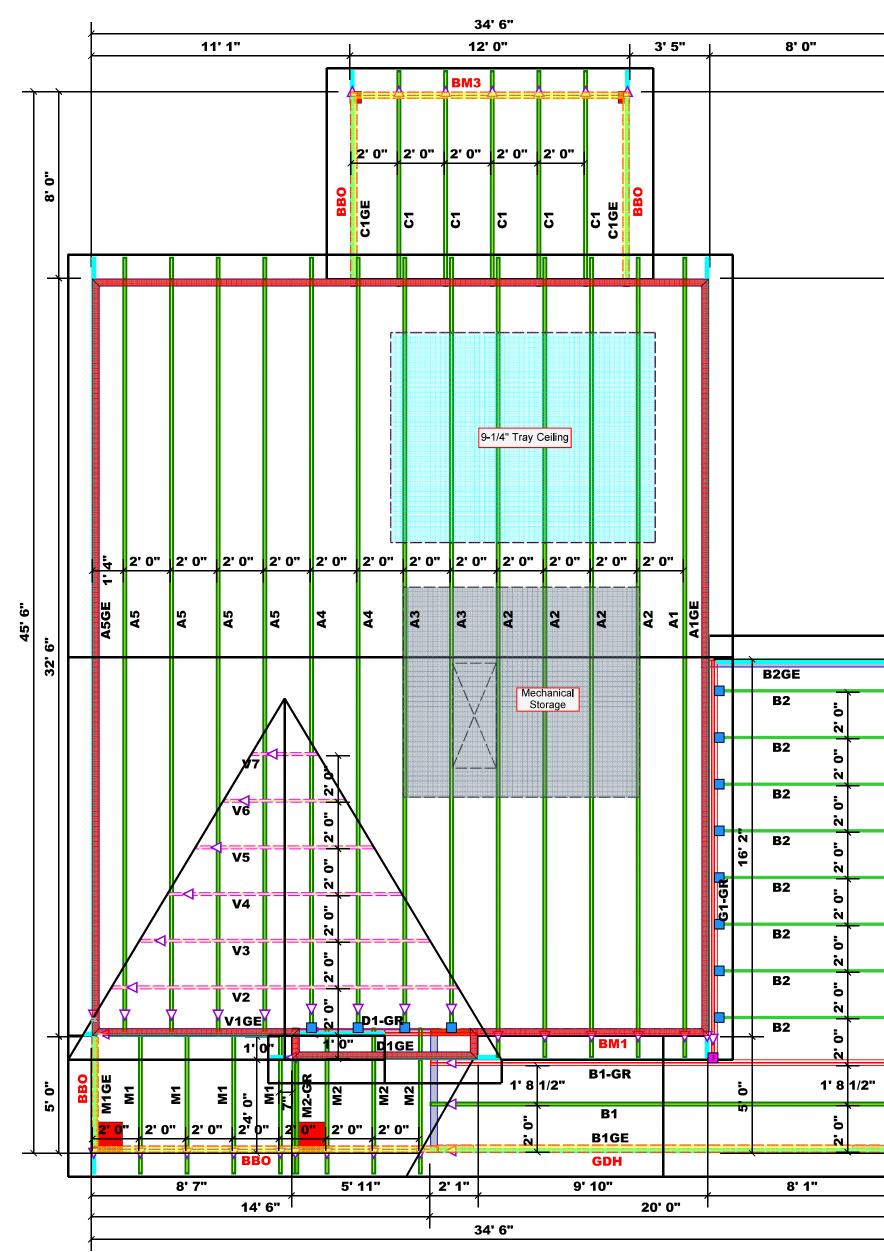


	Builder Development Co. Inc. CITY / CO. Harnett / Harnet	Build 	ROOE       Reilly Road         Fax: (910)         Baring reactions less th         Address of a comply with the survey of a complexity of a		HUS26       USP       12       NA       16d/3-1/2"       16d/3-1/2"         Image:	Ridge Line = 52.07 ft.       Hip Line = 0 ft.         Hip Line = 0 ft.       2nd Floor Walls         Horiz. OH = 115.69 ft.       Tray Ceiling         Decking = 58 sheets       Drop Beam         Connector Information       Nail Information         Sym       Product       Manuf       Qty         Supported       Header       Truss	Considered Load Bearing     Hatch Legend     Roof Area = 1692.08 sq.ft.	Dimension Notes  1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise		50 8		
		Reilly       Faye         Pho       Faye	ROOF         Reilly Road         Fayettevill         Phone: (9'         Fax: (910)         greations less th         do comply with th         (BASED ON TAL	BUILDER	S		Harnett / Harnett		м Нуча 1700 3400 5100 6800 8500 10200 11900 13600	LC	deeme require attache require size an reactio 15000# retaine reactio Tables retaine	T
ROOF & F         Reilly Road Indus         Fayetteville, N.C.         Phone: (910) 864         Fax: (910) 864         Bering reactions less than or equipments. The contractor shatatched Tables (derived from the preserved foods). A registered design proferrationed to design the support sy reactions that exceed 150007.         