PLANS DESIGNED TO THE

MEAN ROOF HEIGHT: 19'-9)"	HEIGHT TO RIDGE: 27'-5"					
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A				
FENESTRATION U-FACTOR	0.35	0.35	0.35				
SKYLIGHT U-FACTOR	0.55	0.55	0.55				
GLAZED FENESTRATION SHGC	0.30	0.30	0.30				
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci				
WALL R-VALUE	15	15	19				
FLOOR R-VALUE	19	19	30				
* BASEMENT WALL R-VALUE	5/13	10/15	10/15				
** SLAB R-VALUE	0	10	10				
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19				
* "10/13" MEANS R-10 SHEATHING INS	LILATION OR R-13 C	AVITY INSULATION					

** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF

DESIGNED FOR WIN								
COMPONENT	& CLA	DDING	DESIG	NED FC	R THE	FOLLO	WING I	OADS
MEAN ROOF	UP T	0 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0		-18.9				
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIN	D SPEED	OF 130 MF	H, 3 SECC	OND GUST	(101 FAS	TEST MILE) EXPOSU	RE "B"
COMPONENT	& CLA	DDING	DESIG	NED FC	R THE	FOLLO	WING I	OADS
COMPONENT MEAN ROOF				NED FC TO 35'				Loads To 45'
		O 30'	30'-1"		35'-1"	TO 40'	40'-1"	TO 45'
MEAN ROOF	UP T	O 30'	30'-1" 17.5	TO 35'	35'-1" 18.2	TO 40' -19.6	40'-1" 18.7	TO 45' -20.2 -23.5
MEAN ROOF ZONE 1	UP T 16.7	O 30' -18.0	30'-1" 17.5 17.5	TO 35' -18.9 -22.1 -22.1	35'-1" 18.2 18.2 18.2	TO 40' -19.6	40'-1" 18.7 18.7 18.7	TO 45' -20.2 -23.5 -23.5

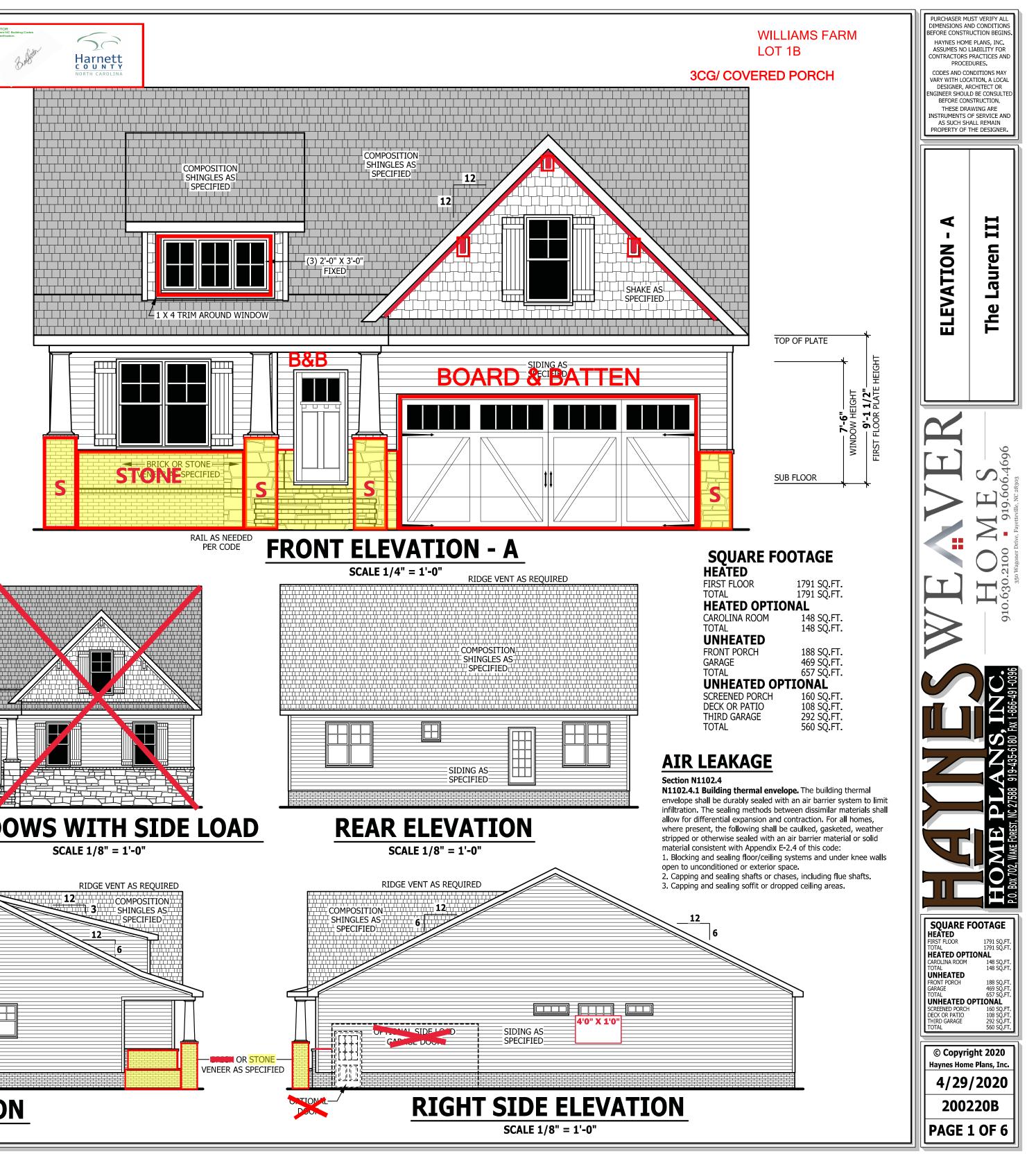
1. *Guards* on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the treads.

