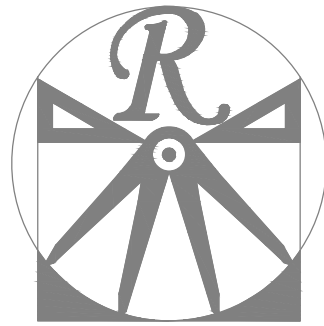


GENERAL NOTES

1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
3. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED.
4. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL SITE REQUIREMENTS.

**LOT 15R WEST PARK
58 WEST PARK LANE
SANFORD, NC**

**PLANS DESIGNED TO THE
2018 NORTH CAROLINA STATE
RESIDENTIAL BUILDING CODE.**



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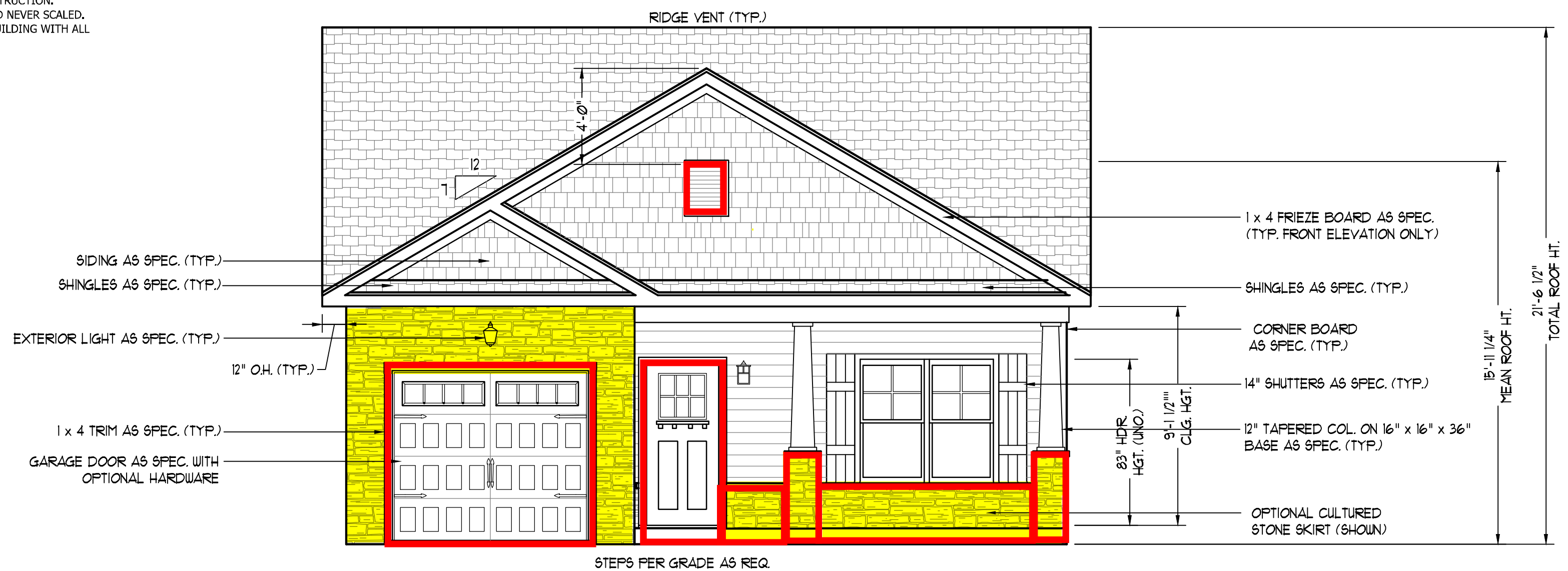
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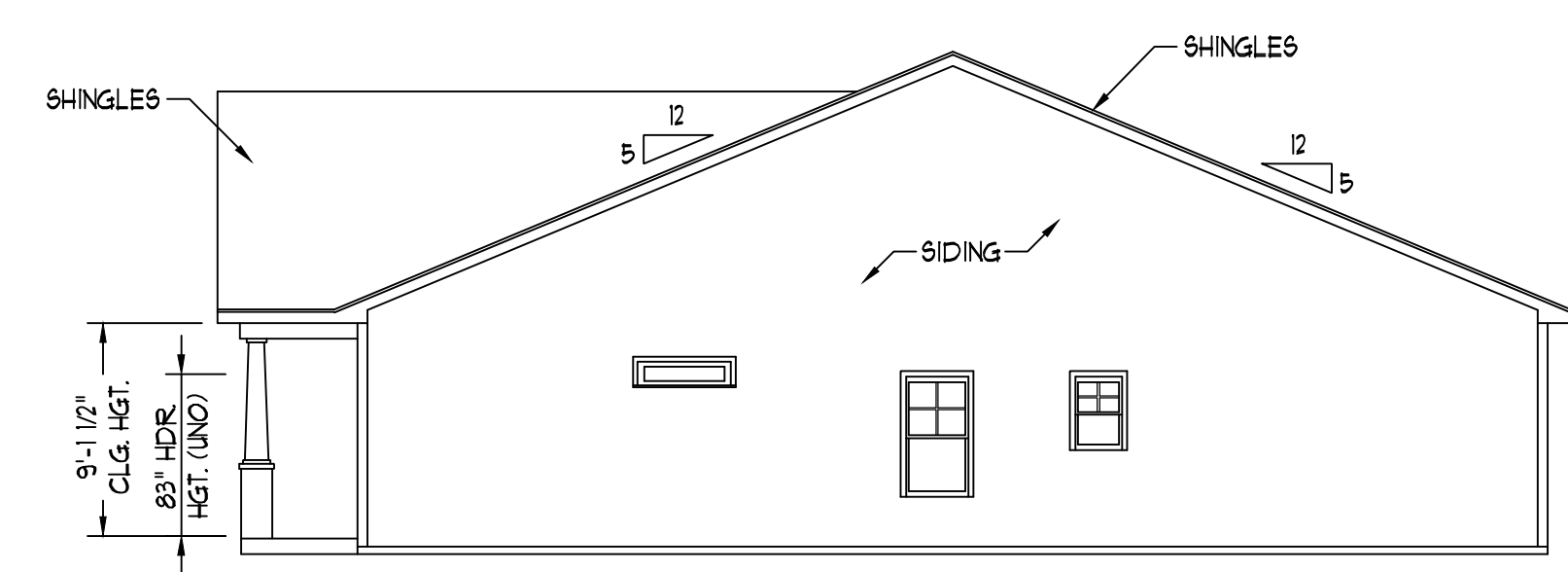
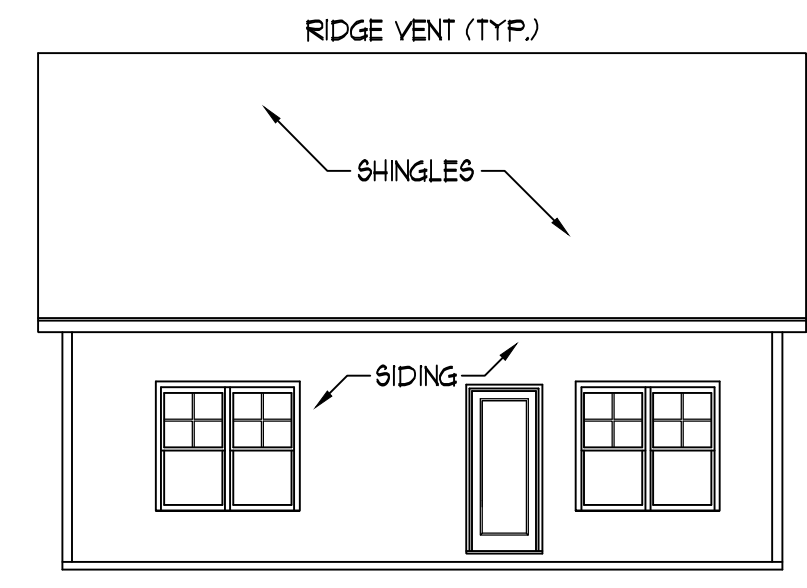
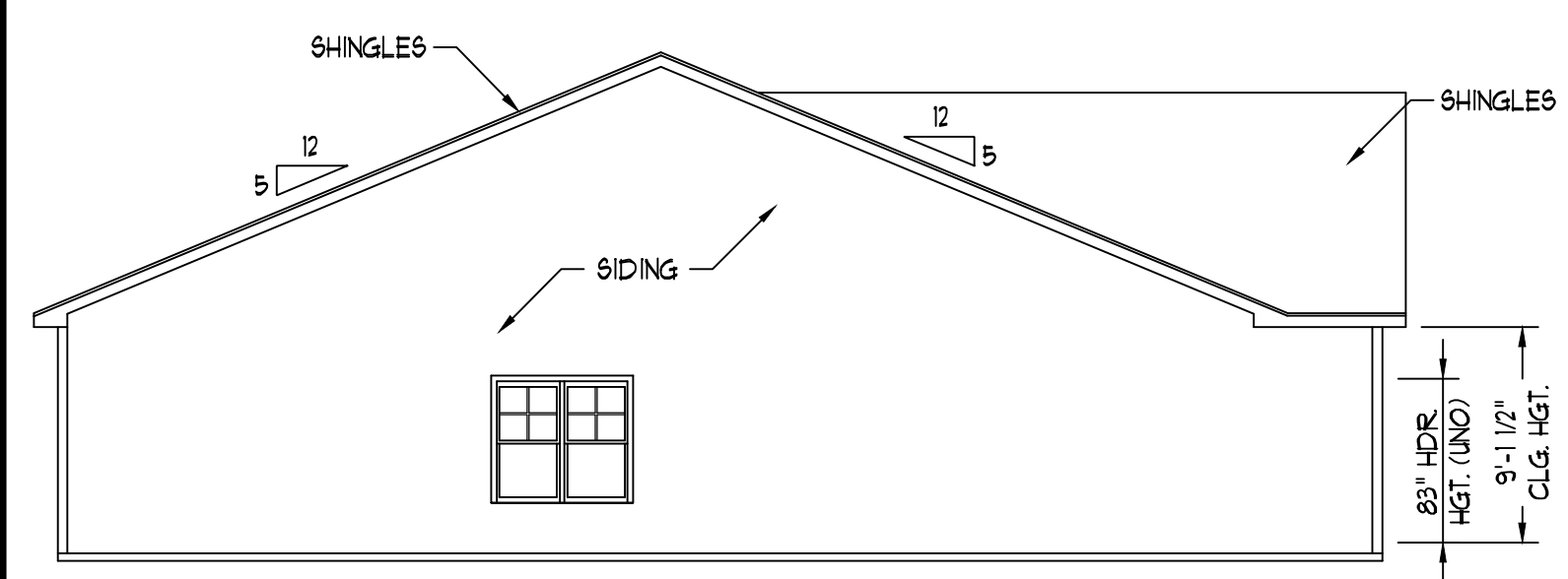
PRICE PROMOTIONS INCENTIVES FEATURES OPTIONS FLOOR PLANS ELEVATIONS DESIGNS MATERIALS AND DIMENSIONS ARE SUBJECT TO CHANGE WITHOUT NOTICE. SQUARE FOOTAGE AND DIMENSIONS ARE ESTIMATED AND HOUSE ON LOT WILL BE DETERMINED BY THE SITE PLAN AND PLOT PLAN. FLOOR PLANS AND ELEVATION RENDERINGS ARE ARTIST CONCEPTIONS. FLOOR PLANS ARE THE COPYRIGHTED PROPERTY OF WEAVER HOMES. ANY REPRODUCTION OR MODIFICATION OF THESE PLANS WITHOUT THE WRITTEN PERMISSION OF WEAVER HOMES IS PROHIBITED. SEE NEW HOME SALES CONSULTANT FOR CURRENT DETAILS. COPYRIGHT © 2020 WEAVER HOMES



NOTICE TO CONTRACTOR
All construction must comply with current NC Building Codes and is subject to field inspection and verification.

