

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 19740-19740A Charleston B Vault Master

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I35910489 thru I35910497

My license renewal date for the state of North Carolina is December 31, 2019.

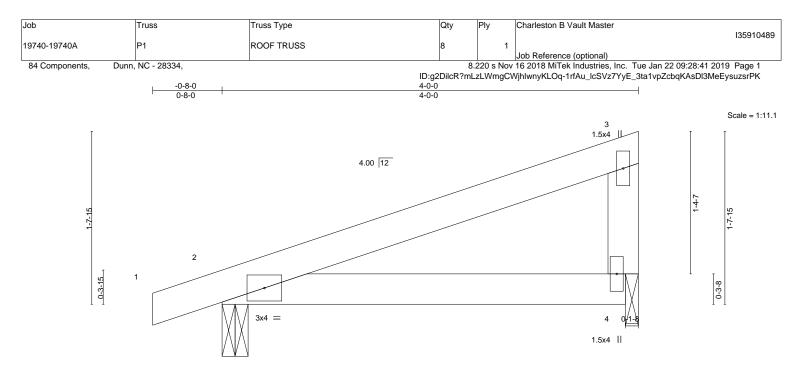
North Carolina COA: C-0844



January 22,2019

Johnson, Andrew

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



			3-10-8 3-10-8				<u>4-0-0</u> 0-1-8	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL)	0.02 4-7	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(CT)	-0.02 4-7	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00 2	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP					Weight: 15 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=198/0-3-0, 4=151/0-1-8 Max Horz 2=45(LC 12) Max Uplift 2=-57(LC 12), 4=-46(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left exposed;

porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



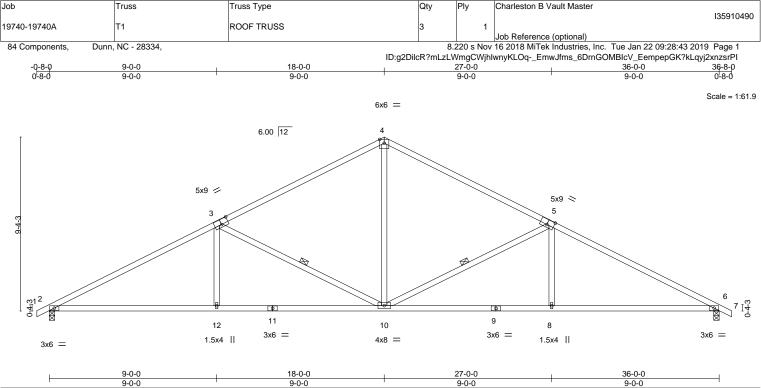


Plate Offs	Plate Offsets (X,Y) [3:0-4-8,0-3-4], [5:0-4-8,0-3-4]											
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.91	Vert(LL)	-0.18	8-18	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.97	Vert(CT)	-0.41	8-18	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.48	Horz(CT)	0.12	6	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-AS						Weight: 171 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

5-10, 3-10

Rigid ceiling directly applied

1 Row at midpt

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

WEBS 2x4 SP No.3

- REACTIONS. (lb/size) 2=1480/0-3-8, 6=1480/0-3-8 Max Horz 2=-158(LC 10) Max Uplift 2=-89(LC 12), 6=-89(LC 12)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-2607/432, 3-4=-1777/377, 4-5=-1777/377, 5-6=-2607/432