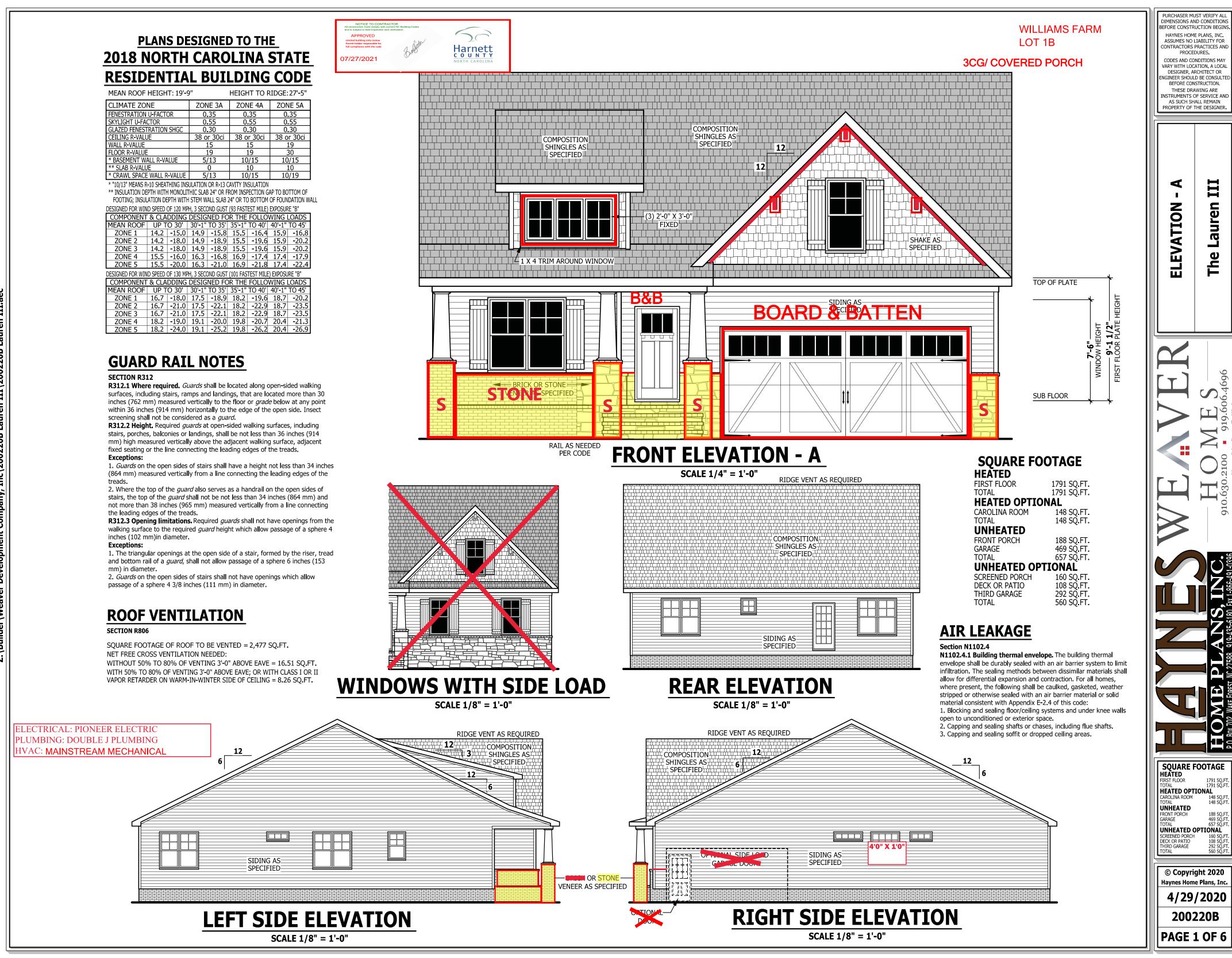
stairs, the top of the *guard* shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

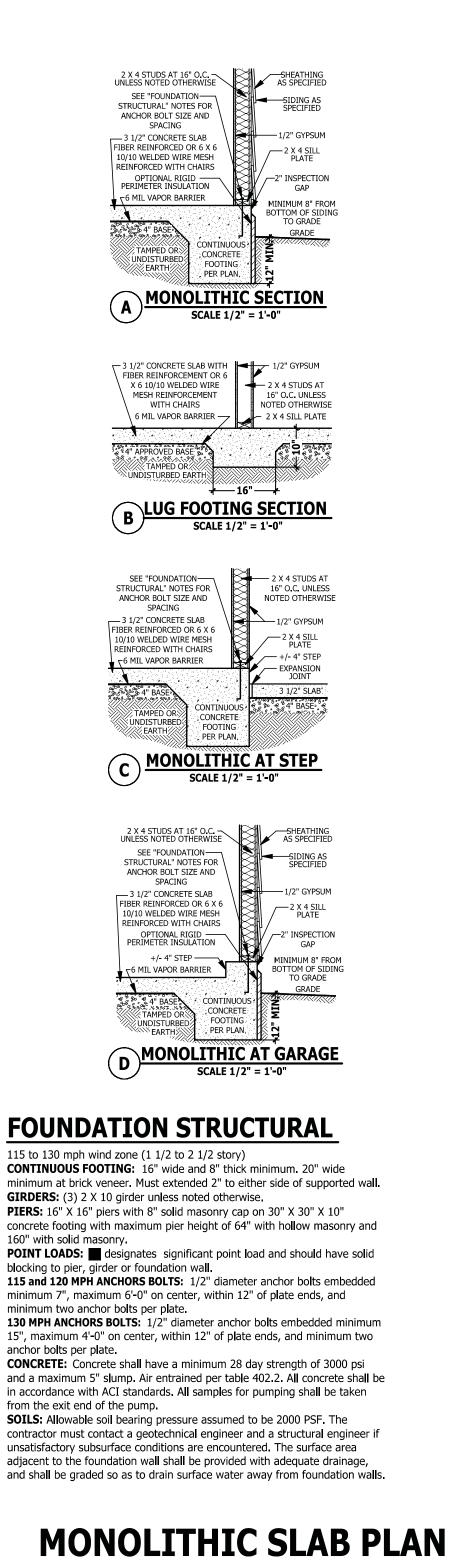
and bottom rail of a *guard*, shall not allow passage of a sphere 6 inches (153

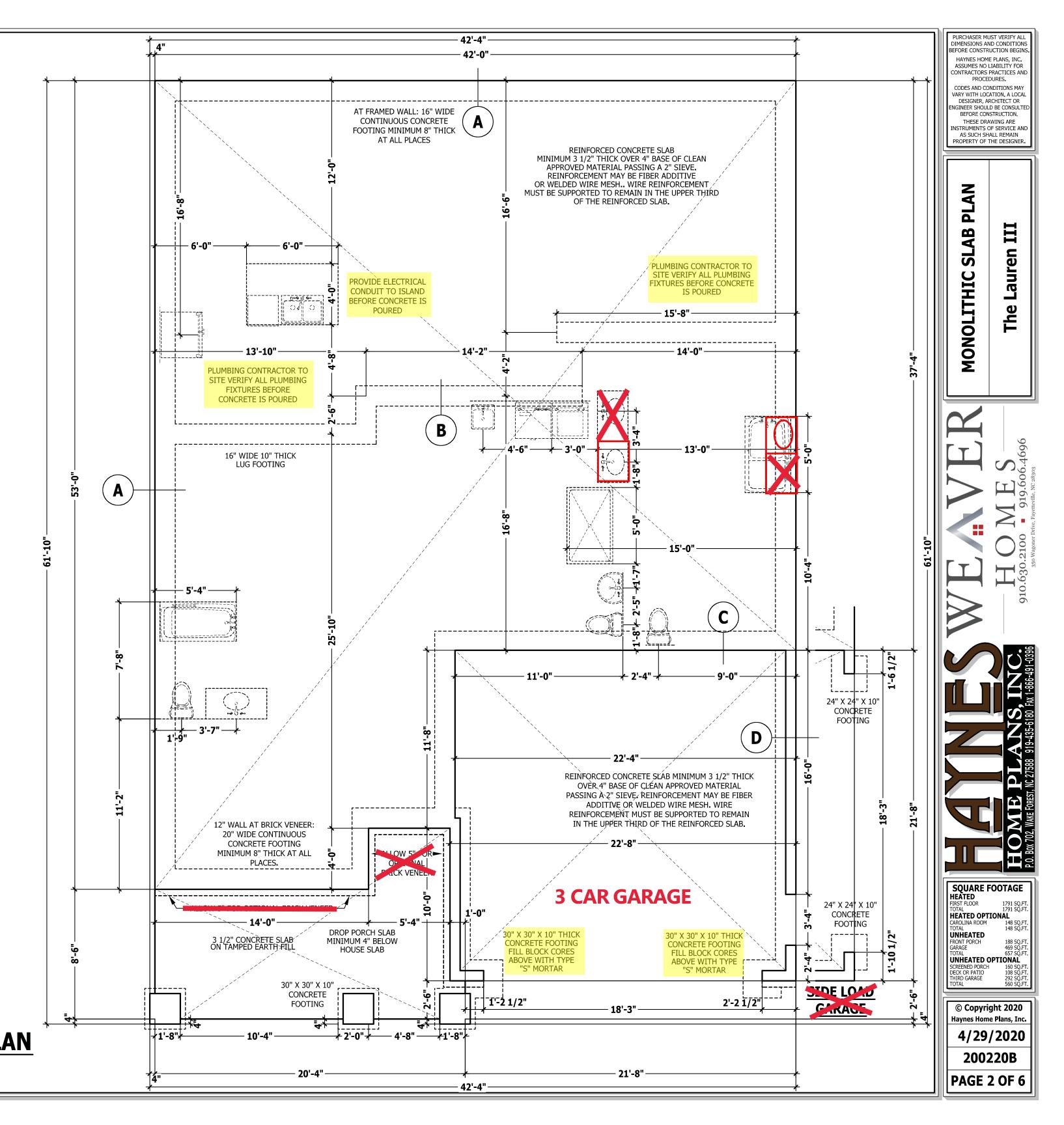
SOUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SO.FT. NET FREE CROSS VENTILATION NEEDED:











SCALE 1/4" = 1'-0"

COVERED REAR PORCH

DWELLING / GARAGE SEPARATION

REFER TO SECTIONS R302.5, R302.6, AND R302.7

WALLS. A minimum 1/2" gypsum board must be installed on all walls supporting floor/ceiling assemblies used for separation required by this section. **STAIRS.** A minimum of 1/2" gypsum board must be installed on the underside and exposed sides of all stairways.

CEILINGS. A minimum of 1/2" gypsum must be installed on the garage ceiling if there are no habitable room above the garage. If there are habitable room above the garage a minimum of 5/8" type X gypsum board must be installed on the garage ceiling. **OPENING PENETRATIONS.** Openings between the garage and residence shall be

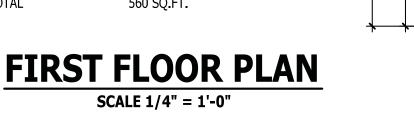
equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute fire-rated doors.

DUCT PENETRATIONS. Ducts in the garage and ducts penetrating the walls or ceilings separating the *dwelling* from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other *approved* material and shall have no openings into the garage.

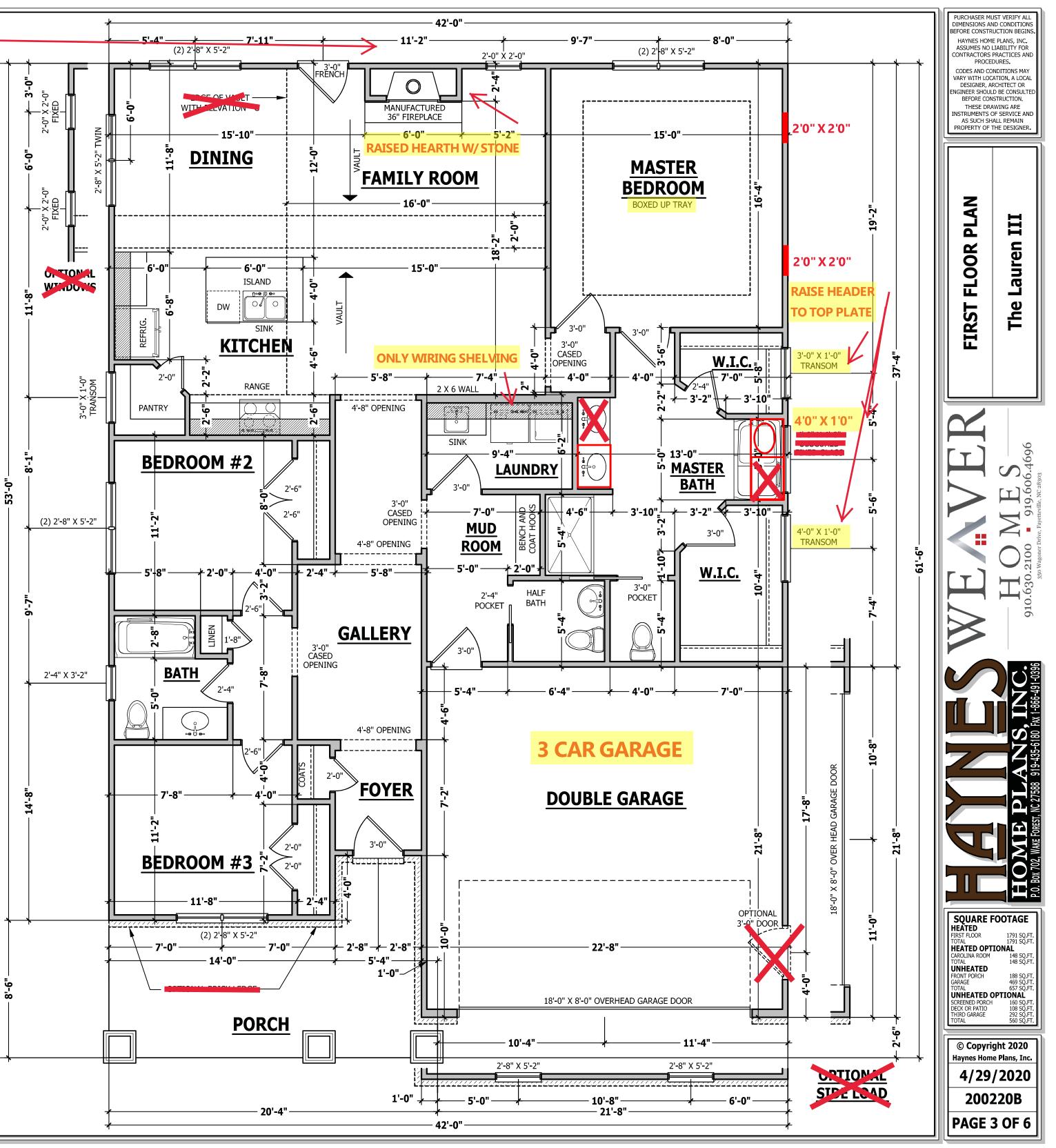
OTHER PENETRATIONS. Penetrations through the separation required in Section R302.6 shall be protected as required by Section R302.11, Item 4.

SQUARE	FOOTAGE
UEATEN	

HEATED 1791 SQ FT 1791 SQ FT FIRST FLOOR TOTAL **HEATED OPTIONAL** 148 SQ FT 148 SQ FT CAROLINA ROOM TOTAL UNHEATED 188 SQ.FT. 469 SQ.FT. 657 SQ.FT. FRONT PORCH GARAGE TOTAL UNHEATED OPTIONAL SCREENED PORCH 160 SQ.FT. 108 SQ FT 292 SQ FT DECK OR PATIO THIRD GARAGE TOTAL 560 SQ.FT.



51



STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code. JOB SITE PRACTICES AND SAFETY: Haynes Home Plans,

Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist

layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 $1/2" \times 3 1/2" \times 1/4"$ steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 $1/2" \times 3 1/2" \times 1/4"$ steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick.

CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length. **HD:** 800 lbs hold down hold down device fastened to the edge

of the brace wall panel closets to the corner. **Methods** Per Table R602.10.1

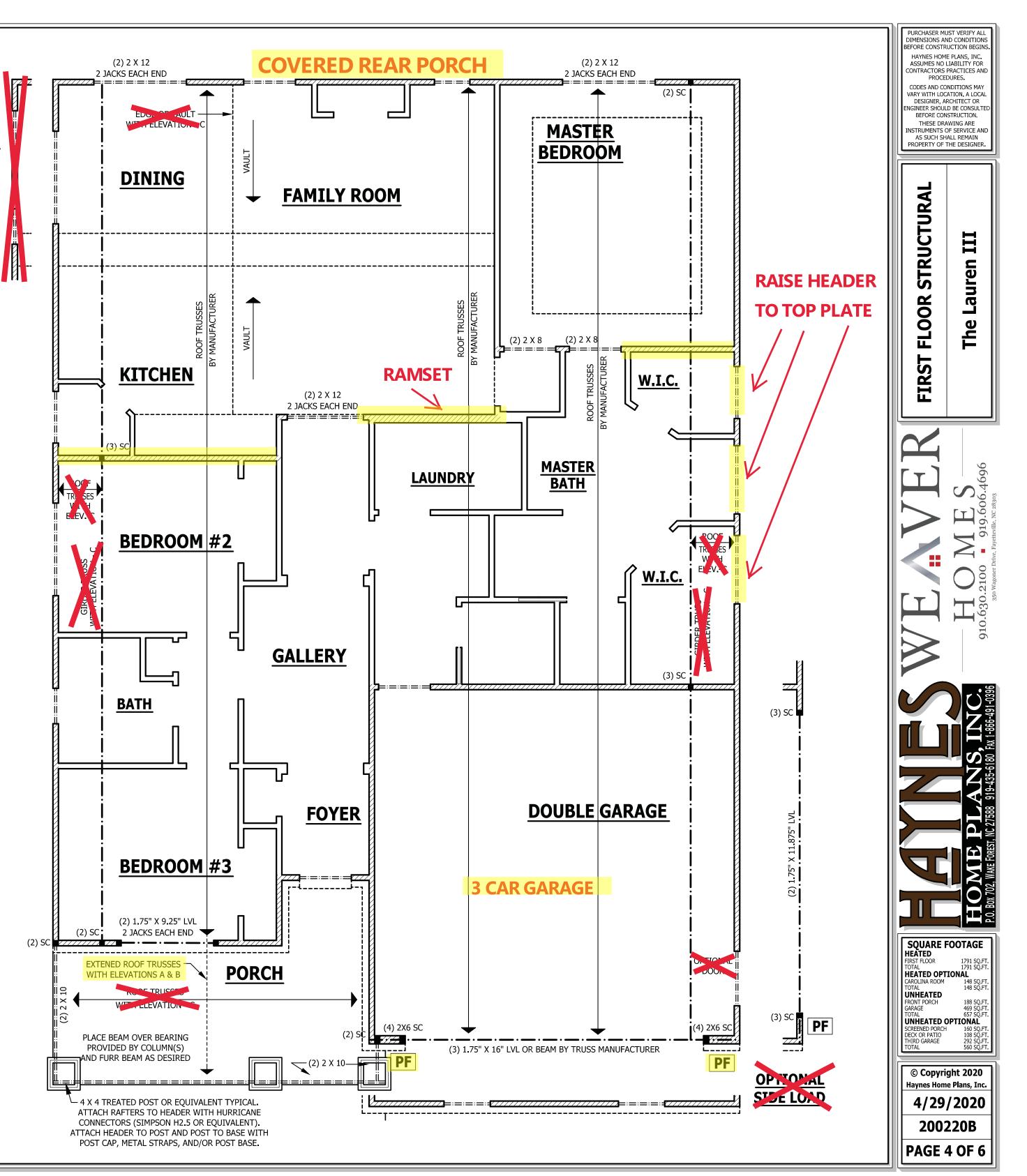
CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or $8d(2 \ 1/2" \ long \ x \ 0.113" \ diameter)$. **CS-SFB:** Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 $1/2" \ long \ x \ 0.12"$ diameter galvanized roofing nails.

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. **PF:** Portal fame per figure R602.10.1

- 6-16D SINKER NAILS FROM KING STUD TO HEADER-PONY WALL HEIGHT TO VARY HEADER PER PLAN - STAP HEADER TO JACK -STUD ON INSIDE 1000 LBS OR Р Р 4000 LBS WITH PONY WALL. : 16D 3" 0 **о** ТОР TOP FASTEN SHEATHING TO-Р© **17** HEADER WITH 8D COMMON ဥ**်** ROWS NAILS NAIL IN 3" GRID AND TO HEIGHT . неіднт ⁻ ---- **10'** FRAMING AT 3" ON CENTER - OPTIONAL SPLICE WITHIN -24" OF MIDDLE OF WALL HEIGHT – JACK STUDS PER PLAN – -SHEATHING DIRECTION -- ANCHORAGE PER FOUNDATION **PORTAL FRAME AT OPENING** PF (METHOD PF PER FIGURE AND SECTION R602.10.1) SCALE 1/4" = 1'-0" **EXTERIOR HEADERS** - (2) 2 X 6 WITH 1 JACK STUD EACH END **UNLESS NOTED OTHERWISE** - KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6 **INTERIOR HEADERS** - LOAD BEARING HEADERS (2) 2 X 6 WITH **1 JACK STUD AND 1 KING STUD EACH END** UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED

FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0"

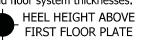


ROOF TRUSS REQUIREMENTS

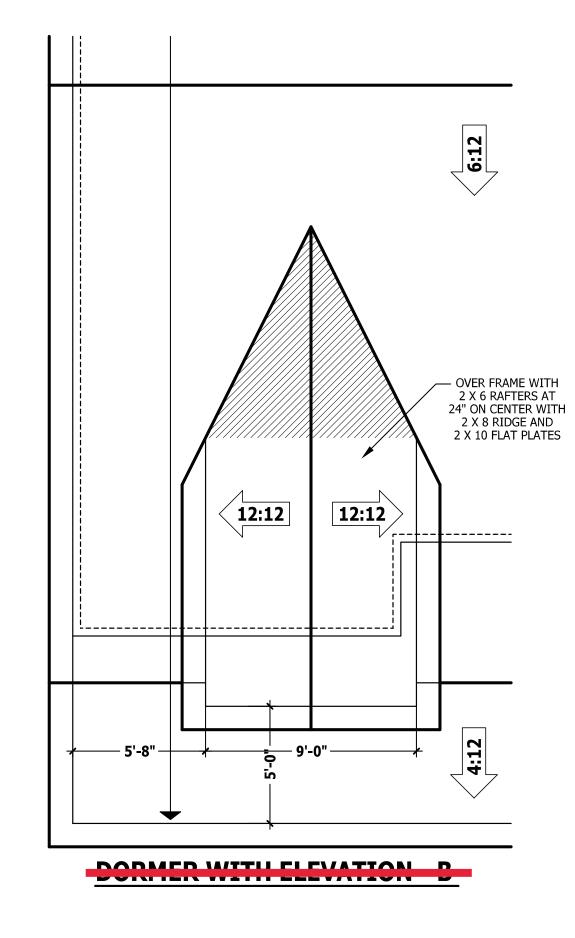
TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

