

Trenco RE: Master - H&H/Topsail/ 818 Soundside Rd Site Information: Edenton, NC 27932 Project Customer: H AND H Project Name: 655409 120MPH Lot/Block: B Subdivision: All Model: Address: State: NC City: Fayetteville General Truss Engineering Criteria & Design Loads (Individual Truss Design **Drawings Show Special Loading Conditions):** Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.2 Wind Code: ASCE 7-10 Wind Speed: 120 mph Design Method: MWFRS (Envelope)/C-C hybrid Wind ASCE 7-10 Roof Load: 40.0 psf Floor Load: N/A psf Mean Roof Height (feet): 25 Exposure Category: C No. Seal# Truss Name Date No. Seal# Truss Name Date 137193666 137193667 A01 A02 35 36 37 38 39 40 137193700 B04 5/24/10 123456789111111111122222222222333333 5/24/19 137193701 B05 24/19 24/19 I37193702 I37193703 B06 B07 137193668 A03 24/10 24/19 137193669 A04 24/19 B10 B11 137193670 A04A 137193704 137193671 A05 137193705 41 42 137193672 A06 B12 24/ 137193706 137193673 A07 137193707 C01 43 44 137193674 A08 137193708 C02 /24/19 137193675 A09 137193709 C03 24/19 45 46 47 137193676 137193677 ČP01 CP02 A10 137193710 5/24/19 A11 137193711 24/19 137193678 137193679 A12 J01 137193712 5/24/10 A13 48 137193713 J02/24/19 49 50 137193714 137193715 A14 137193680 Ĵ03 /24/10 13719 3681 A15 J04 24/19 137193682 A16 137193716 Ĵ05 13719 3683 A17 137193717 Ĵ06 137193684 A18 137193718 J07 13719 3685 137193719 Ĵ08 137193686 J09 137193720 3687 13719372 ĴĨÕ 13719 3688 137193722 13719 13719 3689 13719372 3690 J13 93724 13719 3691 137193725 J14 3692 61 62 63 64 65 66 67 137193726 J15 13719 13719 3693 13719372 J16

137193728

137193731

137193732

137193733

1 of 2

3730

1371937

13719

68

Ĵ17

J18

J19 J20 J21 J22

The truss drawing(s) referenced above have been prepared by

3694

3695

A30

B01

B02

B03

13719

137193696

137193697

137193698

137193699

Incompared on the parameters on the design parameters and the Incompared the design parameters or the design for any particular building. Before use Incompared these designs into the overall building the parameters and the Incompared these designs into the overall building the parameters and the Incompared these designs into the overall building the parameters and the Incompared these designs into the overall building the parameters and the Incompared these designs into the overall building the parameters and the Incompared these designs into the overall building the parameters and the Incompared these designs into the overall building the parameters and the Incompared these designs into the overall building the parameters and the Incompared these designs into the overall building the parameters and the Incompared the parameters on the parameters and the parameters and the Incompared the parameters and the parameters and the parameters and the Incompared the parameters and the parameters and the parameters and the Incompared the parameters and the parameters and the parameters and the Incompared the parameters and the parameters and the parameters and the Incompared the parameters and the Incompared the parameters and the parameters



Sevier, Scott

May 24,2019



RE: Master - H&H/Topsail/

Trenco 818 Soundside Rd Edenton, NC 27932

No.	Seal#	Job ID#	Truss Name	Date
69 70	137193734	Master Master	J23	5/24/19
71	137193736	Master	J25	5/24/19
72 73	137193737 137193738	Master Master	J26 J27	5/24/19 5/24/19
74 75	137193739 137193740	Master Master	V01 V02	5/24/19 5/24/19
76	137193741	Master	V03	5/24/19



being real of use only wind interest contractions. This designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPII Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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	6-2-8	11-11-8	16-11-0	23-5-4	30-5-0					
	6-2-8	5-9-0	4-11-8	6-6-4	6-11-12					
Plate Offsets (X,Y) [2:0-4-1,0-1-0], [8:0-1-5,0-1-4], [10:0-3-0,Edge], [11:0-5-8,0-2-8], [12:0-3-0,0-1-0], [13:0-3-0,Edge]										

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.65 BC 0.78 WB 0.64 Matrix-AS	DEFL.         ir           Vert(LL)         -0.19           Vert(CT)         -0.42           Horz(CT)         0.28           Wind(LL)         0.16	(loc) l/defl 10-11 >999 10-11 >875 8 n/a 12-13 >999	L/d 360 240 n/a 240	PLATES G MT20 2 Weight: 188 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP 2-13,8-	No.2 No.2 *Except* 10: 2x6 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood Rigid ceiling dire 1 Row at midpt	sheathing di ectly applied.	rectly applied. 3-11, 7-11	

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2 *Excep
	2-13,8-10: 2x6 SP N
WEBS	2x4 SP No.3
WEDGE	

Left: 2x4 SP No.3, Right: 2x4 SP No.3

- REACTIONS. (lb/size) 2=1269/0-5-8, 8=1269/0-5-8 Max Horz 2=-305(LC 10) Max Uplift 2=-226(LC 12), 8=-226(LC 13)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-3006/600, 3-5=-1154/402, 5-7=-1304/450, 7-8=-3016/604
- BOT CHORD 2-13=-565/2650, 12-13=-454/2017, 11-12=-258/1488, 10-11=-344/2146, 8-10=-384/2517
- WEBS 3-13=-269/1609, 3-12=-664/259, 3-11=-743/269, 5-11=-239/895, 7-11=-1441/427, 7-10=-150/1554

NOTES-(9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=226. 8=226.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 100322010 SECORE 052. Design valid for use only with MITEK @ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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Edenton, NC 27932

May 24,2019



## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-60, 17-20=-20



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Vert: 1-5=-64, 5-9=-64, 17-20=-21



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9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

#### Continued on page 2

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818 Soundside Road

Edenton, NC 27932

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(IIIIIIIII) May 24,2019

Job	Truss	Truss Type	Qty	Ply	H&H/Topsail/
Master	405	Roof Special	12	1	137193671
	700		12		Job Reference (optional)
Builders FirstSource,	Sumter, SC - 29153,		8.2	40 s May	13 2019 MiTek Industries, Inc. Thu May 23 13:16:46 2019 Page 2
		ID:X87q8	Dmm5Mnji	RaLGAgD	s0yzEGbe-?gdHI7VREUm0kJzrUNA??D37TmfuV8b2lz?TzDcDI

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-60, 13-18=-20, 12-13=-20, 12-21=-20

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## Continued on page 2

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Job	Truss	Truss Type	Qty	F	Ply	H&H/Topsail/	
						13	7193672
Master	A06	ROOF SPECIAL	12		1		
						Job Reference (optional)	
Builders FirstSource,	Sumter, SC - 29153,			8.24	0 s May	13 2019 MiTek Industries, Inc. Thu May 23 13:16:47 2019 Pa	age 2
		ID:X87q8Dmm5MnjRaLGAgDs0yzEGbe-TsBfyTV3?ousMTY114hEYQcJdA3hEVk7qiVWXvzDcDk					

# LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-5=-62, 5-6=-62, 10-15=-21, 9-10=-21, 7-9=-21

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818 Soundside Road Edenton, NC 27932



- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for
- reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
     4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
  - will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) All bearings are assumed to be User Defined crushing capacity of 565 psi.
  - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=209, 2=130.
  - 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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A MITek Affiliate 818 Soundside Road

Edenton, NC 27932



	6-2-8	11-11-8	8 16-11-0	23-5-4	30-	5-0				
	6-2-8	5-9-0	4-11-8	6-6-4	6-11	-12				
Plate Offsets (X,Y) [2:0-4-1,0-1-0], [8:0-1-1,0-1-12], [9:0-3-0,Edge], [10:0-5-8,0-2-8], [11:0-3-0,0-1-0], [12:0-3-0,Edge]										

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.65 BC 0.78 WB 0.65 Matrix-AS	DEFL.         in         (loc)         I/defl         L/d           Vert(LL)         -0.18         9-10         >999         360           Vert(CT)         -0.42         9-10         >877         240           Horz(CT)         0.28         8         n/a         n/a           Wind(LL)         0.16         11-12         >999         240	PLATES         GRIP           MT20         244/190           Weight: 187 lb         FT = 20%

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied.

