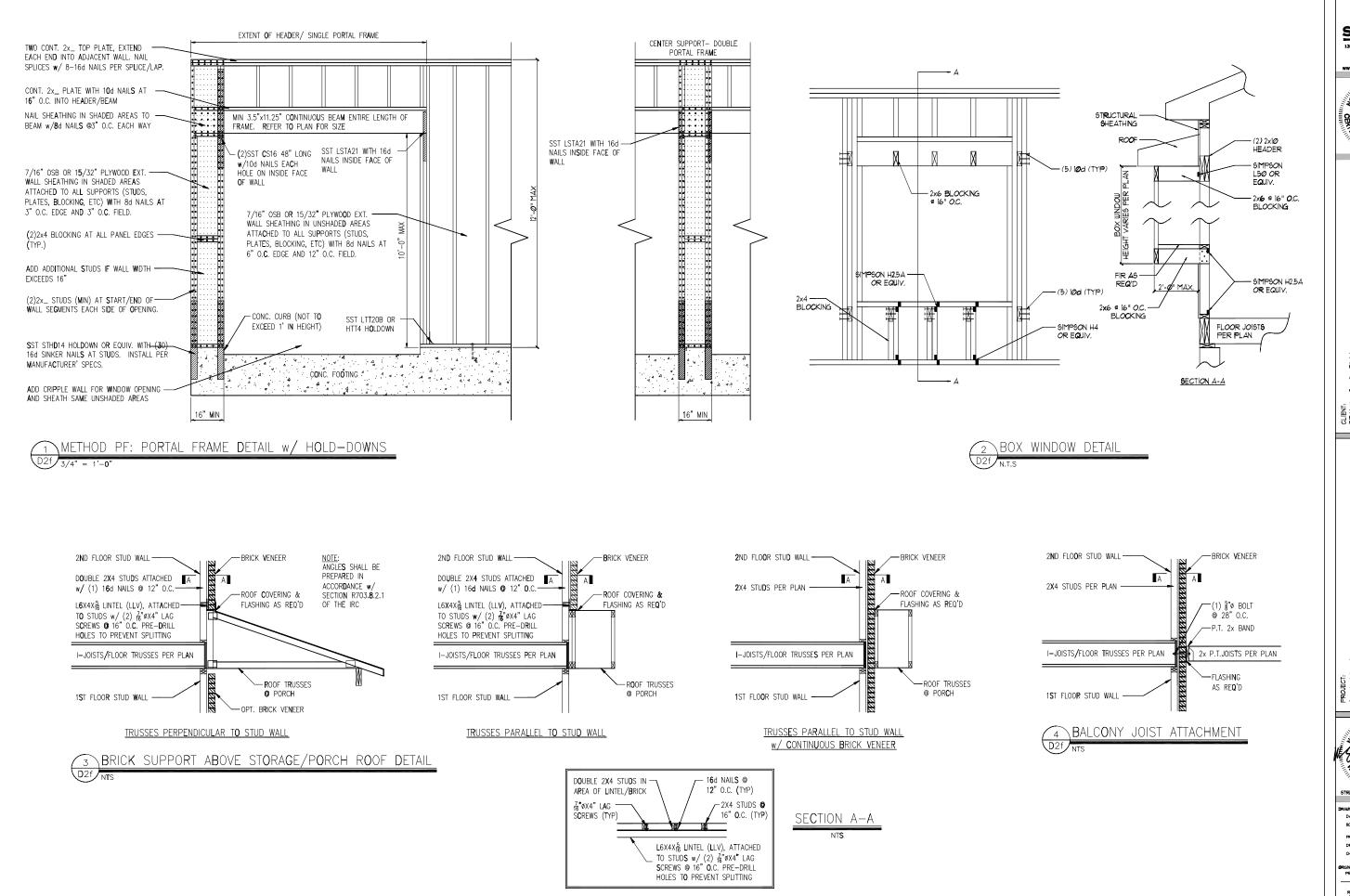


DATE: 3/2/2 8CALE: 22x34 1/4"∗1"-**8"** |bgT 1/8"∗1"-**8"** PROJECT 4 P-19Ø1-1Ø DRAIN BY: LAG CHECKED BY: WAJ

REFER TO GOVER SHEET FOR A COMPLETE LIST OF REVISIONS

D1f

METHOD PF: PORTAL FRAME DETAIL



SUMMIT





Detaí PROJECT: Standard Details Framing

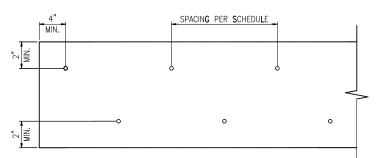


STRUCTURAL MEMBERS ONLY DATE: 3/2/2 8CALE: 22x34 1/4"∗1"-**8"** |bgT 1/8"∗1"-**8"**

PROJECT & P-19Ø1-1ØR DRAWN BY: LAG CHECKED BY: WAJ

REFER TO GOVER SHEET FOR A COMPLETE LIST OF REVISIONS

D2f



ELEVATION VIEW

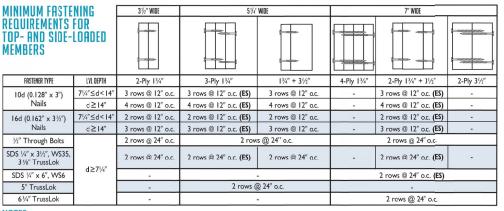
— 10d COMMON NAIL @ 12" O.C.

- SIMPSON C\$16 COIL STRAP OR EQUIV. PER MANUF. SPECIFICATIONS

EACH PLY OR PER CODE

@ 1/3 HEIGHT LOCATIONS