APPROVED
Limited building only review
Permit holder responsible for full compliance with the code

08/04/2021



**PLUMBING: DOUBLE J
HVAC: MAINSTREAM
ELECTRICAL: PIONEER**

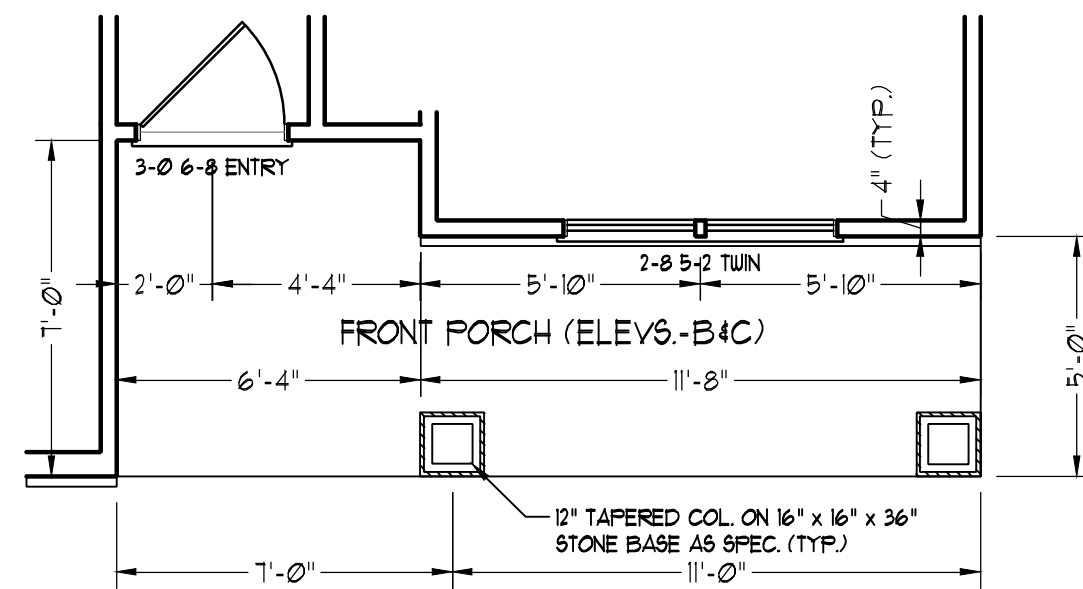
SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.
11x17 PRINTS ARE NOT TO SCALE

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CAROLINA COLLECTION
LEYLAND DRIVE LEFT

DATE: JUNE 15, 2020
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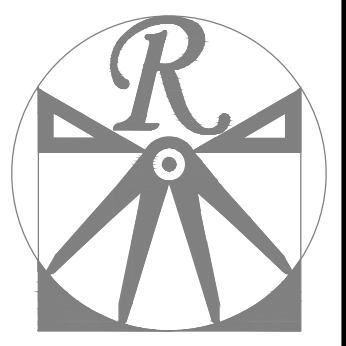
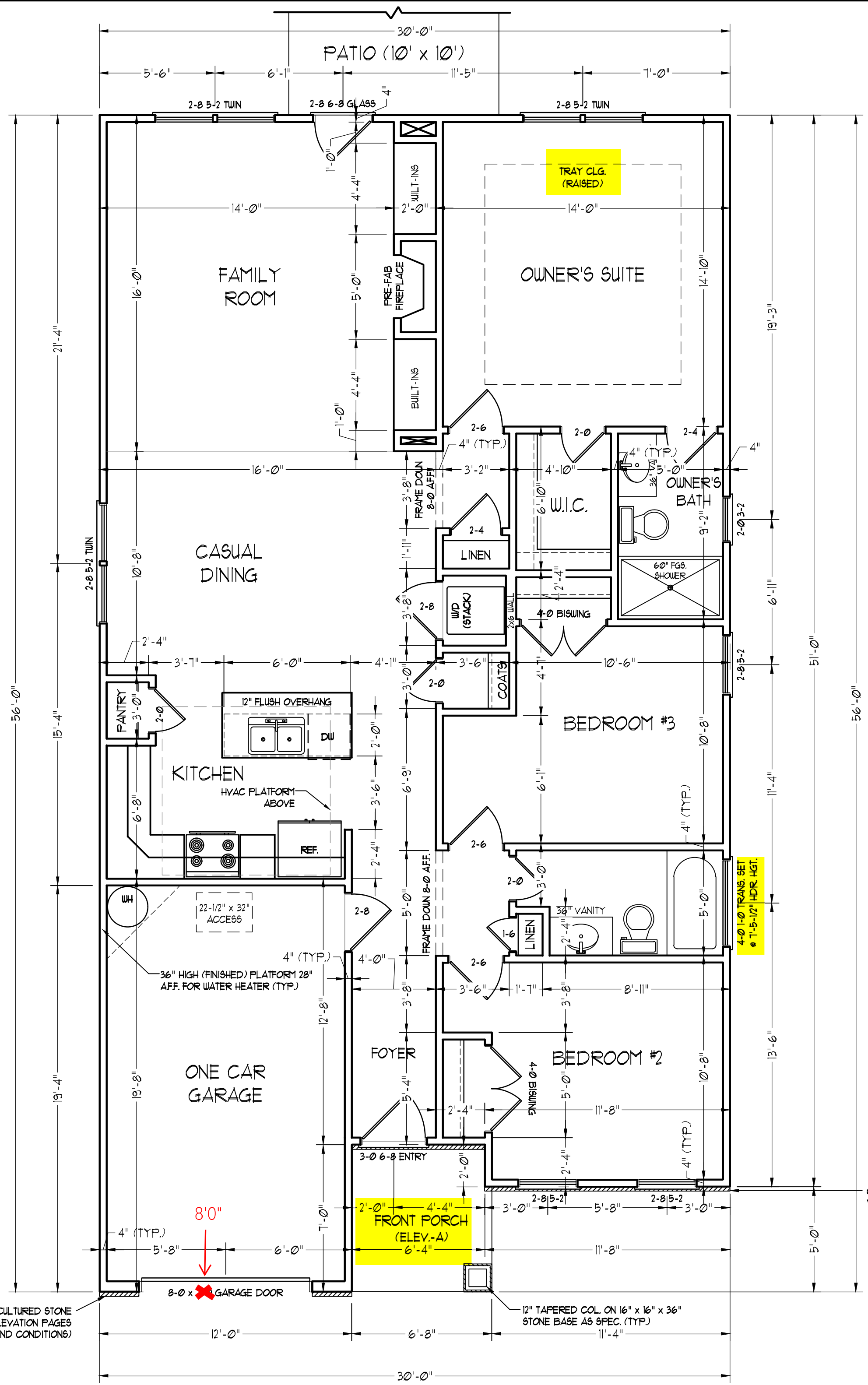
C - ELEVATIONS
A-3

SQUARE FOOTAGE (I.F.S.)	
1st FLOOR:	1295 SQ. FT.
TOTAL:	1295 SQ. FT.
GARAGE:	215 SQ. FT.
FRONT PORCH (ELEV.-A):	46 SQ. FT.
FRONT PORCH (ELEV.-B4C):	102 SQ. FT.
REAR PATIO:	100 SQ. FT.



~~FRONT PORCH AND BEDROOM #2 WINDOW ELEVATIONS B4C~~

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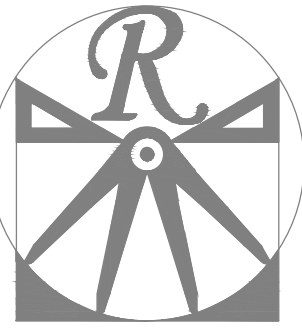
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FIRST FLOOR PLAN
A-4

OPTIONAL CULTURED STONE VENEER (SEE ELEVATION PAGES FOR LOCATIONS AND CONDITIONS)



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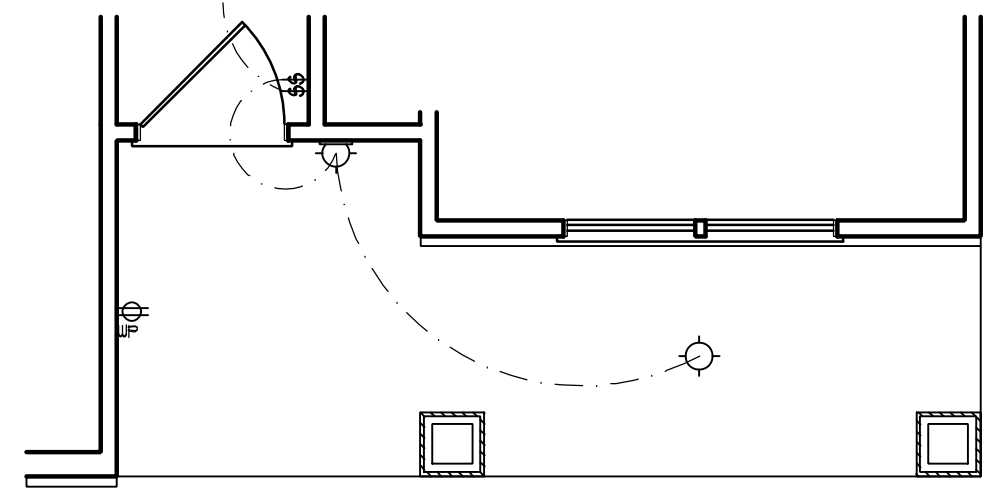
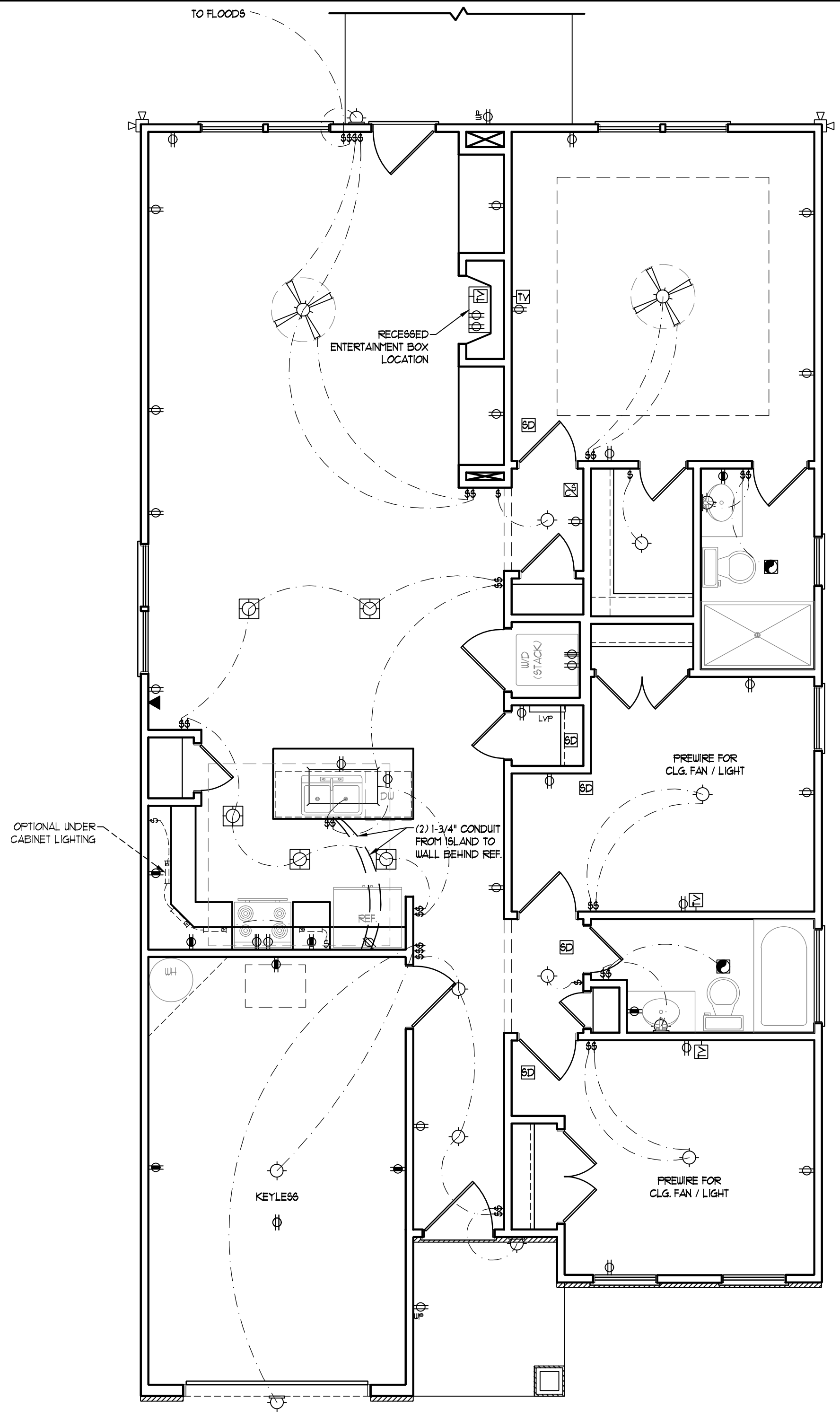
FIRST FLOOR ELECTRICAL PLAN
E-1