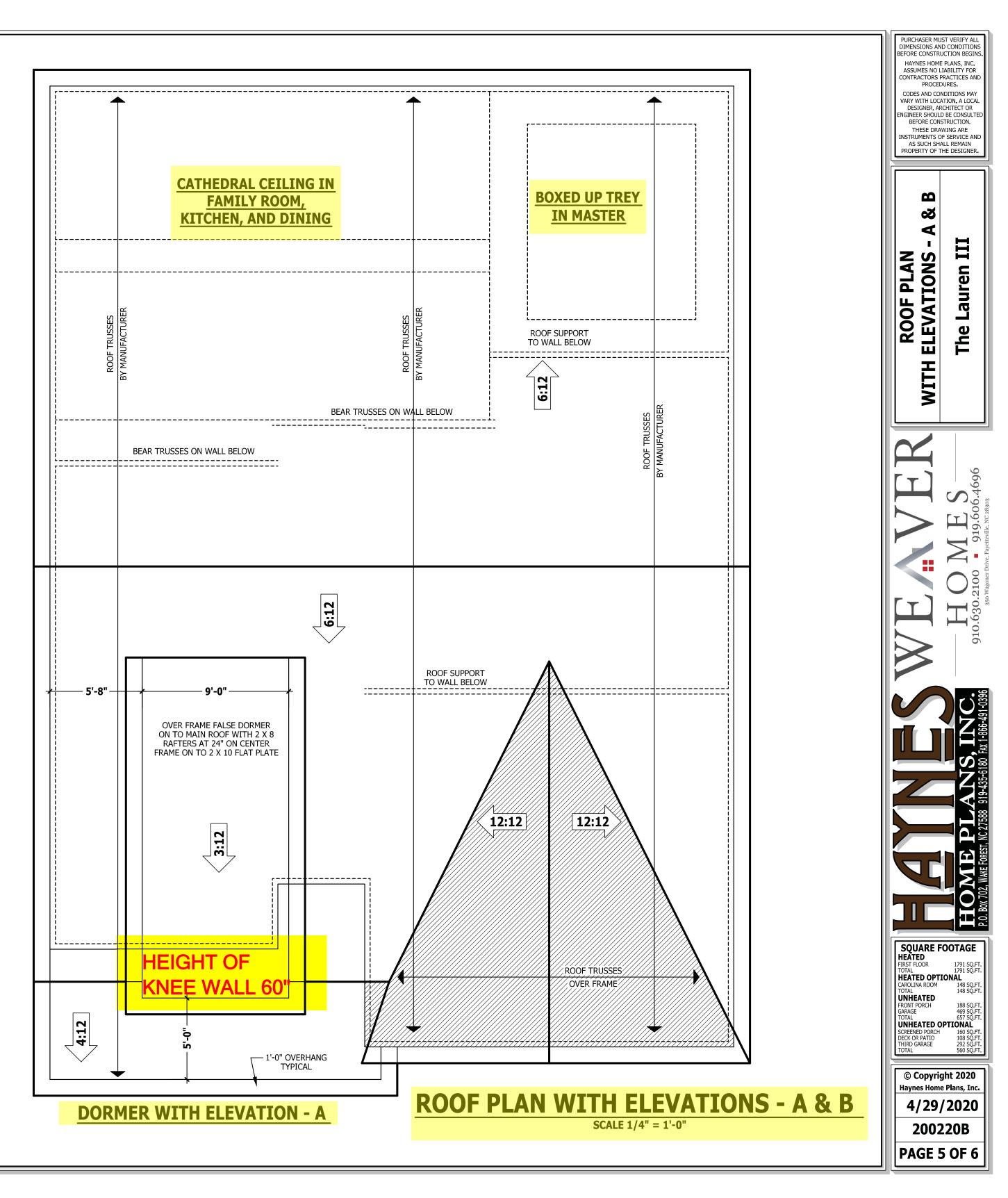
ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

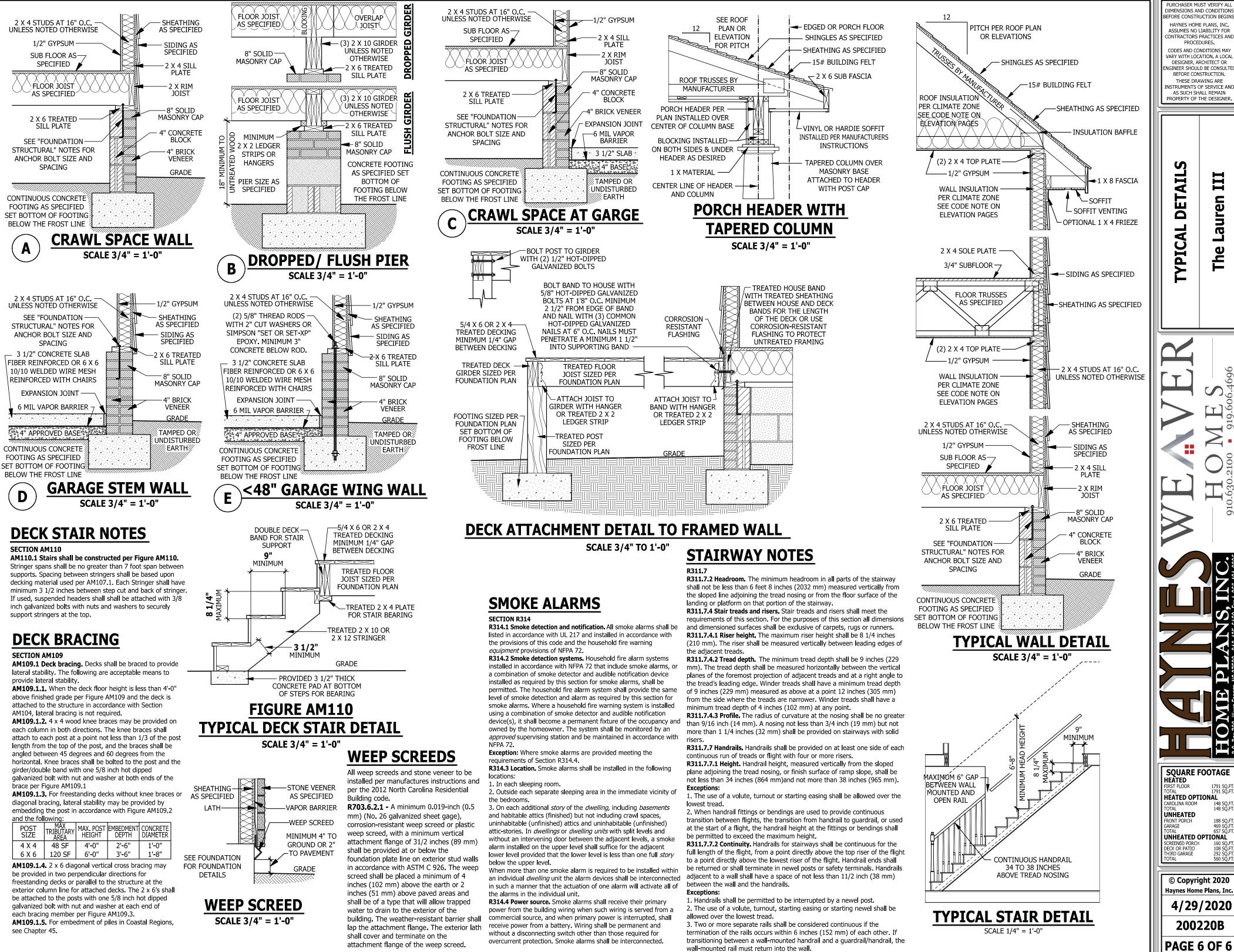
Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.