David L         IDAVID         Signature         David L         IDAVID	Corrections and industrian and indus	SEAL DATE REV. 05/11/21 DATE REV. 05/11/21 SEAL DATE REV. 05/11/21		QUOTE #		DRAWN BY	David Landry		R 3- 6- 13	andry ACK ST	riptive Co II refer to e prescrip ninimum f equired to not greate essional s stem for a ified in the ssional sl	LOO BEA 2830 54-878 -4444
Signature       David Landy         Signature       David Landy         According to the state of t	OF & FLOCK         SEES & BEA         Road Industrial P         tteville, N.C. 2830         ne: (910) 864-878         c: (910) 864-4444         Is less than or equal to 300         by with the prescriptive Cr         of determine the minimum of the order of the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the minimum of the support system for the contractor shall refer to (of determine the support system for the contractor shall refer to (of th	& FLOC & BEA Industrial P e, N.C. 2830 ID) 864-878 864-4444 an or equal to 300 he prescriptive CC from the prescriptive CC for the minimum istuds required to 007 JACK S istuds required to 008 JACK S istuds required to 009 JUII/SI istuds required to 007 JACK S istuds required to 008 JACK S istuds required to 009 JUII/SI istuds required to 009 JUII/SI istuds required to istuds	ACK ST Store A CK ST COC BEA 14444 141 to 300 150 to greater 150 to grea	JOB #	T0521_2780		l annu Nonnis		N2000 N012222223434(14) 4000 2200 26000 70000		ode the otive C founda o supp er than hall be any e attao hall be	0 <b>R M</b> 3 3 3 7

