

PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

MEAN ROOF HEIGHT 25'-8	HEIGHT TO	RIDGE 30'-0"	
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A
FENESTRATION U-FACTOR	0.35	0.35	0.35
SKYLIGHT U-FACTOR	0.55	0.55	0.55
GLAZED FENESTRATION SHGC	0.30	0.30	0.30
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci
WALL R-VALUE	15	15	19
FLOOR R-VALUE	19	19	30
* BASEMENT WALL R-VALUE	5/13	10/15	10/15
** SLAB R-VALUE	0	10	10
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19

* "10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION ** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL DESIGNED FOR WIND SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B"

COMPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS

MEAN ROOF UP TO 30' 30'-1" TO 35' 35'-1" TO 40' 40'-1" TO 45'

ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIN	ID SPEED	OF 130 MF	H, 3 SECO	OND GUST	(101 FAS	TEST MILE	E) EXPOSU	IRE "B"
COMPONENT	& CLA	DDING	DESIG	NED FC	R THE	FOLLO	WING I	LOADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE 5	18.2	-24.0	19.1	-25.2	19.8	-26.2	20.4	-26.9

AIR LEAKAGE

N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code:

- 1. Blocking and sealing floor/ceiling systems and under knee walls open to unconditioned or exterior space.
- 2. Capping and sealing shafts or chases, including flue shafts. 3. Capping and sealing soffit or dropped ceiling areas.

ROOF VENTILATION

R806.1 Ventilation required. Enclosed *attics* and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware cloth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7.

R806.2 Minimum area. The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1/300 when a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.

1. Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only. 2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 1558 SQ.FT. NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 10.39 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 5.16 SQ.FT.

GUARD RAIL NOTES

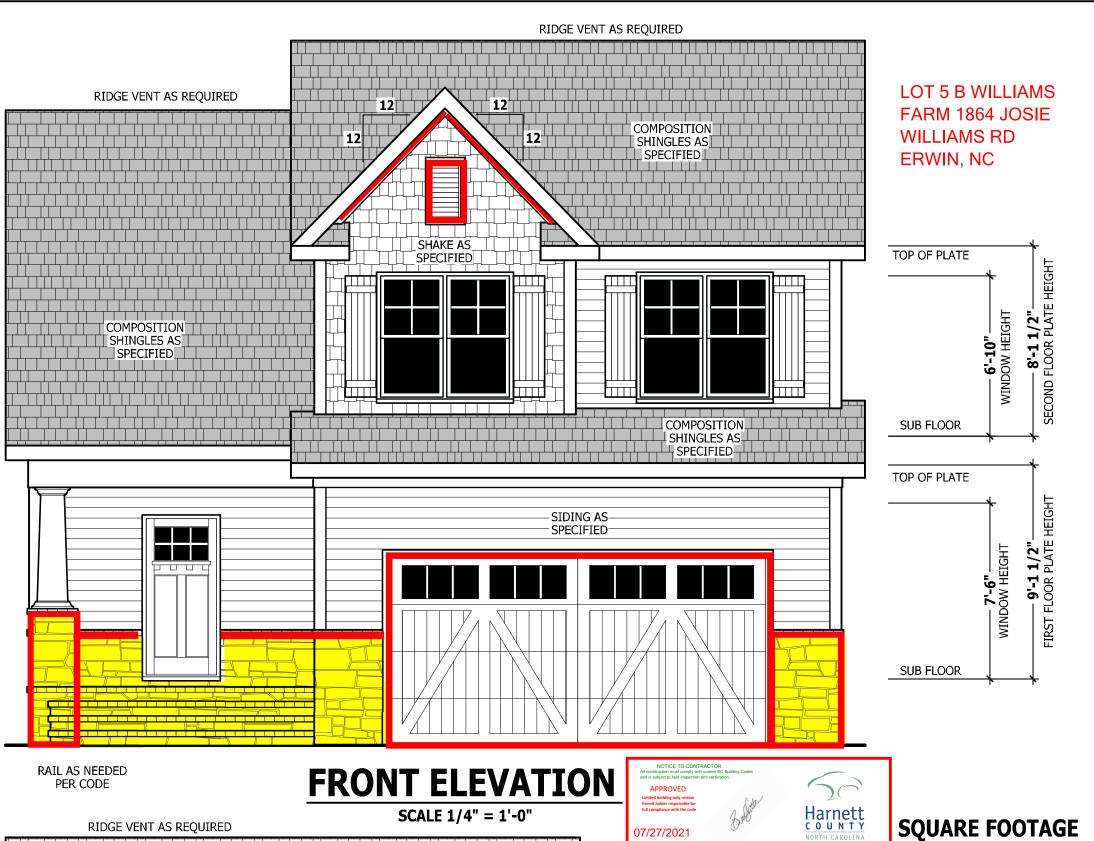
R312.1 Where required. *Guards* shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or *grade* below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a *guard*.

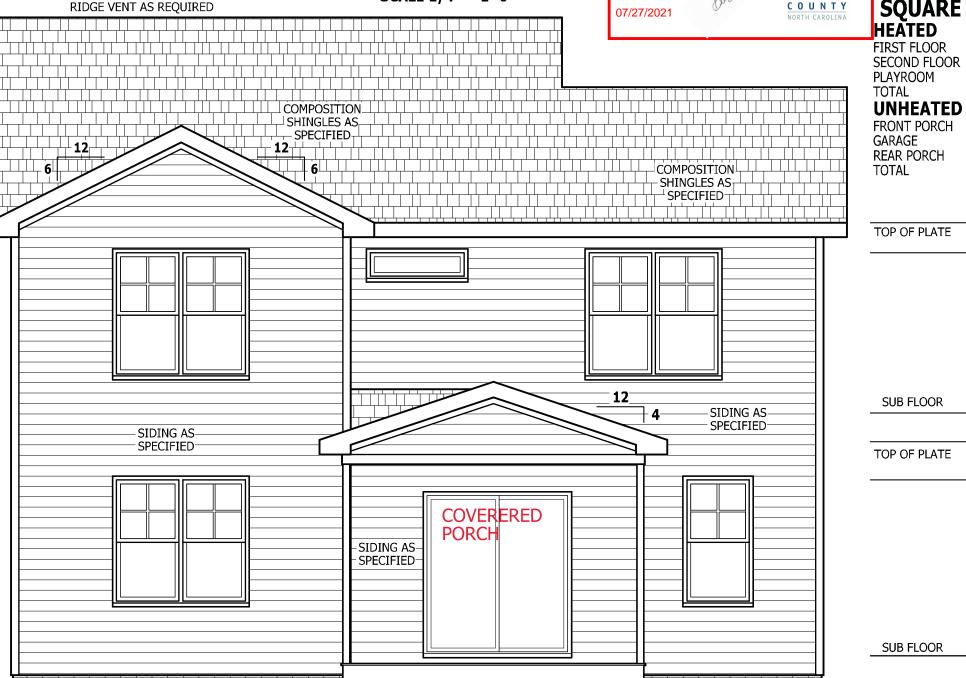
R312.2 Height. Required *guards* at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads. **Exceptions:**

- 1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the
- 2. Where the top of the *guard* also serves as a handrail on the open sides of stairs, the top of the *guard* shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations. Required *guards* shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter. **Exceptions:**

- 1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a *guard*, shall not allow passage of a sphere 6 inches (153 mm) in diameter.
- 2. *Guards* on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.





REAR ELEVATION

SCALE 1/4" = 1'-0"

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DESIGNER, ARCHITECT OR BEFORE CONSTRUCTION. AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER

REAR ELEVATIONS GASTON

Ø 里 **FRONT**

764 SQ.FT.

280 SQ.FT.

1820 SQ.FT.

101 SQ FT

466 SQ.FT.

152 SQ.FT. 719 SQ.FT.

SECOND FLOO

— 9'-1 1/2"-- FLOOR PLATE

SQUARE FOOTAGE HEATED FIRST FLOOR SECOND FLOOR PLAYROOM 776 SQ.FT. 764 SQ.FT. 280 SQ.FT. 1820 SQ.FT. TOTAL UNHEATED FRONT PORCH GARAGE REAR PORCH

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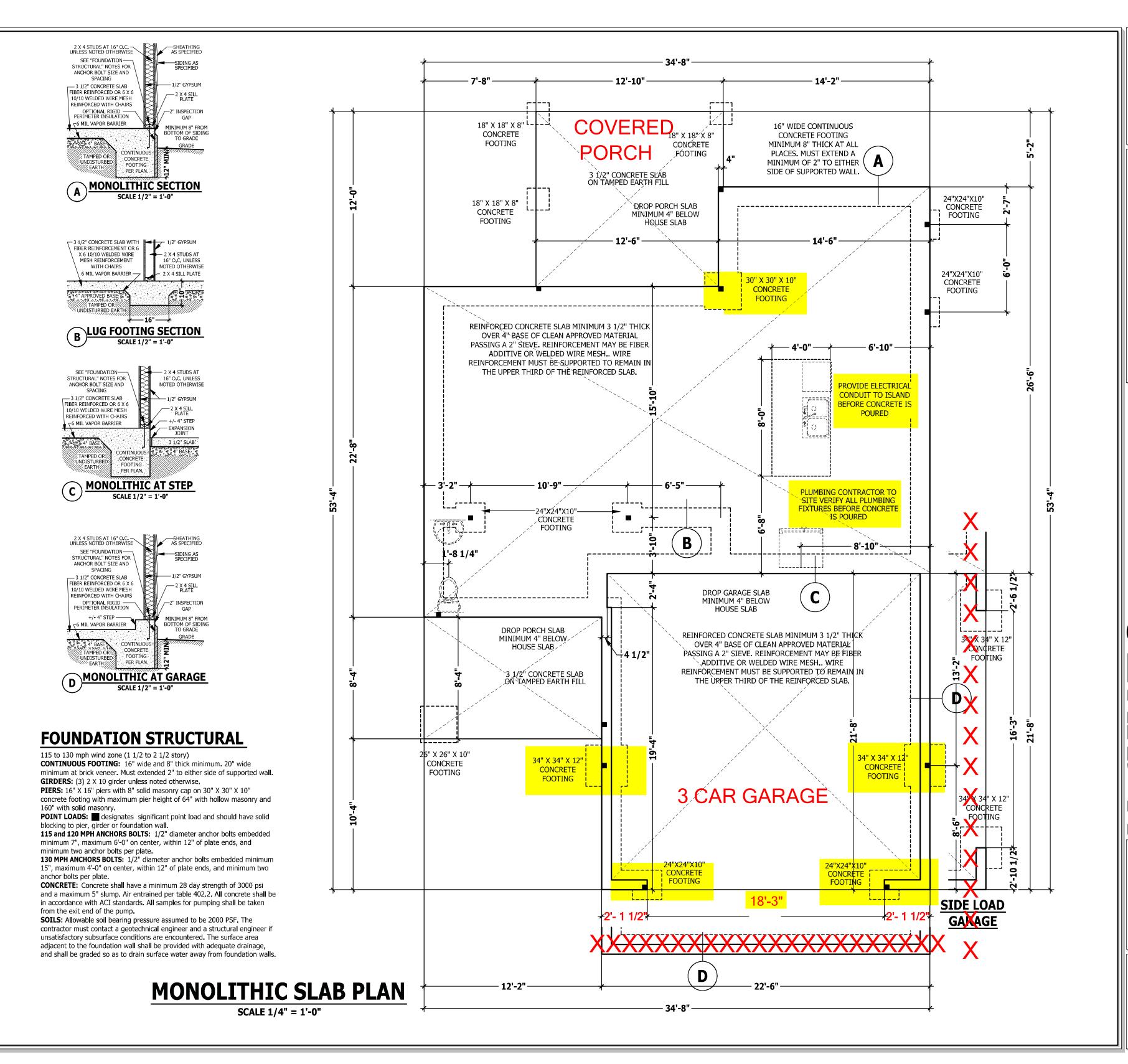
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PLAN STON SLAB MONOLITHIC 4 U Ш

SQUARE FOOTAGE
HEATED
FIRST FLOOR 776 SO.FT.
SECOND FLOOR 764 SO.FT.
PLAYROOM 280 SO.FT.
TOTAL 1893 SO.FT. 776 SQ.FT. 764 SQ.FT. 280 SQ.FT. 1820 SQ.FT. TOTAL

UNHEATED

FRONT PORCH
GARAGE
REAR PORCH
TOTAL

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ATTIC ACCESS

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

- 1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.
- 2. Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.

WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for

Interior walls are drawn as 3 1/2" or as noted 2 X 6 are drawn as 5 1/2", and do not include gypsum.

DWELLING / GARAGE SEPARATION

REFER TO SECTIONS R302.5, R302.6, AND R302.7

WALLS. A minimum 1/2" gypsum board must be installed on all walls supporting floor/ceiling assemblies used for separation required by this section. **STAIRS.** A minimum of 1/2" gypsum board must be installed on the underside and

exposed sides of all stairways. CEILINGS. A minimum of 1/2" gypsum must be installed on the garage ceiling if there are no habitable room above the garage. If there are habitable room above the garage a minimum of 5/8" type X gypsum board must be installed on the garage ceiling.

OPENING PENETRATIONS. Openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute

DUCT PENETRATIONS. Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings

OTHER PENETRATIONS. Penetrations through the separation required in Section R302.6 shall be protected as required by Section R302.11, Item 4.

EXTERIOR WINDOWS AND DOORS

R612.1 General. This section prescribes performance and construction requirements for exterior windows and doors installed in walls. Windows and doors shall be installed and flashed in accordance with the fenestration manufacturer's written installation instructions. Window and door openings shall be flashed in accordance with Section R703.8. Written installation instructions shall be provided by the fenestration manufacturer for each window

R612.2 Window sills. In *dwelling* units, where the opening of an operable window is located more than 72 inches (1829 mm) above the finished *grade* or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches (610 mm) above the finished floor of the room in which the window is located. Operable sections of windows shall not permit openings that allow passage of a 4 inch (102 mm) diameter sphere where such openings are located within 24 inches (610 mm) of the finished floor. Exceptions:

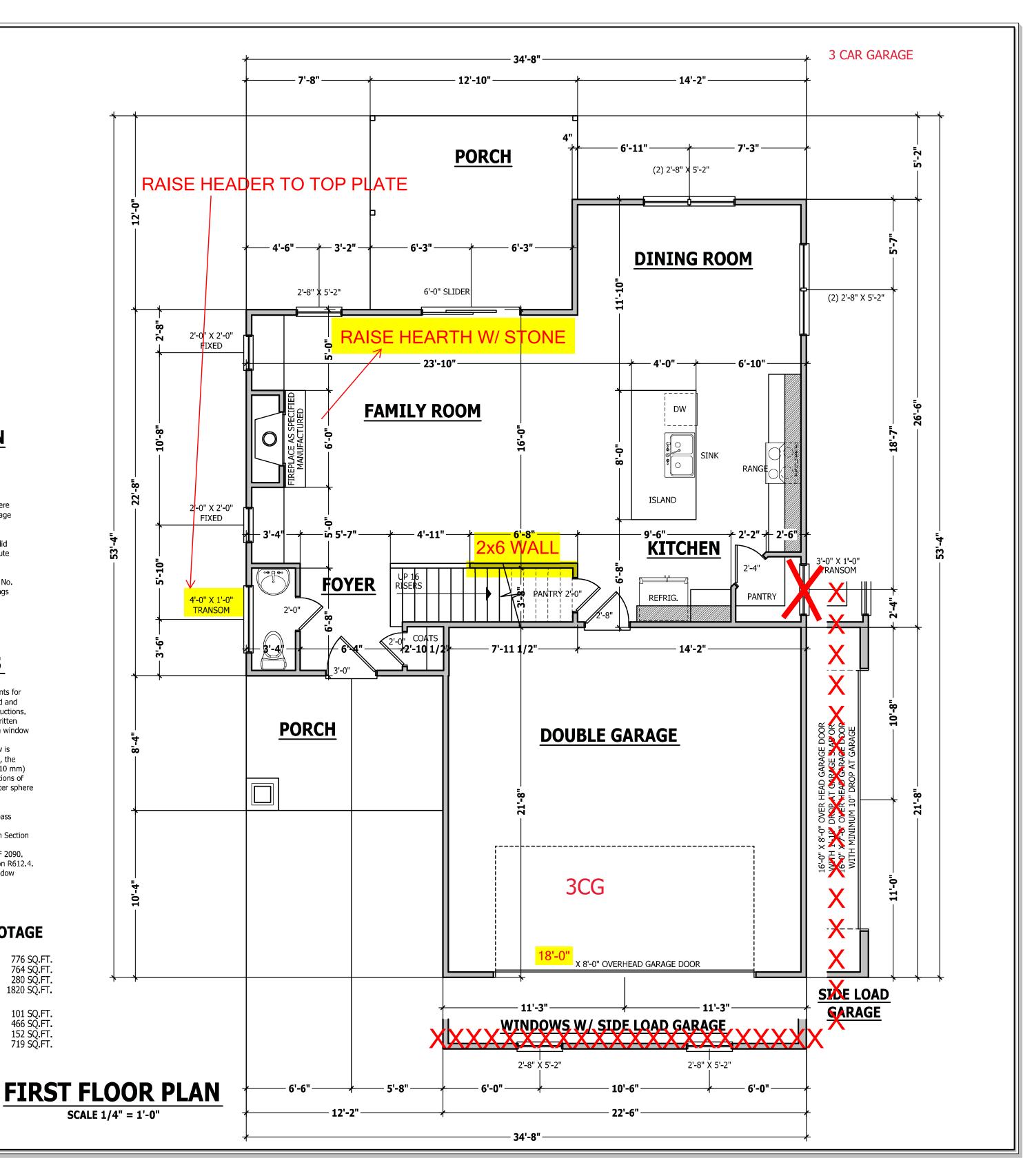
- 1. Windows whose openings will not allow a 4-inch diameter (102 mm) sphere to pass through the opening when the opening is in its largest opened position.
- 2. Openings that are provided with window fall prevention devices that comply with Section
- 3. Openings that are provided with fall prevention devices that comply with ASTM F 2090. 4. Windows that are provided with opening limiting devices that comply with Section R612.4. **R612.3 Window fall prevention devices.** Window fall prevention devices and window guards, where provided, shall comply with the requirements of ASTM F 2090.

SQUARE FOOTAGE HEĂTED

FIRST FLOOR SECOND FLOOR 764 SQ.FT. PLAYROOM 280 SQ.FT. 1820 SQ FT.

UNHEATED FRONT PORCH

101 SQ.FT. 466 SQ.FT. GARAGE REAR PORCH 152 SQ.FT. TOTAL 719 SQ.FT.



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CODES AND CONDITIONS MAY

FIRST FLOOR PLAN STON GA Ш

FIRST FLOOR SECOND FLOOR PLAYROOM

TOTAL

UNHEATED

FRONT PORCH
GARAGE
REAR PORCH
TOTAL

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STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. **FLOOR SHEATHING:** OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick.

CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE - KING STUDS EACH END PER TABLE BELOW

HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE
- NON LOAD BEARING HEADERS TO BE LADDER FRAMED

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length

for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

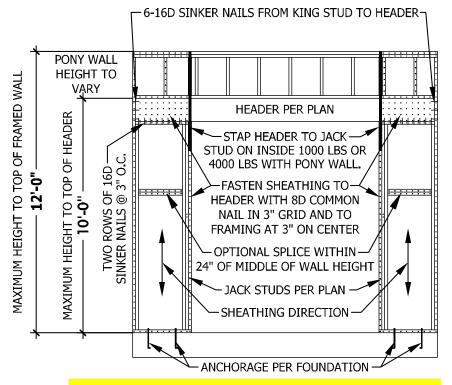
HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter).

CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing nailes

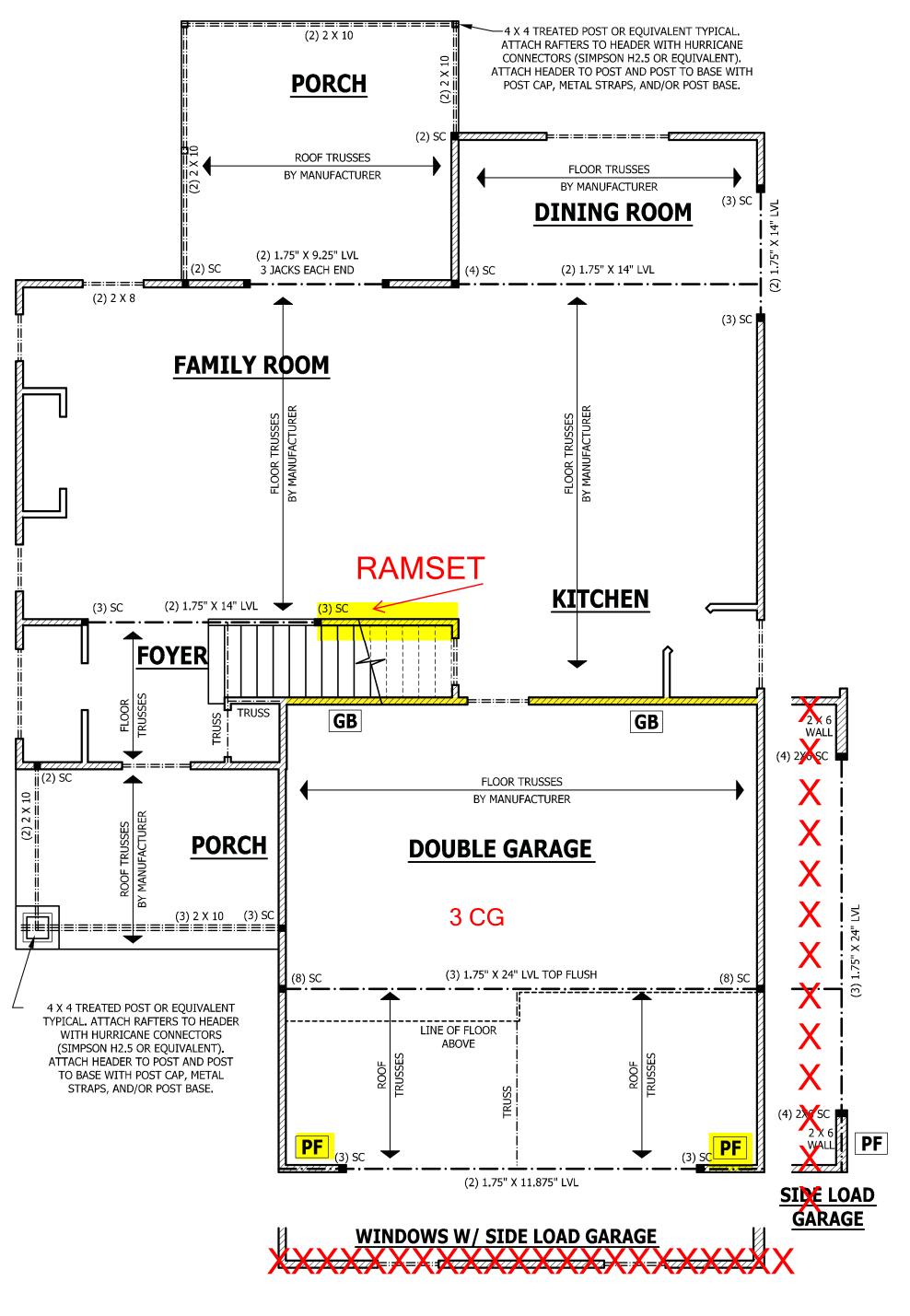
GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. **PF:** Portal fame per figure R602.10.1



PF PORTAL FRAME AT OPENING

(METHOD PF PER FIGURE AND SECTION R602,10,1)

SCALE 1/4" = 1'-0"



FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0"

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DIMENSIONS AND CONDITIONS
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STRUCTURAL

FLOOR

FIRST

SQUARE FOOTAGE HEATED

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FIRST FLOOR SECOND FLOOR PLAYROOM

TOTAL UNHEATED

FRONT PORCH GARAGE REAR PORCH TOTAL

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DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTIO
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. **FLOOR SHEATHING:** OSB or CDX floor sheathing

minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. **ROOF SHEATHING:** OSB or CDX roof sheathing minimum

CONCRETE AND SOILS: See foundation notes.