ELECTRICAL LAYOUT NOTES:

- 1.) BLOCK AND WIRE FOR ALL CEILING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET @ 90" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- ⊕ 110 V OUTLET
- ⊕GFI 110 V GFI OUTLET
- ⊕S 110 V SWITCHED OUTLET
- ⊕B 110 V BASEBOARD OUTLET
- ⊕F 4-FLEX
- ⊕C/F 110 V COUNTER OR FLOOR MOUNTED
- ⊕C/F 220 V COUNTER OR FLOOR MOUNTED 110V GFI
- ⊕W WEATHERPROOF
- ⊕220 220 V OUTLET
- ⊕DC 110 V DEDICATED CIRCUIT
- ⊕DC 220 V DEDICATED CIRCUIT
- ⊕SP 110 V SPECIAL PURPOSE (240 V, ETC.)
- ⊕W 110 V WALL MOUNT LIGHT
- ⊕C 110 V CEILING MOUNT LIGHT
- ⊕P 110 V PENDANT LIGHT
- ⊕R 110 V RECESSED CAN LIGHT
- ⊕M 110 V MINI CAN LIGHT
- ⊕E 110 V EYEBALL LIGHT
- ⊕FL 110 V FLUORESCENT LIGHT
- ⊕UC 110 V UNDERCABINET LIGHT
- ⊕F 110 V FLOOD LIGHT
- ⊕S 110 V SWITCH
- ⊕DS 110 V DIMMER SWITCH
- ⊕T 110 V TELEPHONE
- ⊕D 110 V DATA
- ⊕TD 110 V TELEPHONE AND DATA
- ⊕TV 110 V TV CONNECTION
- ⊕TD 110 V TV/ DATA
- ⊕C 110 V CONDUIT FOR COMPONENT WIRING
- ⊕S 110 V SPEAKER
- ⊕SD 110 V SMOKE/ CH DETECTOR
- ⊕SD 110 V SMOKE DETECTOR
- ⊕E 110 V EXHAUST FAN
- ⊕LVP 110 V LOW VOLTAGE PANEL
- ⊕AL 110 V ALARM PANEL
- ⊕CF 110 V CEILING FAN
- ⊕CF/L 110 V CEILING FAN W/ LIGHT

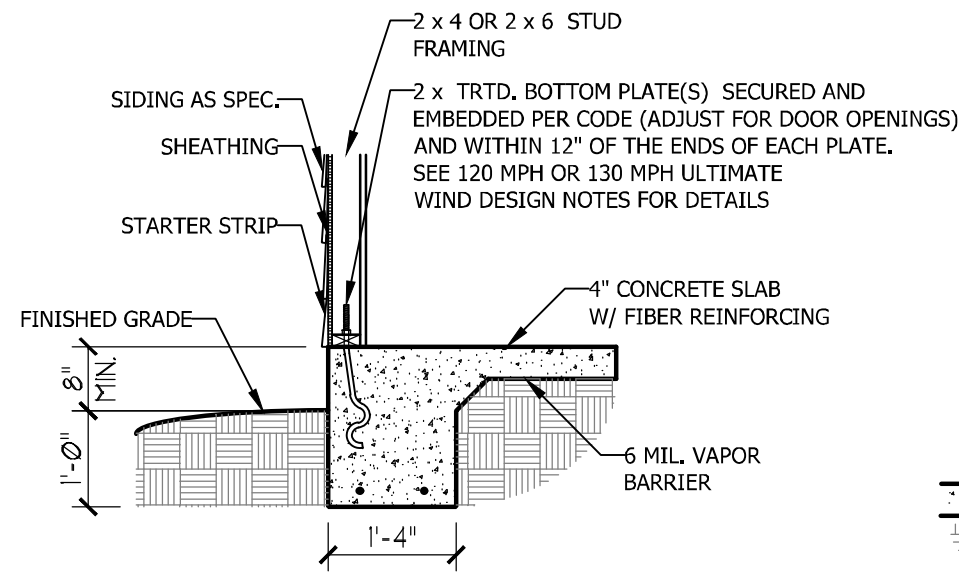


SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.
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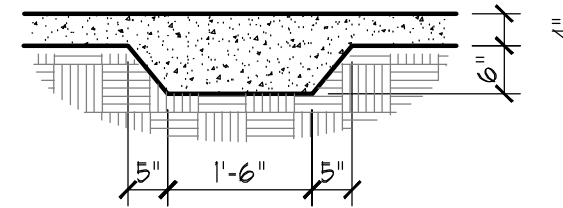
~~FRONT PORCH AND BEDROOM #2 WINDOW ELEVATIONS-B+C~~

ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:

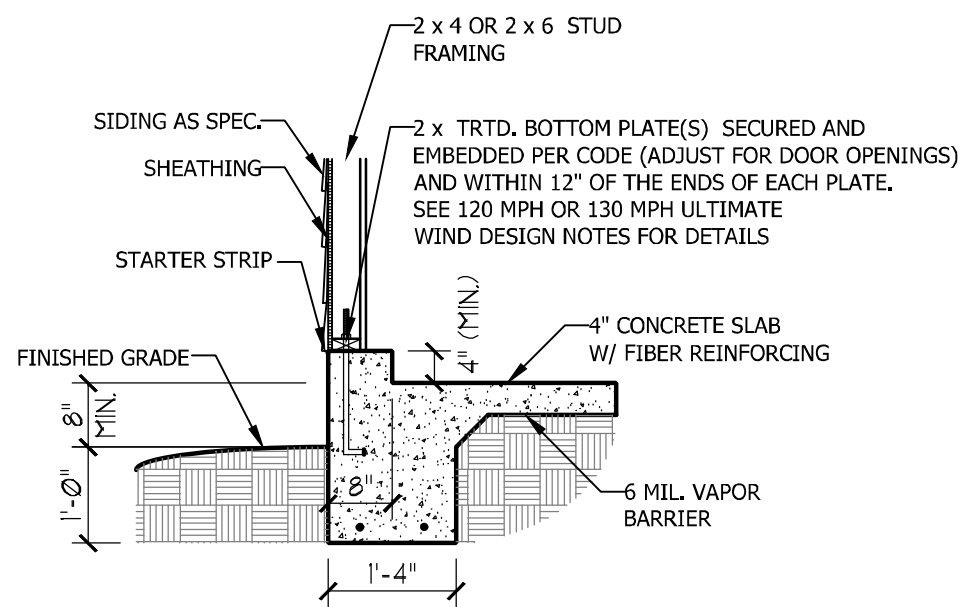
1. STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
2. FOR 120 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 6'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
3. FOR 130 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 4'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
4. MEAN ROOF HEIGHT IS LESS THAN 30 FEET.
5. EXTERIOR WALLS DESIGNED FOR 120 OR 130 MPH WINDS.
6. INSTALL 7/16" OSB SHEATHING ON ALL EXTERIOR WALLS OF ALL STORIES IN ACCORDANCE WITH SECTION R602.10.3 OF THE NRCR, 2018 EDITION.
7. ENERGY EFFICIENCY COMPLIANCE AND INSULATION VALUES OF THE BUILDING TO BE IN ACCORDANCE WITH CHAPTER 11 OF THE NRCR, 2018 EDITION.



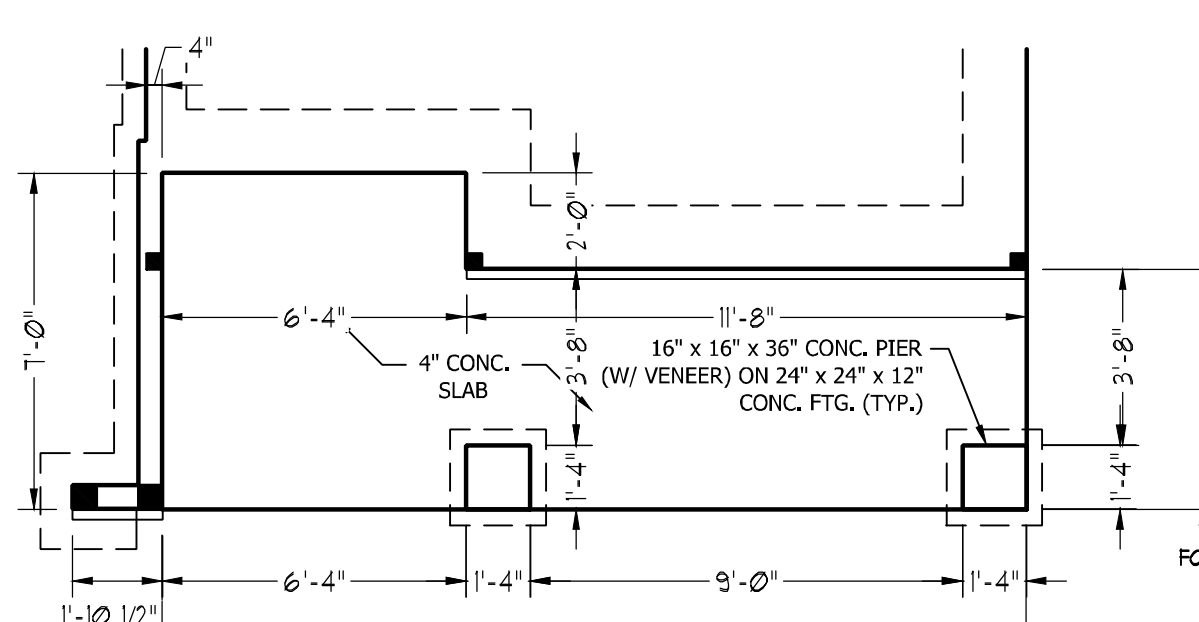
TYPICAL SLAB DETAIL



THICKENED SLAB DETAIL

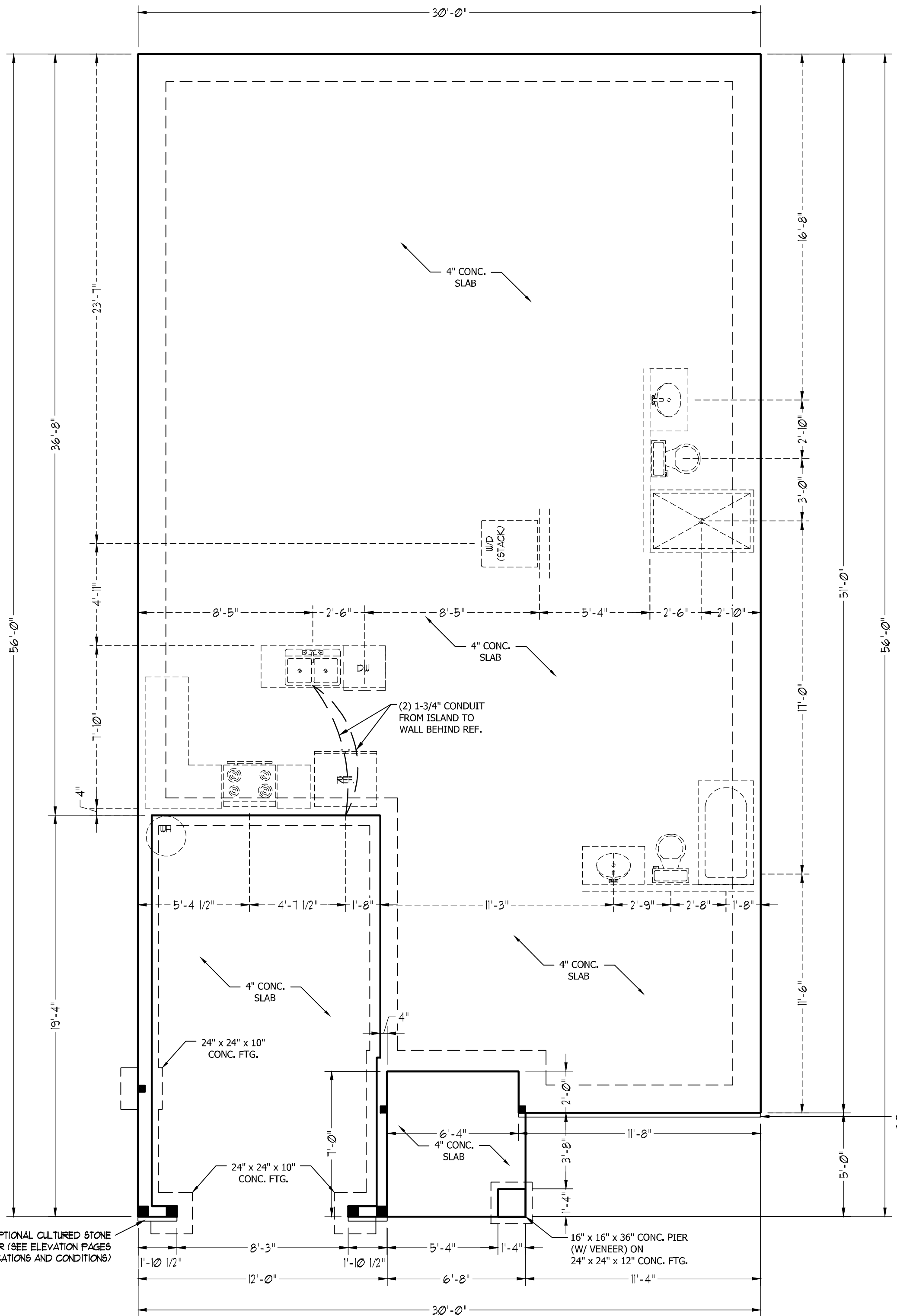


GARAGE CURB DETAIL

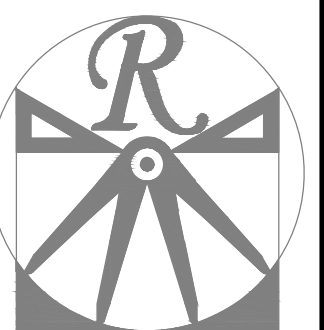


SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.
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~~FRONT PORCH ELEVATIONS B+C~~



