

Client: Weaver Homes Project: Magnolia Elev. C Address: Magnolia Elev. C

Date: 1/25/2021 Input by: Christine Shivy Job Name: Magnolia Elev. C

Project #:

3-Ply - PASSED **Kerto-S LVL** 1.750" X 16.000" BM₁

Level: Level

Reactions UNPATTERNED Ib (Uplift)

Dead

3454

82%

Live

1932

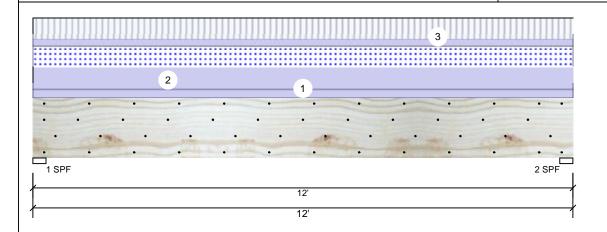
Bearing Length

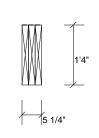
1 - SPF 3.500"

2 - SPF 3.500"

Brg

1





Const

0

Ld. Comb.

D+0.75(L+S) D+0.75(L+S)

Page 1 of 1

Member Information								
Type:	Girder							
Plies:	3							
Moisture Condition:	Dry							
Deflection LL:	480							
Deflection TL:	360							
Importance:	Normal							
Temperature:	Temp <= 100°F							

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: Yes Deck: Not Checked

0 2 1932 3454 1950 0 **Bearings**

3454 / 2912

3454 / 2912

Cap. React D/L lb

Snow

1950

Wind

Total Ld. Case

6366 L

6366 L

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	17729 ft-lb	6'	62010 ft-lb	0.286 (29%)	D+0.75(L+S)	L
Unbraced	17729 ft-lb	6'	17732 ft-lb	1.000 (100%)	D+0.75(L+S)	L
Shear	4565 lb	1'6 5/8"	17920 lb	0.255 (25%)	D+L	L
LL Defl inch	0.066 (L/2116)	6'	0.289 (L/480)	0.230 (23%)	0.75(L+S)	L
TL Defl inch	0.143 (L/968)	6'	0.385 (L/360)	0.370 (37%)	D+0.75(L+S)	L

Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 10'4 1/8" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Wall
2	Uniform			Тор	325 PLF	0 PLF	325 PLF	0 PLF	0 PLF	A2
3	Uniform			Far Face	107 PLF	322 PLF	0 PLF	0 PLF	0 PLF	F1
	Self Weight				19 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 1/8/2023 CSD |



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Project #:

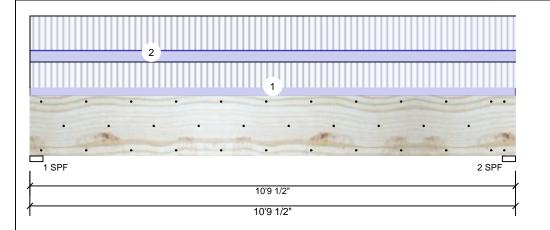
2 - SPF 3.500"

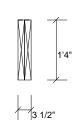
88%

1200 / 3389

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED BM₂

Level: Level





Const

0

0

Ld. Comb. D+L

D+I

0

0

4589 I

Page 1 of 1

Member Information Reactions UNPATTERNED Ib (Uplift) Brg Туре: Girder Application: Floor Snow Wind Live Dead Plies: 2 Design Method: ASD 3389 1200 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 3389 1200 0 2 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case 1 - SPF 3.500" 1200 / 3389 4589 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	11397 ft-lb	5'4 3/4"	34565 ft-lb	0.330 (33%)	D+L	L
Unbraced	11397 ft-lb	5'4 3/4"	11746 ft-lb	0.970 (97%)	D+L	L
Shear	4386 lb	1'6 5/8"	11947 lb	0.367 (37%)	D+L	L
LL Defl inch	0.085 (L/1457)	5'4 3/4"	0.259 (L/480)	0.330 (33%)	L	L
TL Defl inch	0.115 (L/1076)	5'4 3/4"	0.345 (L/360)	0.330 (33%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6. Lateral slenderness ratio based on single ply width

U Lateral Sie	enderness ratio based on	single ply widin.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Far Face	89 PLF	267 PLF	0 PLF	0 PLF	0 PLF	F4	
2	Uniform			Near Face	121 PLF	361 PLF	0 PLF	0 PLF	0 PLF	F2	
	Self Weight				12 PLF						

Notes

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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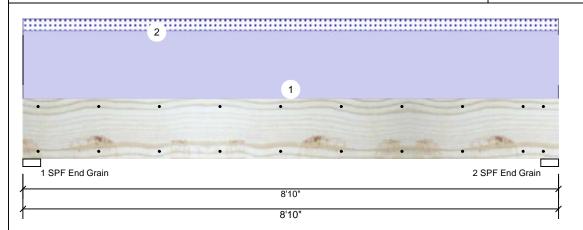
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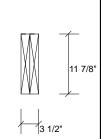
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Project #:

2-Ply - PASSED **Kerto-S LVL** 1.750" X 11.875" **GDH**

Level: Level





Const

0

0

Wind

0

0

Page 1 of 1

Member Information Reactions UNPATTERNED Ib (Uplift) Туре: Girder Application: Floor Brg Live Dead Snow Plies: 2 Design Method: ASD 0 1101 177 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 0 1101 177 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F **Bearings** Analysis Results Analysis Actual Location Allowed

0.236 (24%) D+S

0.100 (10%) D

	Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
	1 - SPF 3.500" End Grain	12% 1101 / 177	1277 L	D+S
cation Allowed Capacity Comb. Case 4'5" 17919 ft-lb 0.122 (12%) D Uniform	2 - SPF 3.500" End Grain	12% 1101 / 177	1277 L	D+S

TL Defl inch **Design Notes**

LL Defl inch

Moment

Shear

Unbraced

1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".

10756 ft-lb

4'5 1/16" 0.209 (L/480) 0.030 (3%) S

4'5 1/16" 0.279 (L/360) 0.140 (14%) D+S

2 Refer to last page of calculations for fasteners required for specified loads.

4'5"

7'7 3/8" 7980 lb

- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.

2185 ft-lb

2536 ft-lb

(L/18257)

0.040 (L/2525)

797 lb

0.006

- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Loads
2	Uniform			Тор	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	2'-0" Gable End
	Self Weight				9 PLF					

Uniform

Notes

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