ATTIC ACCESS

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access. 2. Pull down stair treads, stringers, handrails, and hardware may

Exceptions: 1. Concealed areas not located over the main structure including

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END **UNLESS NOTED OTHERWISE** - KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END **UNLESS NOTED OTHERWISE** - NON LOAD BEARING HEADERS TO BE LADDER FRAMED

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

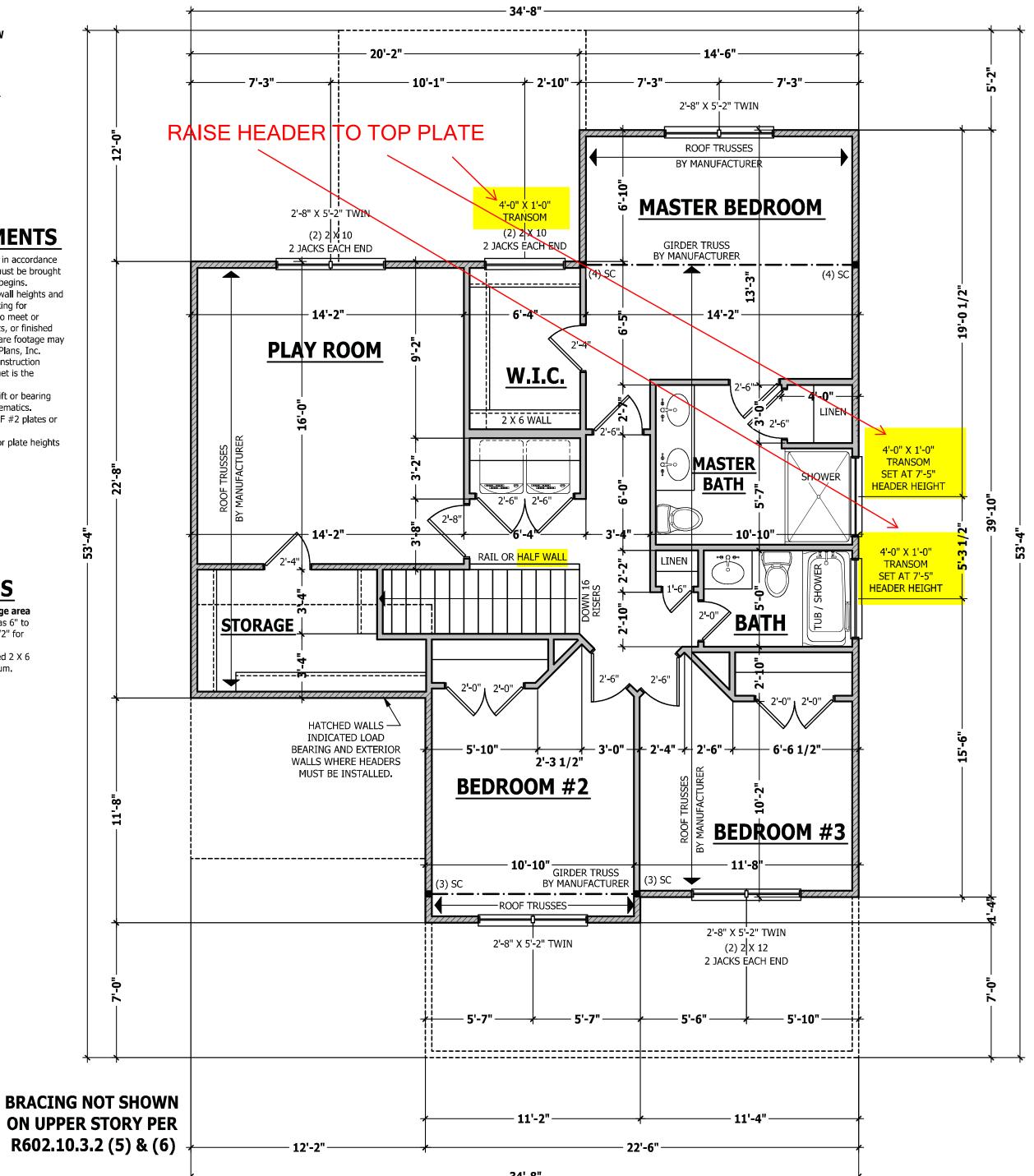
Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for

are drawn as 5 1/2", and do not include gypsum.

Interior walls are drawn as 3 1/2" or as noted 2 X 6



EXTERIOR WINDOWS AND DOORS

protrude into the net clear opening.

R612.1 General. This section prescribes performance and construction requirements for exterior windows and doors installed in walls. Windows and doors shall be installed and flashed in accordance with the fenestration manufacturer's written installation instructions. Window and door openings shall be flashed in accordance with Section R703.8. Written installation instructions shall be provided by the fenestration manufacturer for each window

R612.2 Window sills. In *dwelling* units, where the opening of an operable window is located more than 72 inches (1829 mm) above the finished *grade* or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches (610 mm) above the finished floor of the room in which the window is located. Operable sections of windows shall not permit openings that allow passage of a 4 inch (102 mm) diameter sphere where such openings are located within 24 inches (610 mm) of the finished floor. Exceptions:

1. Windows whose openings will not allow a 4-inch diameter (102 mm) sphere to pass through the opening when the opening is in its largest opened position. 2. Openings that are provided with window fall prevention devices that comply with Section

3. Openings that are provided with fall prevention devices that comply with ASTM F 2090. 4. Windows that are provided with opening limiting devices that comply with Section R612.4. R612.3 Window fall prevention devices. Window fall prevention devices and window guards, where provided, shall comply with the requirements of ASTM F 2090.

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FIRST FLOOR SECOND FLOOR PLAYROOM

TOTAL UNHEATED

FRONT PORCH GARAGE REAR PORCH

SCALE 1/4" = 1'-0"

SECOND FLOOR PLAN

PURCHASER MUST VERIFY ALL BEFORE CONSTRUCTION BEGIN: HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES. CODES AND CONDITIONS MAY

VARY WITH LOCATION, A LOCA

DESIGNER, ARCHITECT OR NGINEER SHOULD BE CONSULTE

BEFORE CONSTRUCTION.

THESE DRAWING ARE

INSTRUMENTS OF SERVICE AND

AS SUCH SHALL REMAIN
PROPERTY OF THE DESIGNER.

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STON

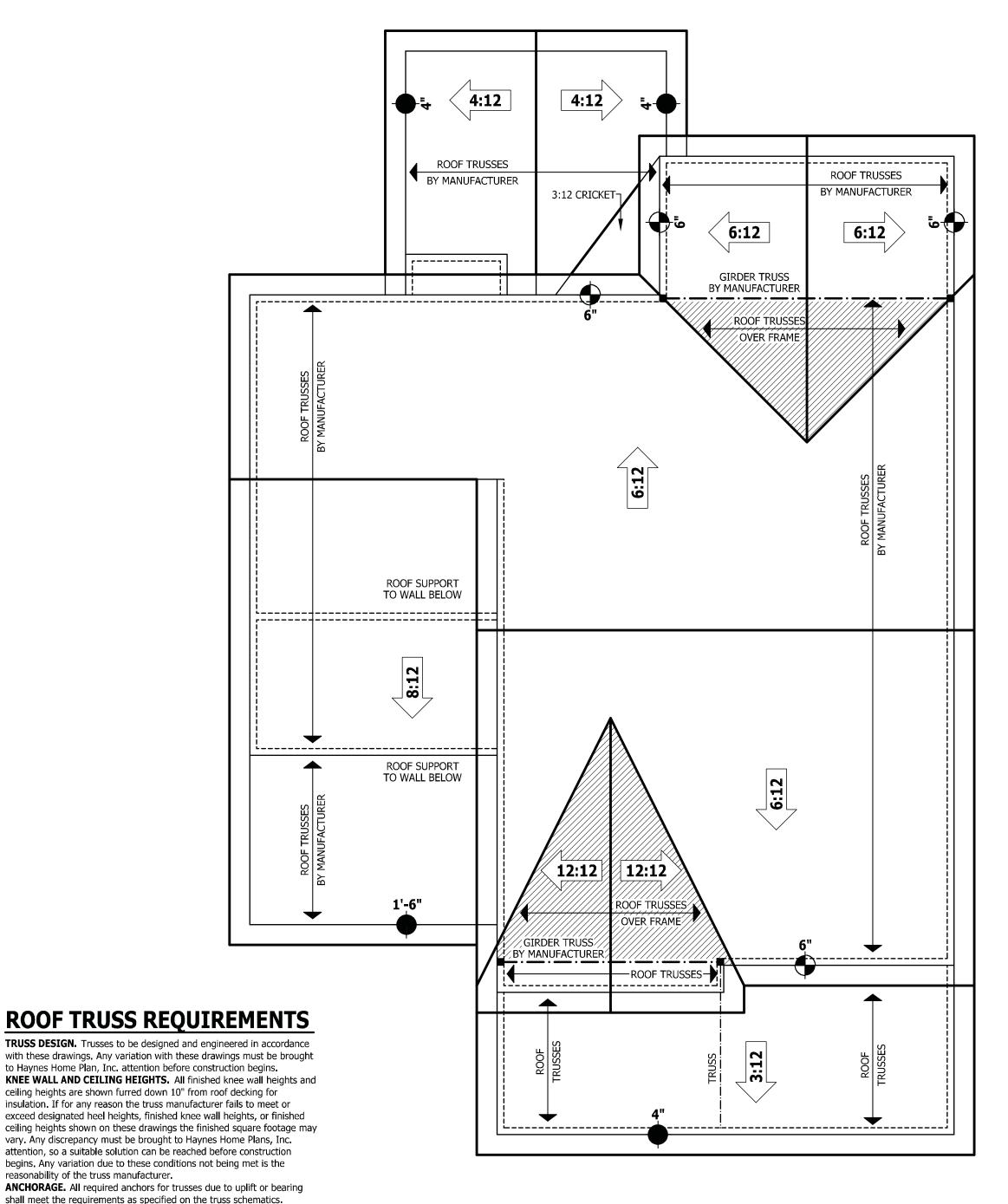
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FLOOR PLAN

SECOND



ROOF PLAN SCALE 1/4" = 1'-0"

BEARING. All trusses shall be designed for bearing on SPF #2 plates or

Plate Heights & Floor Systems. See elevation page(s) for plate heights

HEEL HEIGHT ABOVE SECOND FLOOR PLATE

ledgers unless noted otherwise.

and floor system thicknesses.

HEEL HEIGHT ABOVE

FIRST FLOOR PLATE

PURCHASER MUST VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE CONSTRUCTION BEGINS. HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES.

CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR ENGINEER SHOULD BE CONSULTED BEFORE CONSTRUCTION. THESE DRAWING ARE INSTRUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER.

GASTON **ROOF PLAN**

里

SQUARE FOOTAGE
HEATED
FIRST FLOOR 776 SQ.FT.
SECOND FLOOR 764 SQ.FT.
PLAYROOM 280 SQ.FT.
TOTAL 776 SQ.FT. 764 SQ.FT. 280 SQ.FT. 1820 SQ.FT.