3-10, 7-10

Rigid ceiling directly applied

1 Row at midpt

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2 *Except*
	2-12,8-9: 2x6 SP No.2
WEBS	2x4 SP No.3
WEDGE	

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=1270/0-5-8, 8=1216/0-5-8 Max Horz 2=299(LC 9) Max Uplift 2=-226(LC 12), 8=-203(LC 13)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-3008/624, 3-5=-1155/403, 5-7=-1306/451, 7-8=-3024/628
- BOT CHORD
   2-12=-586/2636, 11-12=-470/2006, 10-11=-269/1481, 9-10=-380/2152, 8-9=-426/2525

   WEBS
   3-12=-281/1601, 3-11=-660/266, 3-10=-744/269, 5-10=-240/896, 7-10=-1447/445, 7-9=-175/1561

**NOTES-** (9)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=226, 8=203.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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![](_page_15_Picture_2.jpeg)

![](_page_16_Figure_0.jpeg)

	L	6-2-8	11-11-	8 ,	16-11-	0		23-5	5-4	l.	30-5-0	
	1	6-2-8	5-9-0		4-11-8	3 '		6-6	-4	1	6-11-12	
Plate Offset	s (X,Y)	[2:0-1-5,0-1-8], [5:0-3-8	,0-1-12], [6:0-3-1	2,0-2-0], [9:0-	<u>1-5,0-1-8], [11:</u>	0-3-0,Edge], [	12:0-5-4	4,0-2-8]	, [14:0-3-0	0,Edge]		
LOADING TCLL 2 TCDL 7 BCLL BCDL 7	(psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/	2-0-0 1.15 1.15 YES FPI2014	CSI. TC ( BC ( WB ( Matrix-)	0.62 0.82 0.67 AS	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.20 -0.44 0.30 0.16	(loc) 11-12 11-12 9 13-14	l/defl >999 >835 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 199 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHOR BOT CHOR WEBS WEDGE Left: 2x4 SP	D 2x4 SP D 2x4 SP 2-14,9- 2x4 SP 9 No.3, Righ	No.2 No.2 *Except* 11: 2x6 SP No.2 No.3 nt: 2x4 SP No.3				BRACING- TOP CHOF BOT CHOF WEBS	- RD RD	Structu 2-0-0 c Rigid c 1 Row	iral wood oc purlins eiling dire at midpt	sheathing d (5-4-5 max.) ectly applied	lirectly applied, except ): 5-6. I. 3-13, 5-12, 8-12	
REACTION	<b>S.</b> (Ib/size Max He Max U	e) 2=1269/0-5-8, 9=12 orz 2=-283(LC 10) plift 2=-219(LC 12), 9=-	269/0-5-8 219(LC 13)									
FORCES. TOP CHOR BOT CHOR WEBS	(lb) - Max. D 2-3=- D 2-14= 3-14= 8-11=	Comp./Max. Ten All fr 3019/624, 3-5=-1390/40 535/2672, 13-14=-458 247/1605, 3-13=-1433 170/1611	orces 250 (lb) or 59, 5-6=-1043/42 /2207, 12-13=-55 /489, 5-13=-153/	less except w 9, 6-8=-1313/ 5/988, 11-12= 518, 6-12=-11	hen shown. 439, 8-9=-3036 -411/2464, 9-1 14/458, 8-12=-1	i/636 1=-421/2542 701/457,						
NOTES- 1) Unbalanc 2) Wind: AS MWFRS 3) Provide a 4) All plates 5) This trus 6) * This trus will fit bet 7) All bearing a capacity of 9) Provide n 2=219, 9: 10) This trus sheetron	(12) ed roof live GE 7-10; V (envelope) for reaction are 5x6 MT s has been ss has been ss has been usen the bugs are assu- t joint(s) 2, of bearing s nechanical =219. ss design re- ck be applie	e loads have been consi ult=120mph (3-second gable end zone and C- s shown; Lumber DOL= ainage to prevent water 120 unless otherwise in designed for a 10.0 psf n designed for a live loa ottom chord and any ot umed to be User Define 9 considers parallel to surface. connection (by others) of equires that a minimum ad directly to the bottom	dered for this det gust) Vasd=95mj C Exterior(2) zon 1.60 plate grip D ponding. dicated. bottom chord live d of 20.0psf on ther members, will d crushing capac grain value using of truss to bearing of 7/16" structura chord.	sign. bh; TCDL=6.0 e; end vertica OL=1.60 e load noncon the bottom cho h BCDL = 10 ity of 565 psi. ANSI/TPI 1 a g plate capabl il wood sheatt	psf; BCDL=6.0 I left and right e current with an rd in all areas o .0psf. Ingle to grain fo le of withstandii hing be applied	psf; h=25ft; C. xposed;C-C f y other live loa where a rectai ormula. Buildii ng 100 lb uplif l directly to the	at. II; E) or mem ads. ngle 3-6 ng desig it at joint e top che	cp C; Er bers an -0 tall b gner sho t(s) exco ord and	nclosed; d forces & y 2-0-0 w puld verify ept (jt=lb) 1/2" gyps	& ride /	Contraction of the second seco	SEAL 44925

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY REENCO A MITEK ATFILIATE 818 Soundside Road Edenton, NC 27932

May 24,2019

![](_page_17_Figure_0.jpeg)

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![](_page_18_Figure_0.jpeg)

- TOP CHORD
- BOT CHORD 2-9=-432/946. 8-9=-190/337
- WEBS 3-9=-459/330, 5-9=-35/277, 6-9=-224/705, 6-8=-716/322

#### NOTES-(10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=161, 8=155.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

![](_page_18_Figure_15.jpeg)

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

![](_page_18_Picture_17.jpeg)

![](_page_19_Figure_0.jpeg)

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![](_page_20_Figure_0.jpeg)

Vert: 1-5=-60, 5-6=-60, 7-13=-20

![](_page_20_Picture_2.jpeg)

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REFERENCE FACE INTERCISES. To according to the operation of a second sec fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

![](_page_20_Picture_4.jpeg)

![](_page_21_Figure_0.jpeg)

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

![](_page_21_Figure_2.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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![](_page_22_Figure_0.jpeg)

Max Uplift All uplift 100 lb or less at joint(s) 2, 26, 28, 29, 30, 31, 32, 25, 23, 22, 21 except 33=-139(LC 12), 20=-137(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 26, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 18 except 33=279(LC 19), 20=277(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 26, 28, 29, 30, 31, 32, 25, 23, 22, 21 except (jt=lb) 33=139, 20=137.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_22_Figure_17.jpeg)

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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

![](_page_23_Figure_0.jpeg)