BOT CHORD 2-12=-272/2268, 10-12=-276/2259, 8-10=-277/2259, 6-8=-273/2268

WEBS 4-10=-120/993, 5-10=-907/252, 5-8=0/381, 3-10=-907/252, 3-12=0/381

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

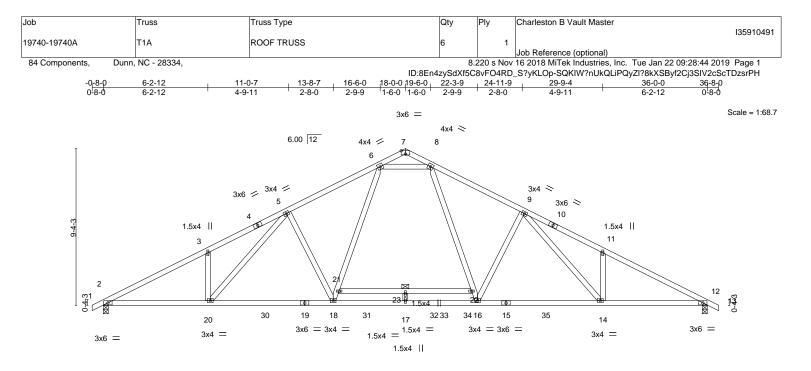
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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1	6-2-12	13-8-7	18-0-0	22-3-9	29-9-4	36-0-0	1
F	6-2-12	7-5-11	4-3-9	4-3-9	7-5-11	6-2-12	
Plate Offsets (X,Y)	[7:0-3-0,Edge]						
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) l/defl l	_/d PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.84	Vert(LL) -0.2	7 18-20 >999 2	40 MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.82	Vert(CT) -0.4	4 18-20 >980 1	80	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.46	Horz(CT) 0.1	2 12 n/a i	n/a	
BCDL 10.0	Code IRC2015/	FPI2014	Matrix-MS			Weight: 205 lb	FT = 20%
BOT CHORD 2x4 WEBS 2x4	SP No.2 SP No.2 SP No.3 *Except* 22: 2x4 SP No.2			BRACING- TOP CHORD BOT CHORD WEBS		athing directly applied or 2-8-12 v applied or 10-0-0 oc bracing. 21-22	2 oc purlins.
(size) 2=1480/0-3-8, 12= × Horz 2=-158(LC 10)	480/0-3-8					

Max Horz 2=-158(LC 10) Max Uplift 2=-89(LC 12), 12=-89(LC 12) Max Grav 2=1517(LC 28), 12=1516(LC 31)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2848/438, 3-5=-2841/544, 5-6=-2142/441, 8-9=-2137/441, 9-11=-2840/544, 11-12=-2848/438

- BOT CHORD
 2-20=-306/2484, 18-20=-213/2079, 17-18=-94/1633, 16-17=-94/1633, 14-16=-214/2076, 12-14=-308/2483

 WEBS
 11-14=-328/180, 3-20=-328/180, 5-18=-592/209, 9-16=-592/209, 5-20=-145/655,
- WEBS 11-14=-328/180, 3-20=-328/180, 5-18=-592/209, 9-16=-592/209, 5-20=-145/655, 9-14=-145/656, 18-21=-96/720, 6-21=-92/728, 8-22=-92/728, 16-22=-97/720, 6-8=-1692/484

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat.
 II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12.

6) Load case(s) 26, 27, 28, 29, 30, 31 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

7) MULTIPLE LOADCASES - This design is the composite result of multiple load cases.

8) User moving load cases exist: Review the load cases for details.

9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard Except:

26) User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-7=-60(F), 7-13=-60(F), 24-27=-20(F)

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Charleston B Vault Master	
			_			135910491
19740-19740A	T1A	ROOF TRUSS	6	1	Job Reference (optional)	
					JOD Reference (optional)	
84 Components,	Dunn, NC - 28334,		8.	220 s Nov	16 2018 MiTek Industries, Inc. Tue Jan	22 09:28:44 2019 Page 2

8.220 s Nov 16 2018 MiTek Industries, Inc. Tue Jan 22 09:28:44 2019 Page 2 ID:8En4zySdXf5C8vFO4RD_S?yKLOp-SQKIW?nUkQLiPQyZI?8kXSByf2Cj3SIV2cScTDzsrPH

LOAD CASE(S)

- 27) 1st User Defined Moving Load User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-7=-60(F), 7-13=-60(F), 18-24=-20(F), 18-31=-50(F=-20), 27-31=-20(F) 28) 2nd User Defined Moving Load - User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-7=-60(F), 7-13=-60(F), 24-31=-20(F), 17-31=-50(F=-20), 17-27=-20(F) 29) 3rd User Defined Moving Load - User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-7=-60(F), 7-13=-60(F), 17-24=-20(F), 17-32=-50(F=-20), 27-32=-20(F) 30) 4th User Defined Moving Load - User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-7=-60(F), 7-13=-60(F), 24-32=-20(F), 32-34=-50(F=-20), 27-34=-20(F)
- 31) 5th User Defined Moving Load User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-7=-60(F), 7-13=-60(F), 24-33=-20(F), 16-33=-50(F=-20), 16-27=-20(F)