TOTAL

UNHEATED

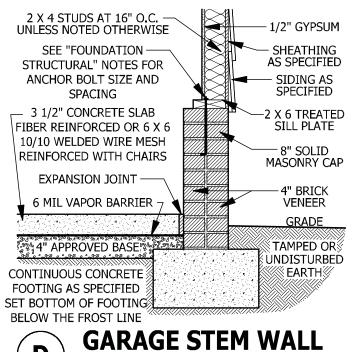
FRONT PORCH
GARAGE
REAR PORCH
TOTAL

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5/19/2020

181035B

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SCALE 3/4" = 1'-0"

DECK STAIR NOTES

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step cut and back of stringer. If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

DECK BRACING

AM109,1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to provide lateral stability.

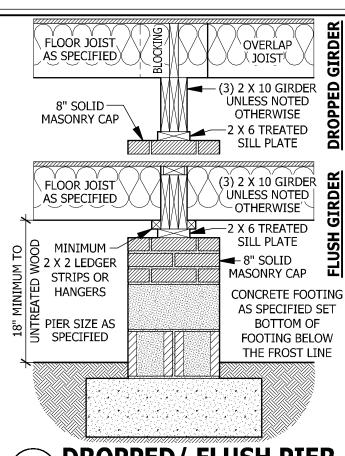
AM109.1.1. When the deck floor height is less than 4'-0" above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

AM109.1.2. 4 x 4 wood knee braces may be provided on each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be bolted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the brace per Figure AM109.1

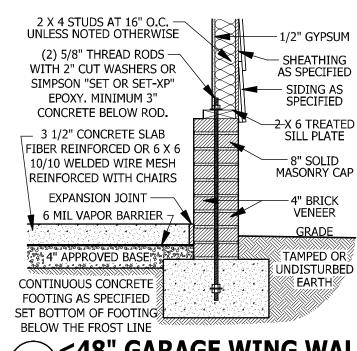
AM109.1.3. For freestanding decks without knee braces or AS SPECIFIED diagonal bracing, lateral stability may be provided by embedding the post in accordance with Figure AM109.2

and the following:									
POST SIZE	MAX TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER					
4 X 4	48 SF	4'-0"	2'-6"	1'-0"					
6 X 6	120 SF	6'-0"	3'-6"	1'-8"					

AM109.1.4. 2 x 6 diagonal vertical cross bracing may be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109.3. AM109.1.5. For embedment of piles in Coastal Regions, see Chapter 45.



DROPPED/FLUSH PIER



<48" GARAGE WING WALL **SCALE 3/4" = 1'-0"**

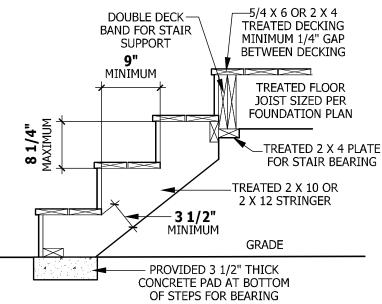


FIGURE AM110 TYPICAL DECK STAIR DETAIL

SHEATHING

LATH-

SEE FOUNDATION

FOR FOUNDATION

DETAILS

WEEP SCREED

SCALE 3/4" = 1'-0"

SCALE 3/4" = 1'-0"

STONE VEENER

AS SPECIFIED

VAPOR BARRIER

WEEP SCREED

MINIMUM 4" TO

GROUND OR 2"

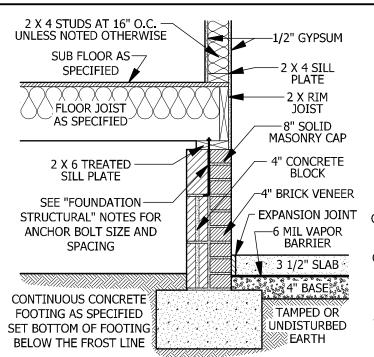
-TO PAVEMENT

GRADE

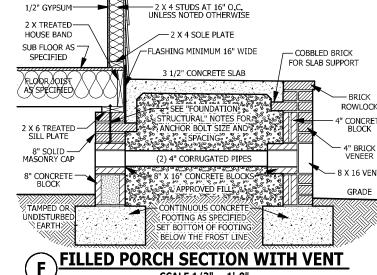
WEEP SCREEDS

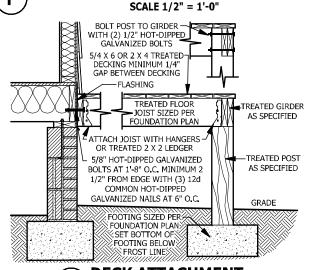
All weep screeds and stone veneer to be installed per manufactures instructions and per the 2012 North Carolina Residential Building code. **R703.6.2.1 -** A minimum 0.019-inch (0.5

mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic weep screed, with a minimum vertical attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier shall lap the attachment flange. The exterior lath shall cover and terminate on the attachment flange of the weep screed.



CRAWL SPACE AT GARAGE SCALE 3/4" = 1'-0"





G DECK ATTACHMENT SCALE 1/2" = 1'-0"

SMOKE ALARMS

R314.1 Smoke detection and notification. All smoke alarms shall be listed in accordance with UL 217 and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

R314.2 Smoke detection systems. Household fire alarm systems a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification device(s), it shall become a permanent fixture of the occupancy and owned by the homeowner. The system shall be monitored by an approved supervising station and be maintained in accordance with

Exception: Where smoke alarms are provided meeting the requirements of Section R314.4.

R314.3 Location. Smoke alarms shall be installed in the following

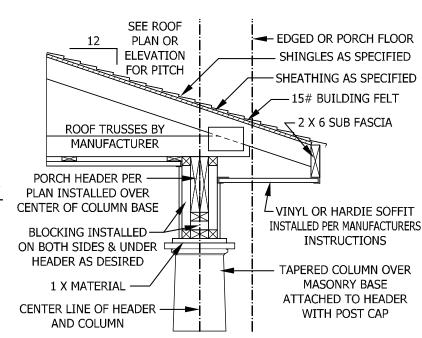
locations 1. In each sleeping room.

2. Outside each separate sleeping area in the immediate vicinity of

the bedrooms. 3. On each additional *story* of the *dwelling*, including *basements* and habitable attics (finished) but not including crawl spaces, uninhabitable (unfinished) attics and uninhabitable (unfinished) attic-stories. In *dwellings* or *dwelling units* with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full *story* below the upper level.

When more than one smoke alarm is required to be installed within an individual *dwelling* unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit.

R314.4 Power source. Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commercial source, and when primary power is interrupted, shall receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected.



PORCH HEADER WITH TAPERED COLUMN

SCALE 3/4" = 1'-0"

CARBON MONOXIDE ALARMS

R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed

R315.2 Where required in existing dwellings. In existing dwellings, where interior alterations, repairs, fuel-fired appliance replacements, or additions requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

R315.3 Alarm requirements. The required carbon monoxide alarms shall be audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with UL 2034 and shall be installed in accordance with this code and the manufacturer's installation instructions.

STAIRWAY NOTES

R311.7.2 Headroom. The minimum headroom in all parts of the stairway shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stairway.

R311.7.4 Stair treads and risers. Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. **R311.7.4.1 Riser height.** The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of the adjacent treads.

R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229 installed in accordance with NFPA 72 that include smoke alarms, or mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a minimum tread depth of 4 inches (102 mm) at any point.

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each continuous run of treads or flight with four or more risers.

R311.7.7.1 Height. Handrail height, measured vertically from the sloped plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm).

1. The use of a volute, turnout or starting easing shall be allowed over the

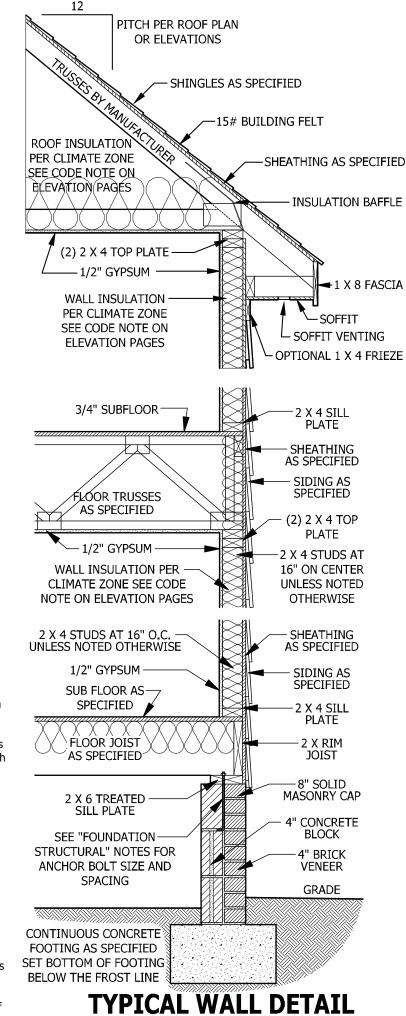
lowest tread. 2. When handrail fittings or bendings are used to provide continuous transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall be permitted to exceed the maximum height.

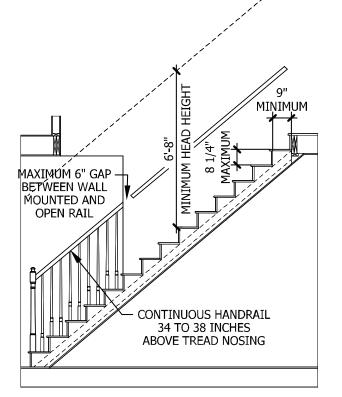
R311.7.7.2 Continuity. Handrails for stairways shall be continuous for the full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails adjacent to a wall shall have a space of not less than 11/2 inch (38 mm) between the wall and the handrails.

Exceptions 1. Handrails shall be permitted to be interrupted by a newel post. 2. The use of a volute, turnout, starting easing or starting newel shall be

allowed over the lowest tread.

3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.





SCALE 3/4" = 1'-0"

TYPICAL STAIR DETAIL

PURCHASER MUST VERIFY ALI BEFORE CONSTRUCTION BEGIN: HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES. CODES AND CONDITIONS MA VARY WITH LOCATION, A LOCA

DESIGNER, ARCHITECT OR NGINEER SHOULD BE CONSULTE BEFORE CONSTRUCTION. THESE DRAWING ARE INSTRUMENTS OF SERVICE AN AS SUCH SHALL REMAIN

