

RE: J0521-2959 Weaver/Lot 4B Williams Farm/Harnett

## Site Information:

Customer: Project Name: J0521-2959 Lot/Block: Address: City:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 18 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E15492391	A1	6/7/2021
2	E15492392	A1SE	6/7/2021
3	E15492393	A2	6/7/2021
4	E15492394	A3	6/7/2021
5	E15492395	A4	6/7/2021
6	E15492396	A5	6/7/2021
7	E15492397	A6	6/7/2021
8	E15492398	A7	6/7/2021
9	E15492399	A8	6/7/2021
10	E15492400	A9	6/7/2021
11	E15492401	A9GE	6/7/2021
12	E15492402	B1	6/7/2021
13	E15492403	B1GE	6/7/2021
14	E15492404	C1	6/7/2021
15	E15492405	C2	6/7/2021
16	E15492406	C3	6/7/2021
17	E15492407	PB	6/7/2021
18	E15492408	PBGE	6/7/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021

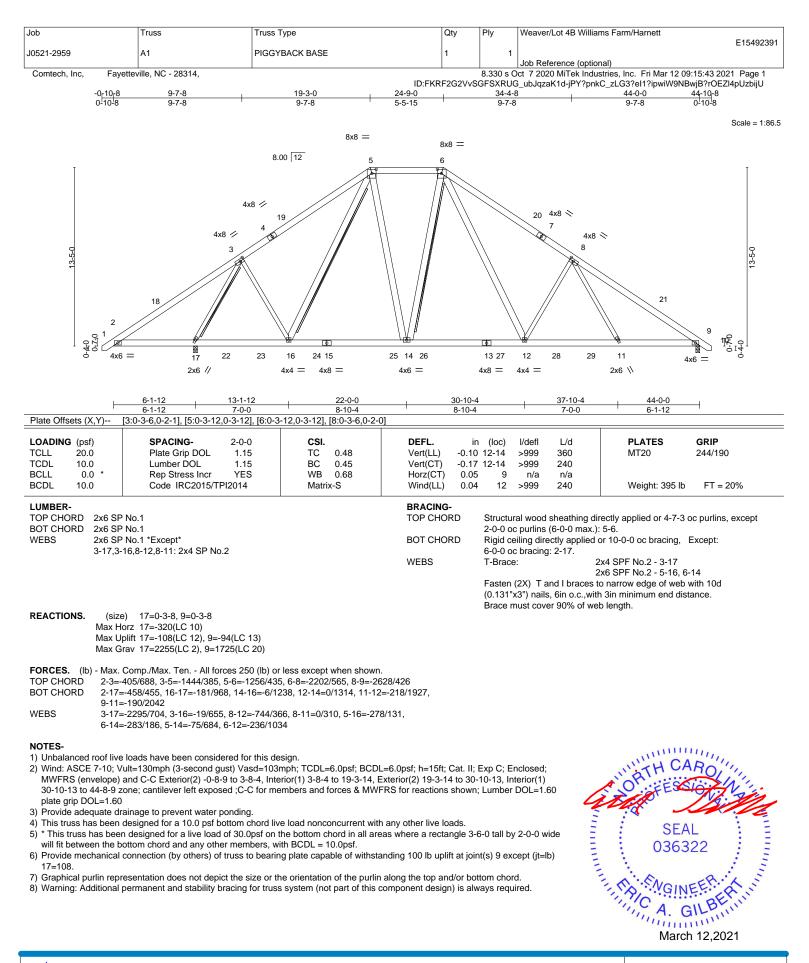
North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