	10-1-15					20-3-1			-		
Plate Offsets (X,Y) [2:0-6-0,0-0-9], [5:0-3-12,0-2-0], [6:0-3-12,0-2-0]				-12,0-2-0], [9:	0-6-4,0-0-13	3]			10-1	-15	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	-0.16 11-13	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.47	Vert(CT)	-0.23 11-13	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.35	Horz(CT)	0.03 9	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-AS	Wind(LL)	0.06 11-19	>999	240	Weight: 192 lb	FT = 20%
	-					BRACING-					

LUMBER-		BRACING-			
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood shea	thing directly applie	d, except
BOT CHORD	2x6 SP No.2		2-0-0 oc purlins (5-4-	11 max.): 5-6.	
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly	applied.	
WEDGE		WEBS	1 Row at midpt	6-13	

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=1269/0-5-8, 9=1269/0-5-8 Max Horz 2=-283(LC 10) Max Uplift 2=-220(LC 12), 9=-220(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1765/457, 3-5=-1536/497, 5-6=-1042/435, 6-8=-1546/503, 8-9=-1766/455 TOP CHORD

BOT CHORD 2-13=-305/1532, 11-13=-29/1005, 9-11=-244/1387

WEBS 3-13=-451/327, 5-13=-139/651, 6-11=-173/728, 8-11=-453/327

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) All bearings are assumed to be User Defined crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=220, 9=220.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_23_Figure_19.jpeg)

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818 Soundside Road Edenton, NC 27932

![](_page_24_Figure_0.jpeg)

![](_page_24_Picture_1.jpeg)

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![](_page_25_Figure_0.jpeg)

- REACTIONS. (lb/size) 7=790/0-3-8, 2=873/0-5-8 Max Horz 2=406(LC 12) Max Uplift 7=-208(LC 12), 2=-142(LC 12) Max Grav 7=928(LC 19), 2=926(LC 19)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-1084/175, 3-5=-943/268
- BOT CHORD 2-8=-358/950, 7-8=-100/332
- WEBS 3-8=-491/359, 5-8=-226/935, 5-7=-782/251

#### NOTES- (8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=208, 2=142.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

![](_page_25_Figure_15.jpeg)

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![](_page_26_Figure_0.jpeg)

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![](_page_26_Picture_3.jpeg)

![](_page_27_Figure_0.jpeg)

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![](_page_28_Figure_0.jpeg)

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
  - Vert: 1-5=-60, 5-6=-60, 7-13=-20

![](_page_28_Picture_4.jpeg)

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🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS ON TIPS REPREVED PAGE MIT-14/3 reference of the second secon fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

![](_page_29_Figure_0.jpeg)

May 24,2019

![](_page_29_Picture_2.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

![](_page_30_Figure_0.jpeg)

Vert: 1-5=-60, 5-9=-60, 17-20=-20

minin May 24,2019

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

![](_page_30_Picture_5.jpeg)

![](_page_31_Figure_0.jpeg)

Plate Offsets (X,Y)	[8:0-3-0,0-0-2], [12:0-3-12,0-2-0], [14:Edge,0-3-8]	

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.64 BC 0.10 WB 0.20 Matrix-S	DEFL.         in         (loc)         l/defl         L/d         PLATES         GRIP           Vert(LL)         0.00         1         n/r         120         MT20         244/190           Vert(CT)         -0.00         1         n/r         120         MT20         244/190           Horz(CT)         -0.00         14         n/a         n/a         MT20         244/190
LUMBER- TOP CHORD 2x4 S	P No.2	1	BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

BOT CHORD

BOT CHORD2x6 SP No.2WEBS2x4 SP No.3OTHERS2x4 SP No.3

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 8-12. Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing; 14-15.

REACTIONS. All bearings 19-11-8.

(lb) - Max Horz 2=345(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 14, 2, 15, 16, 17, 18, 19, 20, 21, 22 except 24=-115(LC 12) Max Grav All reactions 250 lb or less at joint(s) 14, 2, 15, 16, 17, 18, 19, 20, 21, 22, 24

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-432/399, 3-4=-330/311, 4-5=-267/260

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 2, 15, 16, 17, 18, 19, 20, 21, 22 except (jt=lb) 24=115.
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_31_Figure_24.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

![](_page_31_Picture_26.jpeg)

![](_page_32_Figure_0.jpeg)

WEBS 3-9=-459/330, 5-9=0/259, 6-9=-240/697, 6-8=-678/221

#### NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=143, 8=183.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

![](_page_32_Figure_13.jpeg)

![](_page_32_Picture_14.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

![](_page_33_Figure_0.jpeg)

		10-	1-15		20-3-1			30	-5-0	
		10-1-15			10-1-1			10-	1-15	
Plate Off	fsets (X,Y)	[2:0-6-0,0-0-5], [5:0-3-12	,0-2-0], [6:0-3	-12,0-2-0], [9:0-6-0	),0-0-5]					
LOADIN	G (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC 0.4	9 Vert(LL)	-0.16 11-13	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC 0.4	7 Vert(CT)	-0.23 11-13	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.34	4 Horz(CT	0.03 9	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matrix-AS	Wind(LL	0.06 11-19	>999	240	Weight: 193 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied, except

6-13

2-0-0 oc purlins (5-4-11 max.): 5-6.

Rigid ceiling directly applied.

1 Row at midpt

LUMBER-	
---------	--

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x6 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=1294/0-5-8, 9=1294/0-5-8 Max Horz 2=-291(LC 10) Max Uplift 2=-231(LC 12), 9=-231(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1759/452, 3-5=-1530/493, 5-6=-1042/432, 6-8=-1540/499, 8-9=-1760/451
- BOT CHORD 2-13=-297/1531, 11-13=-22/1007, 9-11=-230/1382
- WEBS 3-13=-451/326, 5-13=-136/649, 6-11=-170/724, 8-11=-453/326

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=231, 9=231.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_33_Figure_20.jpeg)

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![](_page_33_Picture_22.jpeg)

![](_page_34_Figure_0.jpeg)

**REACTIONS.** All bearings 30-5-0.

(lb) - Max Horz 2=-243(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 26, 27, 29, 30, 31, 32, 33, 34, 35, 25, 24, 23, 22, 21, 19 Max Grav All reactions 250 lb or less at joint(s) 2, 26, 27, 29, 30, 31, 32, 33, 34, 35, 25, 24, 23, 22, 21, 19

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 26, 27, 29, 30, 31, 32, 33, 34, 35, 25, 24, 23, 22, 21, 19.
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 19.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_34_Figure_19.jpeg)

818 Soundside Road Edenton, NC 27932

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![](_page_35_Figure_0.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

TRENCO A MITek Atfiliate 818 Soundside Road Edenton, NC 27932


May 24,2019

TREENCO A MITEK Affiliate 818 Soundside Road Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