PROPERTY OF THE DESIGNER

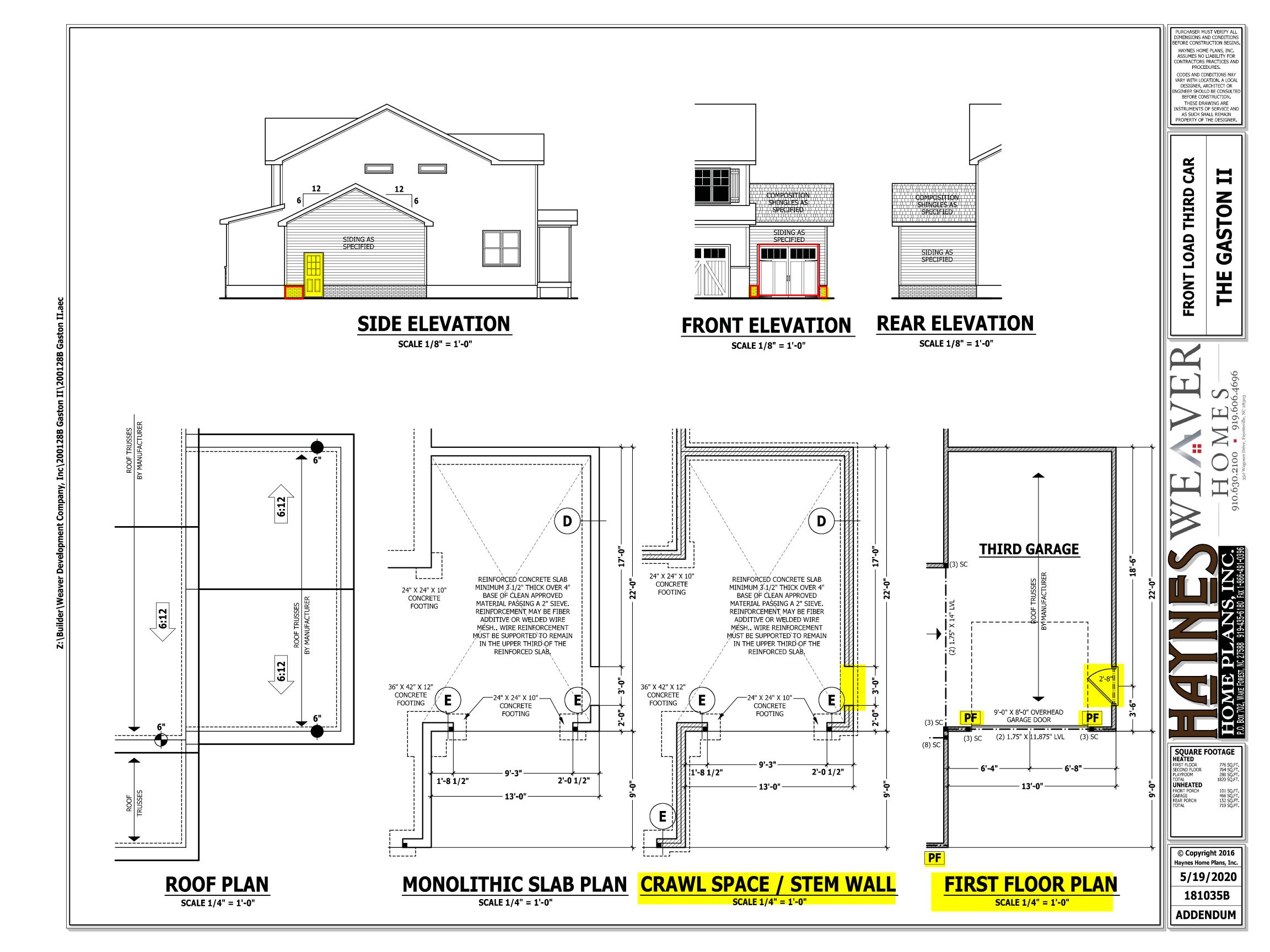
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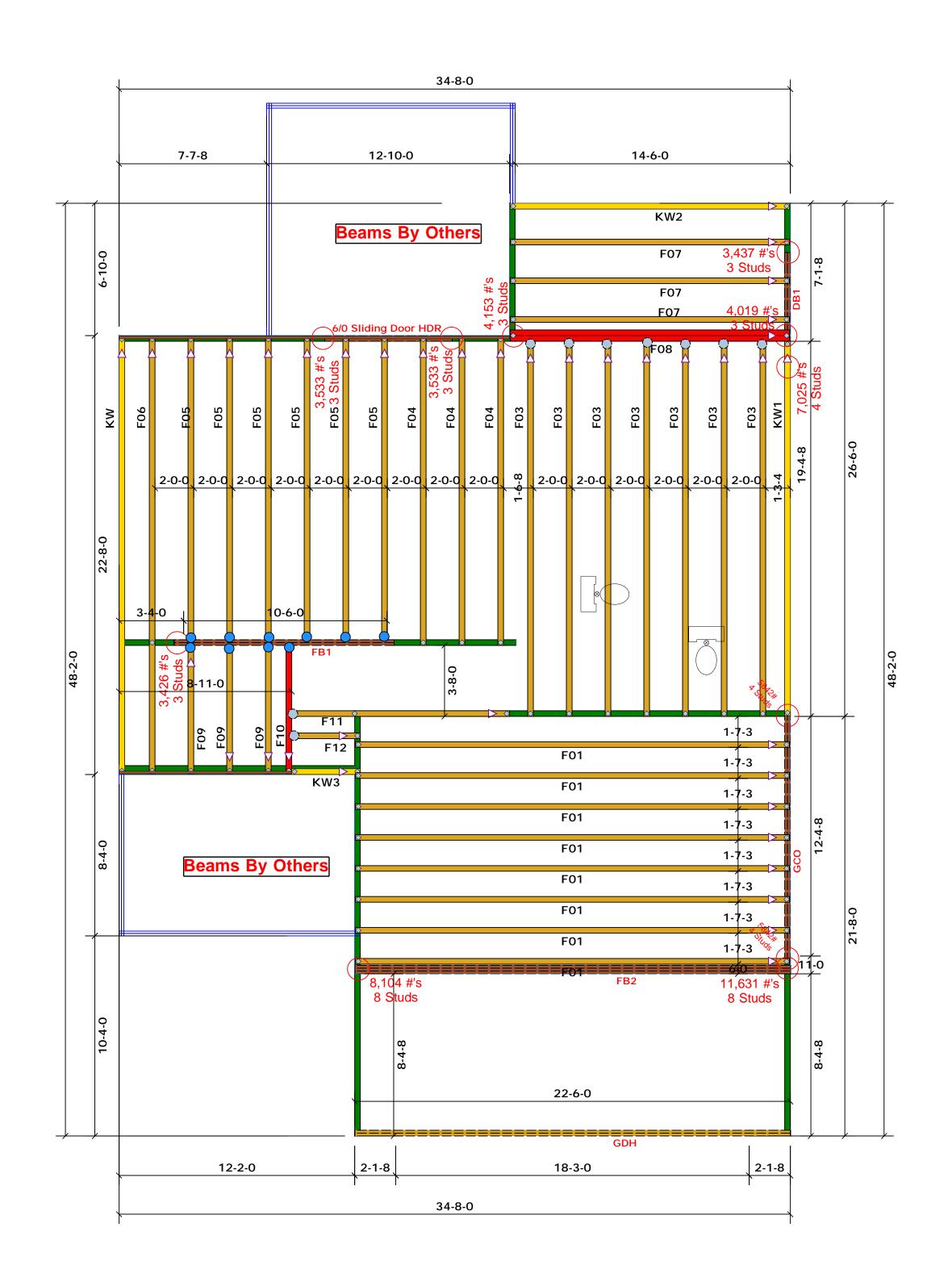
TYPICAL U Ш I

SQUARE FOOTAGE HEATED 776 SQ FT 764 SQ FT 280 SQ FT 1820 SQ FT FIRST FLOOR SECOND FLOOR PLAYROOM TOTAL UNHEATED FRONT PORCH GARAGE REAR PORCH 101 SQ FT 466 SQ FT 152 SQ FT 719 SQ FT

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All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs

	HUS410	USP	10	NA	16d/3-1/2"	16d/3-1/2"
	MSH422	USP	9	Varies	10d/3"	10d/3"

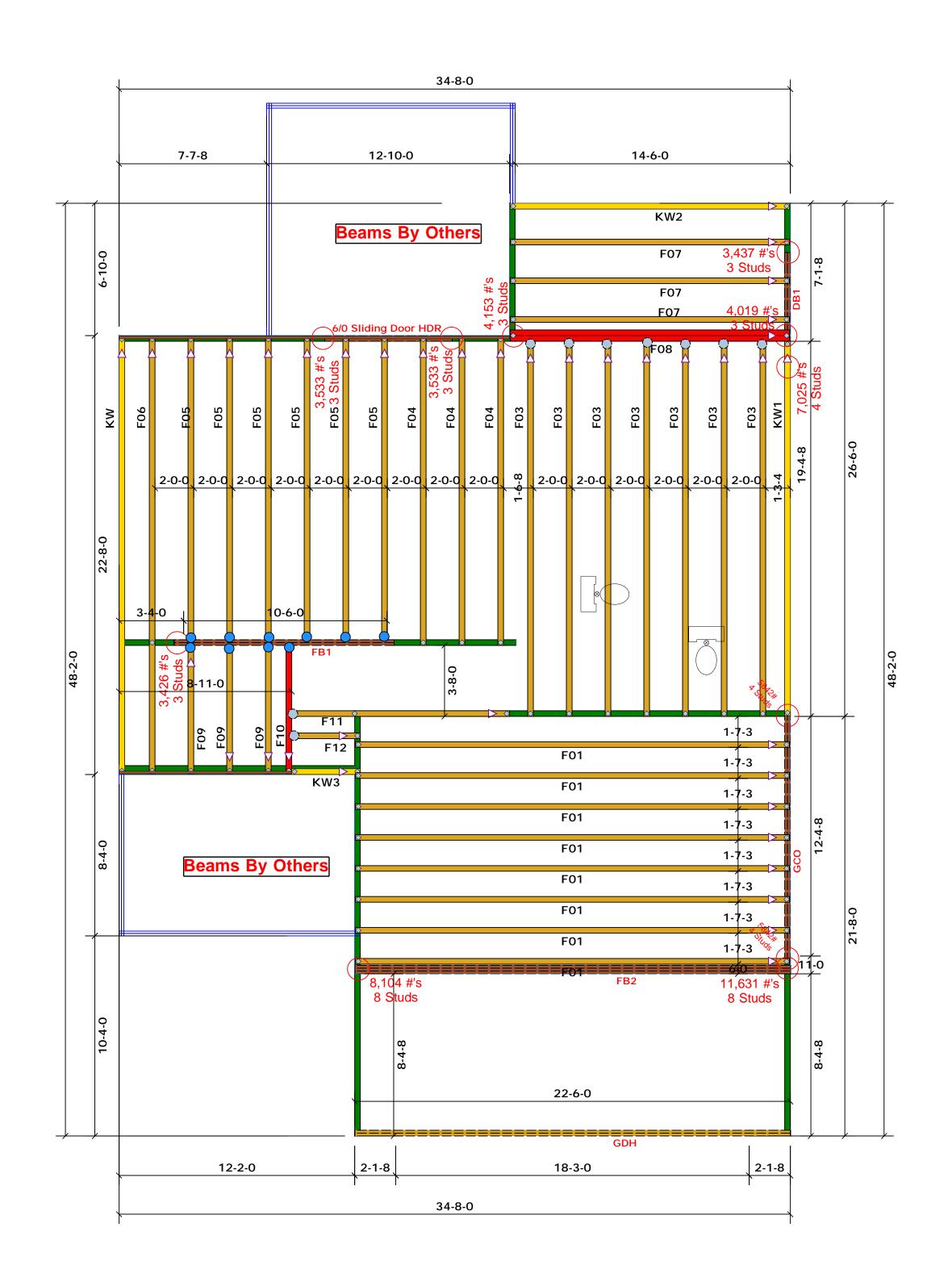
		Products		
PlotID	Length	Product	Plies	Net Qty
6/0 Sliding Door HDR	7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH	23-0-0	1-3/4"x 14" LVL Kerto-S	2	2
GCO	14-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB1	12-0-0	1-3/4"x 14" LVL Kerto-S	2	2
DB1	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB2	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

(04)	HART FOR JA FIDON LABLES R502.5 JACK STUDG SCOURS	5(1) 4 (6())	BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incor the building designer. See individual building designer. See individual sheets for each truss design identified on the placement drawing. The build
S SON	FEADERVEROER 2 9 5 5 6 9 7	9 9 8 FOR ASER	JOB NAME	Lot 5B Williams Farm	ADDRESS	Lot 5B Williams Farm	is responsible for temporary and permanent bracing of the roof and floor sy the overall structure. The design of the truss support structure including he walls, and columns is the responsibility of the building designer. For gener regarding bracing, consult BCSI-B1 and BCSI-B2 provided with the truss de
DAN GAN TRO DE TRO De Tro De Tro De Tro De De De De De De De De De De De De De D	By Design	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	PLAN	Gaston II (181035B) 3Car	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply prescriptive Code requirements. The contractor shall refer to the attractor shall refer to the
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	N/A	DATE REV.	/ /	(derived from the prescriptive Code requirements) to determine the foundation size and number of wood studs required to support react than 3000# but not greater than 15000#. A registered design profess be retained to design the support system for any reaction that excee
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Marshall Naylor	specified in the attached Tables. A registered design professional she retained to design the support system for all reactions that exceed 1
11900 7 13600 8			JOB #	J0521-2777	SALESMAN	Lenny Norris	signature Marshall Naylor