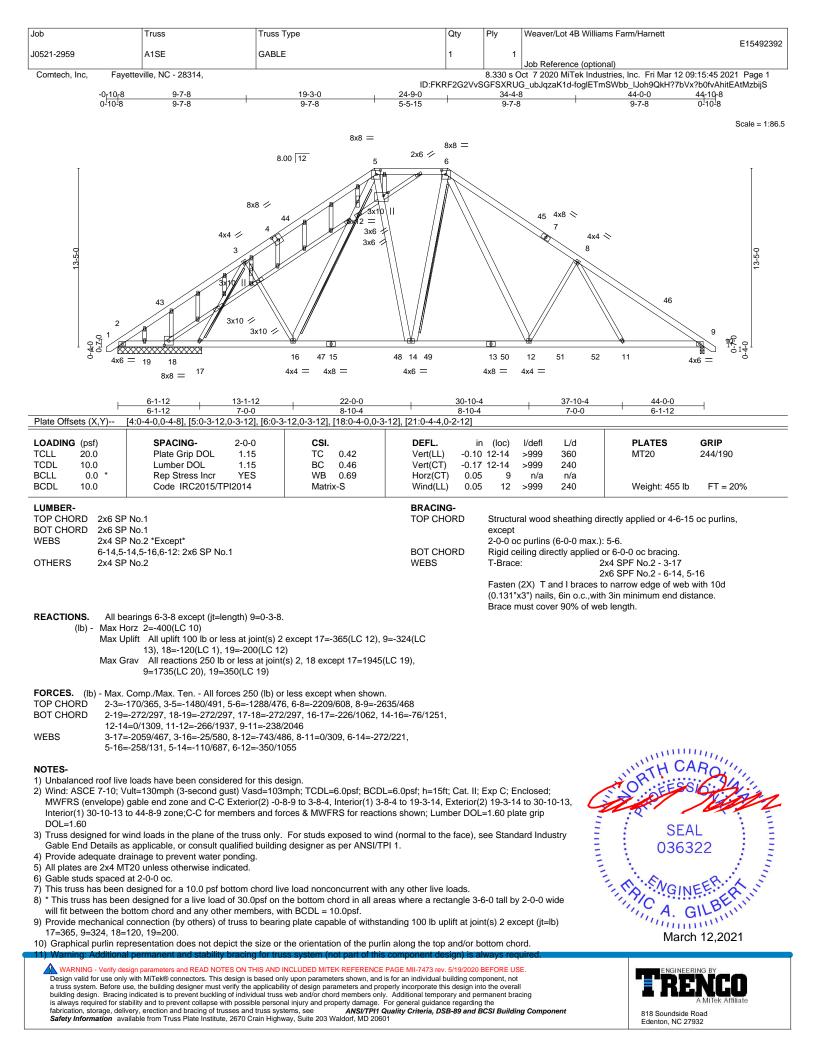


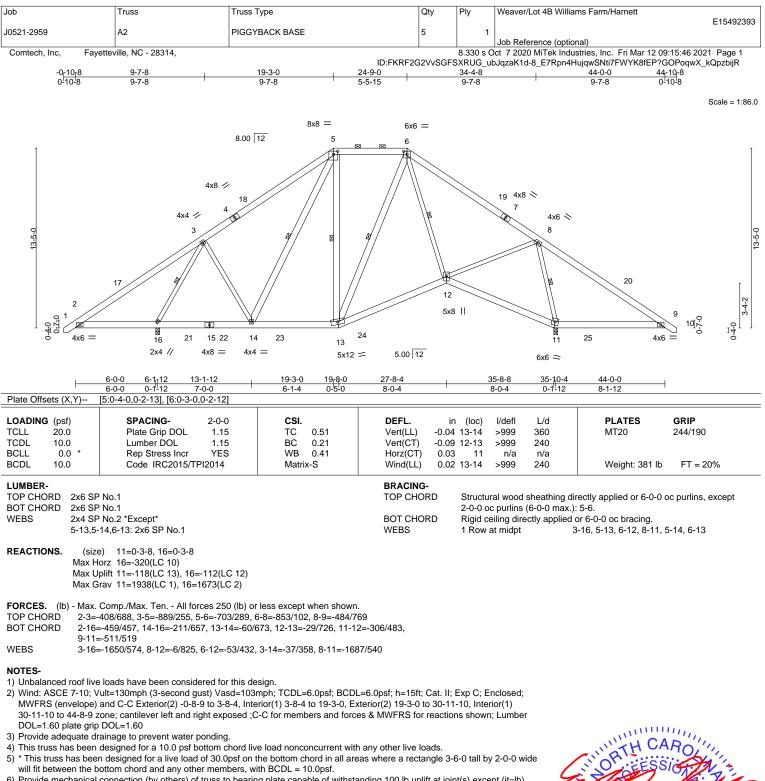
Gilbert, Eric

Trenco 818 Soundside Rd Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



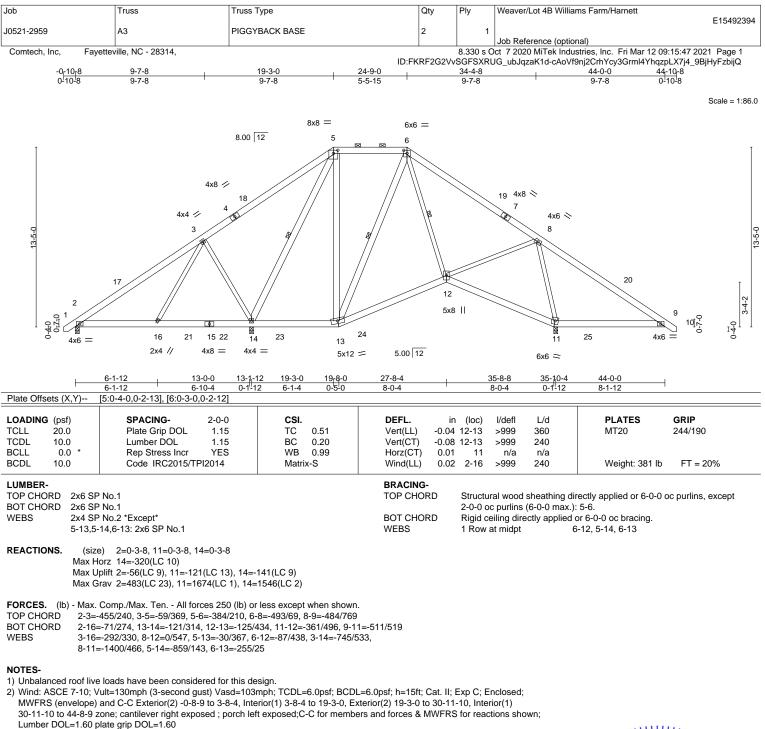


6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=118, 16=112.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 11=121, 14=141.

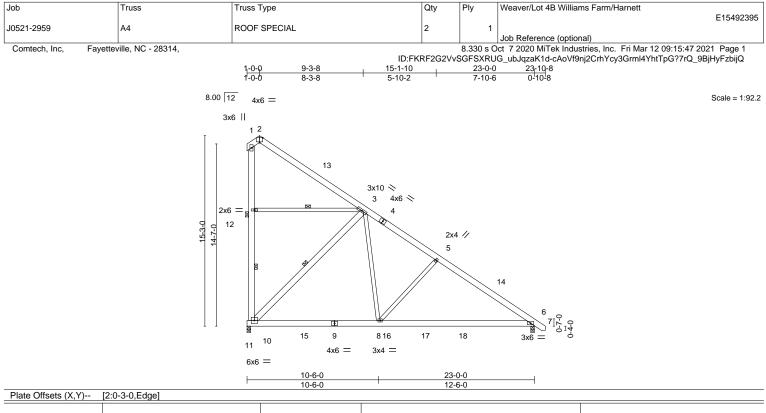
7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



ENGINEERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.29 BC 0.56 WB 0.45 Matrix-S	DEFL.         in           Vert(LL)         -0.13           Vert(CT)         -0.28           Horz(CT)         0.02           Wind(LL)         0.03	6-8 > 6-8 > 6	defl L/d 999 360 970 240 n/a n/a 999 240	PLATES MT20 Weight: 219 lb	<b>GRIP</b> 244/190 FT = 20%
1-10,3 REACTIONS. (siz Max H Max U	P No.1 P No.2 *Except* 10: 2x6 SP No.1		BRACING- TOP CHORD BOT CHORD WEBS JOINTS	except end	d verticals. ng directly applied nidpt	irectly applied or 6-0-0 o or 10-0-0 oc bracing. 10-12, 3-10, 3-12	oc purlins,

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-5=-1052/0, 5-6=-1298/0, 10-12=-259/166, 1-12=-259/166

BOT CHORD 8-10=0/743, 6-8=0/987

WEBS 5-8=-424/234, 3-10=-1116/335, 3-8=-26/850

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 5-4-13, Interior(1) 5-4-13 to 23-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