OPTIONAL CULTURED STONE VENEER (SEE ELEVATION PAGES FOR LOCATIONS AND CONDITIONS)



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REVIEWED BY:

MONO SLAB
FOUNDATION
PLAN

S-1

ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:

1. STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
2. FOR 120 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 6'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
3. FOR 130 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 4'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
4. MEAN ROOF HEIGHT IS LESS THAN 30 FEET.
5. EXTERIOR WALLS DESIGNED FOR 120 OR 130 MPH WINDS.
6. INSTALL 7/16" OSB SHEATHING ON ALL EXTERIOR WALLS OF ALL STORIES IN ACCORDANCE WITH SECTION R602.10.3 OF THE NRCR, 2018 EDITION.
7. ENERGY EFFICIENCY COMPLIANCE AND INSULATION VALUES OF THE BUILDING TO BE IN ACCORDANCE WITH CHAPTER 11 OF THE NRCR, 2018 EDITION.

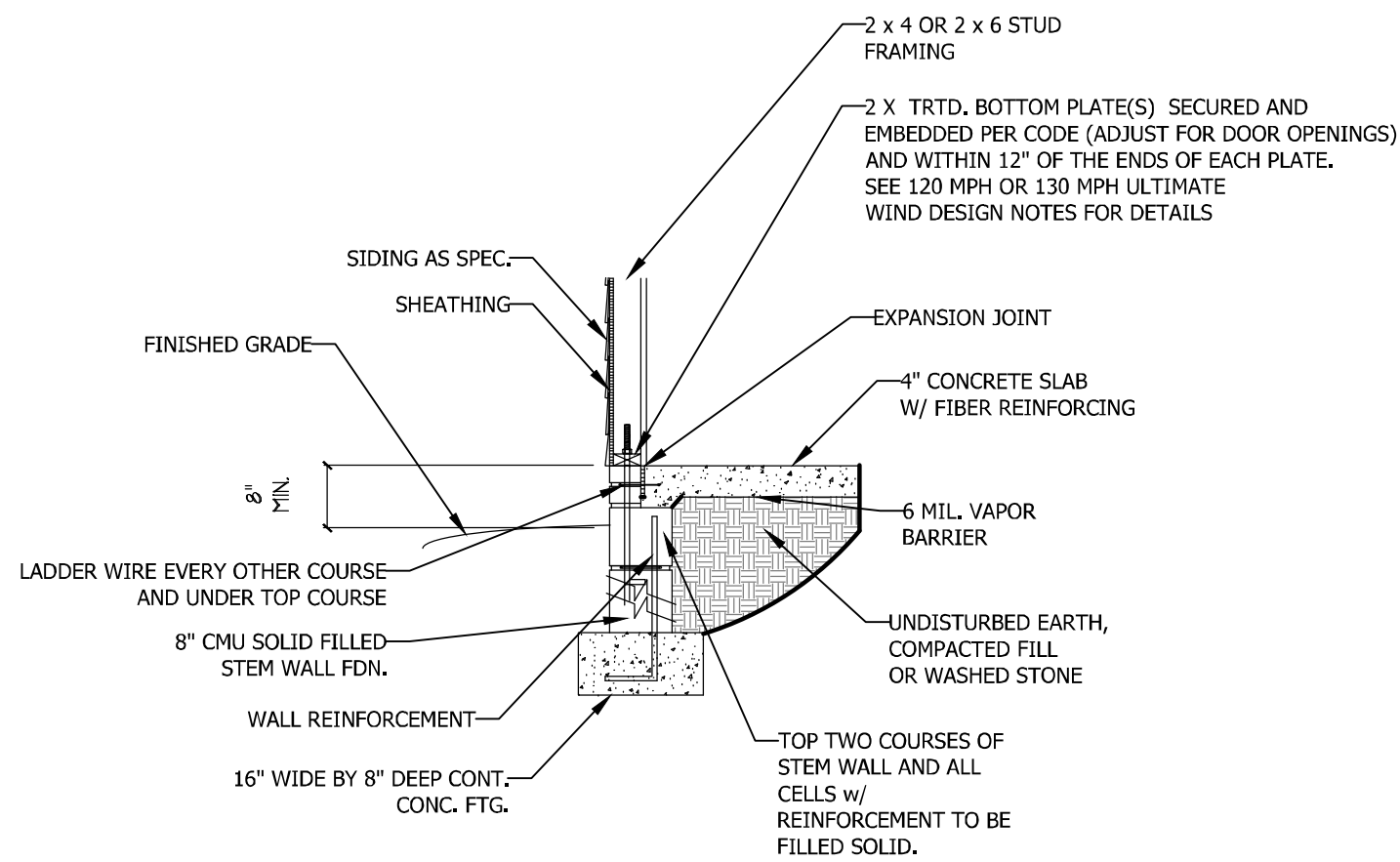
ANCHOR SPACING AND EMBEDMENT		
WIND ZONE	120 MPH	130 MPH
SPACING	6'-0" O.C.	4'-0" O.C.
EMBEDMENT	7"	7" INTO CONCRETE 15" INTO MASONRY

NOTE: HORIZONTAL FOOTING REBAR REQUIRED IN HIGH WIND ZONES ONLY (140-150 MPH)

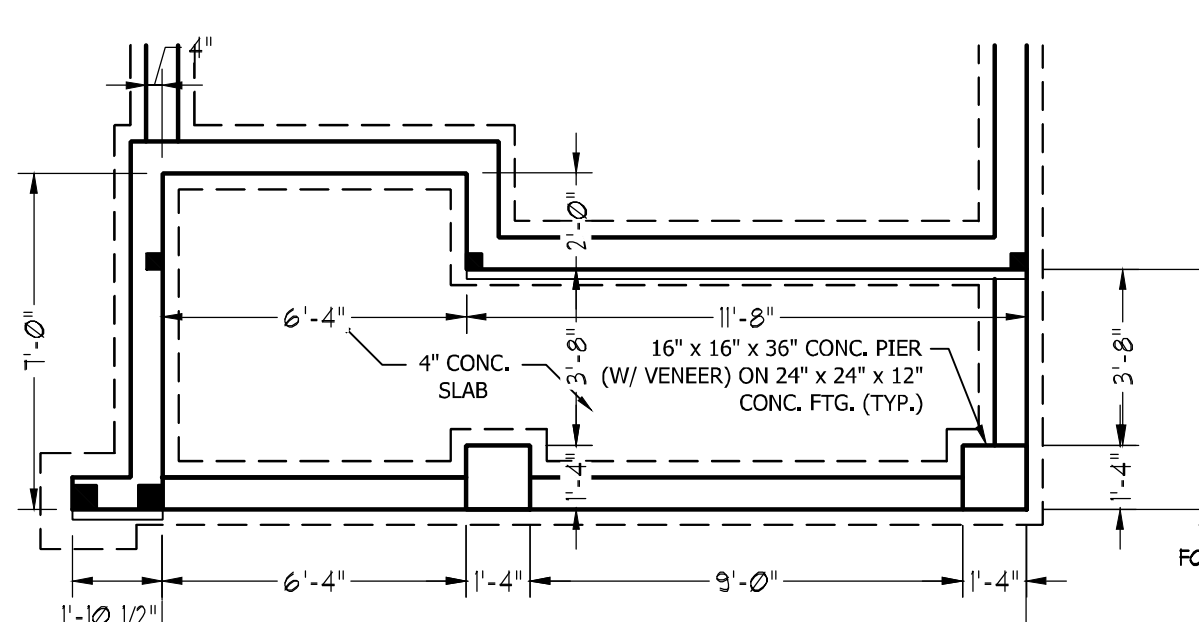
WALL HEIGHT (FEET)	MASONRY WALL TYPE			
	8" CMU	4" BRICK AND 4" CMU	4" BRICK AND 8" CMU	12" CMU
2' OR LESS	UNGROUTED	GROUT SOLID	UNGROUTED	UNGROUTED
3'	UNGROUTED	GROUT SOLID	UNGROUTED	UNGROUTED
4'	GROUT SOLID	GROUT SOLID w/ #4 REBAR @ 48" O.C.	GROUT SOLID	GROUT SOLID w/ #4 REBAR @ 64" O.C.
5'	GROUT SOLID w/ #4 REBAR @ 36" O.C.	N/A	GROUT SOLID w/ #4 REBAR @ 36" O.C.	GROUT SOLID w/ #4 REBAR @ 64" O.C.
6'	GROUT SOLID w/ #4 REBAR @ 24" O.C.	N/A	GROUT SOLID w/ #4 REBAR @ 24" O.C.	GROUT SOLID w/ #4 REBAR @ 64" O.C.
7' OR MORE	ENGINEERED BASED ON SITE CONDITIONS			

STRUCTURAL NOTES:

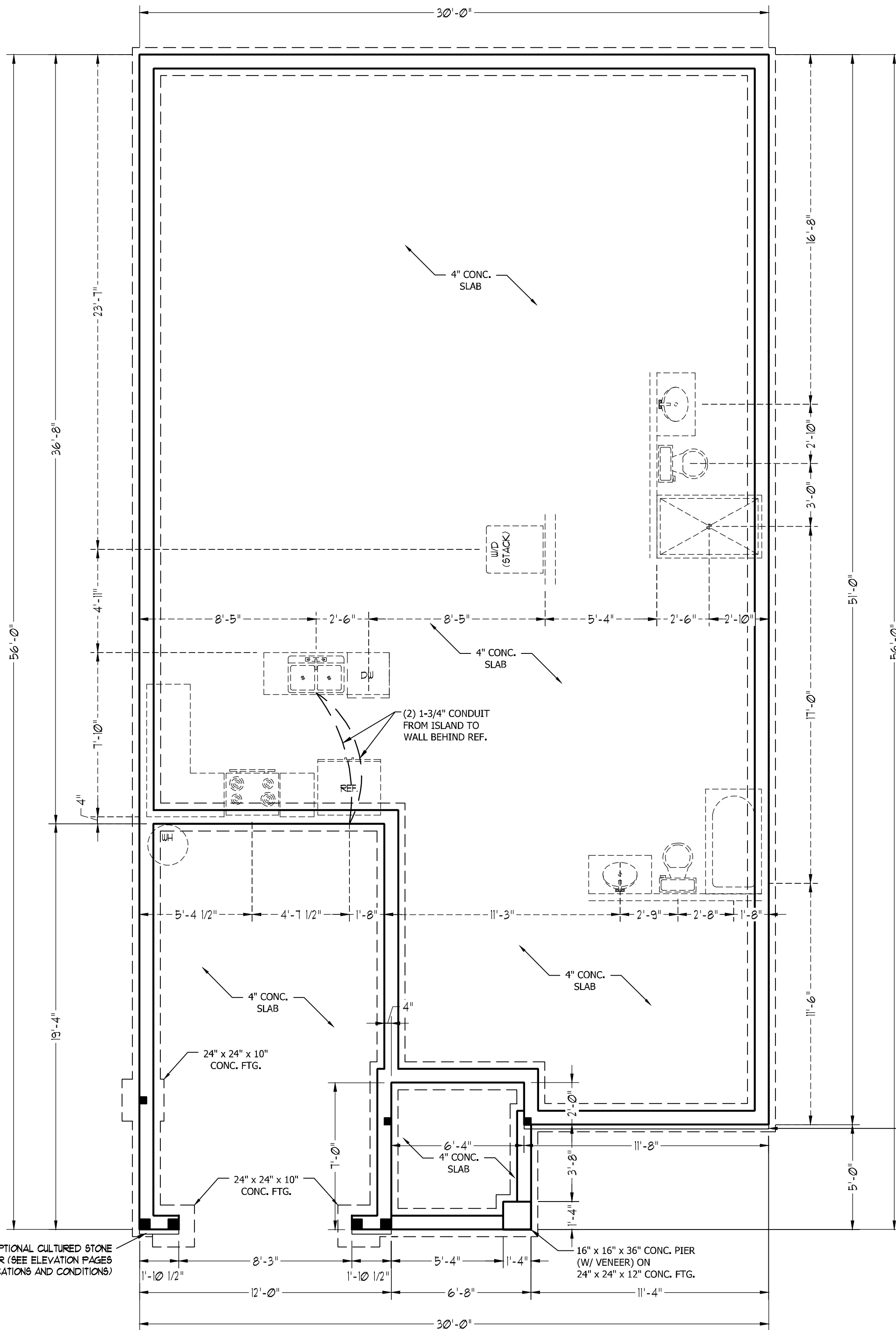
1. TABLE ABOVE APPLIES TO HOUSE FOUNDATION ONLY. TABLE DOES NOT APPLY TO GARAGE FOUNDATION NOT COMMON TO HOUSE.
2. TIE MULTIPLE WYTHES TOGETHER WITH LADDER WIRE @ 16" O.C. VERTICALLY.
3. WALL HGT. IS MEASURED FROM TOP OF FOOTING TO TOP OF WALL.
4. PREP SLAB PER R506.2.1 AND R505.2.2 BASE AND EXCEPTION OF THE 2018 NRCR
5. MINIMUM 24" LAP SPLICE LENGTH.
6. BACKFILL OF CLEAN #57/ #67 WASHED STONE IS PERMITTED.
7. BACKFILL OF WELL DRAINED SAND-GRAVEL MIXTURE SOILS (45 PSF/FT BELOW GRADE) CLASSIFIED AS GROUP 1 ACCORDING TO UNIFIED SOILS CLASSIFICATION SYSTEM IN ACCORDANCE WITH TABLE R405.1 OF THE 2018 NRCR ARE ALLOWABLE.
8. LOCATE REBAR IN CENTER OF FOUNDATION WALL.
9. WHERE REQUIRED, FILL BLOCK SOLID WITH TYPE "S" MORTAR OR 3000 PSI GROUT. USE OF "LOW LIFT GROUTING" METHOD REQUIRED WHEN FILLING WALLS WITH GROUT AT HEIGHTS OF 5' AND GREATER.



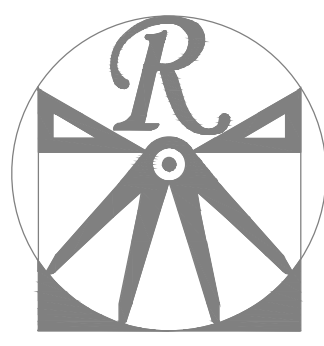
STEM WALL FDN. DETAIL



SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.
11x17 PRINTS ARE NOT TO SCALE



OPTIONAL CULTURED STONE VENEER (SEE ELEVATION PAGES FOR LOCATIONS AND CONDITIONS)



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ENGINEERED BY:
REVIEWED BY:

STEMWALL SLAB FOUNDATION PLAN

S-1

~~FRONT PORCH ELEVATIONS ETC~~

BRACE WALL PANEL NOTES:

EXTERIOR WALLS: ALL EXTERIOR WALLS TO BE SHEATHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THEIR ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

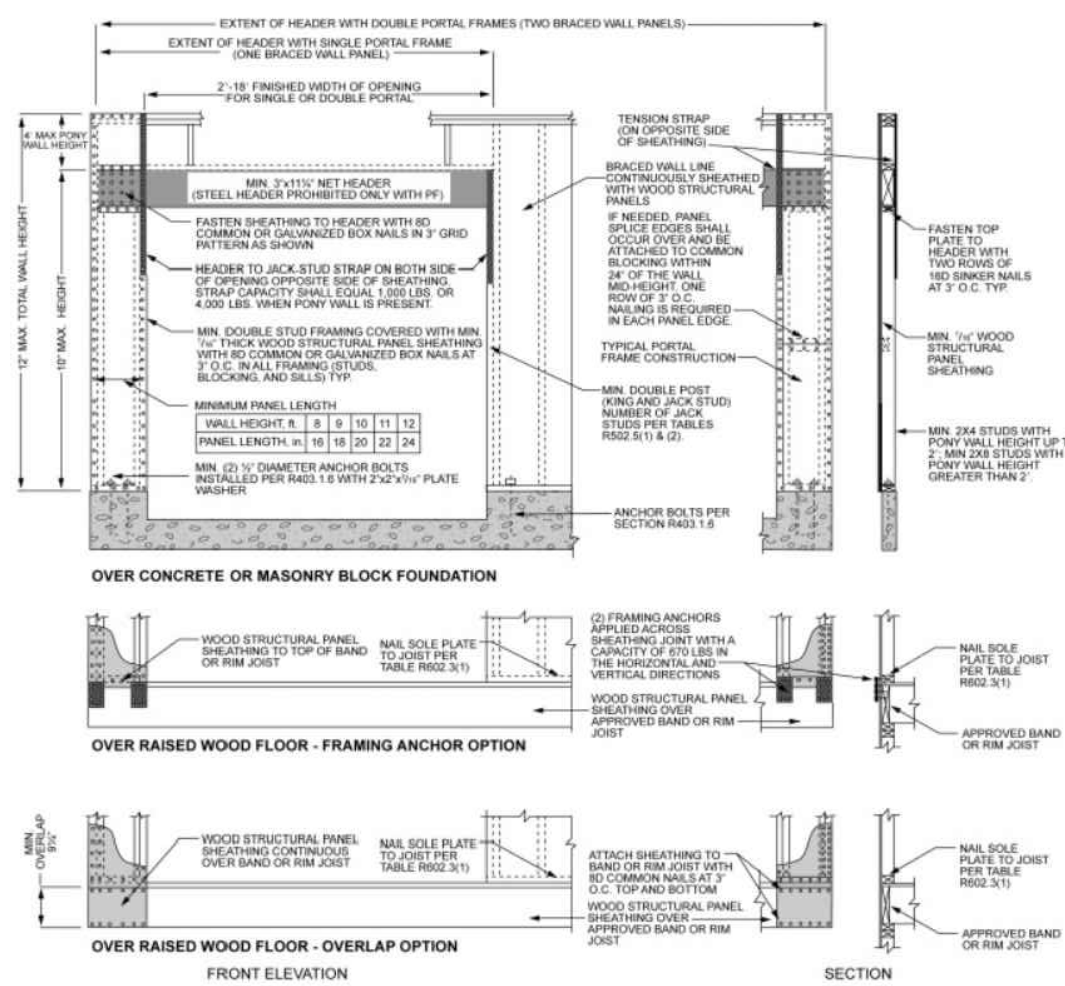
GYPSONUM: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

HD: 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1

STRUCTURAL NOTES:

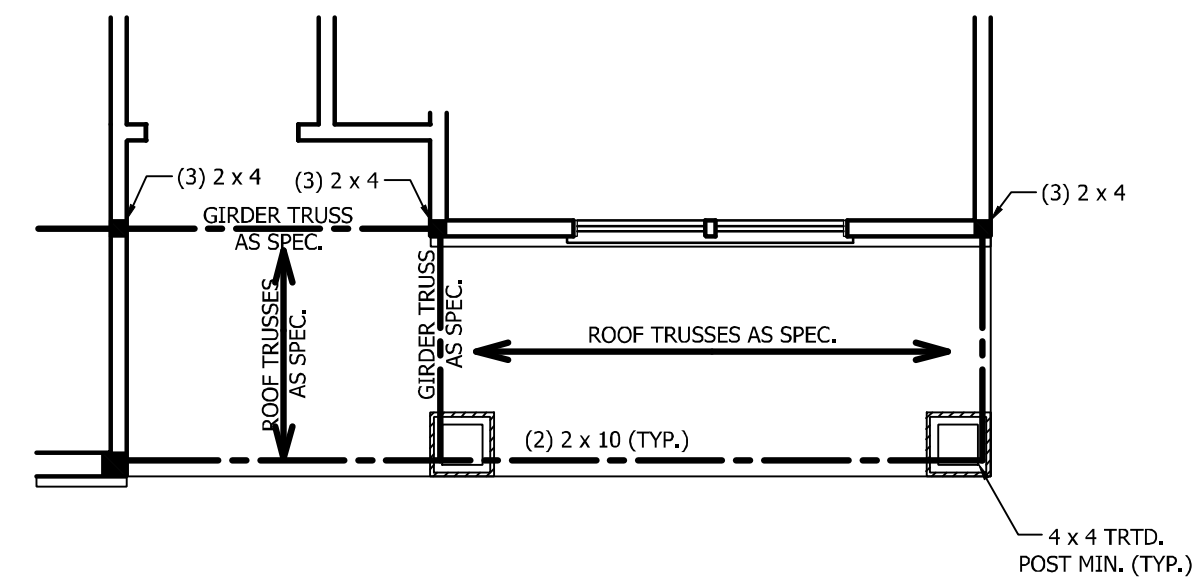
- ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO).
- ALL LOAD BEARING HEADERS TO BE (2) 2 x 4 (UNO).
- INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
- WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO).
- FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.



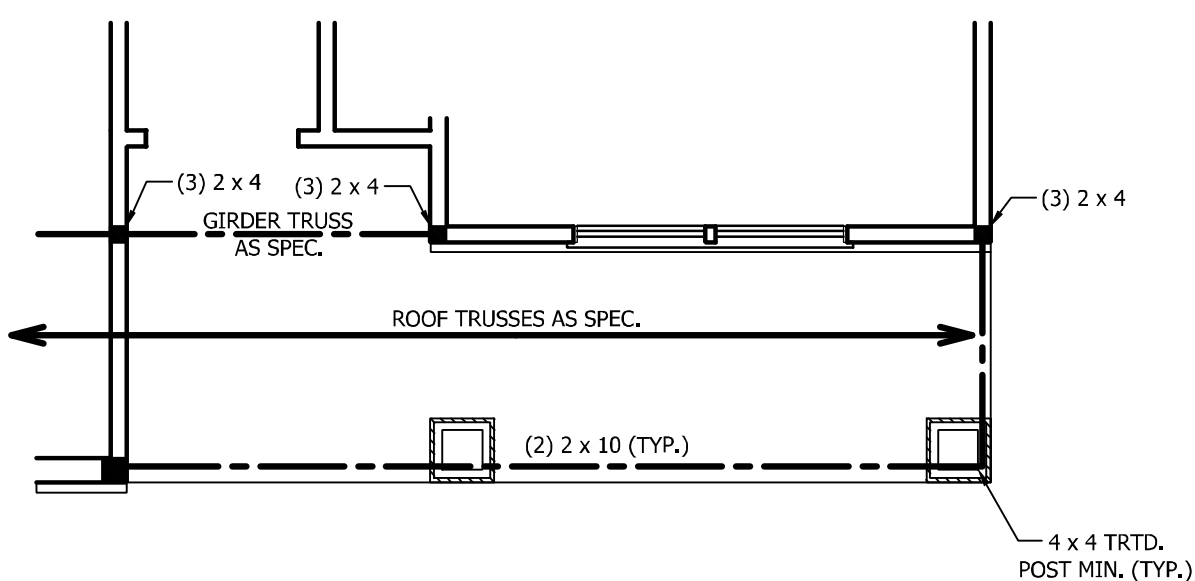
1 inch = 25.4 mm, 1 foot = 305 mm, 1 lb = 4.45 N.