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards



B       Image: An image: A	L			T F	RO RUS Reilly F Fayet Phon	OF & SES Road In teville re: (910)	<b>&amp; FL</b> <b>&amp; B</b> ndustr v, N.C. 0) 864	OOF EAN 28309 -8787	<b>∧</b> S ∧S
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-5-1         -5-1 <td< td=""><td></td><td></td><td></td><td></td><td>(BASEI</td><td>D ON TABU</td><td>ES 85025(I</td><td>l) š (b))</td><td></td></td<>					(BASEI	D ON TABU	ES 85025(I	l) š (b))	
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Considered Load Bearing       Hatch Legend         Hatch Legend       Padded HVAC         Product Area = 169208 sat.       Padded HVAC         Indic. Connector Information       Tray Ceiling         Drop Beam       Tray Ceiling         Drop Beam       Tray Ceiling         Indic. Of 111568 ft.       Header Truss         Indic. Of 111568 ft.       Header Truss         Indic. Of 111568 ft.       Header Truss         Indic. Of 11157 ft.       Indic. Of 11158 ft.         Indic. Of 111 Minute       Of 1112 ft.         Indic. Of 111 Minute       Of 1112 ft.         Indic. Of 111 Minute       Indic. Of 111 Minute         Indic. I			<ol> <li>All exterior wall to wall dimensions are to face of sheathing unless noted otherwise</li> <li>All interior wall dimensions are to face of frame wall unless noted otherwise</li> <li>All exterior wall to truss dimensions are to</li> </ol>	nett	ק				
Hou Line = 0 ft, Houz, OH = 1156, 9 ft, Rated OH = 175, 3 ft, Decking = 56 sheets       Image: Connector Information         Sym       Froduct Manuf Quy       Supported Member       Header       Truss         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets </td <td><b>&gt;</b></td> <td></td> <td></td> <td> </td> <td>/ire Roc</td> <td></td> <td>21</td> <td>andry</td> <td>Vorris</td>	<b>&gt;</b>				/ire Roc		21	andry	Vorris
Hou Line = 0 ft, Houz, OH = 1156, 9 ft, Rated OH = 175, 3 ft, Decking = 56 sheets       Image: Connector Information         Sym       Froduct Manuf Quy       Supported Member       Header       Truss         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets       Image: Sheets       Image: Sheets       Image: Sheets         Image: Drop Beam       Image: Sheets       Image: Sheets </td <td></td> <td></td> <td>Roof Area = 1692.08 sq.ft. Padded HVAC</td> <td>Jannet</td> <td></td> <td>Roof</td> <td>05/11/</td> <td>David L</td> <td>-enny 1</td>			Roof Area = 1692.08 sq.ft. Padded HVAC	Jannet		Roof	05/11/	David L	-enny 1
Connector Information       Nail Information         Sym       Product       Manuf Quy       Supported Member       Header       Truss         HUS26       USP 12       NA       16d/3-1/2"       16d/3-1/2"         Image: HUB210-3       USP 1       Varies       16d/3-1/2"       16d/3-1/2"         Image: HUB210-3       USP 1       Varies       2       2         Image: HUB210-3       Image: HUB210			Hip Line = 0 ft. Horiz. OH = 115.69 ft. Raked OH = 175.3 ft.	•					
Sym       Product       Manuf       Qty       Supported Member       Header       Truss         HUS26       USP       12       NA       16d/3-1/2"       16d/3-1/2"         Image: HUS26       USP       1       Varies       Net Qty       1         Image: HUS26<					DRES	DEL	TE RE	AWN	LES R
N       HUS26       USP       12       NA       16d/3-1/2"       16d/3-1/2"         Image: Constraint of the state			Sum Draduat Manuf Ot Supported Header Truce	IJ	AD	Ň	۵	AD	S¢
Products       Product       Plies       Net Qty         BM1       12'0"       1-3/4"x 16" LVL Kerto-S       2       2         BM3       12'0"       2x10 SPF No.2       2       2         GDH       20'0"       1-3/4"x 11-7/8" LVL Kerto-S       2       2         Mage       Truss Placement Plan       -       -       -         J       Truss Placement Plan       -       -       -       -         J       10       I/4"=1'       -       -       -       -         J       10       1/4"=1'       -       -       -       -       -         J       10       1/4"=1'       -       -       -       -       -       -         J       10       1/4"=1'       -	3		HUS26 USP 12 NA 16d/3-1/2" 16d/3-1/2"						
PlotID       Length       Product       Plies       Net Qty         BM1       12' 0"       1-3/4"x 16" LVL Kerto-S       2       2         BM2       15' 0"       1-3/4"x 16" LVL Kerto-S       2       2         BM3       12' 0"       2x10 SPF No.2       2       2         GDH       20' 0"       1-3/4"x 11-7/8" LVL Kerto-S       2       2         Mage: 1/4 "=1'       Truss Placement Plan       Scale: 1/4"=1'       H <td></td> <td></td> <td></td> <td>ا ن</td> <td></td> <td></td> <td></td> <td></td> <td></td>				ا ن					
BM2 15'0" 1-3/4"x 16" LVL Kerto-S 2 2 BM3 12'0" 2x10 SPF No.2 2 2 GDH 20'0" 1-3/4"x 11-7/8" LVL Kerto-S 2 2 Truss Placement Plan Scale: 1/4"=1' BM2 15'0" 2x10 SPF No.2 2 2 Truss Placement Plan Scale: 1/4"=1' BM2 15'0" 1-3/4"x 11-7/8" LVL Kerto-S 2 2			PlotID Length Product Plies Net Qty	н					
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THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards



**Trenco** 818 Soundside Rd Edenton, NC 27932

Re: J0521-2780 Lot 1 Wire Road

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15716428 thru E15716455

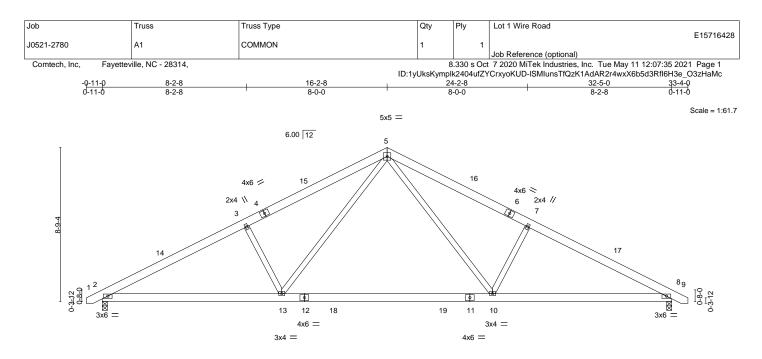
My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 11,2021

Gilbert, Eric **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



	<u>10-2-8</u> 10-2-8		22-2-8 12-0-0		32-5-0 10-2-8	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.28 BC 0.64 WB 0.27 Matrix-S	Vert(CT) -0.47 10-13 Horz(CT) 0.05 8	l/defl L/d >999 360 >824 240 n/a n/a >999 240	MT20 24	<b>RIP</b> 44/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 WFBS

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at

joint 8. 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



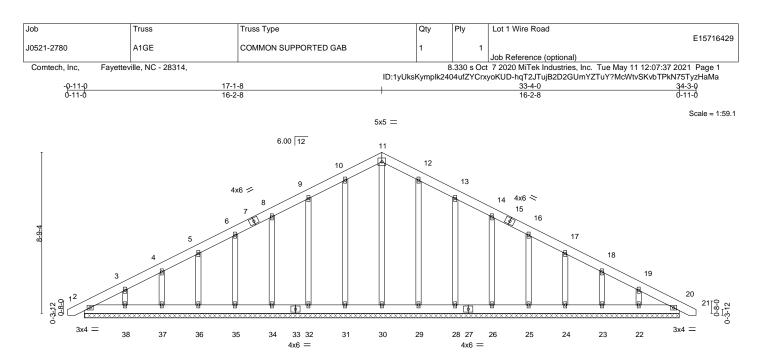
Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 11,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





-0-11-0 0-11-0			33-4-0 32-5-0						34-3-0 0-11-0
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.04 BC 0.02 WB 0.16 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 20 20 20	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 258 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-			BRACING-						

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS.

All bearings 32-5-0. Max Horz 2=-171(LC 17) (lb) -

- Max Uplift All uplift 100 lb or less at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22 Max Grav All reactions 250 lb or less at joint(s) 2, 30, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22, 20
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

TOP CHORD 10-11=-114/284, 11-12=-114/284

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22,
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



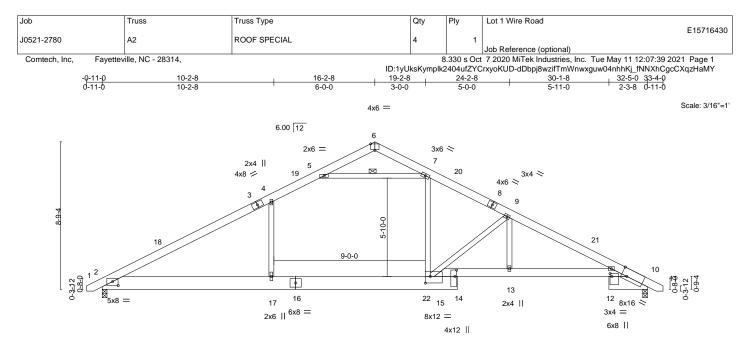
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 11,2021

MARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTE k® connectors. This sk0 includue unterpretended by the presence includue of the sk0 includue and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	10-2-8 10-2-8	16-2-8	19-2-8	21-1-8		24-2-8		-1-8 1-0	32-5-0	4
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0-4						5-	1-0	2-3-0	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.75 BC 0.67 WB 0.70 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.21 -0.38 0.09 0.17	(loc) 17 17 10 2-17	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	MT	ATES 20 ght: 247 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD 2x10 10-11 WEBS 2x4 5 REACTIONS. (s Max Max	SP No.1 SP No.1 *Except* 5: 2x6 SP 2400F 2.0E SP No.2 ize) 2=0-3-8, 10=0-3-8 Horz 2=-110(LC 10) Uplift 2=-90(LC 12), 10=-90(LC 13) Grav 2=1393(LC 2), 10=1353(LC 2)		BRACING- TOP CHOR BOT CHOR WEBS	RD RD	Rigid c		I sheathing d ectly applied t			oc purlins.
TOP CHORD 2-4 BOT CHORD 2-1 WEBS 4-1 NOTES-	<ul> <li>x. Comp./Max. Ten All forces 250 (lb) or =-2217/403, 4-5=-1870/483, 7-9=-2258/5 7=-193/1848, 15-17=-195/1860, 13-15=-3 7=-29/402, 7-15=-114/967, 9-15=-1075/2</li> <li>ve loads have been considered for this definition of the set of th</li></ul>	19, 9-10= <sup>-</sup> 2889/551 771/2525, 10-13=-380/2525 32, 9-13=0/616, 5-7=-1955/-	459							

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; 1CDL=6.0pst; BCDL=6.0pst; BcDL=6.10st; h=15t; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

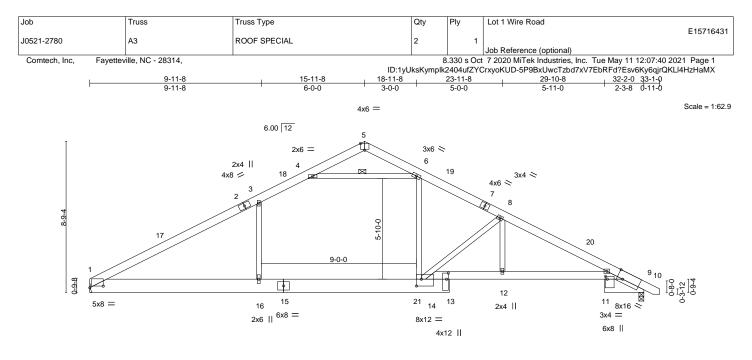
6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	9-11-8	18-11-8	20-10-	-8 23-11-8	29-10-8	32-2-0	
	9-11-8	9-0-0	1-11-	0 3-1-0	5-11-0	2-3-8	
Plate Offsets (X,Y)	[1:0-9-6,0-1-2], [5:0-3-0,Edge], [9:0-4-0	Edge], [11:0-2-0,0-1-4], [13	3:0-4-4,0-1-4], [14:0-3	-8,0-4-12]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.76 BC 0.66 WB 0.70	DEFL. Vert(LL) -0.2 Vert(CT) -0.3 Horz(CT) 0.0	6 16 >99	9 360 9 240	<b>PLATES</b> MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.1	6 1-16 >99	9 240	Weight: 243 lb	FT = 20%
9-14:2	SP No.1 *Except* 2x6 SP 2400F 2.0E P No.2		BRACING- TOP CHORD BOT CHORD WEBS		directly applied or	ctly applied or 3-10-0 10-0-0 oc bracing. 5	) oc purlins.
Max ( Max 0 ORCES. (lb) - Max OP CHORD 1-3= 30T CHORD 1-16	Horz 1=-111(LC 8) Jplift 1=-76(LC 12), 9=-90(LC 13) Grav 1=1345(LC 2), 9=1347(LC 2) . Comp./Max. Ten All forces 250 (lb) oi -2189/401, 3-4=-1853/491, 6-8=-2237/5 =-198/1827, 14-16=-200/1839, 12-14=-3 =-117/966, 3-16=-53/392, 4-6=-1931/47	17, 8-9=-2874/549 875/2511, 9-12=-384/2511	/620				
<ol> <li>Wind: ASCE 7-10; MWFRS (envelope to 32-10-10 zone;C</li> <li>This truss has beer</li> <li>* This truss has beer will fit between the</li> </ol>	re loads have been considered for this dr Vult=130mph (3-second gust) Vasd=103 ) and C-C Exterior(2) 0-0-12 to 4-5-9, int C-C for members and forces & MWFRS for designed for a 10.0 psf bottom chord liv en designed for a live load of 30.0psf on bottom chord and any other members, w	mph; TCDL=6.0psf; BCDL erior(1) 4-5-9 to 15-11-8, E or reactions shown; Lumber re load nonconcurrent with the bottom chord in all area	xterior(2) 15-11-8 to 2 r DOL=1.60 plate grip any other live loads.	20-4-5, Interior( DOL=1.60	1) 20-4-5		