10-0-8 10-0-8 10-0-8				
Plate Offsets (X,Y)	[2:0-1-6,0-2-0], [3:0-3-0,0-2-12], [4:0-3-0	,0-2-12], [5:0-1-6,0-2-0],	[7:0-6-0,0-1-13]	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.25 BC 0.37 WB 0.33 Matrix-AS	DEFL.         in         (loc)         l/defl         L/d         PLATES         GRIP           Vert(LL)         -0.06         7-23         >999         360         MT20         244/190           Vert(CT)         -0.15         7-23         >999         240         MT20         244/190           Horz(CT)         0.09         5         n/a         n/a         Wind(LL)         0.06         7-23         >999         240	
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x4 SF	P No.2 P No.2 P No.3 P No.3		BRACING-         TOP CHORD       Structural wood sheathing directly applied, except 2-0-0 oc purlins (5-8-1 max.): 3-4.         BOT CHORD       Rigid ceiling directly applied.	
REACTIONS. (Ib/size Max H Max U	e) 2=848/0-5-8, 5=848/0-5-8 orz 2=-195(LC 10) plift 2=-127(LC 12), 5=-127(LC 13)			
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           BOT CHORD         2-7=-           WEBS         3-7=0	Comp./Max. Ten All forces 250 (lb) or 1538/283, 3-4=-1653/289, 4-5=-1538/28 230/1191, 5-7=-70/1191 0/797, 4-7=-77/797	less except when shown 4		
<ul> <li>NOTES- (14)</li> <li>1) Unbalanced roof live</li> <li>2) Wind: ASCE 7-10; V MWFRS (envelope)</li> <li>MWFRS for reaction</li> <li>3) Truss designed for v Gable End Details a</li> <li>4) Provide adequate di</li> <li>5) All plates are 2x4 M</li> <li>6) Gable studs spaced</li> <li>7) This truss has been</li> <li>8) * This truss has been</li> <li>8) * All bearings are ass</li> <li>10) Bearing at joint(s) : capacity of bearing</li> <li>11) Provide mechanica 2=127, 5=127.</li> <li>12) This truss design m sheetrock be applie</li> <li>13) Graphical purlin re</li> <li>14) This manufactured particular building in</li> </ul>	e loads have been considered for this det (ult=120mph (3-second gust) Vasd=95m gable end zone and C-C Exterior(2) zon is shown; Lumber DOL=1.60 plate grip D vind loads in the plane of the truss only. s applicable, or consult qualified building rainage to prevent water ponding. T20 unless otherwise indicated. at 2-0-0 oc. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on the vottom chord and any other members. umed to be User Defined crushing capace 2, 5 considers parallel to grain value usin surface. al connection (by others) of truss to beari equires that a minimum of 7/16" structura ed directly to the bottom chord. presentation does not depict the size or to truss is designed as an individual buildirg s the responsibility of the building design	sign. bh; TCDL=6.0psf; BCDL= e; end vertical left and rig OL=1.60 For studs exposed to wir designer as per ANSI/TF e load nonconcurrent with he bottom chord in all are ity of 565 psi. g ANSI/TPI 1 angle to gr. hg plate capable of withs al wood sheathing be app he orientation of the purli ng component. The suita er per ANSI TPI 1 as refe	6.0psf; h=25ft; Cat. II; Exp C; Enclosed; ht exposed;C-C for members and forces & id (normal to the face), see Standard Industry 1. any other live loads. as where a rectangle 3-6-0 tall by 2-0-0 wide ain formula. Building designer should verify tanding 100 lb uplift at joint(s) except (jt=lb) lied directly to the top chord and 1/2" gypsum in along the top and/or bottom chord. bility and use of this component for any erenced by the building code. Kay 24,2019	





LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.35 BC 0.39 WB 0.25 Matrix-AS	DEFL.         ir           Vert(LL)         -0.06           Vert(CT)         -0.14           Horz(CT)         0.06           Wind(LL)         0.08	n (loc) I/defl L/d 7-12 >999 360 7-12 >999 240 5 n/a n/a 7-12 >999 240	PLATES         GRIP           MT20         244/190           Weight: 136 lb         FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP 8-9: 2x4 WEBS 2x4 SP WEDGE Left: 2x4 SP No.3, Righ	No.2 No.2 *Except* 4 SP No.2 No.3 t: 2x4 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di 2-0-0 oc purlins (6-0-0 max.) Rigid ceiling directly applied.	rectly applied, except : 3-4.
REACTIONS. (Ib/size Max Ho Max Up	e) 2=848/0-5-8, 5=848/0-5-8 brz 2=-232(LC 10) blift 2=-136(LC 12), 5=-136(LC 13)				
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           BOT CHORD         2-7=-           WEBS         3-7=0	Comp./Max. Ten All forces 250 (lb) or 1482/226, 3-4=-1158/289, 4-5=-1482/22 128/1171, 5-7=-29/1130 //610, 4-7=-158/695	less except when shown 7			
<ul> <li>NOTES- (11)</li> <li>1) Unbalanced roof live</li> <li>2) Wind: ASCE 7-10; V MWFRS (envelope) MWFRS for reaction.</li> <li>3) Provide adequate dra</li> <li>4) This truss has been</li> <li>6) All bearings are assumed and the second s</li></ul>	loads have been considered for this de- ult=120mph (3-second gust) Vasd=95m gable end zone and C-C Exterior(2) zon s shown; Lumber DOL=1.60 plate grip E ainage to prevent water ponding. designed for a 10.0 psf bottom chord live dottom chord and any other members. urned to be User Defined crushing capad 5 considers parallel to grain value using urface. connection (by others) of truss to bearin quires that a minimum of 7/16" structural I directly to the bottom chord. presentation does not depict the size or f truss is designed as an individual buildin	sign. ph; TCDL=6.0psf; BCDL= e; end vertical left and rig OCL=1.60 e load nonconcurrent with he bottom chord in all are city of 565 psi. I ANSI/TPI 1 angle to gra g plate capable of withsta wood sheathing be appli the orientation of the purli og component. The suita	=6.0psf; h=25ft; Cat. II; E ht exposed;C-C for men any other live loads. as where a rectangle 3- in formula. Building desi anding 100 lb uplift at joir ed directly to the top cho in along the top and/or b bility and use of this com	Exp C; Enclosed; nbers and forces & 6-0 tall by 2-0-0 wide igner should verify nt(s) except (jt=lb) ord and 1/2" gypsum ottom chord. nponent for any	SEAL 044925

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







	<u> </u>	3	20-1 10-0	-0 )-8	
Plate Offsets (X,Y)	[2:0-1-11,0-1-2], [3:0-3-0,0-2-12], [4:0-3-	0,0-2-12], [5:0-1-11,0-1-2]	, [7:0-6-0,0-1-12]		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.24 BC 0.38 WB 0.31 Matrix-AS	DEFL.         in         (loc)         //det           Vert(LL)         -0.07         7-23         >999           Vert(CT)         -0.15         7-23         >999           Horz(CT)         0.10         5         n/           Wind(LL)         0.05         7-23         >999	I         L/d         PLATE           3         360         MT20           3         240            a         n/a            9         240         Weigh	ES GRIP 244/190 t: 153 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP OTHERS 2x4 SP WEDGE Left: 2x4 SP No.3, Righ REACTIONS. (Ib/size Max He	No.2 No.2 No.3 No.3 it: 2x4 SP No.3 it: 2x4 SP No.3 it: 2=873/0-5-8, 5=873/0-5-8 orz 2=-204(LC 10)		BRACING- TOP CHORD Structural wo 2-0-0 oc purl BOT CHORD Rigid ceiling	od sheathing directly applied, ins (5-9-3 max.): 3-4. directly applied.	except
<ul> <li>FORCES. (lb) - Max. I MAX UK</li> <li>FORCES. (lb) - Max. I TOP CHORD 2-3= BOT CHORD 2-7=-2 WEBS 3-7=0</li> <li>NOTES- (14)</li> <li>1) Unbalanced roof live</li> <li>2) Wind: ASCE 7-10; V MWFRS (envelope) - MWFRS (envelope) - MW</li></ul>	oli 2=-204(LC 12), 5=-138(LC 13) Comp./Max. Ten All forces 250 (lb) or 1508/261, 3-4=-1584/258, 4-5=-1508/26 220/1155, 5-7=-62/1155 1/756, 4-7=-72/766 Loads have been considered for this det ult=120mph (3-second gust) Vasd=95m gable end zone and C-C Exterior(2) zon s shown; Lumber DOL=1.60 plate grip D vind loads in the plane of the truss only. s applicable, or consult qualified building ainage to prevent water ponding. T20 unless otherwise indicated. at 2-0-0 oc. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on the ottom chord and any other members. Jerned to be User Defined crushing capaca 2, 5 considers parallel to grain value usin surface. I connection (by others) of truss to beari equires that a minimum of 7/16" structura ad directly to the bottom chord. presentation does not depict the size or t truss is designed as an individual building truss is designed as an individual building truss is designed as an individual building designed truss is designed as an individual building designed tr	less except when shown. 2 sign. bh; TCDL=6.0psf; BCDL= e; end vertical left and rigl OL=1.60 For studs exposed to win designer as per ANSI/TP e load nonconcurrent with he bottom chord in all area ity of 565 psi. g ANSI/TPI 1 angle to gra ng plate capable of withsta al wood sheathing be appl he orientation of the purlir g component. The suitat	5.0psf; h=25ft; Cat. II; Exp C; Enclosed it exposed;C-C for members and force d (normal to the face), see Standard Ir 1. any other live loads. is where a rectangle 3-6-0 tall by 2-0-0 in formula. Building designer should v anding 100 lb uplift at joint(s) except (j ed directly to the top chord and 1/2" g along the top and/or bottom chord. ility and use of this component for any ranced by the building code	d; es & ndustry 0 wide verify t=lb) typsum	SEAL 044925
					May 27,2010