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Lauren

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1791 SQ.F 1791 SQ.F

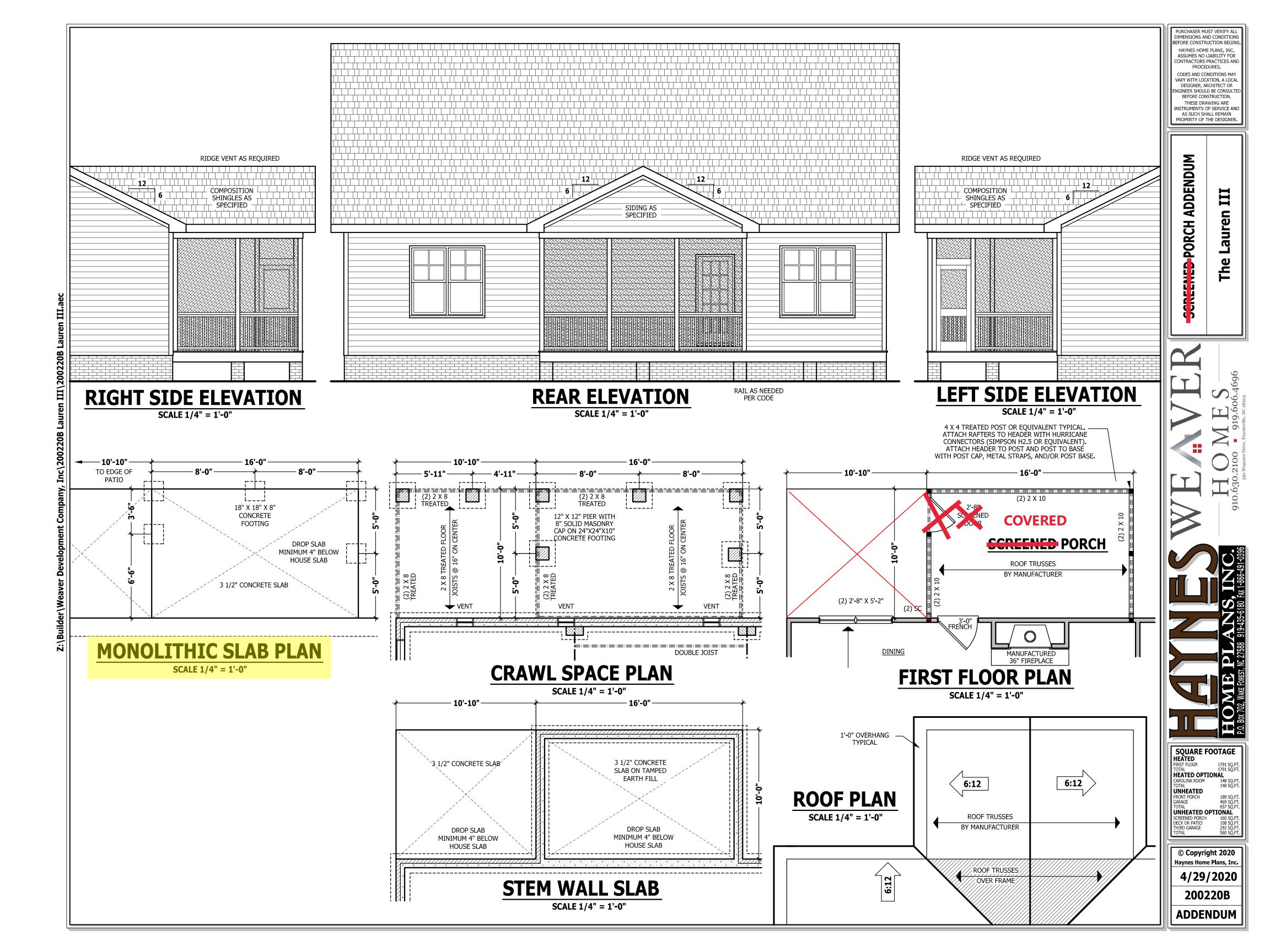
148 SQ.FT 148 SQ.FT

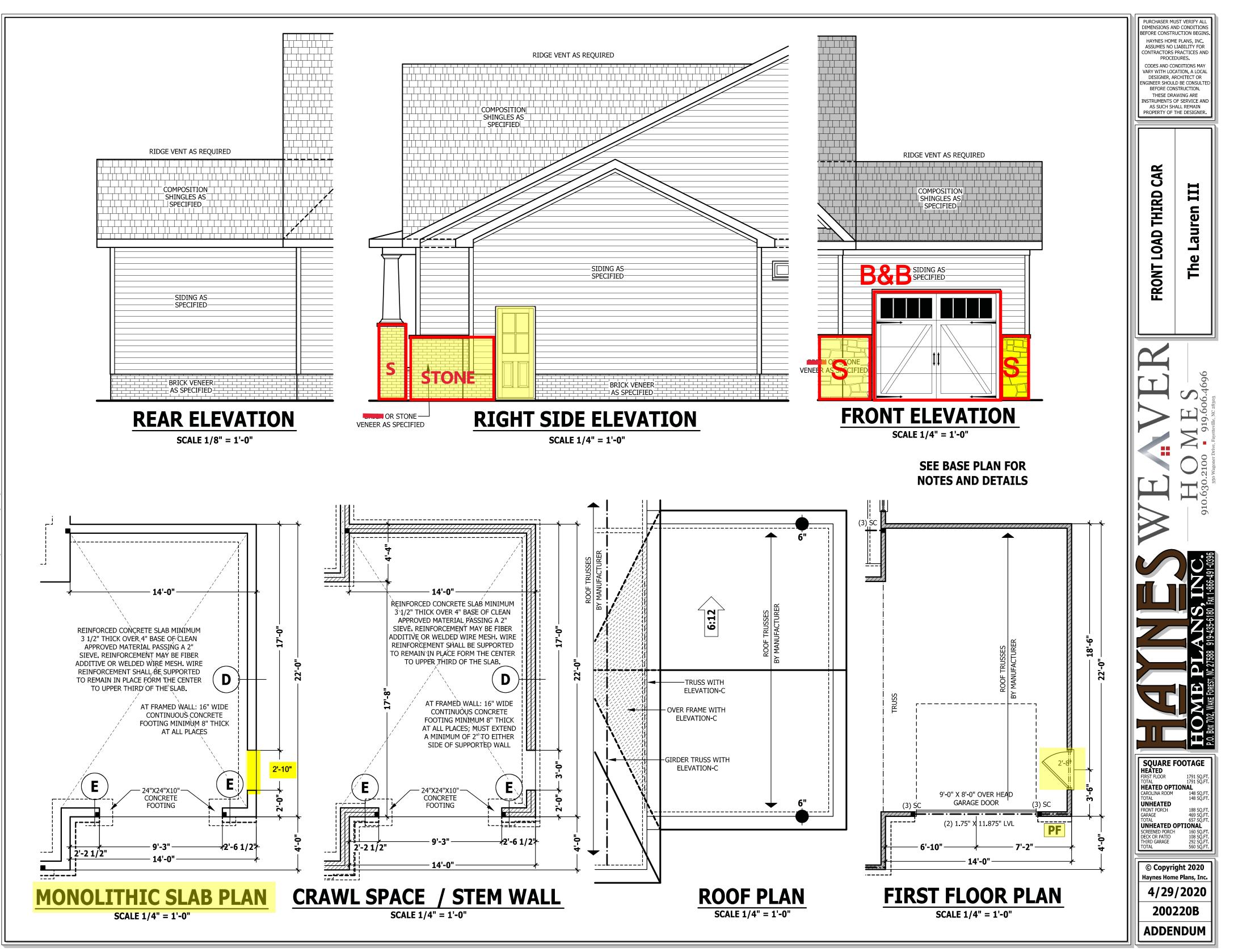
188 SO F

108 SO.

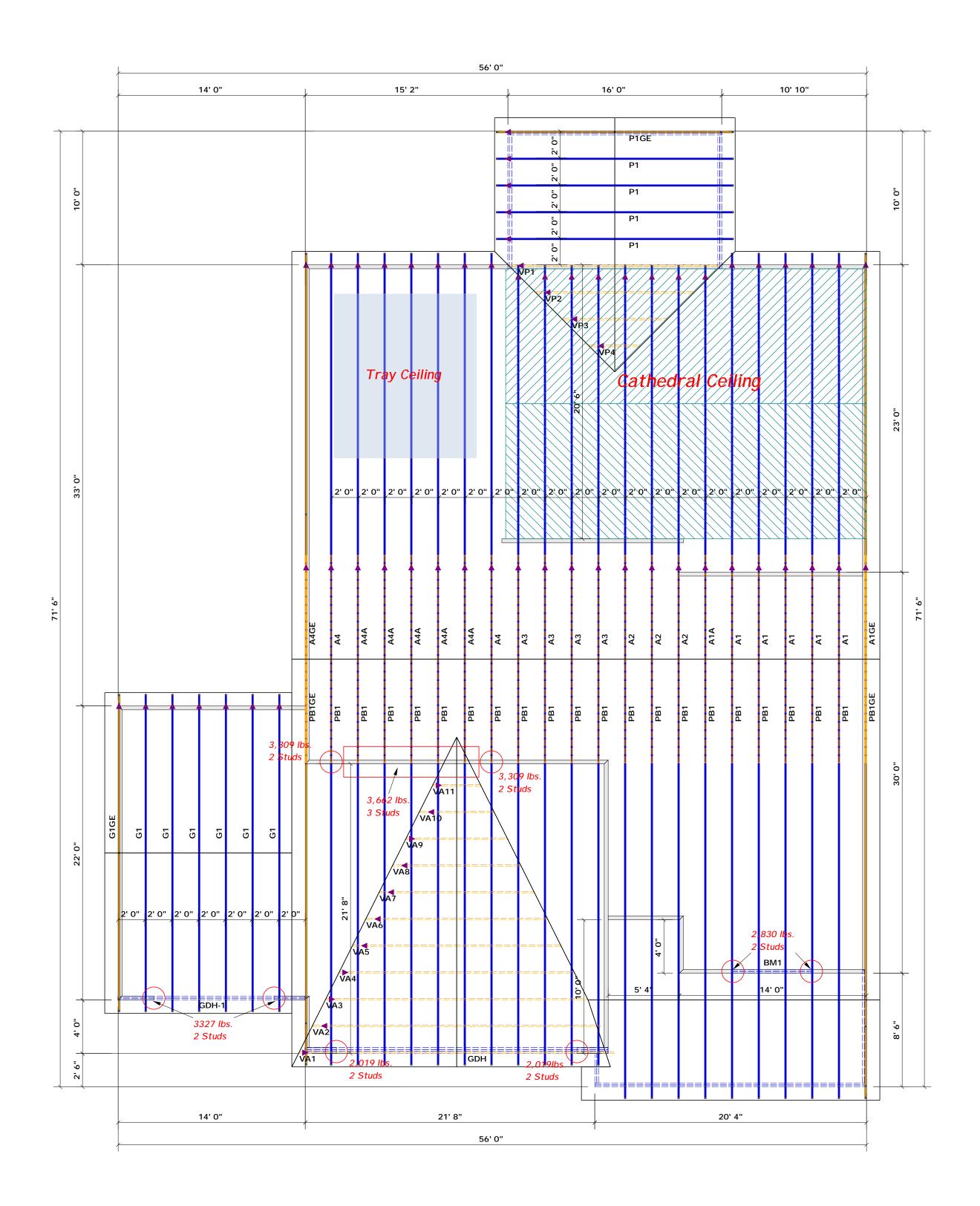
292 SQ FT 560 SO FT

wall-mounted rail must return into the wall.





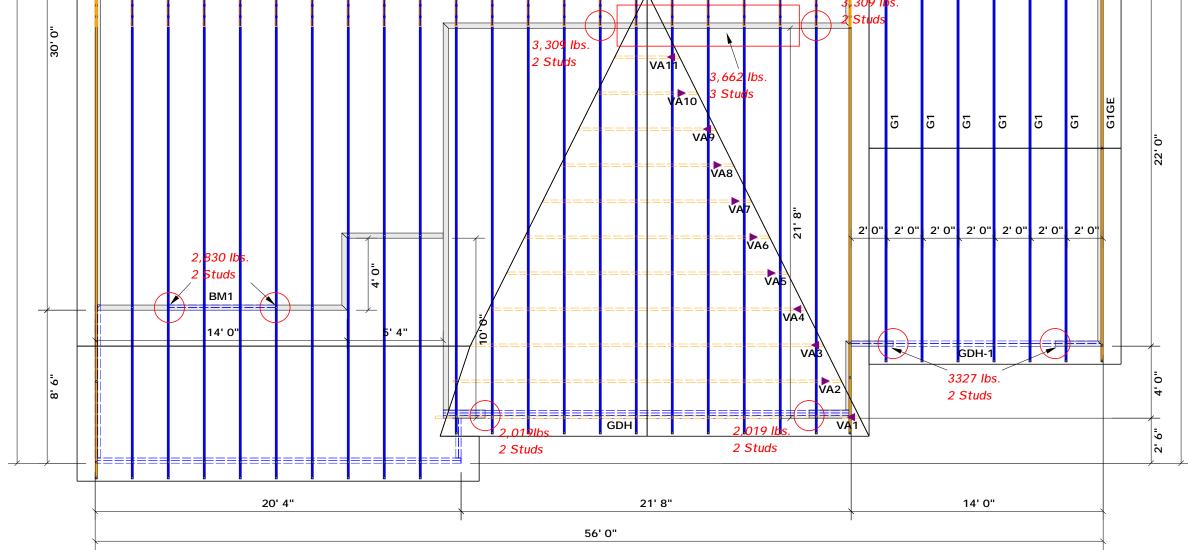
Z:\Builder\Weaver Development Company, Inc\200220B Lauren III\200220B Lauren III.



Г					Beam Legend		
	All Truss Reactions are Less		PlotID	Length	Product	Plies	Net Qty
	than 3,000 lbs. Unless Noted Otherwise.		BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
▲ = Denotes Left End of Truss			GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
(Reference Engineered Truss Drawing)	Denotes Reaction Greater than 3,000 lbs.		GDH	23' 0"	1-3/4"x 16" LVL Kerto-S	3	3
Do Not Erect Trusses Backwards		Truss Placement Plan SCALE: 3/16" = 1'	L				

6	CHART FOR JAG BASES ON 1 ABLES (\$502.5) OF JACK STUDG (COURCE)	0.4.000	BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