5) Refer to girder(s) for truss to truss connections.

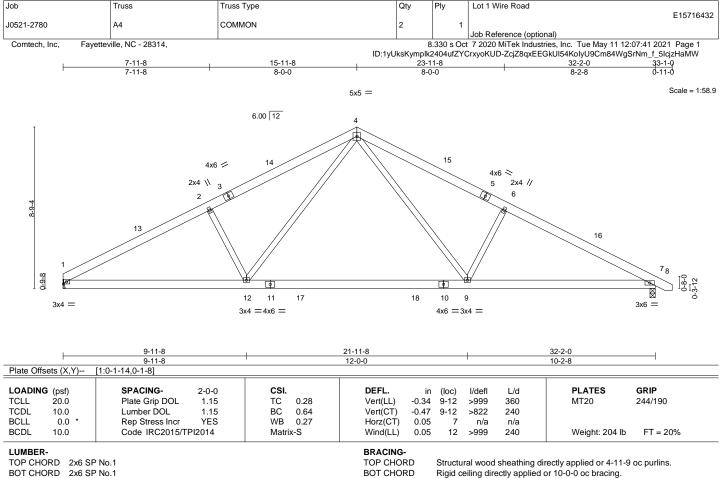
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss evaluat and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rector and bracing of trusses and truss systems, see **ANSTPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932



BOT CHORD 2x6 SP No.1 2x4 SP No 2 WEBS

REACTIONS. 1=Mechanical, 7=0-3-8 (size) Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13) Max Grav 1=1278(LC 1), 7=1331(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

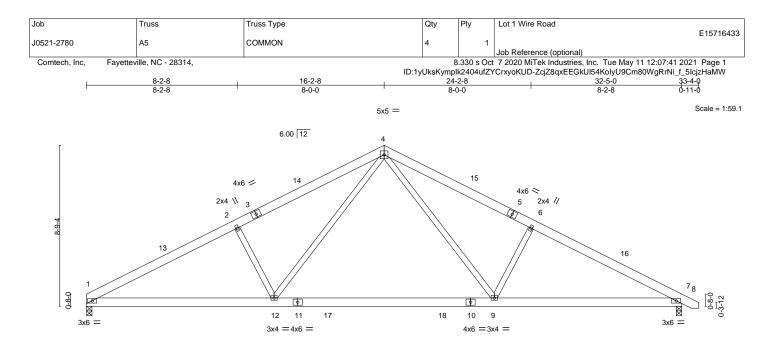
5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTerk® connectors. This design is back into ALD DED will the REFERENCE FAGE MIF/473 few. 519/2020 beFVRE USE. Design valid for use only with MTerk® connectors. This design is backed only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Compon Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	10-2-8 10-2-8	22-2-8 12-0-0			4	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.65 Vert(CT) -0 WB 0.27 Horz(CT) 0	in (loc) 0.34 9-12 0.47 9-12 0.05 7 0.05 12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	PLATES GRII MT20 244/ Weight: 206 lb FT	