	10	)-0-8		10-0-8			
Plate Offsets (X,Y)	[2:0-1-13,0-1-12], [3:0-2-4,0-3-4], [4:0-2	-4,0-3-4], [5:0-1-13,0-1-12]	]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.34 BC 0.37 WB 0.25 Matrix-AS	DEFL.         in           Vert(LL)         -0.06           Vert(CT)         -0.14           Horz(CT)         0.07           Wind(LL)         0.07	(loc) l/defl 7-12 >999 7-12 >999 5 n/a 7-12 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 139 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP 8-9: 2x WEBS 2x4 SP WEDGE Left: 2x4 SP No.3, Righ	No.2 No.2 *Except* 4 SP No.2 No.3 nt: 2x4 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural woc 2-0-0 oc purlir Rigid ceiling d	d sheathing dir is (6-0-0 max.): irectly applied.	ectly applied, except 3-4.	
REACTIONS. (Ib/size Max H Max U	e) 2=873/0-5-8, 5=873/0-5-8 orz 2=-242(LC 10) plift 2=-146(LC 12), 5=-146(LC 13)						
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           BOT CHORD         2-7=-           WEBS         3-7=0	Comp./Max. Ten All forces 250 (lb) or 1455/206, 3-4=-1121/271, 4-5=-1455/20 121/1151, 5-7=-14/1096 //595, 4-7=-158/683	less except when shown. 16					
NOTES- (11) 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) MWFRS for reaction 3) Provide adequate dr 4) This truss has been will fit between the b 6) All bearings are assist 7) Bearing at joint(s) 2.	e loads have been considered for this de ult=120mph (3-second gust) Vasd=95m gable end zone and C-C Exterior(2) zor s shown; Lumber DOL=1.60 plate grip I ainage to prevent water ponding. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on t ottom chord and any other members. Juned to be User Defined crushing capat 5 considers parallel to grain value using	sign. ph; TCDL=6.0psf; BCDL= e; end vertical left and rigi OL=1.60 e load nonconcurrent with he bottom chord in all area sity of 565 psi. I ANSI/TPI 1 angle to grain	6.0psf; h=25ft; Cat. II; E ht exposed;C-C for merr any other live loads. as where a rectangle 3-6 n formula. Building desi	xp C; Enclosed hbers and forces 6-0 tall by 2-0-0 aner should ver	s & wide	ORTH	CAROLINI,

7) Bearing at joint(s) 2, 5 considers parallel to grain value using ANSI/TPTT angle to grain formula. Building designer should verify capacity of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=146, 5=146.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

#### LUMBER-

WEDGE

 TOP CHORD
 2x6 SP No.2

 BOT CHORD
 2x6 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=873/0-5-8, 10=873/0-5-8 Max Horz 2=265(LC 11) Max Uplift 2=-151(LC 12), 10=-151(LC 13)

 
 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1574/255, 3-4=-1466/350, 4-5=-1291/241, 5-6=-1130/218, 6-7=-1154/260, 7-8=-1291/243, 8-9=-1458/329, 9-10=-1574/238

 BOT CHORD
 2-18=-267/1357, 17-18=-295/1402, 16-17=-221/1348, 15-16=-61/1163, 14-15=-30/1129, 13-14=-65/1193, 12-13=-92/1222, 10-12=-72/1184

 WEBS
 6-15=-148/1090, 4-16=-456/343, 8-14=-445/376

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) All bearings are assumed to be User Defined crushing capacity of 565 psi.

9) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=151, 10=151.

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



ERGINEERING BY ERECO AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932





🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



May 24,2019

ENGINEERING BY ERENCED AMITEK Affiliate 818 Soundside Road Edenton, NC 27932



May 24,2019





LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.56 BC 0.85 WB 0.15 Matrix-AS	DEFL.         in           Vert(LL)         -0.13           Vert(CT)         -0.28           Horz(CT)         0.02           Wind(LL)         0.11	(loc)         I/defl         L/d           6-9         >999         360           6-9         >844         240           4         n/a         n/a           6-9         >999         240	PLATES         GRIP           MT20         244/190           Weight: 91 lb         FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS.	2x6 SP No.2 2x4 SP No.2 2x4 SP No.3 (Ib/size) 2=836/0-5-8, 4=836/0-5-8 Max Horz 2=85(LC 16) Max Uplift 2=-164(LC 12), 4=-164(LC 13)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir Rigid ceiling directly applied.	ectly applied.
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	- Max. Comp./Max. Ten All forces 250 (lb) or 2-3=-1235/403, 3-4=-1235/403 2-6=-241/1080, 4-6=-241/1080 3-6=0/404	less except when shown.			

**NOTES-** (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be User Defined crushing capacity of 565 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=164, 4=164.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







						10 11 0						
						19-11-0						
Plate Offse	ets (X,Y)	[2:0-2-11,Edge], [2:0-0-0	,0-1-1], [5:0-3-	0,0-2-4], [9:0	-3-0,0-2-4], [1	12:0-2-11,Edge], [1	2:Edge	e,0-1-1],	[16:0-3-0	),0-3-0]		
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.16	Vert(LL)	0.00	13	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	0.01	13	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	12	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 91 lb	FT = 20%
LUMBER-						BRACING-						
ГОР СНО	RD 2x4 SF	P No.2				TOP CHOR	D	Structu	ral wood	sheathing di	rectly applied or 6-0-0	oc purlins, except
вот сно	RD 2x4 SF	P No.2						2-0-0 o	c purlins	(6-0-0 max.)	5-9.	
OTHERS	2x4 SF	P No.3				BOT CHOR	D	Rigid c	eiling dire	ctly applied	or 10-0-0 oc bracing.	
WEDGE								0	Ū	, ,,	Ū	
_eft: 2x4 S	P No.2, Rig	ht: 2x4 SP No.2										

19-11-0

REACTIONS. All bearings 19-11-0.