MULTI-PLY STUD CONNECTION DETAIL



side are to be staggered up to one-hall the o.c. spacing, but maintaining the fastene-clearances above and

(3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the

SECTION VIEW

STEEL BEAM

PER PLAN

STEEL BEAM -

PER PLAN

COPE END OF STEEL

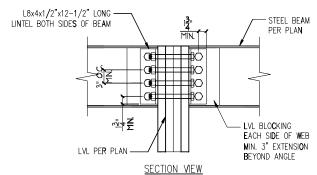
AS REQ'D TO CLEAR

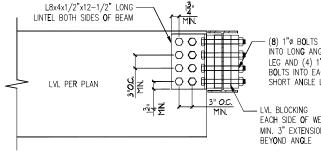
WEB OF STEEL BEAM

- STEEL BEAM PER PLAN

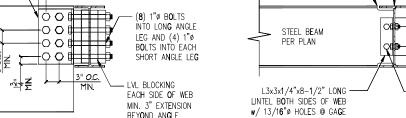
(2) 3/4"ø BOLTS

ÈACH ANGLE LEG





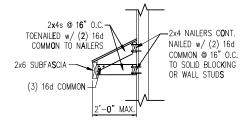
ELEVATION VIEW







ELEVATION VIEW



GABLE ROOF RETURN

SÜMMIT

SUMMIT LEDGE OF THE OF T

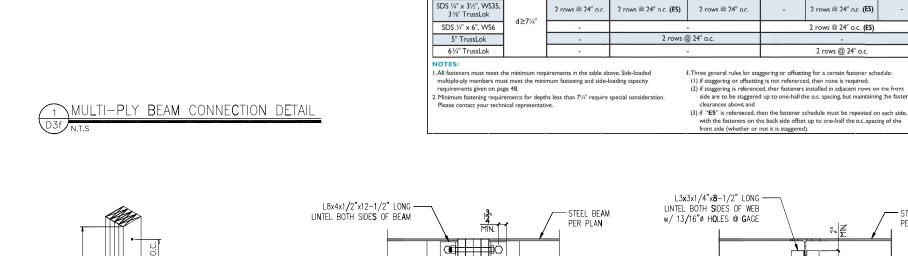
PROJECT: Standard Details Framing Details