FIGURE R602.10.1
METHOD PF—PORTAL FRAME CONSTRUCTION

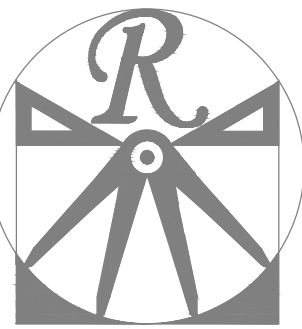
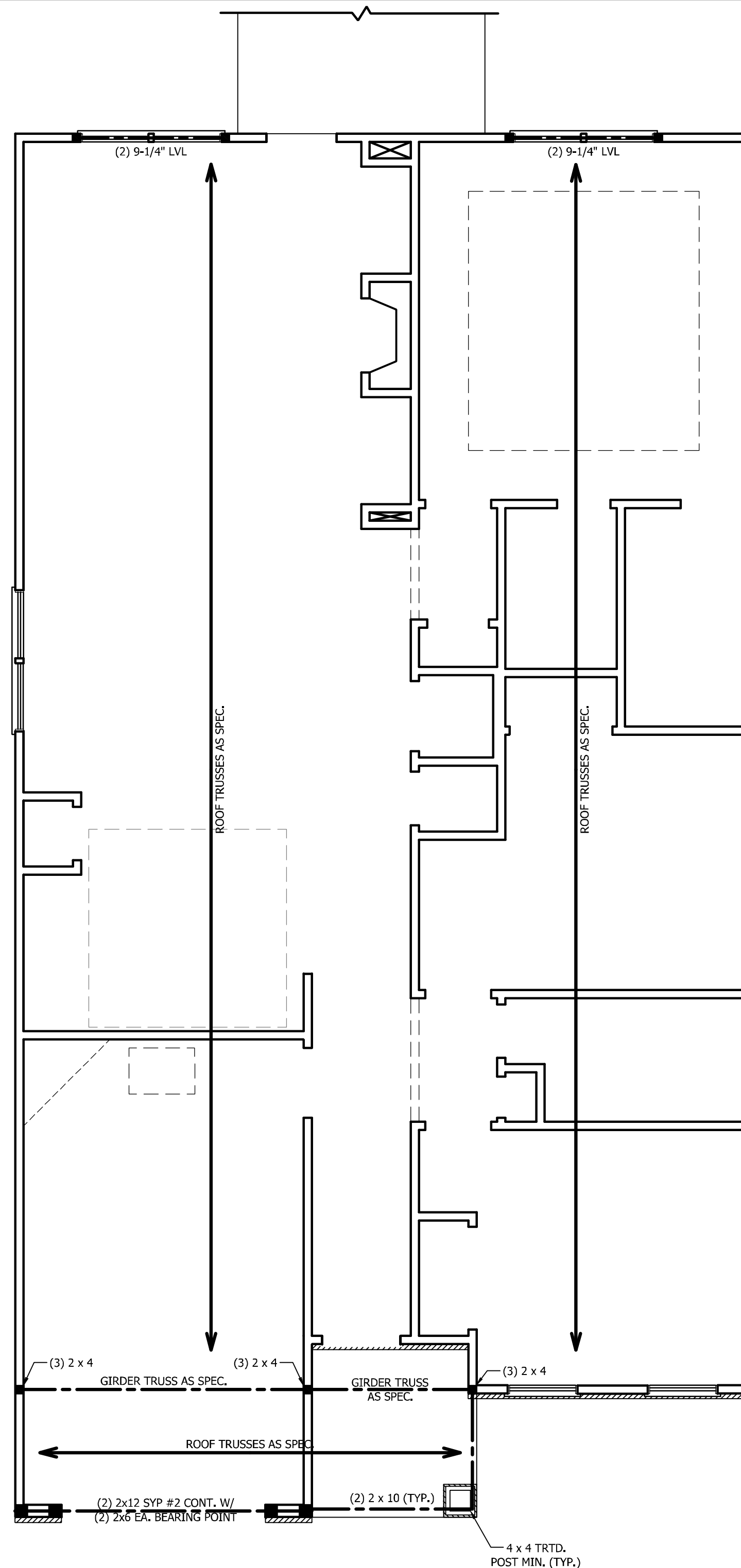
FRONT PORCH
AND BEDROOM #2
WINDOW
ELEVATION B



FRONT PORCH
AND BEDROOM #2
WINDOW
ELEVATION C



SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.
11x17 PRINTS ARE NOT TO SCALE



RENAISSANCE
RESIDENTIAL DESIGN, INC.

RALEIGH, NC 27612
(919) 649-4128
WWW.RDNCAROLINA.COM

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WEAVER HOMES
CAROLINA COLLECTION
LEYLAND DRIVE LEFT

DATE: JUNE 15, 2020
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

SECOND FLOOR
FRAMING PLAN

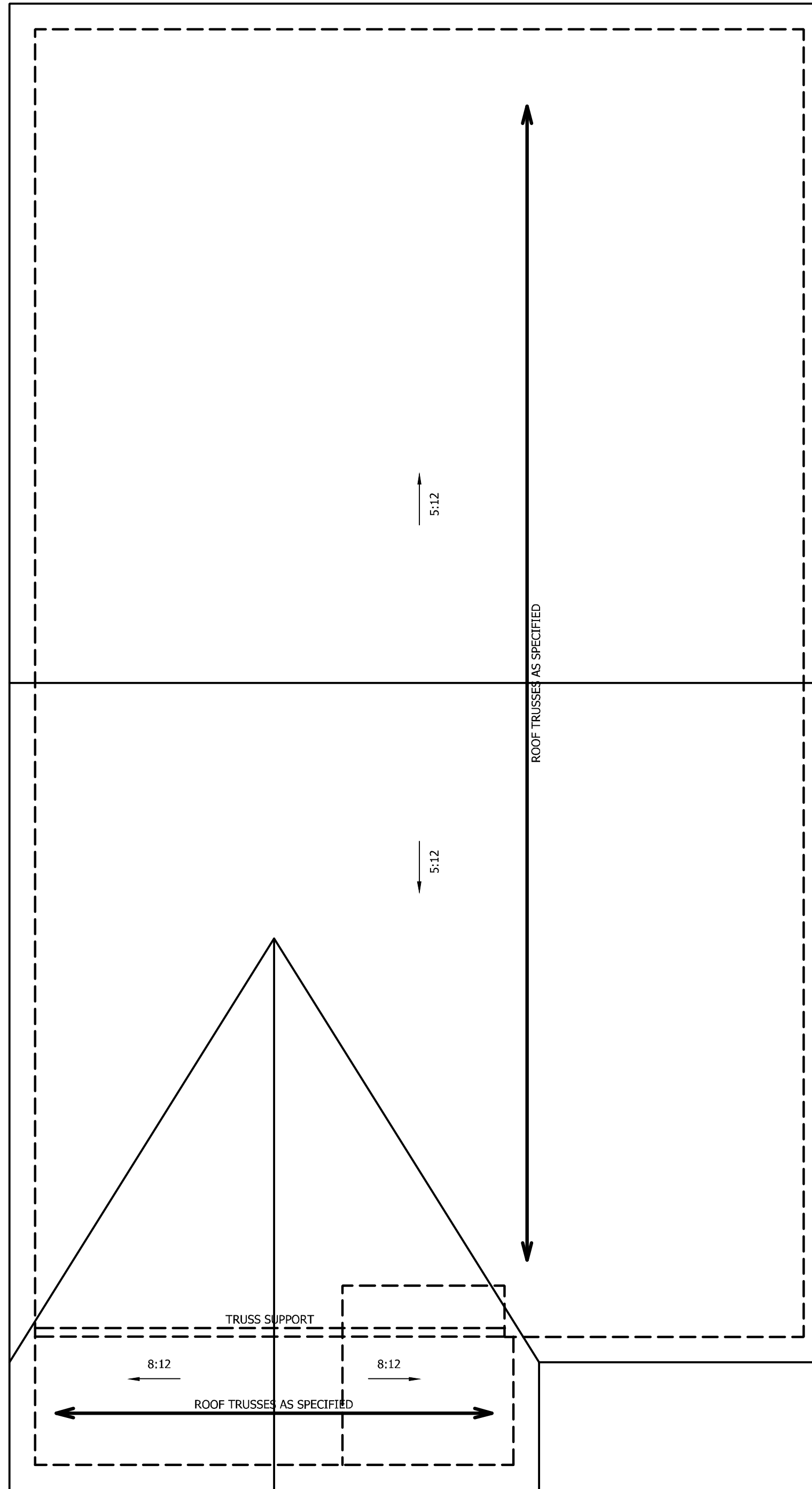
S-2

ATTIC VENT CALCULATION:

1756 SQ. FT. OF ATTIC DIVIDED BY
150 REQUIRES 11.7 SQ. FT. OF NET
FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

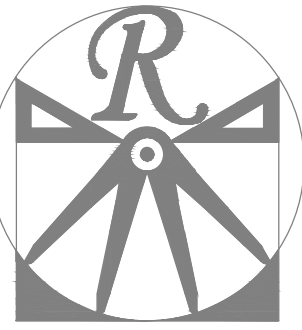
1. ALL FRAMING LUMBER TO BE #2 SPF (UNO).
2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
5. REFER TO SECTION R802.11 OF THE 2018 NCR FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



ELEVATION A

SCALE NOTE: 18x24 PRINTS ARE
TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE



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WEAVER HOMES
CAROLINA COLLECTION
LEYLAND DRIVE LEFT

DATE: JUNE 15, 2020

REV.:

SCALE: 1/4" = 1'-0"

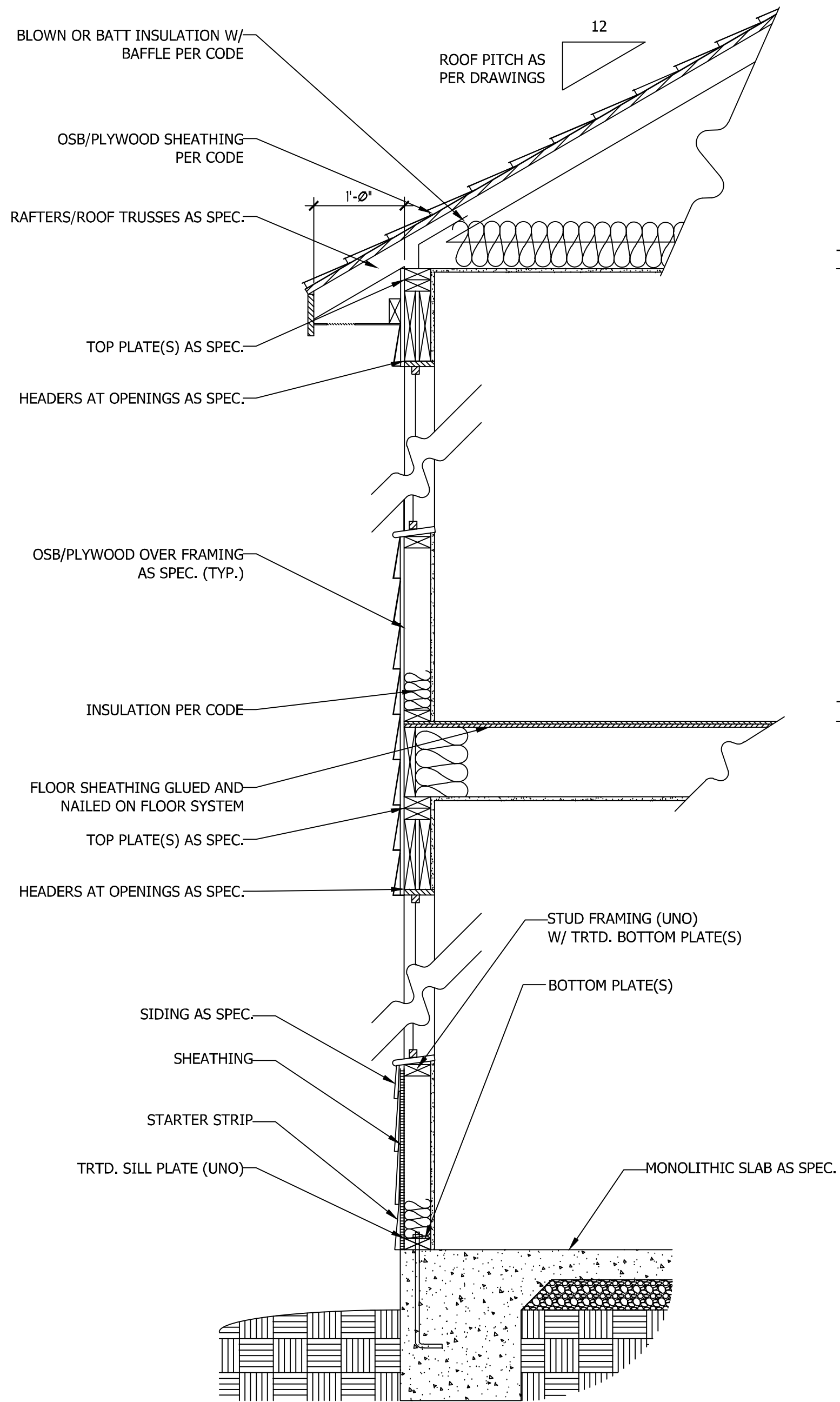
DRAWN BY: WG

ENGINEERED BY:

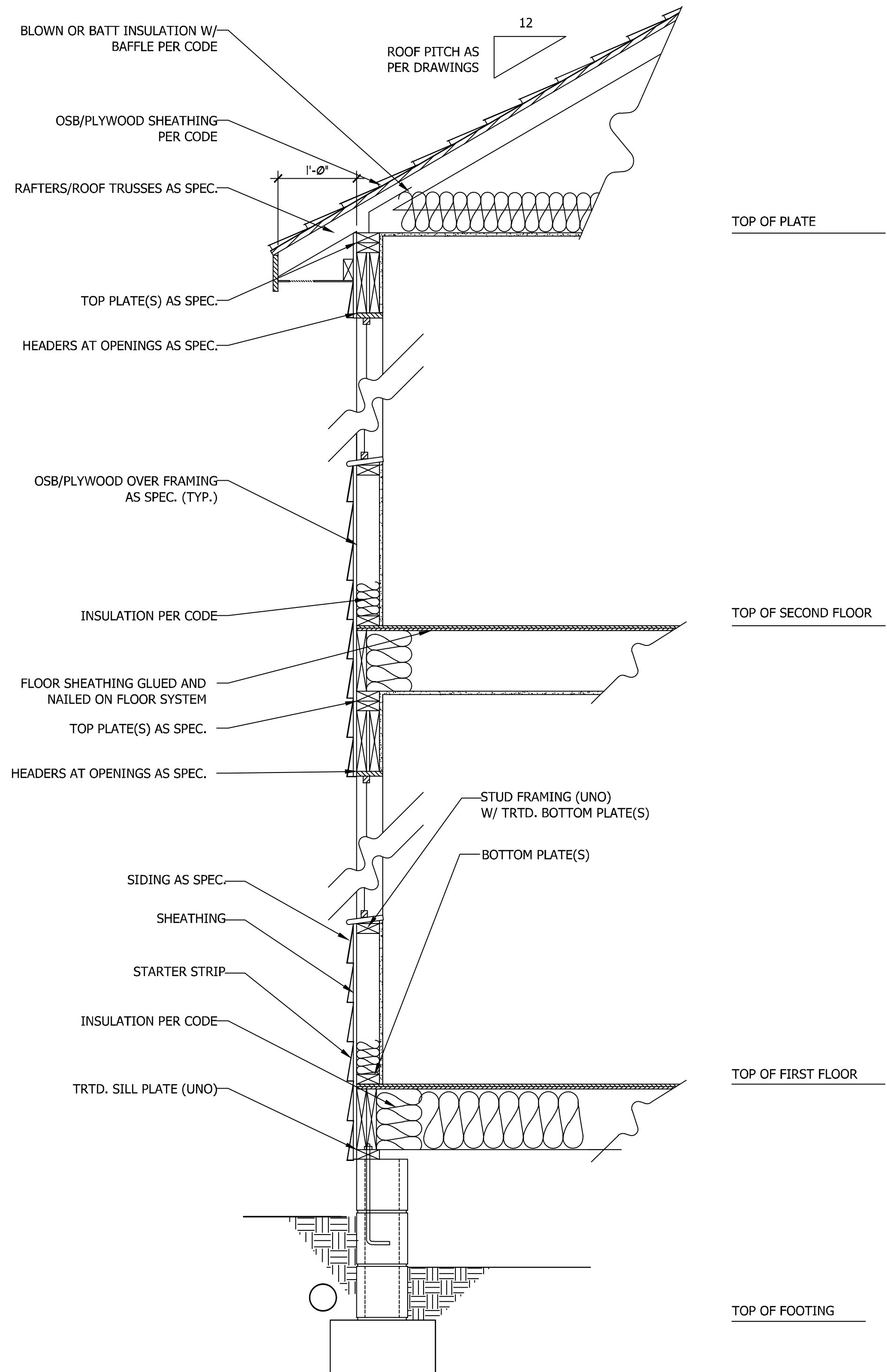
REVIEWED BY:

ROOF PLAN
ELEVATION-A

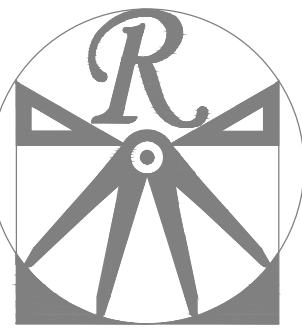
S-3



**WALL SECTION W/ SLAB
W/ STD. SIDING SHOWN (NTS)**



**WALL SECTION W/ CRAWL SPACE
W/ STD. SIDING SHOWN (NTS)**



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WEAVER HOMES
CAROLINA COLLECTION
LEYLAND DRIVE LEFT

DATE: JUNE 15, 2020

REV.:

SCALE: 1/4" = 1'-0"

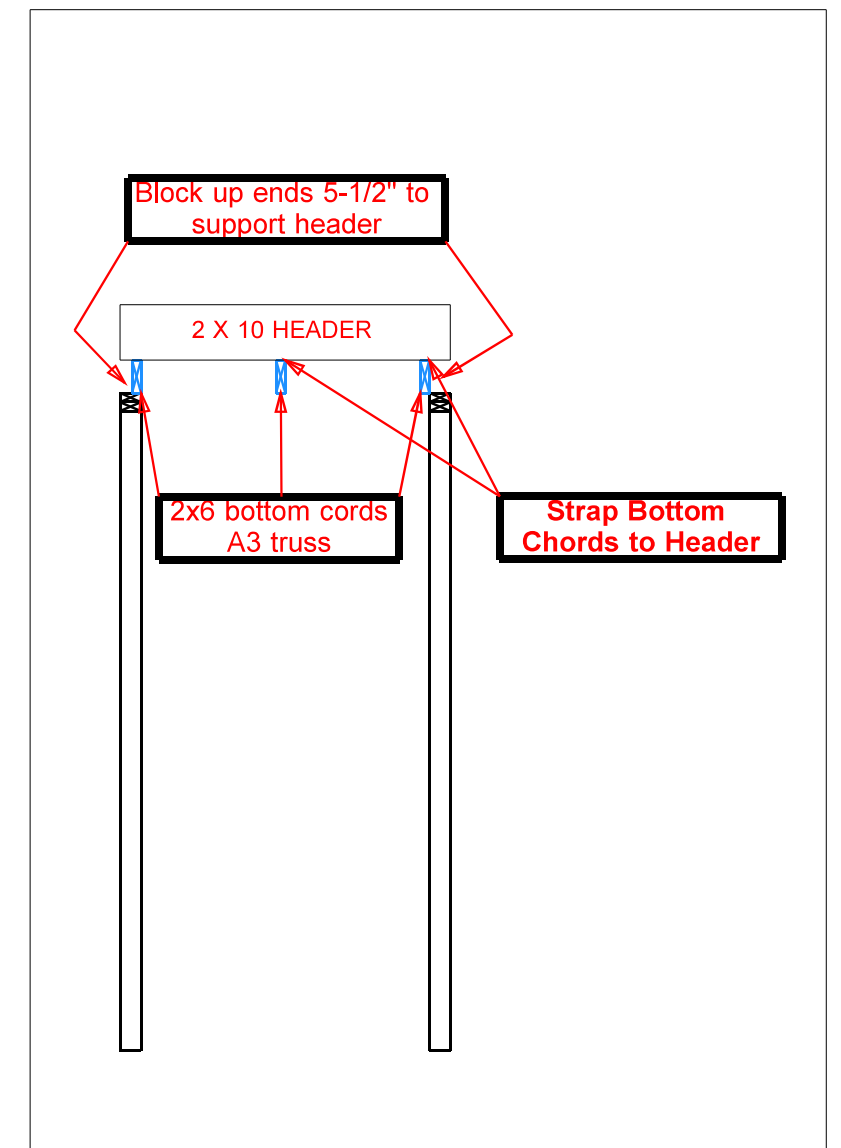
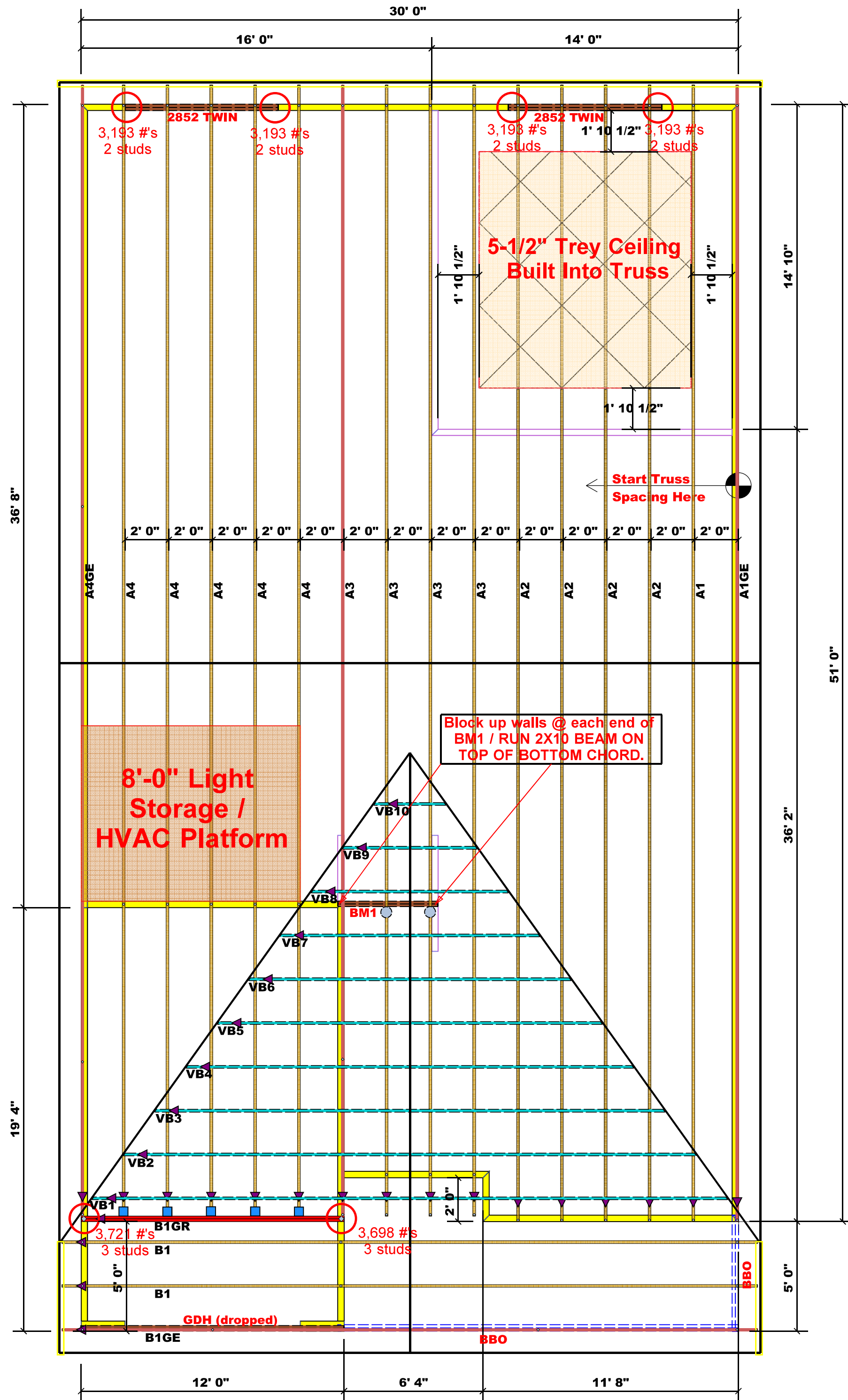
DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:

TYPICAL WALL SECTIONS

D-1



Truss Placement Plan
SCALE: 1/4" = 1'-0"

■	HUS26	USP	5	NA	16d/3-1/2"	16d/3-1/2"
●	MSH422	USP	2	Varies	10d/3"	10d/3"