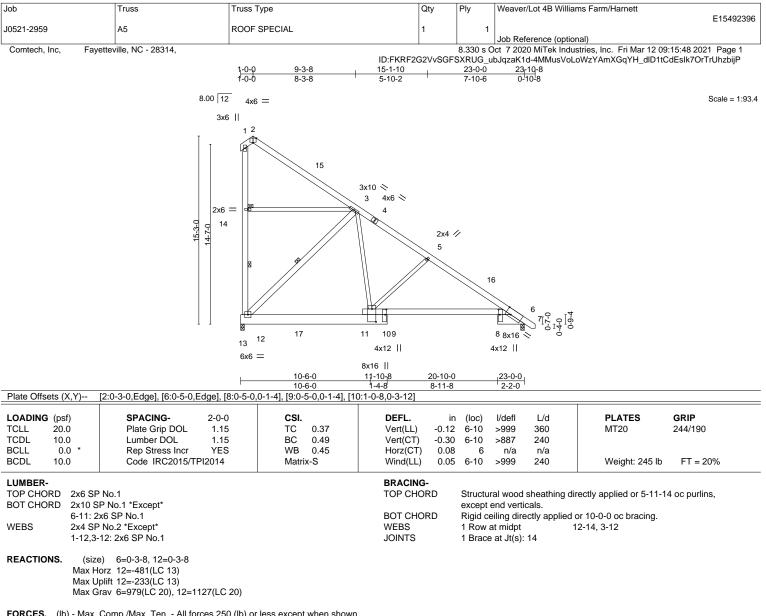
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=236.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-5=-1079/0, 5-6=-1350/0, 12-14=-256/166, 1-14=-257/166

- BOT CHORD 10-12=0/827, 6-10=0/1044
- WEBS 5-10=-432/206, 3-12=-1146/302, 3-10=0/874

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 5-4-13, Interior(1) 5-4-13 to 23-8-9 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12 = 233



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Job	Truss	uss Type	Qty	Ply	Weaver/Lot 4B Williams	Farm/Harnett	
J0521-2959		DOF SPECIAL	1				E15492397
				2	Job Reference (optiona		
Comtech, Inc, Faye	etteville, NC - 28314, 1 1	1-0-0 9-3-8 1-0-0 8-3-8			0ct 7 2020 MiTek Industr bJqzaK1d-YZwG4rpzZp 23 <u>r</u> 10-8 0-10-8		
	8.00 12	4x6 =					Scale = 1:93.4
		1 2 1 2 1 3 1 12 17 11 13 12 17 11 13 12 17 11 13 12 17 11 13 12 17 11 13 12 17 11 13 12 17 15 15 15 15 15 15 15 15 15 15	4x6 4 2x4 /2 5 109 4x12    6	11	6 8 8x16 xx12		
		10-6-0 1-	10-8         20-10-0           4-8         8-11-8		23-0-0		
Plate Offsets (X,Y)	[2:0-3-0,Edge], [6:0-5-0,Edge], [8:0-	-5-0,0-1-4], [9:0-5-4,0-1-4], [10:1-0	-4,0-4-0]				
LOADING(psf)TCLL20.0TCDL10.0BCLL0.0BCDL10.0	SPACING- 3-6-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.37 BC 0.47 WB 0.65 Matrix-S	DEFL.         in           Vert(LL)         -0.11           Vert(CT)         -0.27           Horz(CT)         0.07           Wind(LL)         0.05	(loc) 6-10 6-10 6 6-10	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 490 lb	<b>GRIP</b> 244/190 FT = 20%
6-11: 2: WEBS 2x4 SP 1-12,3- REACTIONS. (size Max Hu Max U	No.1 P No.1 *Except* (6 SP No.1 No.2 *Except* 12: 2x6 SP No.1 ) 6=0-3-8, 12=0-3-8 orz 12=-842(LC 13) olift 12=-407(LC 13) rav 6=1713(LC 20), 12=1971(LC 2)	0)	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	2-0-0 oc (Switche Rigid ce 1 Row a	: purlins (6-0-0 max.), e ed from sheeted: Spacii iling directly applied or t midpt 12- at Jt(s): 2, 1, 14	except end verticals ng > 2-8-0). 10-0-0 oc bracing. 14	NRO, Mar
FORCES. (lb) - Max. TOP CHORD 1-2=- 1-14= BOT CHORD 10-2 WEBS 5-10= NOTES- 1) 2-ply truss to be con Top chords connecte Bottom chords connected Bottom chords connected as	Comp./Max. Ten All forces 250 (I 333/223, 2-3=-379/107, 3-5=-1889/ -449/291 =0/1448, 6-10=0/1827 -757/361, 3-12=-2006/528, 3-10=0/ nected together with 10d (0.131"x3' ad as follows: 2x6 - 2 rows staggere scted as follows: 2x10 - 2 rows staggere follows: 2x4 - 1 row at 0-9-0 oc, 2x6	b) or less except when shown. 0, 5-6=-2362/0, 12-14=-448/290, /1529 ") nails as follows: ad at 0-9-0 oc. gered at 0-9-0 oc, 2x6 - 2 rows sta 5 - 2 rows staggered at 0-9-0 oc.			A DELITION OF THE REAL OF T	SEA 0363	EER. KI
<ul> <li>ply connections have</li> <li>3) Unbalanced roof live</li> <li>4) Wind: ASCE 7-10; V MWFRS (envelope) reactions shown; Lui</li> <li>5) This truss has been will fit between the bit 7) Provide mechanical 12=407.</li> </ul>	red equally applied to all plies, exc been provided to distribute only lo loads have been considered for thi ult=130mph (3-second gust) Vasd= and C-C Exterior(2) 0-4-4 to 5-4-13 mber DOL=1.60 plate grip DOL=1.6 designed for a 10.0 psf bottom chor o designed for a live load of 30.0psf bottom chord and any other member connection (by others) of truss to be esentation does not depict the size	ads noted as (F) or (B), unless oth is design. 403mph; TCDL=6.0psf; BCDL=6.0; 1, Interior(1) 5-4-13 to 23-8-9 zone; 30 rd live load nonconcurrent with any on the bottom chord in all areas w 's, with BCDL = 10.0psf. earing plate capable of withstandir	erwise indicated. Opsf; h=15ft; Cat. II; E C-C for members an v other live loads. vhere a rectangle 3-6 ng 100 lb uplift at joint	typ C; Er d forces -0 tall by (s) exce	aclosed; & MWFRS for 2-0-0 wide ot (jt=lb)		