ND TON 10 TON 10 FOUND	FEADER/STRDER	100 Lack	JOB NAME	Lot 1B Williams Farm	ADDRESS	Lot 1B Williams Farm	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			PLAN	Lauren III / Elev. A / 3 Car / CP	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	4/29/20	DATE REV.	11	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9			JOB #	J0521-2779	SALESMAN	Lenny Norris	Signature Curtis Quick	Fax: (910) 864-4444

+															56	' 0"								
X				10' 10)"						16' 0"				\uparrow			15	' 2"				14' 0"	
										P1GE			2' 0"		┿══╬									
10'0"										P1 P1			2'0" 2'0"											10' 0"
	3							8	9	P1 P1		{	2' 0" 2' 0"	g										
													VP2	VPI										
-0						Cat	hee	t ra	Ce	ilin		20'.6"						Tray	Cei	ling				
23																								-0
	2'0	y" 2	Y 04	2'0"	2'0"	2'0"	2' 0"	2 [*] 0 [*]	2,0,	2'0"	2'0"	2'0"	2'0"	2'0"	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"	2' 0"			33.
A1GE		A1	A1	A1	A1	A1	A1A	A2	A2	A2	A3	A3	A3	A3	A4	A4A	A4A	A4A	A4A	A4A	A4	A4GE		
PB1GE		PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1	PB1GE		