(lb) - Max Horz 2=-63(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 17, 18, 19, 16, 15, 12 except 20=-118(LC 12), 14=-118(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 17, 18, 19, 16, 15, 12 except 20=310(LC 23), 14=310(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (13)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 17, 18, 19, 16, 15, 12 except (jt=lb) 20=118, 14=118.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







				12-0-0					
LOADING	(psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.50	Vert(LL) 0.01	5	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.35	Vert(CT) 0.02	5	n/r	120		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.08	Horz(CT) 0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 41 lb	FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD OTHERS

2x4 SP No.2 2x4 SP No.3

REACTIONS. 2=228/12-0-0, 4=228/12-0-0, 6=549/12-0-0 (lb/size) Max Horz 2=-41(LC 17) Max Uplift 2=-78(LC 8), 4=-83(LC 9), 6=-77(LC 8) Max Grav 2=235(LC 23), 4=235(LC 24), 6=549(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. 3-6=-366/234 WEBS

NOTES-(10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7) will fit between the bottom chord and any other members.

- 8) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- 10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







	6-0-0 6-0-0			<u> </u>				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2015/TPI2014	<b>CSI.</b> TC 0.38 BC 0.43 WB 0.10 Matrix-AS	DEFL. i Vert(LL) 0.0 Vert(CT) -0.0 Horz(CT) 0.0	n (loc) 9 6-12 9 6-12 1 4	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 41 lb	<b>GRIP</b> 244/190 FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=503/0-3-0, 4=503/0-3-0 Max Horz 2=-41(LC 13) Max Uplift 2=-244(LC 8), 4=-244(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-902/994, 3-4=-902/994

BOT CHORD 2-6=-877/826, 4-6=-877/826

3-6=-334/267 WEBS

NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=244. 4=244.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.





LUMBER-



3x4 =

LOADING (psf) ICLL 20.0 ICDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	<b>CSI.</b> TC 0.81 BC 0.52	<b>DEFL.</b> in Vert(LL) -0.07 Vert(CT) 0.07	n (loc) 1 1	l/defl n/r n/r	L/d 120 120	PLATES MT20	<b>GRIP</b> 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-P	Horz(CT) 0.00	) 4	n/a	n/a	Weight: 22 lb	FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.2

REACTIONS. (lb/size) 4=254/6-6-0, 2=277/6-6-0 Max Horz 2=78(LC 9) Max Uplift 4=-69(LC 12), 2=-79(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (9)

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



2x4 ||

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





	<u>0-6-8</u> 0-6-8		<u>6-6-0</u> 5-11-8	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.47 BC 0.39 WB 0.00 Matrix-AS	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         0.14         4.9         >547         240           Vert(CT)         -0.10         4-9         >734         240           Horz(CT)         -0.00         2         n/a         n/a	PLATES         GRIP           MT20         244/190           Weight: 22 lb         FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

REACTIONS. (lb/size) 2=303/0-3-0, 4=228/0-1-8 Max Horz 2=76(LC 8) Max Uplift 2=-154(LC 8), 4=-128(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (9)

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
   This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=154, 4=128.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

LUMBER-



		0-6-8	1-5-8		
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.07	DEFL. in (loc Vert(LL) -0.00	c) I/defl L/d 9 >999 360	PLATES         GRIP           MT20         244/190
BCDL 10.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-MP	Vert(CT)         -0.00         2           Horz(CT)         -0.00         2           Wind(LL)         0.00         2	9 >999 240 2 n/a n/a 9 >999 240	Weight: 7 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD

REACTIONS. (lb/size) 4=39/0-3-8, 2=143/0-3-8 Max Horz 4=26(LC 8) Max Uplift 4=-16(LC 9), 2=-48(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(8)

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearings are assumed to be: Joint 4 User Defined crushing capacity of 425 psi, Joint 2 User Defined crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 2-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing





	0-6-8		<u>6-4-8</u> 5-10-0	I	
Plate Offsets (X,Y)	[2:0-4-5,0-0-5]				
LOADING(psf)TCLL20.0TCDL10.0BCLL0.0*	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNO	CSI. TC 0.45 BC 0.50 WB 0.00	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         0.07         5-9         >999         240           Vert(CT)         -0.08         5-9         >936         240           Horz(CT)         0.00         5         n/a         n/a	PLATES         GRIP           MT20         244/190	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP		Weight: 27 lb FT = 20%	

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.2WEBS2x4 SP No.2

BRACING-TOP CHORD Structur except 6 BOT CHORD Rigid ce

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

# REACTIONS. (lb/size) 5=503/Mechanical, 2=477/0-3-0 Max Horz 2=78(LC 4) Max Uplift 5=-296(LC 4), 2=-250(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone; cantilever left exposed ; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=296, 2=250.
- 6) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-20, 2-5=-20 Concentrated Loads (lb)

Vert: 5=-154(F) 10=-154(F) 11=-146(F)







REACTIONS. (Ib/size) 2=252/0-3-0, 4=159/0-1-8 Max Horz 2=75(LC 8) Max Uplift 2=-126(LC 8), 4=-94(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (9)

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
   This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=126.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







	0-6-8		4-7-8				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.27 BC 0.22 WB 0.00 Matrix-AS	DEFL.         in           Vert(LL)         0.04           Vert(CT)         -0.03           Horz(CT)         -0.00	(loc) l/defl 4-8 >999 4-8 >999 2 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 18 lb	<b>GRIP</b> 244/190 FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

REACTIONS. (lb/size) 4=166/Mechanical, 2=258/0-3-0 Max Horz 2=78(LC 8) Max Uplift 4=-99(LC 8), 2=-127(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
   This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.

5) Refer to girder(s) for truss to truss connections.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=127.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







		0-6-8 0-6-8		5-2-0 4-7-8	
LOADIN TCLL TCDL BCU	<b>G</b> (psf) 20.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Bep Stress Incr. YES	CSI. TC 0.10 BC 0.17 WB 0.05	DEFL.         in         (loc)         //defl         L/d         PLATES         GRIP           Vert(LL)         -0.01         5-9         >999         360         MT20         244/190           Vert(CT)         -0.03         5-9         >999         240         MT20         244/190	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-AS	Wind(LL)         0.03         5-9         >999         240         Weight: 20 lb         FT = 20%	

# LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins: 3-4. Rigid ceiling directly applied.

REACTIONS. (lb/size) 2=258/0-3-0, 5=166/Mechanical Max Horz 2=50(LC 8) Max Uplift 2=-136(LC 8), 5=-91(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (10)

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
   Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2=136.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.