March 12,2021

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Job	Truss	ss Type	Qty	Ply	Weaver/Lot 4B William	s Farm/Harnett	
J0521-2959		OF SPECIAL	1				E15492398
	etteville, NC - 28314,			<b>2</b>	Job Reference (option Oct 7 2020 MiTek Indus		0.15.51 2021 Page 1
Contech, inc, Fay		0-p 9-3-8 0-0 8-3-8	D:FKRF2G2VvSGFS> 15-1-10 5-10-2	RUG_ub 23-0-0 7-10-6	JqzaK1d-Ux10VWqD5F 23-10-8 0-10-8	RL71EFrVhrhEOrWkQc	ad3aCZ4phV50zbijM
	8.00 12	4x6 =					Scale = 1:93.4
	3x6    $2x6 = $ $4x6 = $ $4x6 = $ $4x6 = $ $14$ $14$ $14$ $13$ $13$	1   17   11 3   12   17   11 10.6-0   11	4x6 ≈ 4 2x4 ∞ 5 109 4x12    6    10-β 20-10-0		6 7 8 8 8 4x12    123-0-0		
Plate Offsets (X,Y)	⊢ [2:0-3-0,Edge], [6:0-5-0,Edge], [8:0-5	10-6-0 1	-4-8 8-11-8		2-2-0		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 4-3-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.46 BC 0.58 WB 0.79 Matrix-S	DEFL.         in           Vert(LL)         -0.13           Vert(CT)         -0.32           Horz(CT)         0.09	(loc) 6-10 6-10 6 6-10	l/defl L/d >999 360 >835 240 n/a n/a >999 240	PLATES MT20 Weight: 490 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x10 S 6-11: 2 WEBS 2x4 SP 1-12,3- REACTIONS. (size Max H			BRACING- TOP CHORD BOT CHORD WEBS JOINTS	2-0-0 oo (Switch Rigid ce 1 Row a	c purlins (6-0-0 max.), ed from sheeted: Spac eiling directly applied o at midpt 12 at Jt(s): 2, 1, 14	except end verticals sing > 2-8-0). r 10-0-0 oc bracing. 2-14	ROLIN
FORCES. (lb) - Max. TOP CHORD 1-2=- 1-14= BOT CHORD 10-12 WEBS 5-10= NOTES- 1) 2-ply truss to be con Top chords connected Bottom chords connected Bottom chords connected as 2) All loads are conside ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; V	rav 6=2081(LC 20), 12=2394(LC 20) Comp./Max. Ten All forces 250 (lb) 404/270, 2-3=-460/130, 3-5=-2294/0 -545/353 =0/1758, 6-10=0/2219 -919/438, 3-12=-2436/641, 3-10=0/1 nected together with 10d (0.131"x3") ad as follows: 2x6 - 2 rows staggered acted as follows: 2x10 - 2 rows staggered becent as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 becent as follows: 2x10 - 2 rows staggered becent as follows: 2x6 - 2 rows staggered becent as follo	) or less except when shown. , 5-6=-2868/0, 12-14=-544/352, 1857 1 nails as follows: d at 0-9-0 oc. gered at 0-9-0 oc, 2x6 - 2 rows st - 2 rows staggered at 0-9-0 oc. pt if noted as front (F) or back (B ids noted as (F) or (B), unless oth idesign. 103mph; TCDL=6.0psf; BCDL=6.	) face in the LOAD Control of the LOAD Control of the LOAD Control of the termination of the LOAD Control	Exp C; Ei	section. Ply to	SEA 0363	EER. R. IIII
reactions shown; Lui 5) This truss has been 6) * This truss has been will fit between the b 7) Provide mechanical 12=495.	and C-C Exterior(2) 0-4-4 to 5-4-13, mber DOL=1.60 plate grip DOL=1.60 designed for a 10.0 psf bottom chord n designed for a live load of 30.0psf ottom chord and any other members connection (by others) of truss to bea esentation does not depict the size of	) I live load nonconcurrent with any on the bottom chord in all areas , with BCDL = 10.0psf. aring plate capable of withstandir	y other live loads. vhere a rectangle 3-6 ng 100 lb uplift at join	-0 tall by (s) exce	r 2-0-0 wide pt (jt=lb)		

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# ENGINEERING BY REPART AMITER Affiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 4B Willia	ms Farm/Harnott	
	A8		1		Weaver/Lot 4D Willia	nis i ann/namett	E15492399
J0521-2959 Comtech, Inc, Fayett	teville, NC - 28314,	ROOF TRUSS	1	2	Job Reference (optio	nal) stries, Inc. Fri Mar 12 0	
	6x	$f = 0.0 \ 3.8-7$ $6 \times 8 =$ $6 \times 8 =$ $3 \times 6 \neq 8.0$ $3 \times 6 \neq 8$	5-1-10 10-5-3 00 12 4x12 4 4 6x8	23-0 7-10 ×x6 ≈	-0 23-10-8	5RL71EFrVhrhEOrSUG	lgX3f?Z4phV50zbijM Scale = 1:91.0
		$8x8 = \frac{13}{5}$ 8x8 M18SHS    $13   8x8 = \frac{1}{5}$ 13   9-0-9   1 9-0-9   1	x8 = 2x6   0-9-15 15-1-10 1-9-6 4-3-11	 23-0 7-10			
	2:0-4-0,Edge], [4:0-6-0,0-2-8], [4					<b></b>	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-8-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NC Code IRC2015/TPI2014	TC 0.73 BC 0.33	DEFL.         i           Vert(LL)         -0.2           Vert(CT)         -0.4           Horz(CT)         0.0           Wind(LL)         0.1	6 10 1 7	l/defl L/d >999 360 >587 240 n/a n/a >999 240	PLATES MT20 M18SHS Weight: 563 lb	<b>GRIP</b> 244/190 244/190 FT = 20%
BOT CHORD       5-8: 2x6         BOT CHORD       2x10 SP         WEBS       2x6 SP N         6-9,6-10,       6-9,6-10,         REACTIONS.       (size)         Max Hor       Max Upi         Max Gra       FORCES.         FOR CHORD       3-4=-80         BOT CHORD       10-12=	2400F 2.0E No.1 *Except* 3-14: 2x4 SP No.2 12=0-3-8, 7=0-3-8 rz 12=-633(LC 13) ift 12=-7(LC 13) av 12=2288(LC 21), 7=1482(LC comp./Max. Ten All forces 25/ 08/104, 4-6=-829/128, 6-7=-22 119/629, 9-10=0/1720, 7-9=0/	(lb) or less except when shown. 33/92, 12-14=-1091/133	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	except e Rigid ce 1 Row a	end verticals. eiling directly applied at midpt at Jt(s): 14	rectly applied or 6-0-0 or 10-0-0 oc bracing. 12-14 TH C SEA 0363	
<ol> <li>2-ply truss to be connected Bottom chords connected Bottom chords connected Webs connected as for</li> <li>All loads are considere ply connections have I</li> <li>Unbalanced roof live I</li> <li>Wind: ASCE 7-10; Vul MWFRS (envelope) an reactions shown; Lum</li> <li>All plates are MT20 pla</li> <li>This truss has been de 7) * This truss has been de will fit between the bot</li> <li>Ceiling dead load (10.</li> <li>Bottom chord live load</li> </ol>	cted as follows: 2x10 - 2 rows s pllows: 2x6 - 2 rows staggered a ed equally applied to all plies, e been provided to distribute only oads have been considered for It=130mph (3-second gust) Vas nd C-C Exterior(2) 0-4-4 to 5-4 ber DOL=1.60 plate grip DOL= ates unless otherwise indicated esigned for a 10.0 psf bottom c designed for a live load of 30.0 ttom chord and any other mem 0 psf) on member(s). 4-14; Wa (40.0 psf) and additional bottc connection (by others) of truss	gered at 0-9-0 oc, 2x6 - 2 rows sta aggered at 0-9-0 oc. tt 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc xcept if noted as front (F) or back loads noted as (F) or (B), unless this design. d=103mph; TCDL=6.0psf; BCDL= 13, Interior(1) 5-4-13 to 23-8-9 zc 1.60 ord live load nonconcurrent with osf on the bottom chord in all area	c. (B) face in the LOAD of otherwise indicated. =6.0psf; h=15ft; Cat. II; one;C-C for members a any other live loads. as where a rectangle 3 s).4-10 lied only to room. 10-1	; Exp C; Ei and forces -6-0 tall by 2	section. Ply to nclosed; & MWFRS for	A.C.	
						Marc	h 12,2021