All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs

	HUS410	USP	10	NA	16d/3-1/2"	16d/3-1/2"
	MSH422	USP	9	Varies	10d/3"	10d/3"

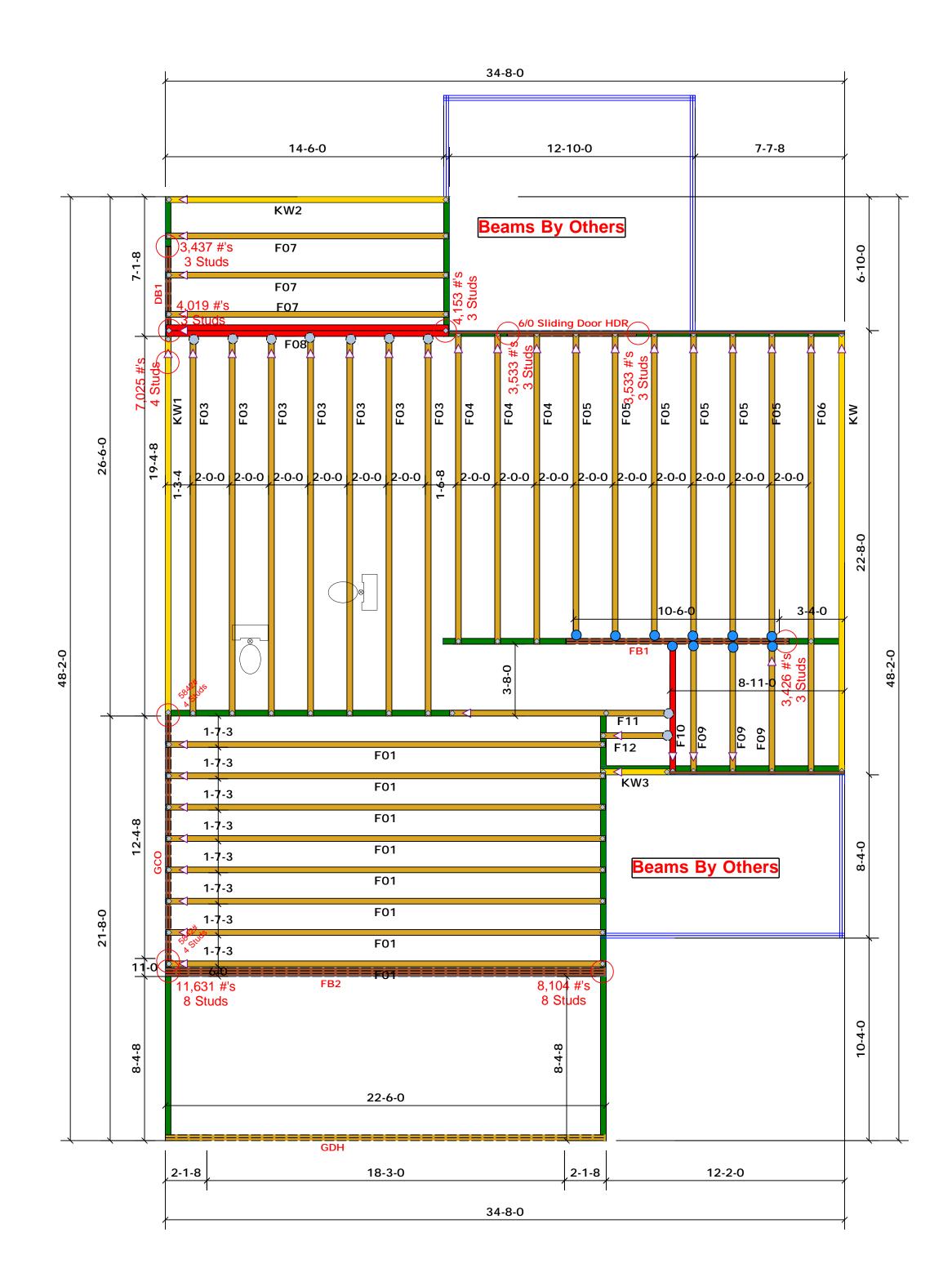
		Products		
PlotID	Length	Product	Plies	Net Qty
6/0 Sliding Door HDR	7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH	23-0-0	1-3/4"x 14" LVL Kerto-S	2	2
GCO	14-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB1	12-0-0	1-3/4"x 14" LVL Kerto-S	2	2
DB1	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB2	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

(04)	HART FOR JA FIDON LABLES R502.5 JACK STUDG SCOURS	5(1) 4 (6())	BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incor the building designer. See individual building designer. See individual sheets for each truss design identified on the placement drawing. The build
S SON	FEADERVEROER 2 9 5 5 6 9 7	9 9 8 FOR ASER	JOB NAME	Lot 5B Williams Farm	ADDRESS	Lot 5B Williams Farm	is responsible for temporary and permanent bracing of the roof and floor sy the overall structure. The design of the truss support structure including he walls, and columns is the responsibility of the building designer. For gener regarding bracing, consult BCSI-B1 and BCSI-B2 provided with the truss de
DAN GAN TRO DE TRO De Tro De Tro De Tro De De De De De De De De De De De De De D	By Design	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	PLAN	Gaston II (181035B) 3Car	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply prescriptive Code requirements. The contractor shall refer to the attractor shall refer to the
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	N/A	DATE REV.	/ /	(derived from the prescriptive Code requirements) to determine the foundation size and number of wood studs required to support react than 3000# but not greater than 15000#. A registered design profess be retained to design the support system for any reaction that excee
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Marshall Naylor	specified in the attached Tables. A registered design professional she retained to design the support system for all reactions that exceed 1
11900 7 13600 8			JOB #	J0521-2777	SALESMAN	Lenny Norris	signature Marshall Naylor





All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs

HUS410	USP	10	NA	16d/3-1/2"	16d/3-1/2"
MSH422	USP	9	Varies	10d/3"	10d/3"

		Products		
PlotID	Length	Product	Plies	Net Qty
6/0 Sliding Door HDR	7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH	23-0-0	1-3/4"x 14" LVL Kerto-S	2	2
GCO	14-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB1	12-0-0	1-3/4"x 14" LVL Kerto-S	2	2
DB1	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB2	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS (045Fb ON 140Fb 98025()) 1-00() SUMMER OF JACK STUDS 4(3) (1805 B) (4 CM OF		BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUS These trusses are of the building design sheets for each trus	
z 3 £	PEADER/STROER	z (5 a	JOB NAME	Lot 5B Williams Farm	ADDRESS	Lot 5B Williams Farm	is responsible for to the overall structure walls, and columns regarding bracing, o
CND REACTED (0° TO) 3EQ D STUDS (BND PENCTIO COT AU SOUTH CIPSE BND - VARIO	OT 10) COT TO COT TO COT TO T	PLAN	Gaston II (181035B) 3Car	MODEL	Floor	or online @ sbcindu Bearing reactions prescriptive Code
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 ! 6600 2 10200 3	SEAL DATE	N/A	DATE REV.	/ /	(derived from the foundation size an than 3000# but no be retained to des
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Marshall Naylor	specified in the at retained to design
11900 7 13600 8 15300 o			JOB #	J0521-2777	SALESMAN	Lenny Norris	1

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 1500#

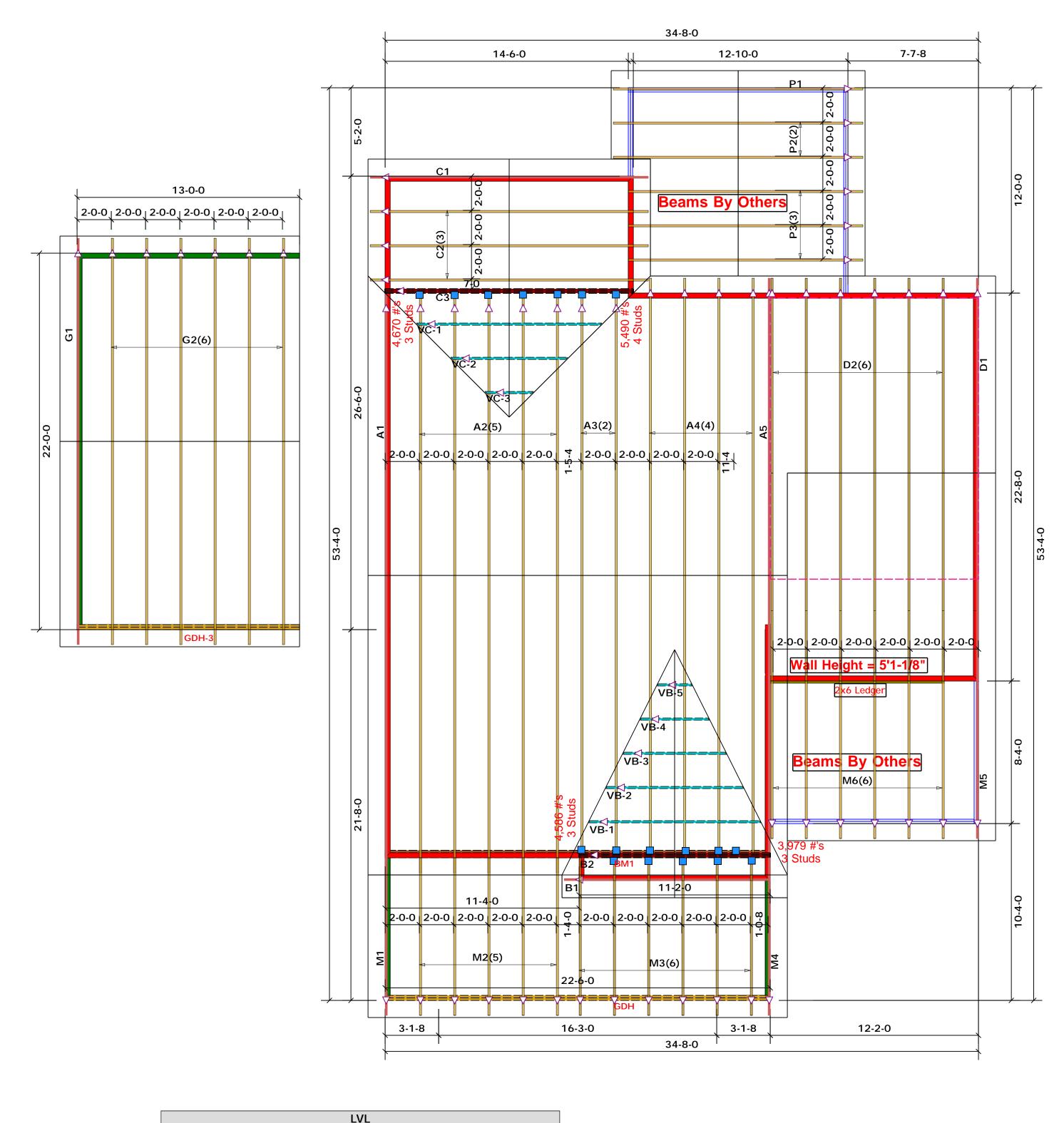
Marshall Naylor

ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

соттесн



LVL Net Qty Plies Product PlotID Length 2 1-3/4"x 11-7/8" LVL Kerto-S 13' 0" GDH-3