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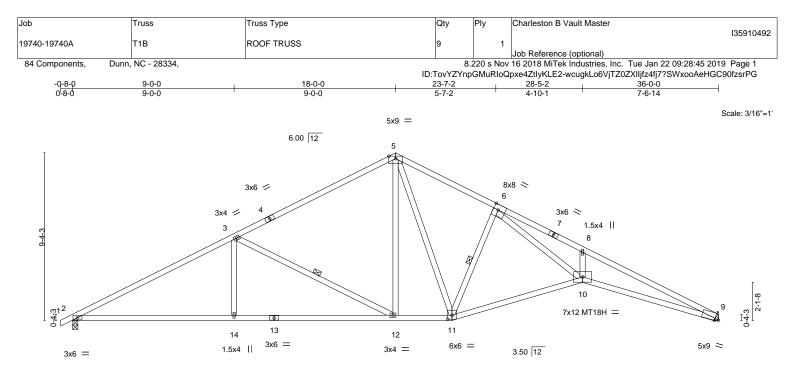


Plate Offsets (X,Y) [9-0-0 9-0-0 6:0-3-4,Edge], [9:0-2-7,Edge], [11:0-3	18-0-0 9-0-0 -0,0-2-10]	21-1-12 3-1-12	28-5-2 7-3-6	36-0-0 7-6-14
LOADING (psf) FCLL 20.0 FCDL 10.0 3CLL 0.0 3CDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.87 BC 0.95 WB 0.93 Matrix-AS	Vert(LL) -0.4	in (loc) l/defl L/d 1 10-11 >999 240 9 10-11 >484 180 2 9 n/a n/a	PLATES GRIP MT20 244/190 MT18H 244/190 Weight: 184 lb FT = 20%
7-9: 2x4	No.2 *Except* I SP DSS No.2 *Except*		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing dir Rigid ceiling directly applied. 1 Row at midpt 3	ectly applied. -12, 6-11

- BOT CHORD
 2x4 SP No.2 *Except*

 9-10: 2x4 SP No.1

 WEBS
 2x4 SP No.3 *Except*

 6-10: 2x4 SP No.2
- REACTIONS. (Ib/size) 2=1480/0-3-8, 9=1440/Mechanical Max Horz 2=157(LC 11) Max Uplift 2=-89(LC 12), 9=-70(LC 12)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-2622/436, 3-5=-1772/378, 5-6=-1954/448, 6-8=-5671/942, 8-9=-5744/830
- BOT CHORD 2-14=-291/2285, 12-14=-291/2285, 11-12=-64/1465, 10-11=-244/2261, 9-10=-689/5265
- WEBS 3-14=0/392, 3-12=-931/253, 5-12=-30/559, 6-11=-1239/282, 6-10=-562/3805,
 - 8-10=-278/193, 5-11=-156/679

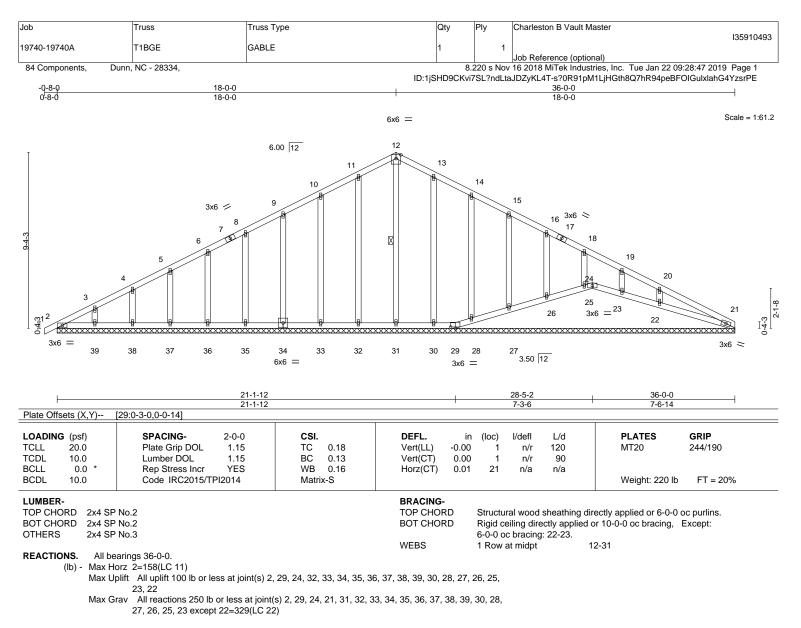
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 9.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 29, 24, 32, 33, 34, 35, 36, 37, 38, 39, 30, 28, 27, 26, 25, 23, 22.