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Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 4B Williams	Farm/Harnett	
J0521-2959	A9	ROOF TRUSS	3	1			E15492400
		ROOF TRUSS	3		Job Reference (optional		
Comtech, Inc, Fay	etteville, NC - 28314,	1-0-0 3-8-6	ID:FKRF2G2VvS0 15-1-10 10-5-4				η?vo4PjJTR2dTzĎijL
		6x8 =					Scale = 1:94.6
	14-7-0	6x6 =	5 6 10 9	3x6 ≈			
		6x10 M18SHS    7x14 M18SHS					
			5x8 — 1¦0-9-15 15-1-10 <sub>1</sub>	23-0	-0 ,		
Plate Offsets (X,Y)	[2:0-4-0 Edge] [4:0-2-12 0-6-8	9-0-9 , [5:0-4-0,Edge], [12:0-5-0,0-0-8]	1-9-6 4-3-11	7-10			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0 Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YE Code IRC2015/TPI2014	0 <b>CSI.</b> 5 TC 0.96 5 BC 0.45 S WB 0.60	DEFL. ir Vert(LL) -0.32 Vert(CT) -0.69 Horz(CT) 0.01 Wind(LL) 0.26	10 10 7	l/defl L/d >844 360 >392 240 n/a n/a >999 240	PLATES MT20 M18SHS Weight: 282 lb	<b>GRIP</b> 244/190 244/190 FT = 20%
BOT CHORD 2x10 S WEBS 2x6 SP 6-9,6-1 REACTIONS. (size Max H	P No.1 *Except* 6 SP No.1 P 2400F 2.0E ' No.1 *Except* 0,3-14: 2x4 SP No.2 e) 12=0-3-8, 7=0-3-8 orz 12=-475(LC 13) plift 12=-5(LC 13)		BRACING- TOP CHORD BOT CHORD WEBS JOINTS	Rigid ce 1 Row a	ral wood sheathing direct eiling directly applied or a at midpt 12- e at Jt(s): 14		end verticals.
Max G FORCES. (lb) - Max. TOP CHORD 3-4=- BOT CHORD 10-12	rav 12=1716(LC 21), 7=1111(I Comp./Max. Ten All forces 2 606/78, 4-6=-622/96, 6-7=-167 2=-89/471, 9-10=0/1290, 7-9=0,	50 (lb) or less except when shown. 5/69, 12-14=-818/100	69				
<ol> <li>Wind: ASCE 7-10; V MWFRS (envelope) reactions shown; Lu</li> <li>All plates are MT20</li> <li>This truss has been will fit between the b</li> <li>Ceiling dead load (1</li> <li>Bottom chord live load</li> </ol>	and C-C Exterior(2) 0-4-4 to 5- mber DOL=1.60 plate grip DOL plates unless otherwise indicate designed for a 10.0 psf bottom n designed for a live load of 30. ottom chord and any other mer 0.0 psf) on member(s). 4-14; V ad (40.0 psf) and additional bot connection (by others) of truss	asd=103mph; TCDL=6.0psf; BCDL= 4-13, Interior(1) 5-4-13 to 23-8-9 zc =1.60 sd. chord live load nonconcurrent with 0psf on the bottom chord in all area	ine;C-C for members an any other live loads. as where a rectangle 3- s).4-10 lied only to room. 10-12	6-0 tall by	& MWFRS for / 2-0-0 wide	SEA 0363	EER ER III



CO

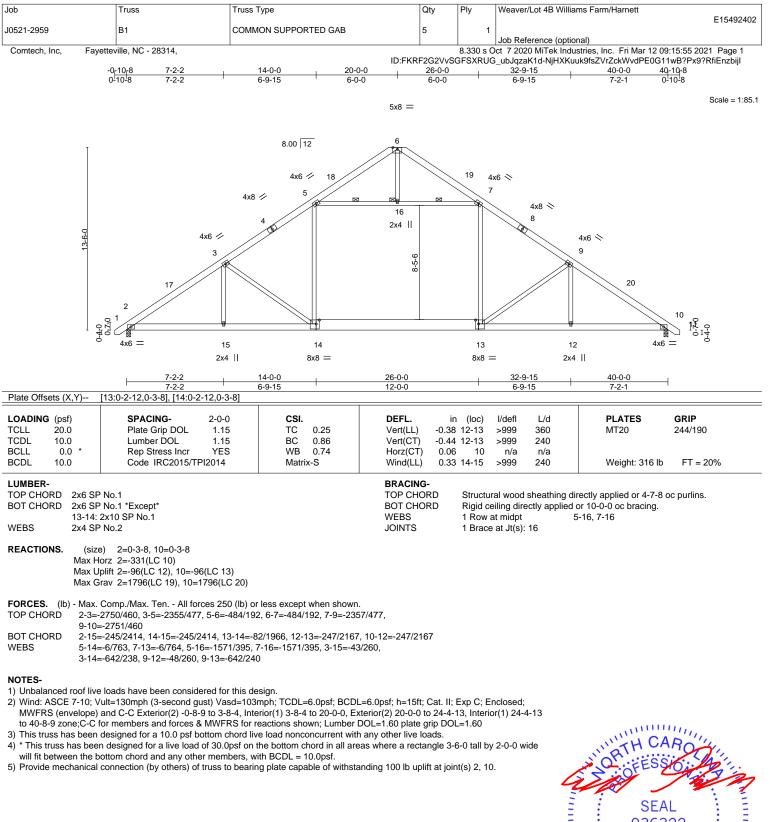
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