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs



= 1st Level Wall

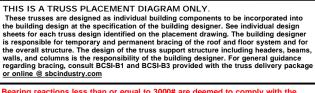
= 2nd Level Wall

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

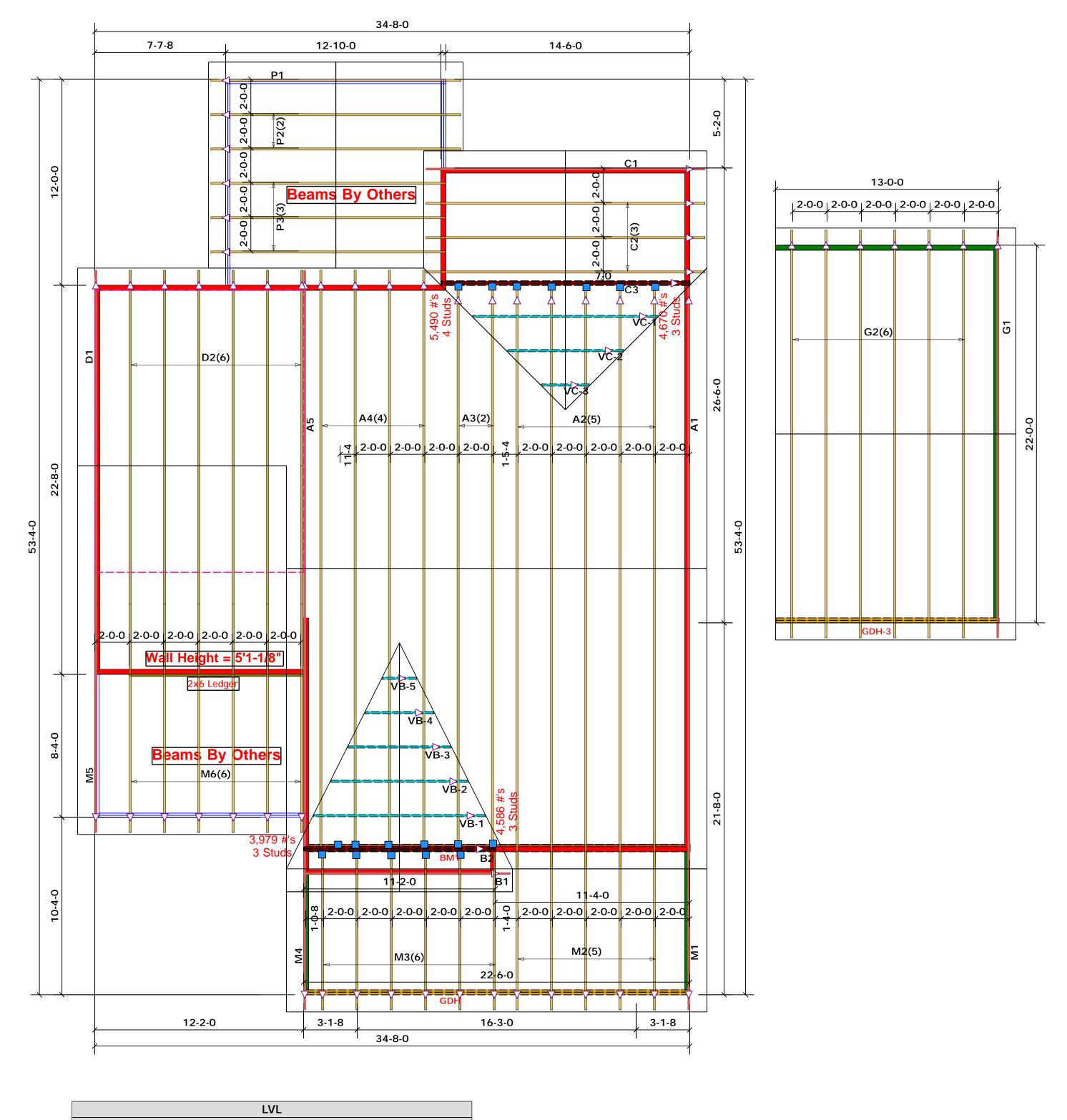
LO.	AD (CHAR	T FO	R J	4CK	STUD	5
			N TABLES				
No	nr(s c		STUBS A EADERN			A EMB OF	
CND REACTION (OT FU)	SEC DISTUDS FOR CORN HEADER		NOTION OF THE PROPERTY OF THE	REQUESTUDS FOR CORN - CARER		END NIACTION (0° TO)	REQ'D STUDS FOR (4) MY HEADER
1700	1		2550	1		3400	1
3400	2		5100	2		6600	2
5100	3		7650	3		10200	3
6800	4		10200	4		13600	4
8500	5		12750	5		17000	5
10200	á		15300	6			
11900	7						
13600	8						
15300	9						

BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS These tru the building
JOB NAME	OB NAME Lot 5B Williams Farm		Lot 5B Williams Farm	is respon the overa walls, and regarding
PLAN	Gaston II (181035B) 3Car	MODEL	Roof	or online Bearing of prescript
SEAL DATE	N/A	DATE REV.	/ /	(derived foundation than 3000 be retain
QUOTE #		DRAWN BY	Marshall Naylor	specified retained
JOB #	J0521-2776	SALESMAN	Lenny Norris	



Marshall Naylor

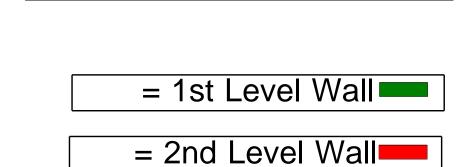




Net Qty Plies Product PlotID Length 2 1-3/4"x 11-7/8" LVL Kerto-S 13-0-0 GDH-3

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs



18 USP

HUS26

NA

16d/3-1/2" 16d/3-1/2"

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

LO.	AD (HAR	T FO	RJ	ACK S	STUD	5
			N LABLES				
NLA	MMC-5 C		ISTUDIO A FEADERIA			4 EVID 51	
END REACTION (07 PU)	SEC DISTURS FOR COMPANY HEADER		CUL ATO	MRQ IS STUDS FOR CIPAN - EMBER		IND 8.04TJCN (01.10)	REQUESTADS FOR (4) RLY HEADER
1700	1		2550	1		3400	1
3400	2		5100	2		6600	2
5100	3		7650	3		10200	3
6800	4		10200	4		13600	4
8500	5		12750	5		17000	5
10200	á		15300	6			
11900	7						
13600	8						
15300	9						

	BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS These true the building
	JOB NAME	Lot 5B Williams Farm	ADDRESS	Lot 5B Williams Farm	is respons the overall walls, and regarding
	PLAN	Gaston II (181035B) 3Car	MODEL	Roof	Bearing represcripti
-	SEAL DATE	AL DATE N/A		/ /	(derived foundation than 3000 be retained
	QUOTE #		DRAWN BY	Marshall Naylor	specified retained t
-	JOB #	J0521-2776	SALESMAN	Lenny Norris	



Marshall Naylor





Project: Address:

Weaver Homes

Gaston II (181035B)

6/2/2021

Job Name: Gaston II (181035B)

Marshall Naylor

Page 1 of 1

D+S

Input by:

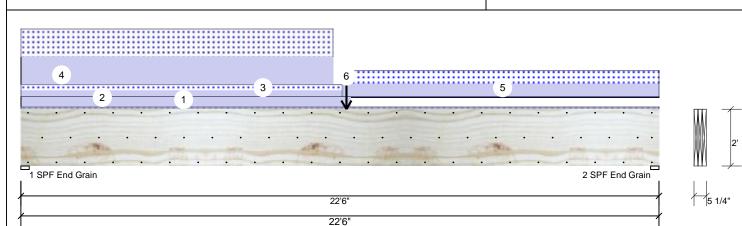
Date:

Project #: evel: Level

Kerto-S LVL FB₂

1.750" X 24.000"

3-Ply - PASSED



Member Information Type: Girder Plies: 3 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: Yes Deck: Not Checked

Reactions UNPATTERNED lb (Uplift)								
Brg	Live	Dead	Snow	Wind	Const			
1	225	6536	5095	0	0			
2	225	4429	3676	0	0			

Analysis Results Analysis Actual Comb. Case Location Allowed Capacity 0.499 (50%) D+S Moment 65477 ft-lb 11'5 3/4" 131295 ft-lb L Unbraced 65477 ft-lb 11'5 3/4" 65903 ft-lb 0.994 (99%) D+S L 10093 lb 2'2 5/8" 0.327 (33%) D+S Shear 30912 lb ī LL Defl inch 0.226 (L/1171) 11'1 11/16" 0.552 (L/480) 0.410 (41%) S L TL Defl inch 0.501 (L/528) 11' 7/8" 0.735 (L/360) 0.680 (68%) D+S L

Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 6536 / 5095 11631 L D+S

End Grain

Grain

4429 / 3676 8104 L 2 - SPF 3.500" End

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 3'11 5/8" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

/ Lateral 3	Chachicss ratio basea c	in single ply width.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Tie-In	0-0-0 to 22-6-0	0-6-0	Far Face	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	1' Floor	
2	Part. Uniform	0-0-0 to 11-7-8		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
3	Part. Uniform	0-0-0 to 11-4-0		Near Face	79 PLF	0 PLF	79 PLF	0 PLF	0 PLF	M2	
4	Part. Uniform	0-0-0 to 11-0-0		Тор	341 PLF	0 PLF	341 PLF	0 PLF	0 PLF	A2	
5	Part. Uniform	11-4-0 to 22-6-0		Near Face	164 PLF	0 PLF	164 PLF	0 PLF	0 PLF	M3	
6	Point	11-5-12		Тор	2293 lb	0 lb	2293 lb	0 lb	0 lb	B2	
	Self Weight				28 PLF						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 2/26/2023

Manufacturer Info



Weaver Homes

Project: Address:

Gaston II (181035B)

Date: 6/2/2021

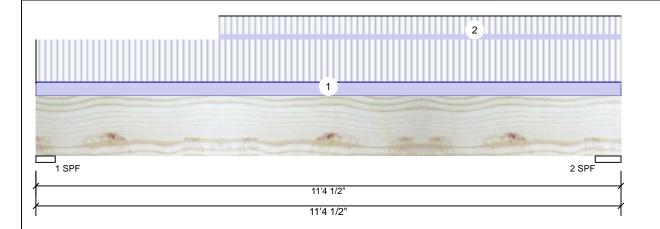
Marshall Naylor Job Name: Gaston II (181035B)

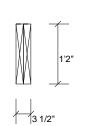
Project #:

Input by:

1.750" X 14.000" 2-Ply - PASSED **Kerto-S LVL** FB₁

Level: Level





Page 1 of 1

Member Infor	mation			Reaction	ns UNPAT	TERNED	lb (Uplift)		
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	2129	771	0	0	0
Moisture Condition	n: Dry	Building Code:	IBC 2012	2	2523	904	0	0	0
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	360	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearing	S				
				Bearing	Length	Cap. Re	eact D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	4.500"	43%	771 / 2129	2899 L	D+L
				2 - SPF	6.000"	38%	904 / 2523	3426 L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8168 ft-lb	5'9 3/16"	26999 ft-lb	0.303 (30%)	D+L	L
Unbraced	8168 ft-lb	5'9 3/16"	10258 ft-lb	0.796 (80%)	D+L	L
Shear	2446 lb	9'9 1/4"	10453 lb	0.234 (23%)	D+L	L
LL Defl inch	0.090 (L/1419)	5'8 3/16"	0.266 (L/480)	0.340 (34%)	L	L
TL Defl inch	0.122 (L/1044)	5'8 3/16"	0.354 (L/360)	0.340 (34%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	106 PLF	318 PLF	0 PLF	0 PLF	0 PLF	F5	
2	Part. Uniform	3-6-8 to 11-4-8		Тор	44 PLF	132 PLF	0 PLF	0 PLF	0 PLF	F9	
	Self Weight				11 PLF						

Notes

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation

 1. UVI beams must not be out or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

Handling & Installation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Project:

Address: Gaston II (181035B)

Weaver Homes

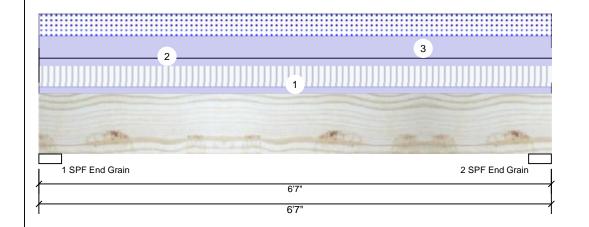
Date: 6/2/2021

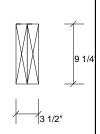
Marshall Naylor Job Name: Gaston II (181035B)