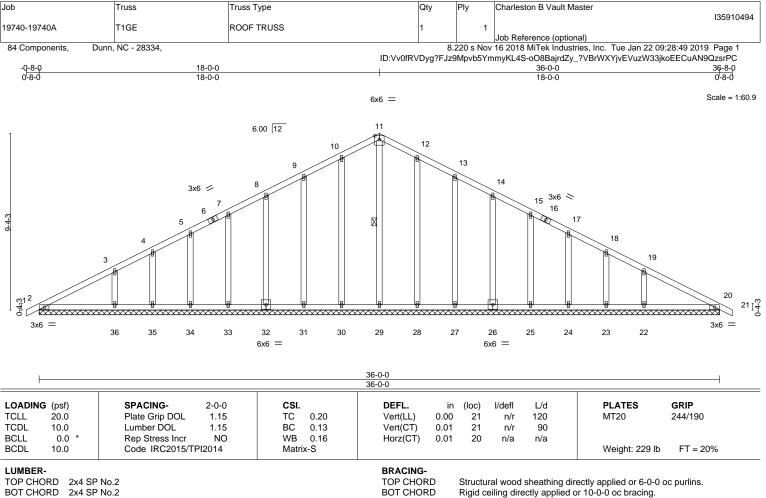
10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 24, 28, 27, 26, 25, 23, 22.



818 Soundside Road Edenton, NC 27932

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¹⁾ Unbalanced roof live loads have been considered for this design.



WEBS

1 Row at midpt

11-29

2x4 SP No.2 BOT CHORD OTHERS 2x4 SP No.3

REACTIONS. All bearings 36-0-0.

(lb) -Max Horz 2=-158(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 30, 31, 32, 33, 34, 35, 36, 28, 27, 26, 25, 24, 23, 22 All reactions 250 lb or less at joint(s) 2, 29, 30, 31, 32, 33, 34, 35, 28, 27, 26, 25, 24, 23, 20 Max Grav except 36=320(LC 21), 22=320(LC 22)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

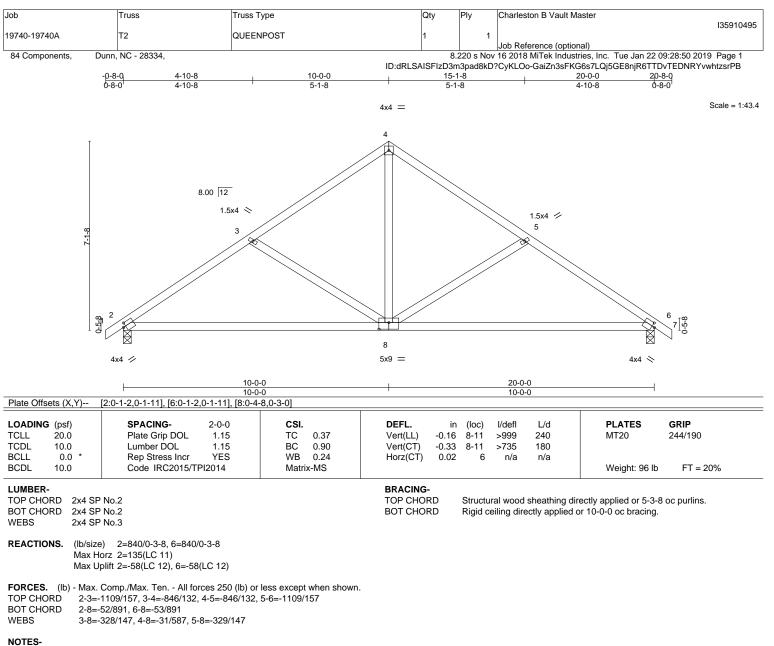
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 30, 31, 32, 33, 34, 35, 36, 28, 27, 26, 25, 24, 23, 22.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLODED MITER REFERENCE FAGE MIT F14's rev. Involved to Bercher Gosc. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