Estimation

Name	Selection	Formula	Calculation
Roof Area	1st Floor	Roof Area	2049.22
Roof Decking	1st Floor	Roof Decking	70 sheets

BEAM LEGEND

PlotID	Length	Product	Plies	Net Qty
2852 TWIN	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4
GDH (dropped)	12' 0"	2x12 SP No.2	2	2

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

LOAD CHART FOR JACK STUDS

BASED ON TABLES 10.10.1 AND 10.10.2
NUMBER OF JACK STUDS REQUIRED PER SHEET OF ROOF DECKING

Roof Decking (PSF)	Roof Decking (PSF)	Roof Decking (PSF)	Roof Decking (PSF)	Roof Decking (PSF)	Roof Decking (PSF)
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13500	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

BUILDER	Weaver Development Co. Inc.
JOB NAME	Lot 15 West Park
PLAN	Leyland Elev. " C "
SEAL DATE	Seal Date
QUOTE #	Quote #
JOB #	J0820-3790

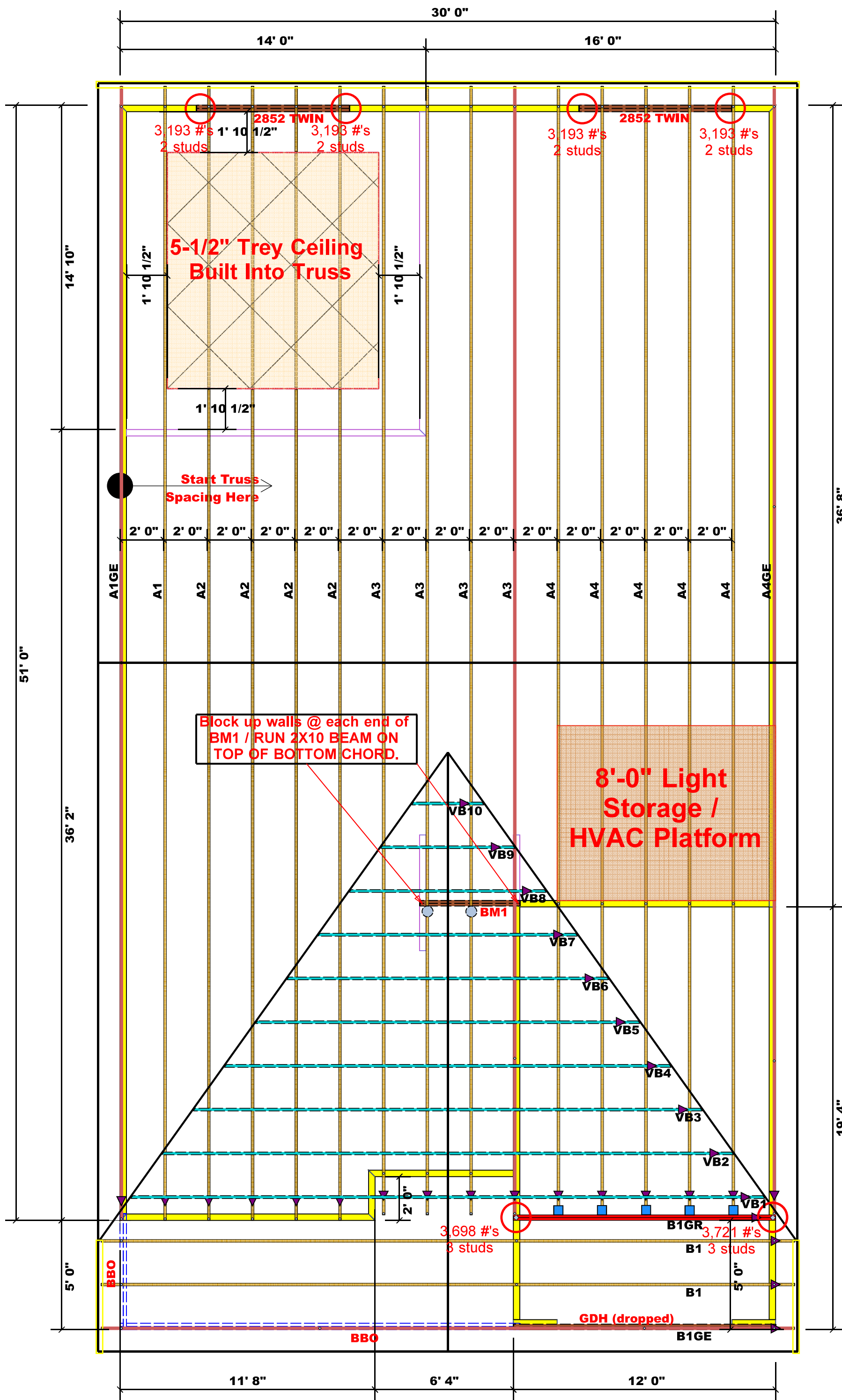
CITY / CO.	Dunn / Harnett
ADDRESS	Lot 15 West Park
MODEL	ROOF
DATE REV.	/ /
DRAWN BY	Lenny Norris
SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: Lenny Norris
Lenny Norris

comTECH
ROOF & FLOOR TRUSSES & BEAMS
Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444



Truss Placement Plan
SCALE: 1/4" = 1'-0"

■	HUS26	USP	5	NA	16d/3-1/2"	16d/3-1/2"
●	MSH422	USP	2	Varies	10d/3"	10d/3"

Estimation

Name	Selection	Formula	Calculation
Roof Area	1st Floor	Roof Area	2049.22
Roof Decking	1st Floor	Roof Decking	70 sheets

BEAM LEGEND

PlotID	Length	Product	Plies	Net Qty
2852 TWIN	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	4
GDH (dropped)	12' 0"	2x12 SP No.2	2	2

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

LOAD CHART FOR JACK STUDS

BASED ON TABLES SET FORTH IN THE PRESCRIBITIVE CODE REQUIREMENTS. SEE INDIVIDUAL DESIGN SHEETS FOR EACH TRUSS IDENTIFIED ON THE ATTACHED TABLES (DERIVED FROM THE PRESCRIBITIVE CODE REQUIREMENTS) TO DETERMINE THE MINIMUM FOUNDATION SIZE AND NUMBER OF WOOD STUDS REQUIRED TO SUPPORT REACTIONS GREATER THAN 3000# BUT NOT GREATER THAN 15000#. A REGISTERED DESIGN PROFESSIONAL SHALL BE RETAINED TO DESIGN THE SUPPORT SYSTEM FOR ANY REACTION THAT EXCEEDS THOSE SPECIFIED IN THE ATTACHED TABLES. A REGISTERED DESIGN PROFESSIONAL SHALL BE RETAINED TO DESIGN THE SUPPORT SYSTEM FOR ALL REACTIONS THAT EXCEED 15000#.

REACTION (LBS)	NO. OF STUDS	MIN. FOUNDATION SIZE (INCHES)
1700	1	3400
3400	2	6800
5100	3	10200
6800	4	13600
8500	5	17000
10200	6	17000
11900	7	17000
13600	8	17000
15300	9	17000

BUILDER	Weaver Development Co. Inc.
JOB NAME	Lot 15 West Park
PLAN	Leyland Elev. " C "
SEAL DATE	Seal Date
QUOTE #	Quote #
JOB #	J0820-3790

CITY / CO.	Dunn / Harnett
ADDRESS	Lot 15 West Park
MODEL	ROOF
DATE REV.	//
DRAWN BY	Lenny Norris
SALES REP.	Lenny Norris

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These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCS1-B1 and BCS1-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: Lenny Norris
Lenny Norris

comTECH

ROOF & FLOOR TRUSSES & BEAMS

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Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

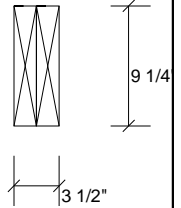
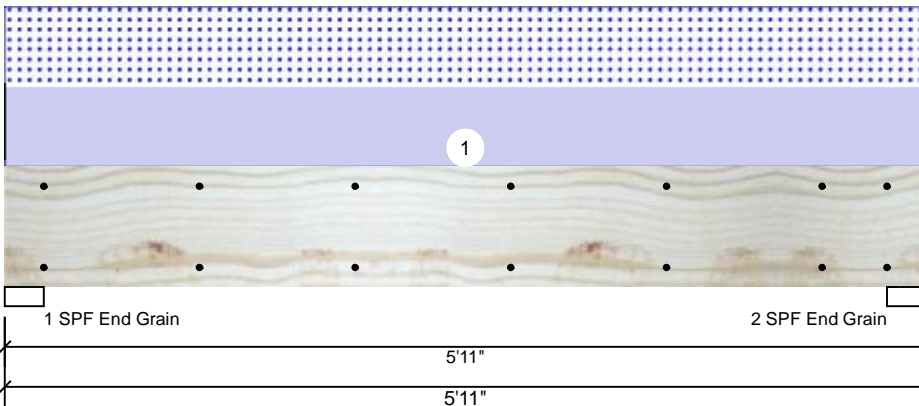


Client: WEAVER
 Project:
 Address:

Date: 9/17/2020
 Input by: Lenny Norris
 Job Name: LEYLAND
 Project #:

2852 TWIN Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	1607	1586	0	0
2	0	1607	1586	0	0

Bearings

Bearing	Length	Cap.	React D/L	Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	35%	1607 / 1586	3193	L	D+S	
2 - SPF End Grain	3.000"	35%	1607 / 1586	3193	L	D+S	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4143 ft-lb	2'11 1/2"	14423 ft-lb	0.287 (29%)	D+S	L
Unbraced	4143 ft-lb	2'11 1/2"	11027 ft-lb	0.376 (38%)	D+S	L
Shear	2158 lb	4'11 1/2"	7943 lb	0.272 (27%)	D+S	L
LL Defl inch	0.032 (L/2081)	2'11 1/2"	0.139 (L/480)	0.230 (23%)	S	L
TL Defl inch	0.064 (L/1034)	2'11 1/2"	0.185 (L/360)	0.350 (35%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	536 PLF	0 PLF	536 PLF	0 PLF	0 PLF	A2 TRUSS / A4
	Self Weight				7 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us
 ICC-ES: ESR-3633

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



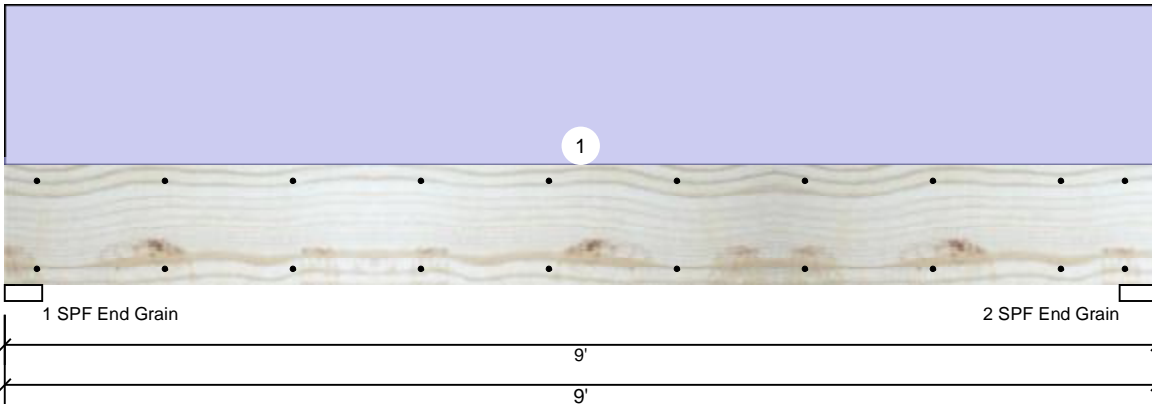


Client: WEAVER
 Project:
 Address:

Date: 9/17/2020
 Input by: Lenny Norris
 Job Name: LEYLAND
 Project #:

GDH SP #2 2.000" X 12.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	900	0	0	0
2	0	900	0	0	0

Bearings

Bearing	Length	Cap. React	D/L Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	15%	900 / 0	900	Uniform	D
2 - SPF End Grain	3.500"	15%	900 / 0	900	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1824 ft-lb	4'6"	3560 ft-lb	0.512 (51%)	D	Uniform
Unbraced	1824 ft-lb	4'6"	3175 ft-lb	0.575 (57%)	D	Uniform
Shear	667 lb	1'2"	3544 lb	0.188 (19%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.048 (L/2132)	4'6"	0.285 (L/360)	0.170 (17%)	D	Uniform

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	

Manufacturer Info

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



This design is valid until 2/26/2023