Input by:

6/0 SLIDER **Kerto-S LVL** 1.750" X 9.250" 2-Ply - PASSED

Project #: Level: Level





Page 1 of 1

Member Information							
Type:	Girder						
Plies:	2						
Moisture Condition:	Dry						
Deflection LL:	480						
Deflection TL:	360						
Importance:	Normal						
Temperature:	Temp <= 100°F						

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reaction	ns UNPAT	TERNED IL	(Uplift)			
Brg	Live	Dead	Snow	Wind	Const	
1	1060	1887	1113	0	0	
2	1060	1887	1113	0	0	

Analysis Results Analysis Actual Comb. Case Location Allowed Capacity 0.347 (35%) D+0.75(L+S) L Moment 5009 ft-lb 3'3 1/2" 14423 ft-lb Unbraced 5009 ft-lb 3'3 1/2" 10451 ft-lb 0.479 (48%) D+0.75(L+S) L 2448 lb 1' 7943 lb 0.308 (31%) D+0.75(L+S) L Shear LL Defl inch 0.042 (L/1741) 3'3 1/2" 0.153 (L/480) 0.280 (28%) 0.75(L+S) L TL Defl inch 0.091 (L/807) 3'3 1/2" 0.204 (L/360) 0.450 (45%) D+0.75(L+S) L

Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 33% 1887 / 1629 3516 L D+0.75(L+S)

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

End Grain				
2 - SPF 3.500" End Grain	33%	1887 / 1629	3516 L	D+0.75(L+S)

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	108 PLF	322 PLF	0 PLF	0 PLF	0 PLF	F4
2	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
3	Uniform			Тор	338 PLF	0 PLF	338 PLF	0 PLF	0 PLF	A4
	Self Weight				7 PLF					

Notes

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Weaver Homes

Project:

Address: Gaston II (181035B) Date: 6/2/2021

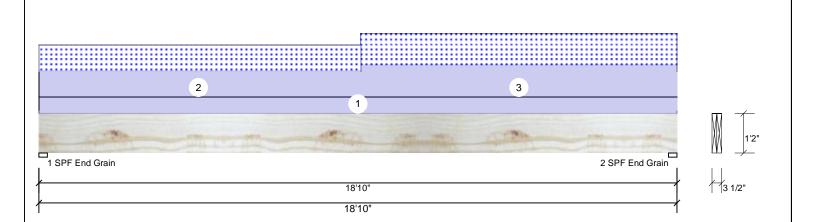
Marshall Naylor Job Name: Gaston II (181035B) Page 1 of 1

Project #:

Input by:

Kerto-S LVL 1.750" X 14.000" Front GDH 2-Ply - PASSED Level: Level

Reactions UNPATTERNED Ib (Uplift)



Type:	Girder		Applicati	ion: FI	oor		Brg	Live	Dead	Snow	Wind	Const
Plies:	2		Design I	Method: A	SD		1	0	1619	952	0	0
Moisture Cor	ndition: Dry		Building	Code: IB	C 2012		2	0	1720	1052	0	0
Deflection LL	.: 480		Load Sh	aring: N	0							
Deflection TL	_: 360		Deck:	N	ot Checked							
Importance:	Normal											
Temperature	: Temp <=	100°F										
							Bearings	S				
							Bearing	Length	Cap. R	React D/L lb	Total Ld. Case	Ld. Comb.
							1 - SPF End	3.000"	28%	1619 / 952	2571 L	D+S
Analysis R	esults		!				Grain					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2-SPF	3.000"	30%	1720 / 1052	2772 L	D+S
Moment	12090 ft-lb	9'8 7/8"	31049 ft-lb	0.389 (39%)) D+S	L	End Grain					
Unbraced	12090 ft-lb	9'8 7/8"	12111 ft-lb	0.998	D+S	L						

TL Defl inch Design Notes

Shear

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.

17'5 3/4" 12021 lb

- 3 Top loads must be supported equally by all plies.
- 4 Top must be laterally braced at a maximum of 8'7 7/8" o.c.
- 5 Bottom braced at bearings.

2360 lb

0.491 (L/451)

LL Defl inch 0.184 (L/1202)

Member Information

6 Lateral slenderness ratio based on single ply width.

o Eatoral olorido	moco rano bacca em	omigio pij maan									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	wall	
2	Part. Uniform	0-0-0 to 9-6-0		Тор	96 PLF	0 PLF	96 PLF	0 PLF	0 PLF	M2	
3	Part. Uniform	9-6-0 to 18-10-0		Тор	117 PLF	0 PLF	117 PLF	0 PLF	0 PLF	M3	
	Self Weight				11 PLF						

ī.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Indicating & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 1. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

(100%)

9'6 3/16" 0.461 (L/480) 0.400 (40%) S

9'5 13/16" 0.615 (L/360) 0.800 (80%) D+S

0.196 (20%) D+S

6. For flat roofs provide proper drainage to prevent ponding

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Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 2/26/2023 CSD |

Manufacturer Info



Client: Weaver Homes

Project:

Address: Gaston II (181035B) Date: 6/2/2021

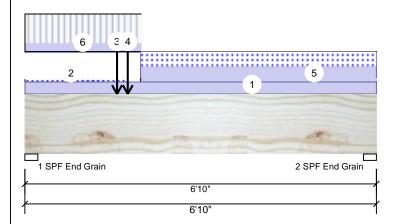
Input by: Marshall Naylor Job Name: Gaston II (181035B)

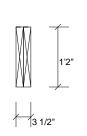
Project #:

Kerto-S LVL Window Hdr. 1.750" X 14.000" 2-Ply - PASSED

Level: Level

Reactions UNPATTERNED Ib (Uplift)





Page 1 of 1

Member Inform	nation
Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

_			` ' '			
Brg	Live	Dead	Snow	Wind	Const	
1	2861	3387	1990	0	0	
2	873	1906	1168	0	0	

Analysis Results Case Analysis Actual Location Allowed Comb. Capacity 11172 ft-lb 0.360 (36%) D+0.75(L+S) L Moment 2' 31049 ft-lb Unbraced 11172 ft-lb 2' 15735 ft-lb 0.710 (71%) D+0.75(L+S) L 6425 lb 1'4 1/4" 12021 lb 0.534 (53%) D+0.75(L+S) L Shear LL Defl inch 0.033 (L/2343) 2'7 5/8" 0.161 (L/480) 0.200 (20%) 0.75(L+S) L TL Defl inch 0.067 (L/1165) 2'8 7/8" 0.215 (L/360) 0.310 (31%) D+0.75(L+S) L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

В	earings	S					
	Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1	1 - SPF End Grain	3.000"	77%	3387 / 3638	7025	L	D+0.75(L+S)
	2 - SPF End Grain	3.000"	38%	1906 / 1531	3437	L	D+0.75(L+S)

o Latoral diorido	milodo ratio badda dir c	migio pry wiatri.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
2	Tie-In	0-0-0 to 2-0-0	1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	2' ROOF	
3	Point	1-9-8		Тор	1040 lb	3115 lb	0 lb	0 lb	0 lb	F08	
4	Point	2-0-0		Тор	2385 lb	0 lb	2385 lb	0 lb	0 lb	C3	
5	Part. Uniform	2-3-0 to 6-10-0		Тор	160 PLF	0 PLF	160 PLF	0 PLF	0 PLF	C2	
6	Part. Uniform	2-3-0 to 0-0-0		Тор	97 PLF	300 PLF	0 PLF	0 PLF	0 PLF	F07	
	Self Weight				11 PI F						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Project: Address:

Weaver Homes

Gaston II (181035B)

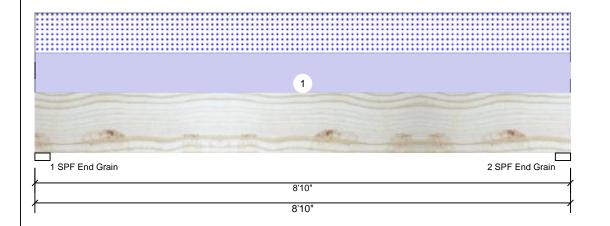
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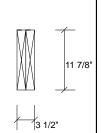
Marshall Naylor Job Name: Gaston II (181035B)

Project #:

1.750" X 11.875" GDH-2 **Kerto-S LVL** 2-Ply - PASSED

Level: Level





Page 1 of 1

Member Inform	ation
Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked Reactions UNPATTERNED Ib (Uplift) Brg Live Wind Const Dead Snow 0 1145 1104 0 0 1 0 0 2 1145 1104 0

Analysis Results Comb. Analysis Actual Location Allowed Capacity Case 0.199 (20%) D+S Moment 4554 ft-lb 4'5" 22897 ft-lb L Unbraced 4554 ft-lb 4'5" 10675 ft-lb 0.427 (43%) D+S L 1650 lb 10197 lb 0.162 (16%) D+S Shear 1'2 1/8" ī LL Defl inch 0.036 (L/2845) 4'5 1/16" 0.211 (L/480) 0.170 (17%) S L TL Defl inch 0.073 (L/1397) 4'5 1/16" 0.282 (L/360) 0.260 (26%) D+S L

Bearings

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1145 / 1104 D+S 1 - SPF 3.000" 2249 L End Grain 2 - SPF 3.000" 1145 / 1104 2249 L D+S End Grain

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	250 PLF	0 PLF	250 PLF	0 PLF	0 PLF	G2
	Self Weight				9 PLF					

Notes

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- Damaged Beams must not be used Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





GCO

Client:

Weaver Homes

Project:

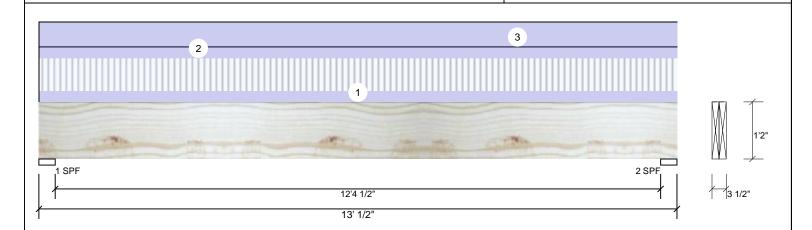
Address: Gaston II (181035B) Date: 6/2/2021

Input by: Marshall Naylor Job Name: Gaston II (181035B) Page 1 of 1

Project #:

1.750" X 14.000" 2-Ply - PASSED **Kerto-S LVL**

Level: Level



Member Infor	mation			Reaction	ons UNPAT	TERNED Ib	(Uplift)		
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	2374	3468	0	0	0
Moisture Condition	n: Dry	Building Code:	IBC 2012	2	2374	3468	0	0	0
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearing	gs				
				Bearing	g Length	Cap. Read	ct D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	4.000"	98% 346	88 / 2374	5842 L	D+L
				2 - SPF	4.000"	98% 346	88 / 2374	5842 L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case					
Moment	17498 ft-lb	6'6 1/4"	26999 ft-lb	0.648 (65%)	D+L	L					
Unbraced	17498 ft-lb	6'6 1/4"	17570 ft-lb	0.996 (100%)	D+L	L					
Shear	4554 lb	1'5 1/4"	10453 lb	0.436 (44%)	D+L	L					
LL Defl inch	0.142 (L/1059)	6'6 1/4"	0.312 (L/480)	0.450 (45%)	L	L					
TL Defl inch	0.349 (L/430)	6'6 1/4"	0.625 (L/240)	0.560 (56%)	D+L	L					

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top must be laterally braced at a maximum of 5'6 3/4" o.c.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	122 PLF	364 PLF	0 PLF	0 PLF	0 PLF	F01
2	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Uniform			Тор	274 PLF	0 PLF	0 PLF	0 PLF	0 PLF	A1
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be out or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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