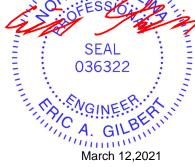
818 Soundside Road Edenton, NC 27932

RENGINEERING BY

Job	Truss Truss	Туре	Qty	Ply	Weaver/Lot 4B Williar	ns Farm/Harnett	
J0521-2959	A9GE COM	MON SUPPORTED GAB	1	1			E15492401
Comtech, Inc, Faye	etteville, NC - 28314,					stries, Inc. Fri Mar 12 09:	
	1-0-0 1-0-0		1D:FKRF2G2VVSGF 23-0-0 22-0-0	SXRUG_ub	0.00228K10-QK9mVCs102 23-10-8 0-10-8	2crGXPDc6t9KpwzRDR8	XevvsX7Ab9vzbijK
	8.00 12 5x5 =		22-0-0		0-10-8		Scale = 1:89.8
	500 - 500 -						
		3 4 5 6 6 7 7 8 8 8 8 8 8 8	4x6 7 8 9 10 10 10 10 10 10 10 10 10 10	11 12	$ \begin{array}{c} 13 \\ 14 \\ 16 \\ 3x4 = \end{array} $	SEA 0363	
			23-0-0				
Plate Offsets (X,Y)	[7:0-2-9,Edge], [23:0-4-0,0-4-8]		23-0-0				
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.		in (loc)	l/defl L/d	PLATES	GRIP
TCLL         20.0           TCDL         10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.06 BC 0.06	Vert(LL) 0.0 Vert(CT) 0.0	00 14	n/r 120 n/r 120	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.16 Matrix-S	Horz(CT) 0.0	01 14	n/a n/a	Weight: 280 lb	FT = 20%
(lb) - Max Ho Max Up	9 No.1 9 No.1	=-100(LC 11)		except of Rigid co 1 Row a T-Brace Fasten (0.131")	end verticals. eiling directly applied of at midpt 1 e: 2 (2X) T and I braces to	-27 x4 SPF No.2 - 3-25, 4-2 x6 SPF No.2 - 2-26 o narrow edge of web w a 3in minimum end dista	24, 5-23, 6-22 ⁄ith 10d
Wax G	19, 18, 17, 16 except 14=414(LC 1		1, 20,				
TOP CHORD         5-6=-7           11-12         11-12           BOT CHORD         26-27           21-22         21-22	Comp./Max. Ten All forces 250 (lb) o 254/197, 6-8=-329/256, 8-9=-404/314, 2=-629/491, 12-13=-705/550, 13-14=-71 7=-536/694, 25-26=-536/694, 24-25=-53 2=-536/694, 20-21=-536/694, 19-20=-53 7=-536/694, 14-16=-536/694	9-10=-479/373, 10-11=-554 94/626 36/694, 23-24=-536/694, 22	2-23=-536/694,				
<ol> <li>Wind: ASCE 7-10; V MWFRS (envelope) forces &amp; MWFRS for</li> <li>Truss designed for w Gable End Details as</li> <li>All plates are 2x4 MI</li> <li>Gable requires contin</li> <li>Gable studs spaced</li> <li>This truss has been of</li> <li>* This truss has been of</li></ol>	e loads have been considered for this d fult=130mph (3-second gust) Vasd=103 gable end zone and C-C Corner(3) 0-4 r reactions shown; Lumber DOL=1.60 p vind loads in the plane of the truss only s applicable, or consult qualified buildin T20 unless otherwise indicated. nuous bottom chord bearing. at 2-0-0 oc. designed for a 10.0 psf bottom chord lin n designed for a live load of 30.0psf on ottom chord and any other members. connection (by others) of truss to beari 7 except (jt=lb) 16=116, 14=100. Il permanent and stability bracing for tru	Bmph; TCDL=6.0psf; BCDL= -4 to 5-4-13, Exterior(2) 5-4 value grip DOL=1.60 . For studs exposed to wind g designer as per ANSI/TPI ve load nonconcurrent with the bottom chord in all area ng plate capable of withstar	4-13 to 23-8-9 zone;C- d (normal to the face), l 1. any other live loads. as where a rectangle 3 nding 100 lb uplift at jo	C for mem , see Stand 3-6-0 tall by pint(s) 27, 2	ubers and dard Industry y 2-0-0 wide 26, 25, 24, 23,	March	12,2021
Design valid for use only a truss system. Before us building design. Bracing is always required for sta fabrication, storage, deliv	sign parameters and READ NOTES ON THIS AND with MiTek® connectors. This design is based on use, the building designer must verify the applicable indicated is to prevent buckling of individual trust ability and to prevent collapse with possible perso very, erection and bracing of trusses and truss sys- aible form Turus Plate heriture. 2670 Ccaine Lindo	nly upon parameters shown, and is lity of design parameters and propers s web and/or chord members only. nal injury and property damage. Foc stems, see <b>ANS/TP11 Qu</b>	s for an individual building co erly incorporate this design Additional temporary and p or general guidance regardin uality Criteria, DSB-89 and	omponent, not into the overa permanent bra ng the	t III acing	B18 Soundside R	NG BY NCCO A MITek Atfiliate

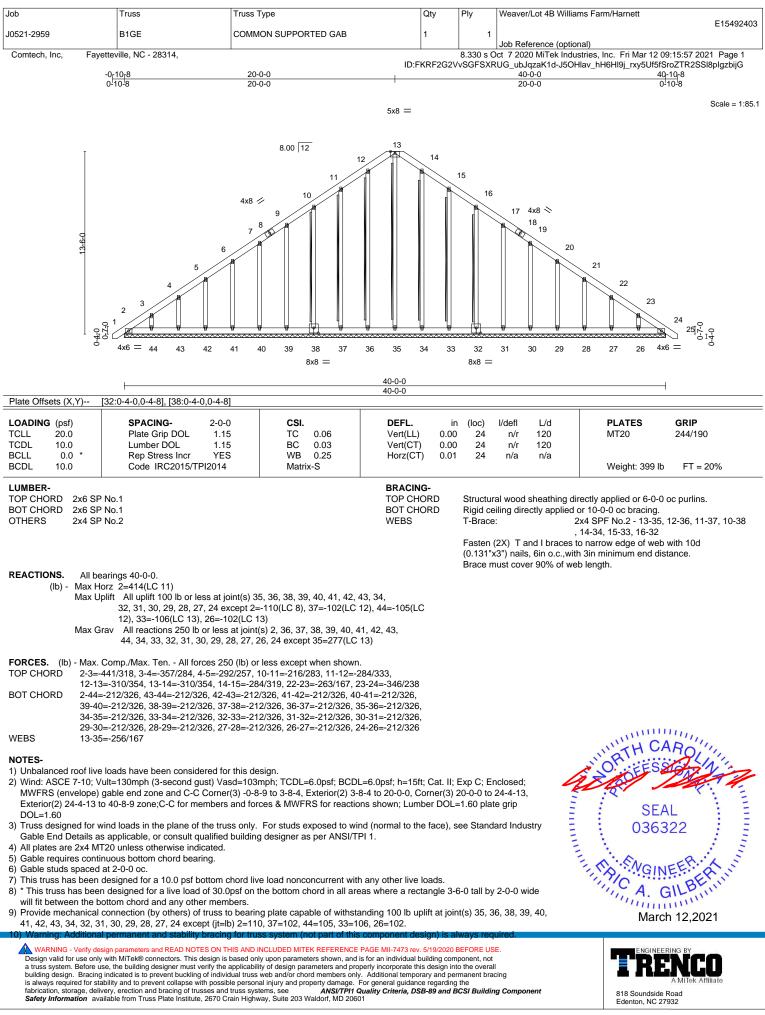
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