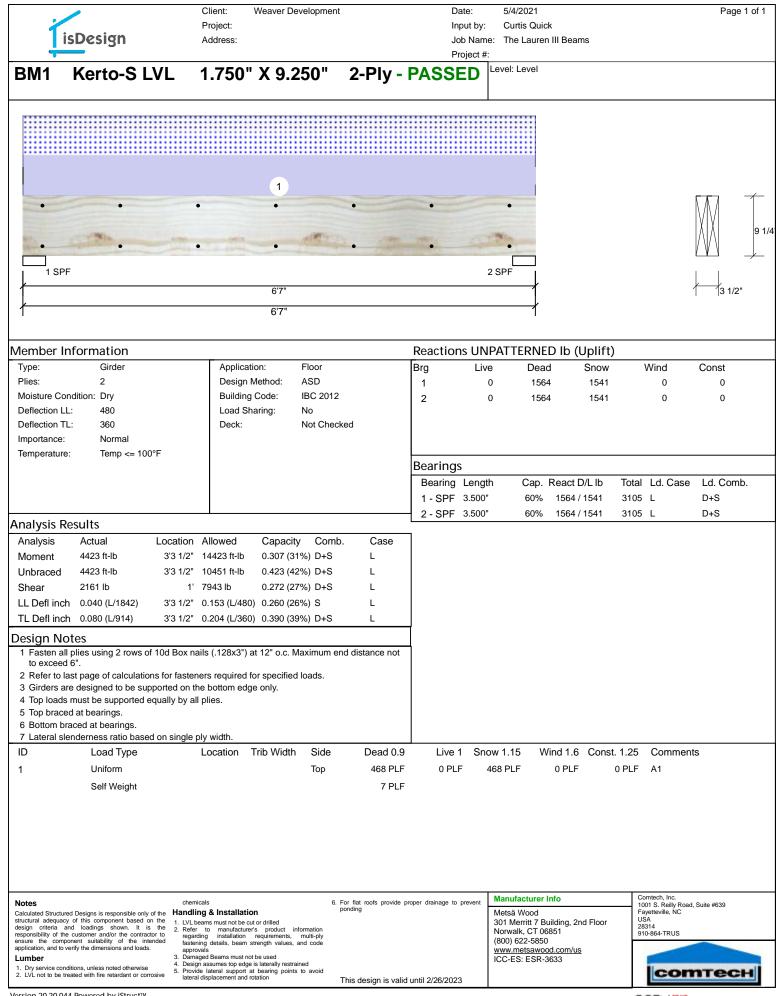
Г					Beam Legend		
	All Truss Reactions are Less		PlotID	Length	Product	Plies	Net Qty
	than 3,000 lbs. Unless Noted Otherwise.		BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
▲ = Denotes Left End of Truss			GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
(Reference Engineered Truss Drawing)	Denotes Reaction Greater than 3,000 lbs.		GDH	23' 0"	1-3/4"x 16" LVL Kerto-S	3	3
Do Not Erect Trusses Backwards		Truss Placement Plan SCALE: 3/16" = 1'	L				

	(04NEb c	RT FOR JAC ON 1 ABLES (2502-51) X STUDO (2000)) 4 (bi))	BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
z		FEADERVEIRDER	ACTOON 105 FOC	JOB NAME	Lot 1B Williams Farm	ADDRESS	Lot 1B Williams Farm	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
RND RUA AU	150 28	and grade and grade to and grade	UN NU UN UN UN	PLAN	Lauren III / Elev. A / 3 Car / CP	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
170 340 510	02	2550 1 5100 2 7650 3	3400 1 6600 2 10200 3	SEAL DATE	4/29/20	DATE REV.	/ /	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
	05	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
1190 1360 1530	8 00			JOB #	J0521-2779	SALESMAN	Lenny Norris	Signature Curtis Quick	Fax: (910) 864-4444

Í	sDesign	Client: Weaver Development Project: Address:	Date: 5/4/2021 Page Input by: Curtis Quick Job Name: The Lauren III Beams
		750" V 46 000" 2 Dby	Project #:
GDH	Kerto-5 LVL	.750" X 16.000" 3-Ply	- PASSED
		1	
•	• • •	• • • • • • •	· · · · · · · · · · · · · · · · · · ·
	- THE	1000	
1 SPF			
<u> </u>		18'10"	5 1/4"
r		18'10"	ſ
ember lı _{ype:}	nformation _{Girder}	Application: Floor	Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Snow Wind Const
lies:	3	Design Method: ASD	1 0 1127 951 0 0
Noisture Co	ndition: Dry L: 480	Building Code: IBC 2012 Load Sharing: Yes	2 0 1127 951 0 0
eflection T		Deck: Not Checked	
nportance:			
emperature	e: Temp <= 100°F		Bearings
			Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb
			1 - SPF 3.500" 27% 1127 / 951 2078 L D+S
nalysis R	esults		2 - SPF 3.500" 27% 1127 / 951 2078 L D+S
Analysis	Actual Location		e
/loment		62010 ft-lb 0.151 (15%) D+S L 10984 ft-lb 0.850 (85%) D+S L	
Jnbraced Shear		10984 ft-lb 0.850 (85%) D+S L 20608 lb 0.084 (8%) D+S L	
		0.460 (L/480) 0.170 (17%) S L	
L Defl inc	h 0.171 (L/1288) 9'5 1/16"	0.613 (L/360) 0.280 (28%) D+S L	
esign No			
 Fasten al to exceed 		ils (.128x3") at 12" o.c. Maximum end distance r	lot
	ast page of calculations for fasten re designed to be supported on th		
3 Girders a 4 Top loads	s must be supported equally by all		
3 Girders a 4 Top loads 5 Top brace	s must be supported equally by all ed at bearings. raced at bearings.		
3 Girders a 4 Top loads 5 Top brace 6 Bottom br 7 Lateral slo	ed at bearings. raced at bearings. enderness ratio based on single p	•	
3 Girders a 4 Top loads 5 Top brace 6 Bottom bi	ed at bearings. raced at bearings. enderness ratio based on single p	oly width. Location Trib Width Side Dead Top 101	

GDH-1	Design	Address:	Job Name: The Lauren III Bea Project #:	ams
	Kerto-S LVL	1.750" X 11.875" 2-P	y - PASSED	
•	• • •		• • • •	••••
□ 1 SPF	170	and the second	and and	
<u> </u>		14'		3 1/2"
<u>}</u>		14'		
				(1. 115)
ember Inf	formation Girder	Application: Floor	Reactions UNPATTERNED Ib (Brg Live Dead	(Uplift) Snow Wind Const
lies:	2	Design Method: ASD	1 0 1696	1631 0 0
loisture Conc eflection LL:	dition: Dry 480	Building Code: IBC 2012 Load Sharing: No	2 0 1696	1631 0 0
eflection TL:		Deck: Not Checked		
nportance:	Normal			
emperature:	Temp <= 100°F		Bearings	
			Bearing Length Cap. React	D/L lb Total Ld. Case Ld. Comb.
				/1631 3327 L D+S
nalysis Re	sults		2 - SPF 3.500" 64% 1696	/1631 3327 L D+S
Analysis		Allowed Capacity Comb. Ca	e	
loment Inbraced		' 22897 ft-lb 0.476 (48%) D+S L ' 10911 ft-lb 0.998 D+S L		
JIDIACEU		(100%)		
Shear		' 10197 lb 0.269 (27%) D+S L		
L Defl inch		' 0.339 (L/480) 0.580 (58%) S L ' 0.451 (L/360) 0.880 (88%) D+S L		
L Defl inch				
	lies using 2 rows of 10d Box r	nails (.128x3") at 12" o.c. Maximum end distance	not	
esign Not 1 Fasten all p		eners required for specified loads.		
esign Not Fasten all p to exceed 6		the hetter ender and the		
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