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

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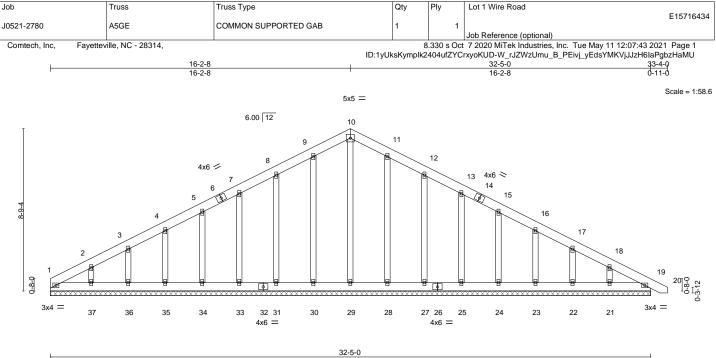
Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 11,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			32-5-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES	CSI. TC 0.04 BC 0.02 WB 0.16	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         0.00         19         n/r         120           Vert(CT)         0.00         19         n/r         120           Horz(CT)         0.00         19         n/a         n/a	PLATES         GRIP           MT20         244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	· · ·	Weight: 256 lb FT = 20%
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 32-5-0

- Max Horz 1=-175(LC 17) (lb) -
  - Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except 37=-101(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22,

21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-114/284, 10-11=-114/284

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.

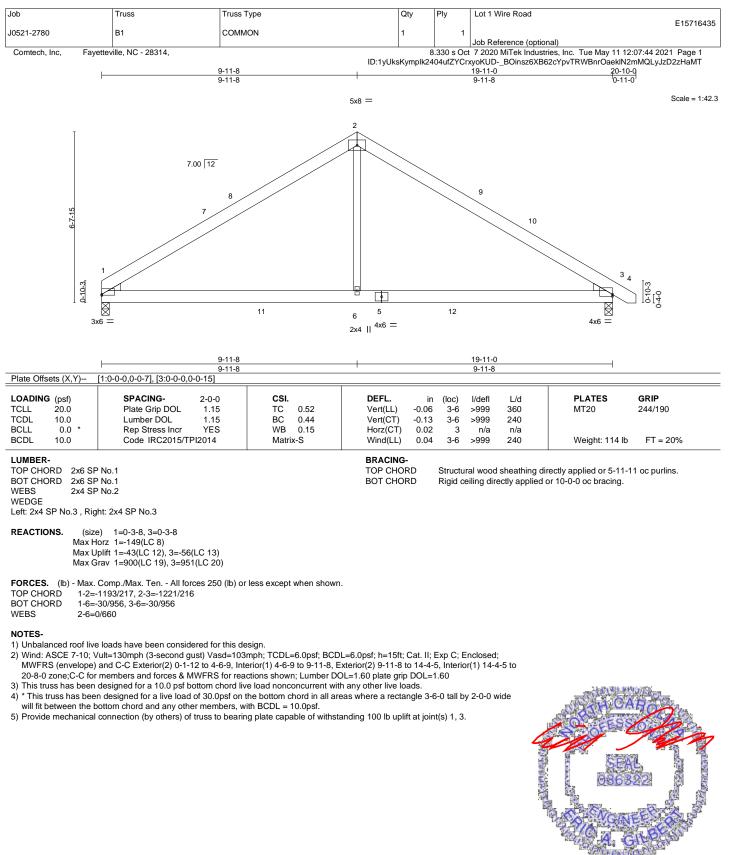


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid of use only with MTek® connectors. This sket intoCLOBED with REFERENCE FAGE MIF 4/3 FeV, 519/2020 BEFORE 052. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Braching indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