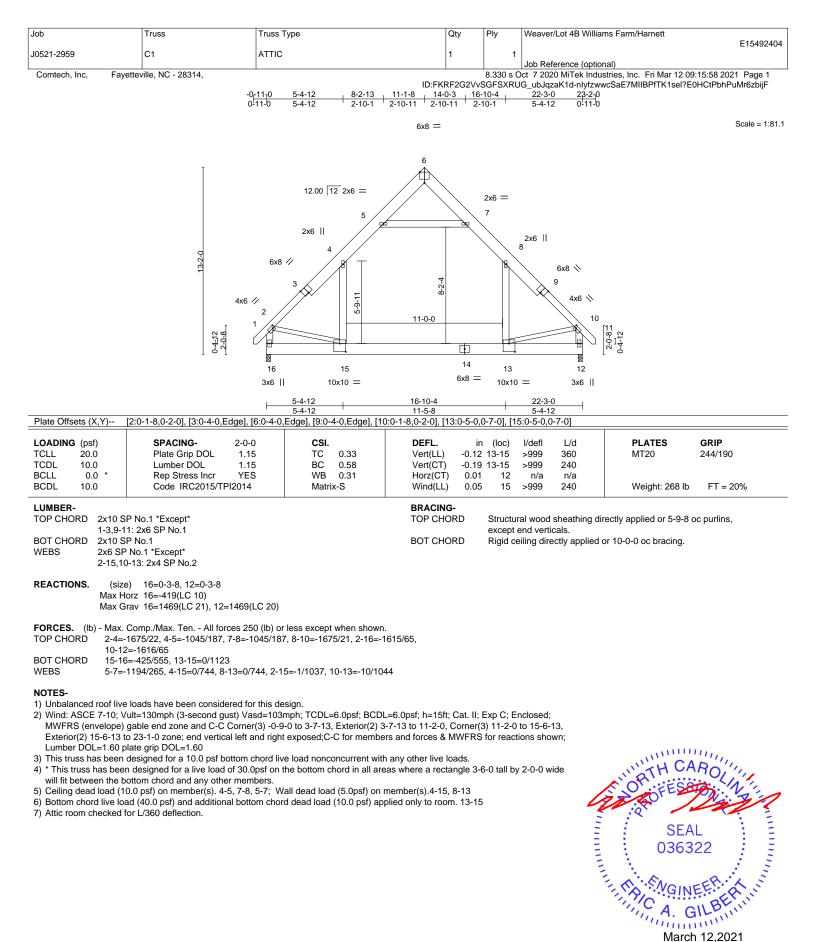


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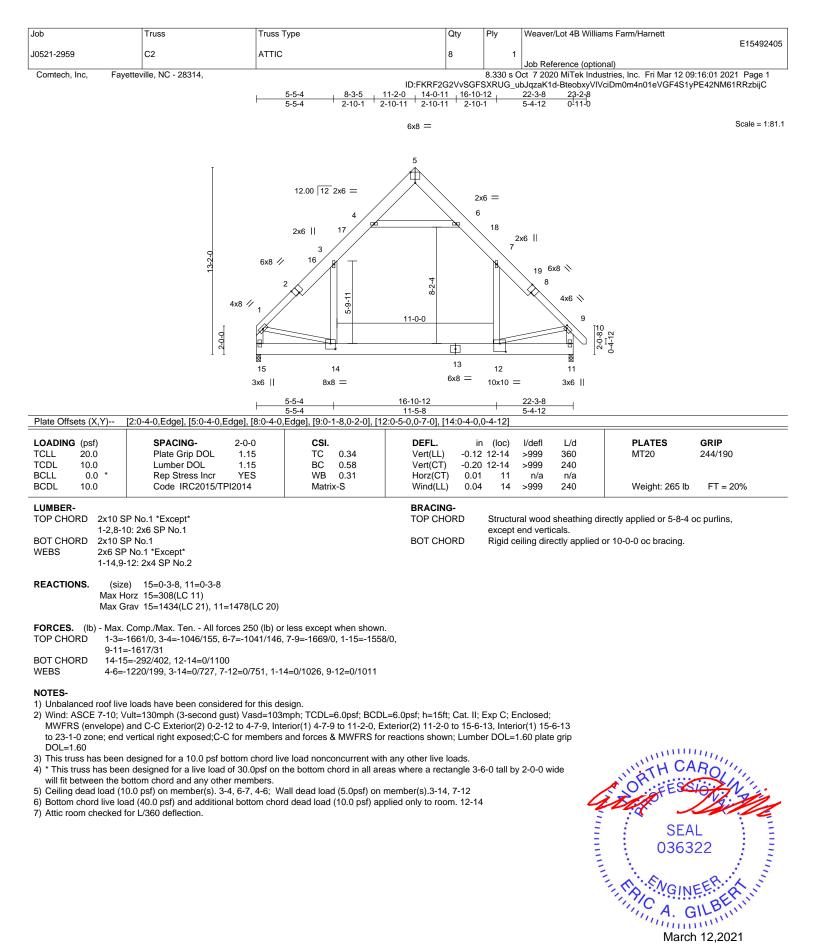
Edenton, NC 27932