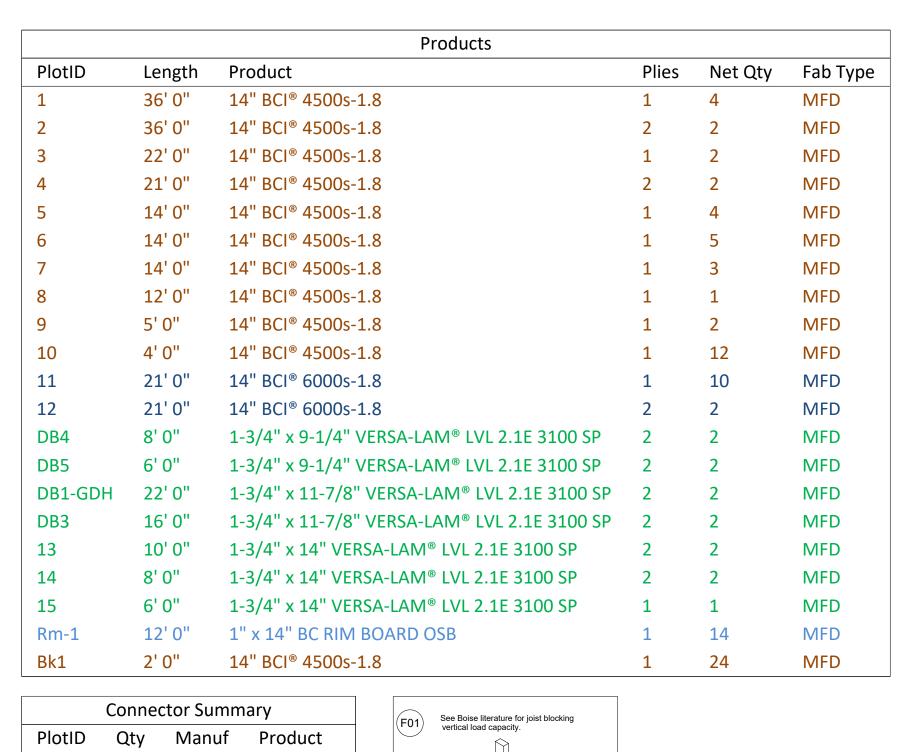
DATE: 3/2/2 8CALE: 22x34 1/4"∗1"-**6"** lbt1 1/8"∗1"-**6"** PROJECT 4 P-1907-10R DRAIIN BY: LAG CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D3f



D R Horton Penwell Elev.B 36 Morgan North



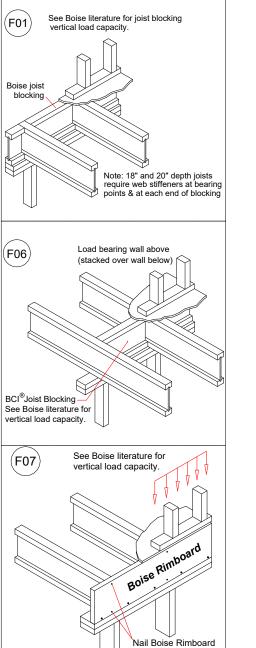
| Connector Summary | | | | | | |
|-------------------|-----|-------|----------|--|--|--|
| PlotID | Qty | Manuf | Product | | | |
| H1 | 23 | USP | THF17140 | | | |
| Н3 | 1 | USP | THD414 | | | |
| | | | | | | |

DB3

Rm-1

F07

Rm-1



All I-Joist and Versa-Lam Beams Must be Installed per The Boise Cascade Installation Guide!

Squash Blocks Required Under The Ends Of All LVL And Point Loads For Load Transfer - See Details

| Revisions: | BY: | |
|------------|-----|--|
| | | |
| | | |
| | | |
| | | |
| 06-25-18 | | |
| | | |



BC FRAMER II

Sheet: 2/3

Plan Date: 10/18/2019

Structural Date: 05/28/2021 By: GAT

Second Floor Layout

Rm-1

(F07)

F07

DB4

(F07)

Rm-1

11

11

11

12

11

11

11

11

11

11

Rm-1

Rm-1

DB5