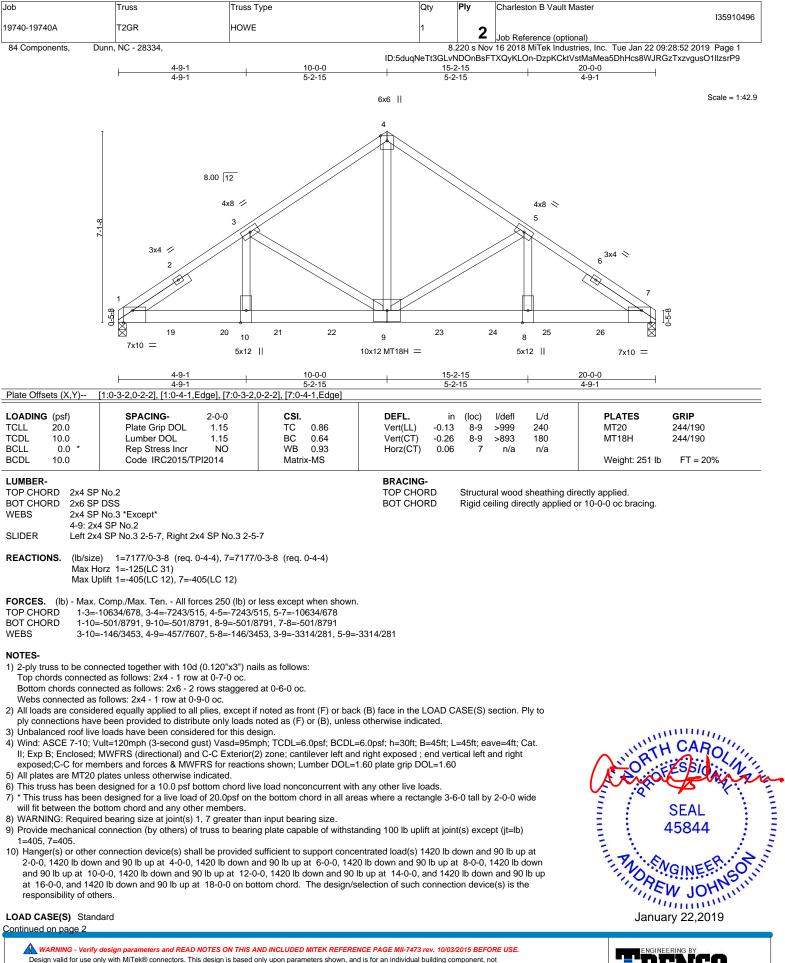
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





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818 Soundside Road Edenton, NC 27932

[Job	Truss	Truss Type	Qty	Ply	Charleston B Vault Master
						135910496
	19740-19740A	T2GR	HOWE	1	2	
					2	Job Reference (optional)
	84 Components, Dunn,	NC - 28334,		8.2	220 s Nov	16 2018 MiTek Industries, Inc. Tue Jan 22 09:28:52 2019 Page 2

ID:5duqNeTt3GLvNDOnBsFTXQyKLOn-DzpKCktVstMaMea5DhHcs8WJRGzTxzvgusO1llzsrP9

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

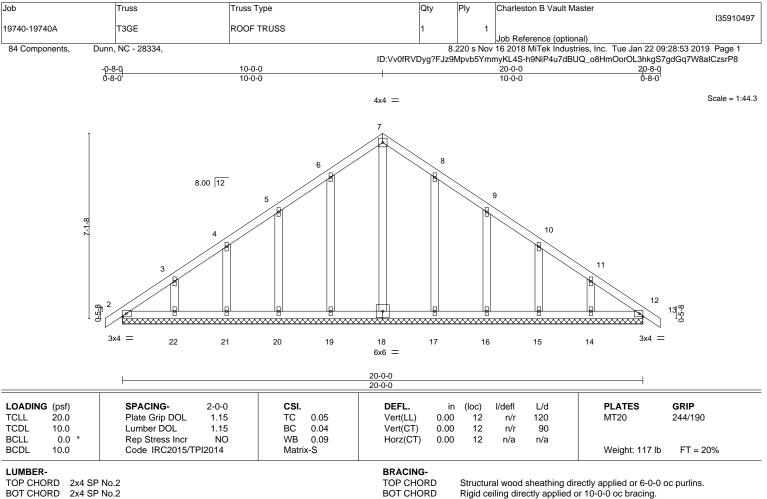
Uniform Loads (plf) Vert: 11-15=-20, 1-4=-60, 4-7=-60

Concentrated Loads (lb)

Vert: 9=-1420(F) 19=-1420(F) 20=-1420(F) 21=-1420(F) 22=-1420(F) 23=-1420(F) 24=-1420(F) 25=-1420(F) 26=-1420(F)

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2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS. All bearings 20-0-0.

Max Horz 2=-135(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14

Max Grav All reactions 250 lb or less at joint(s) 2, 18, 19, 20, 21, 22, 17, 16, 15, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