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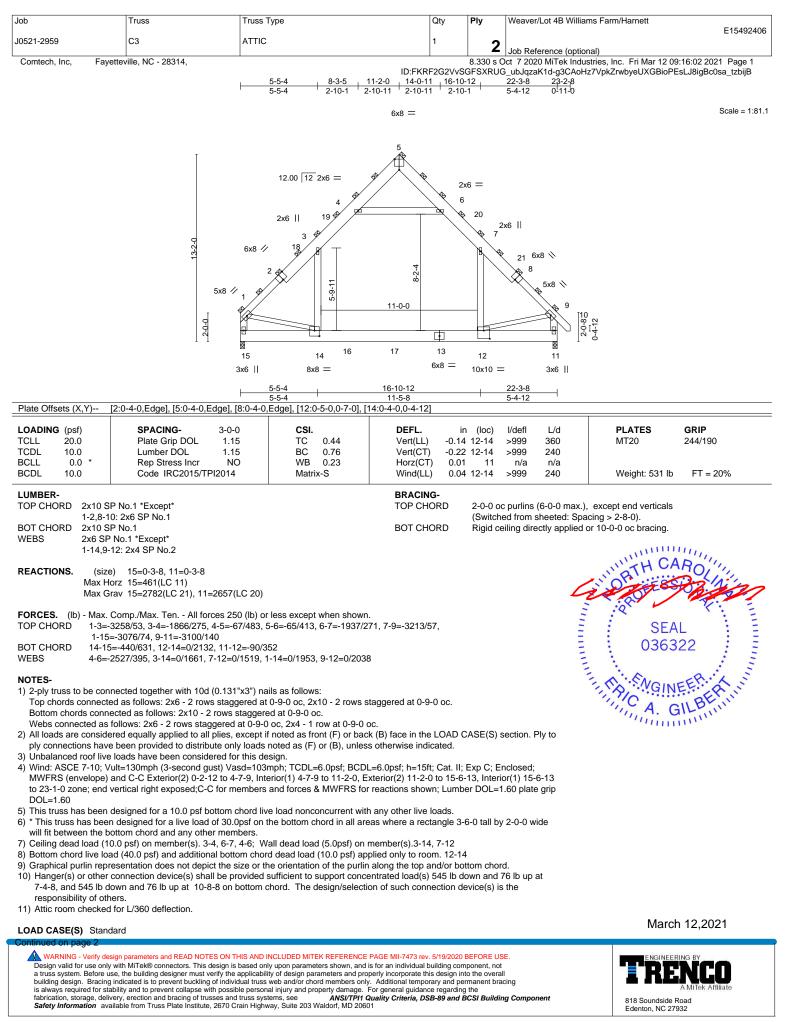
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818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 4B Williams Farm/Harnett
					E15492406
J0521-2959	C3	ATTIC	1	2	
				<b></b>	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,			8.330 s C	ct 7 2020 MiTek Industries, Inc. Fri Mar 12 09:16:02 2021 Page 2

ID:FKRF2G2VvSGFSXRUG\_ubJqzaK1d-g3CAoHz7VpkZrwbyeUXGBioPEsLJ8igBc0sa\_tzbijB

### LOAD CASE(S) Standard

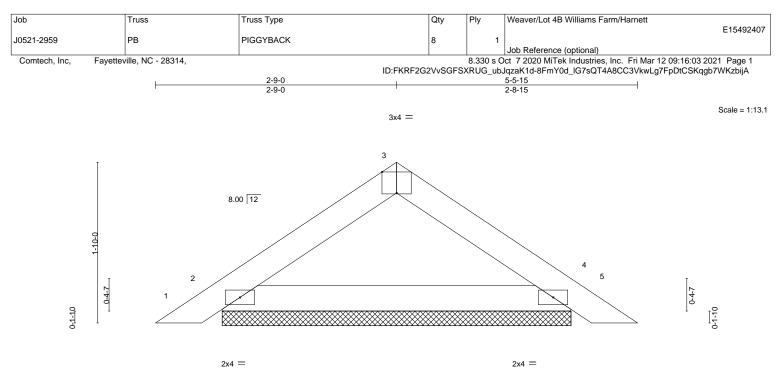
Lond CASE(0) Statuting
 Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 14-15=-30, 12-14=-60, 11-12=-30, 1-3=-90, 3-4=-120, 4-5=-90, 5-6=-90, 6-7=-120, 7-9=-90, 9-10=-90, 4-6=-30 Drag: 3-14=-15, 7-12=-15

Concentrated Loads (lb)

Vert: 16=-300(B) 17=-300(B)

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2x4 =

Rigid ceiling directly applied or 10-0-0 oc bracing.

.OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) 0.00 4 n/r 120	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.14	Vert(CT) 0.00 5 n/r 120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 4 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 16 lb FT = 20%

BOT CHORD

5-5-15

BOT CHORD 2x4 SP No.1

REACTIONS. (size) 2=3-11-11, 4=3-11-11 Max Horz 2=-40(LC 10)

Max Uplift 2=-16(LC 12), 4=-16(LC 13) Max Grav 2=189(LC 1), 4=189(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

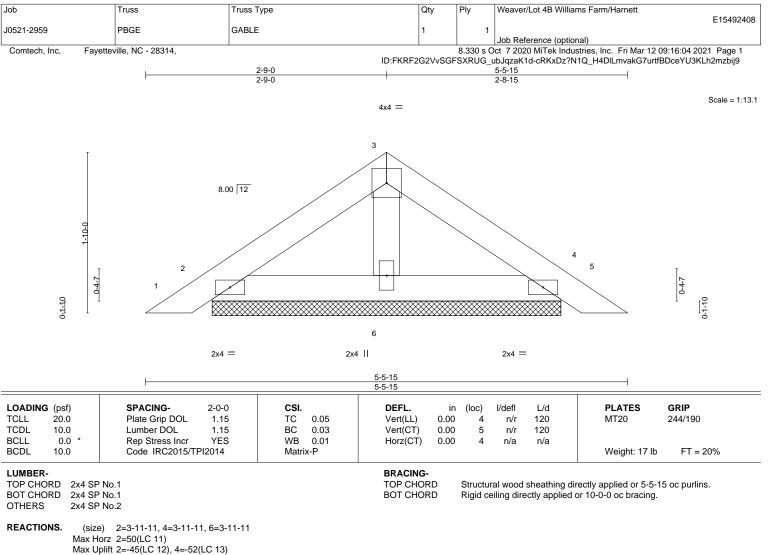
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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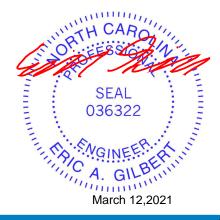


Max Grav 2=119(LC 1), 4=119(LC 1), 6=139(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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