	0-6-8			5-2-0					1
	0-6-8			4-7-8					
Plate Offsets (X,	Y) [2:0-3-0,0-2-8]								
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.19	Vert(LL)	0.01 4-6	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.16	Vert(CT)	-0.01 4-6	>999	240		
BCLL 0.0	* Rep Stress Incr	NO	WB 0.00	Horz(CT)	0.00 4	l n/a	n/a		
BCDL 10.0	Code IRC2015	/TPI2014	Matrix-MR					Weight: 17 lb	FT = 20%
LUMBER-				BRACING					

TOP CHORD 2x6 SP No.2 \*Except\* TOP CHORD Structural wood sheathing directly applied or 5-2-0 oc purlins, 2-3: 2x4 SP No.2 except end verticals, and 2-0-0 oc purlins: 2-3. BOT CHORD 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2x4 SP No.2

#### REACTIONS. (lb/size) 4=174/Mechanical, 1=189/0-3-0 Max Horz 1=16(LC 4)

Max Uplift 4=-92(LC 4), 1=-97(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(11)

1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone; cantilever left exposed ; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) Provide adequate drainage to prevent water ponding.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 1.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 2-7=-60, 2-3=-60, 1-4=-20 Concentrated Loads (lb) Vert: 11=-1(B) 12=-1(B)



818 Soundside Road Edenton, NC 27932

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPERVICE PAGE MIT-14/3 refer to 1000 SEC. Design valid for use only with MITER design connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



			0-6-8	1		1-5-8			1		
LOADING	(psf) 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC	0.03	DEFL. Vert(LL)	in -0.00	(loc) 5	l/defl >999	L/d 360	PLATES MT20	<b>GRIP</b> 244/190
TCDL	10.0	Lumber DOL 1.15	BC	0.03	Vert(CT)	-0.00	5	>999	240		21.1.100
BCLL BCDL	0.0 * 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB Matri	0.00 ix-MP	Horz(CT) Wind(LL)	-0.00 0.00	3 9	n/a >999	n/a 240	Weight: 7 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

```
LUMBER-
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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (Ib/size) 3=27/Mechanical, 4=11/Mechanical, 2=143/0-3-0 Max Horz 2=28(LC 8) Max Uplift 3=-16(LC 8), 4=-7(LC 9), 2=-79(LC 8) Max Grav 3=27(LC 1), 4=21(LC 3), 2=143(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (7)

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4, 2.
- 7) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 2-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





3x4 =

Plate Offsets	(X,Y)	[3:0-0-11,0-1-8	1

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.61 BC 0.42 WB 0.00 Matrix-S	DEFL. ir Vert(LL) -0.01 Vert(CT) 0.01 Horz(CT) 0.00	n (loc) l/defl L/d 1 n/r 120 1 n/r 120 0 4 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 25 lb         FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x6 S	P No.2 P No.2 P No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=267/6-11-0, 2=291/6-11-0 Max Horz 2=79(LC 8)

Max Uplift 4=-78(LC 12), 2=-77(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(9)

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.









📣 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPERVICE PAGE MIT-14/3 refer to 1000 SEC. Design valid for use only with MITER design connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





		0-6-8	1-10-8	
LOADING (psf)	SPACING- 2-0-0	<b>CSI.</b>	<b>DEFL.</b> in (loc) I/defl L/d	PLATES GRIP
TCDL 10.0	Lumber DOL 1.15	BC 0.14	Vert(CT) -0.00 4-9 >999 240	101120 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-MP	Horz(CT) -0.00 2 n/a n/a	Weight: 8 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 4=61/0-1-8, 2=154/0-3-0 Max Horz 2=30(LC 8)

Max Uplift 4=-28(LC 8), 2=-80(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-** (7)

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearings are assumed to be: Joint 2 User Defined crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 7) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 2-5-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing





	0-6-8		2-5-8					
LOADING (psf)SPACING-TCLL 20.0Plate Grip DOTCDL 10.0Lumber DOLBCLL 0.0 *Rep Stress InBCDL 10.0Code IRC201	2-0-0 L 1.15 1.15 cr YES 5/TPI2014	<b>CSI.</b> TC 0.06 BC 0.05 WB 0.00 Matrix-MP	DEFL. in Vert(LL) 0.00 Vert(CT) -0.00 Horz(CT) -0.00	(loc) 9 4-9 3	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 10 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 3=57/Mechanical, 4=27/Mechanical, 2=173/0-3-0 Max Horz 2=38(LC 8) Max Uplift 3=-33(LC 8), 4=-14(LC 8), 2=-92(LC 8) Max Grav 3=57(LC 1), 4=41(LC 3), 2=173(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

 Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left

exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be User Defined crushing capacity of 565 psi.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4, 2.



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





	<u>0-6-8</u>	1-10-1	5-2-0
	0-6-8	1-3-9	3-3-15
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.21 BC 0.11 WB 0.03 Matrix-MP	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         5-9         >999         360         MT20         244/190           Vert(CT)         -0.01         5-9         >999         240         MT20         244/190           Horz(CT)         0.00         5         n/a         n/a         Wind(LL)         0.00         5-9         >999         240

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 2x6 SP No.2 BOT CHORD WEBS

2x4 SP No.2

REACTIONS. 5=167/Mechanical, 2=306/0-3-0 (lb/size) Max Horz 2=41(LC 4) Max Uplift 5=-90(LC 4), 2=-177(LC 4)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone; cantilever left exposed ; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) Provide adequate drainage to prevent water ponding.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2 = 177

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 22 lb down and 35 lb up at 1-10-1, and 23 lb down and 35 lb up at 4-0-12 on top chord, and 11 lb down and 28 lb up at 2-0-12, and 11 lb down and 28 lb up at 4-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 5-6=-20 Concentrated Loads (Ib)

Vert: 3=-1(B) 10=-2(B) 11=-7(B) 12=-8(B)



Structural wood sheathing directly applied or 5-2-0 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPERVICE PAGE MIT-14/3 refer to 1000 SEC. Design valid for use only with MITER decomponent, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



	0-6-8		5-2-0 4-7-8			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.15 WB 0.05 Matrix-AS	DEFL.         in         (loc)           Vert(LL)         -0.01         5-10           Vert(CT)         -0.02         5-10           Horz(CT)         -0.00         2           Wind(LL)         0.02         5-10	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 25 lb	<b>GRIP</b> 244/190 FT = 20%

TOP CHORD

BOT CHORD

2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied.

LUMBER-

- TOP CHORD
   2x6 SP No.2 \*Except\*

   3-4: 2x4 SP No.2

   BOT CHORD
   2x4 SP No.2

   WEBS
   2x4 SP No.3
- REACTIONS. (lb/size) 2=268/0-3-0, 5=170/Mechanical Max Horz 2=62(LC 8)

Max Uplift 2=-142(LC 8), 5=-95(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2=142.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied, except end verticals, and





TOP CHORD

BOT CHORD

#### LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

 WEDGE
 Left: 2x4 SP No.3

REACTIONS. (lb/size) 2=283/0-3-0, 4=158/0-1-8 Max Horz 2=82(LC 8) Max Uplift 2=-149(LC 8), 4=-94(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=149.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.





Plate Offsets (X,Y)	0-6-8 0-6-8 [2:0-4-0,0-0-4]		6-9-8 6-3-0	
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	<b>CSI.</b> TC 0.52	DEFL. in (loc) I/defl L/d Vert(LL) 0.09 5-9 >875 240	PLATES         GRIP           MT20         244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	BC 0.61 WB 0.00 Matrix-MP	Vert(CT) -0.11 5-9 >759 240 Horz(CT) 0.00 5 n/a n/a	Weight: 29 lb FT = 20%

#### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.2WEBS2x4 SP No.2

BRACING-TOP CHORD Si ex BOT CHORD Ri

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

# REACTIONS. (lb/size) 5=408/Mechanical, 2=449/0-3-0 Max Horz 2=83(LC 4)

Max Uplift 5=-238(LC 4), 2=-237(LC 4)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone; cantilever left exposed ; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=238, 2=237.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 147 lb down and 115 lb up at 3-0-12, and 150 lb down and 117 lb up at 5-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
  - Vert: 1-3=-60, 3-4=-20, 2-5=-20

Concentrated Loads (lb)

Vert: 10=-147(F) 11=-150(F)







- 8) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
  - Uniform Loads (plf) Vert: 1-4=-60, 2-5=-20 Concentrated Loads (lb) Vert: 6=-146(F) 5=-154(F) 11=-154(F)

SEAL 044925 WGINEEP





	0-6-8
	0-6-8
Diata Offacta (V.V)	[2:0 4 12 0 0 4]

LOADING	i (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP			
TCLL	20.0	Plate Grip DOL 1.15	TC 0.29	Vert(LL) 0.05 4-8 >999 240 MT20 244/190			
TCDL	10.0	Lumber DOL 1.15	BC 0.24	Vert(CT) -0.04 4-8 >999 240			
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.00 4 n/a n/a			
BCDL	10.0	Code IRC2015/TPI2014	Matrix-AS	Weight: 28 lb FT = 20%			

# LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.2WEBS2x6 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

REACTIONS. (Ib/size) 2=307/0-3-0, 4=218/0-1-8 Max Horz 2=76(LC 8) Max Uplift 2=-157(LC 8), 4=-122(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

 Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be User Defined crushing capacity of 565 psi.

- 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=157, 4=122.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







BRACING-TOP CHORD

BOT CHORD

IIM	RF	R-

TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.2WEBS2x6 SP No.2

REACTIONS. (Ib/size) 2=249/0-3-0, 4=155/0-1-8 Max Horz 2=75(LC 9) Max Uplift 2=-72(LC 8), 4=-45(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.





REACTIONS. (lb/size) 5=9/5-11-8, 2=166/5-11-8, 6=356/5-11-8 Max Horz 2=75(LC 9) Max Uplift 5=-5(LC 9), 2=-48(LC 8), 6=-102(LC 12)

# NOTES-

 Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-4-8 to 2-7-8, Exterior(2) 2-7-8 to 6-4-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.602) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2 except (jt=lb) 6=102.

7) Non Standard bearing condition. Review required.





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-6=-267/322



		0-6-8	1-5-11	1	5-4-4							
		0-6-8	0-11-3	1				3-10-	9			
Plate Offse	ets (X,Y)	[3:0-6-0,0-1-12], [6:0-1-7	11,0-0-8]									
LOADING TCLL TCDL BCLL	(psf) 20.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 NO	CSI. TC BC WB	0.30 0.29 0.01	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.04 -0.06	(loc) 5-6 5-6	l/defl >999 >999	L/d 240 240	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0	Code IRC2015/T	PI2014	Matri	k-MP	1012(01)	0.00	2	n/a	n/a	Weight: 18 lb	FT = 20%
TOP CHORD 2x4 SP No.2				BRACING- TOP CHOR	D	Structu	ral wood	sheathing di	rectly applied or 5-4-4	oc purlins,		

BOT CHORD

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

REACTIONS. (lb/size) 5=172/Mechanical, 2=276/0-3-0

Max Horz 2=30(LC 4) Max Uplift 5=-93(LC 5), 2=-152(LC 4)

Max Grav 5=175(LC 20), 2=276(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;

MWFRS (envelope) gable end zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 565 psi.

7) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2=152.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 5-7=-20

Concentrated Loads (lb) Vert: 6=-1(B) 13=-1(B) SEAL 044925 MGINEEPHERIN May 24,2019

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTP11** Quality Criteria, DSB-89 and BCSI Building Component **Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. **Base Street Street** 

except end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.




	<u>0-6-8</u>   <u>0-6-8</u>		<u>5-4-4</u> 4-9-12	
LOADING     (psf)       TCLL     20.0       TCDL     10.0       BCLL     0.0       BCDL     10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.10 BC 0.17 WB 0.06 Matrix-AS	DEFL.     in     (loc)     //defl     L/d     PLA*       Vert(LL)     -0.01     7-11     >999     360     MT20       Vert(CT)     -0.03     7-11     >999     240       Horz(CT)     0.00     7     n/a     n/a       Wind(LL)     0.03     7-11     >999     240	TES     GRIP       0     244/190       ht: 21 lb     FT = 20%

# LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD

WEBS 2x4 SP No.3

REACTIONS. 2=261/0-3-0, 7=180/Mechanical (lb/size) Max Horz 2=45(LC 8) Max Uplift 2=-139(LC 8), 7=-89(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 565 psi.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 2 = 139

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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TOP CHORD

BRACING-

BOT CHORD

Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins: 3-5. Rigid ceiling directly applied.



	0-6-8			
Plate Offsets (X,Y)	[3:0-3-0,Edge]			
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.22	Vert(LL) 0.04 7-11 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.20	Vert(CT) -0.03 7-11 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.04	Horz(CT) -0.00 2 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-AS		Weight: 21 lb FT = 20%
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

2-0-0 oc purlins: 3-5.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=261/0-3-0, 7=180/Mechanical

Max Horz 2=67(LC 8) Max Uplift 2=-133(LC 8), 7=-95(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; C-C for
- members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

All bearings are assumed to be User Defined crushing capacity of 565 psi.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 2=133.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Structural wood sheathing directly applied, except end verticals, and

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 100322010 SECORE 052. Design valid for use only with MITEK @ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Plate Offsets (X	(,Y)	[2:0-8-0,0-1-2], [4:Edge,0	-3-14]	-		1					1	
LOADING (psf	)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	) )	Plate Grip DOL	1.15	тс	0.41	Vert(LL)	0.07	4-8	>999	240	MT20	244/190
TCDL 10.0	)	Lumber DOL	1.15	BC	0.51	Vert(CT)	-0.08	4-8	>999	240		
BCLL 0.0	) *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	)	Code IRC2015/TF	PI2014	Matri	k-MS						Weight: 30 lb	FT = 20%
LUMBER-						BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.2WEBS2x6 SP No.2

REACTIONS. (Ib/size) 2=529/0-3-0, 4=507/0-1-8 Max Horz 2=81(LC 19) Max Uplift 2=-273(LC 4), 4=-277(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-258/104

### NOTES-

1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;

MWFRS (envelope) gable end zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be User Defined crushing capacity of 565 psi.

5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=273, 4=277.
- 8) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 2-4=-20

Concentrated Loads (lb)

Vert: 9=-155(F) 10=-160(F) 11=-163(F)



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



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0-6-8	6-11-0	1	
0-6-8	6-4-8	1	
Plate Offsets (X,Y) [3:0-0-11,0-1-8], [4:Edge,0-1-8]			
LOADING (psf) SPACING- 2-0-0	CSI. DEFL. in (loc) I/defl L/d	PLATES GRIP	
TCLL 20.0 Plate Grip DOL 1.15	TC 0.34 Vert(LL) 0.09 4-9 >895 240	MT20 244/190	
TCDL 10.0 Lumber DOL 1.15	BC 0.28 Vert(CT) -0.07 4-9 >999 240		
BCLL 0.0 * Rep Stress Incr YES	WB 0.00 Horz(CT) -0.00 2 n/a n/a		
BCDL 10.0 Code IRC2015/TPI2014	Matrix-AS	Weight: 25 lb FT = 20%	
BCDL 10.0 Code IRC2015/TPI2014	Matrix-AS	Weight: 25 lb $FI = 20\%$	

### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x6 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

REACTIONS. (Ib/size) 2=316/0-3-0, 4=241/0-1-8 Max Horz 2=80(LC 8) Max Uplift 2=-160(LC 8), 4=-135(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

 Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=160, 4=135.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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0-6-8

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x6 SP No.2

REACTIONS. (lb/size) 2=249/0-3-0, 4=155/0-1-8 Max Horz 2=74(LC 8) Max Uplift 2=-123(LC 8), 4=-93(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=123.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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May 24,2019



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) All bearings are assumed to be User Defined crushing capacity of 565 psi.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 13, 11 except (jt=lb) 14=156, 10=152.

Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
This manufactured truss is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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