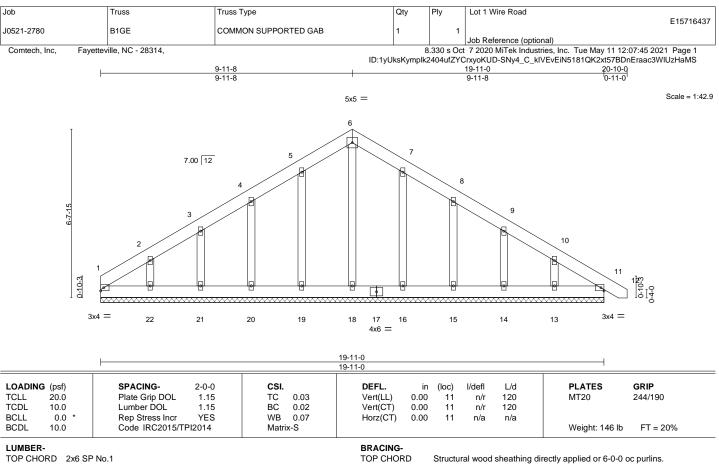
May 11,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type		Qty	Ply	Lot 1 Wire Road		
JOD J0521-2780	B1-GR	COMMON GIRDER		Qty 1		FOL 1 MILE KOSO		E15716436
	Fayetteville, NC - 28314,			•	220 c Or	Job Reference (option 7 2020 MiTek Industrie		12-07-46 2021 Dees 4
Comtech, Inc,	4-11-8	9-11-8	ID:1yUksKym		IfZYCrxyok	UD-wZWSBY?M3pMm	rsyHbsYfsGU_oXTP	WaWjpGo3HwzHaMR
F	4-11-8	5-0-0		5-0-0			11-8	<u>20-10-0</u> 0-11-0
			5x12					Scale = 1:39.5
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	7.00 12		/\`\\\`	$\searrow$				
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0-10-3	8			\$				0-10-3 0-1-0-3
I d	\$	9	8	7			×	1914
5x8		5x8 =	6x8 =				5x8 =	I
F	7-9-0		12-2-0	_		19-11-0		1
Plate Offsets (X,Y)	[7:0-8-0,0-1-12]		4-5-0			7-9-0		
OADING (psf)	SPACING- 2-0	0 <b>CSI</b> .	DEFL.	ir	n (loc)	l/defl L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.1 Lumber DOL 1.1		Vert(LL) Vert(CT)	-0.07 -0.14		>999 360 >999 240	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr N Code IRC2015/TPI2014	O WB 0.55	Horz(CT Wind(LL	) 0.03	3 5	n/a n/a >999 240	Weight: 348 I	b FT = 20%
LUMBER-			BRACIN			210		
TOP CHORD 2x			TOP CH	ORD		al wood sheathing dire		
WEBS 2x6	10 SP 2400F 2.0E 6 SP No.1 *Except*		BOT CH	JRD	Rigia ce	iling directly applied o	r 10-0-0 oc bracing.	
	7,2-9: 2x4 SP No.2							
	(size) 1=0-3-8, 5=0-3-8 ax Horz 1=-148(LC 6)							
	ax Uplift 1=-423(LC 8), 5=-642(LC 9) ax Grav_1=4234(LC 2), 5=6219(LC							
	/lax. Comp./Max. Ten All forces 2		in.					
TOP CHORD 1	-2=-7519/795, 2-3=-7422/798, 3-4=	-10665/1144, 4-5-10716/1136						
	-9=-679/6256, 7-9=-581/6251, 5-7= }-7=-953/8888, 4-7=-281/487, 3-9=-							
OTES-								
	connected together with 10d (0.13 nected as follows: 2x6 - 2 rows stac							
Bottom chords of	connected as follows: 2x10 - 4 rows d as follows: 2x6 - 2 rows staggered	staggered at 0-2-0 oc.	100					
2) All loads are cor	nsidered equally applied to all plies,	except if noted as front (F) or ba	ack (B) face in the		CASE(S) s	ection. Ply to		
<ol> <li>Unbalanced roo</li> </ol>	have been provided to distribute or f live loads have been considered f	or this design.					- Colores	
	10; Vult=130mph (3-second gust) V ope); Lumber DOL=1.60 plate grip I		DL=6.0psf; h=15f	t; Cat. II;	Exp C; Ei	nclosed;	ATT	ABOL MA
	een designed for a 10.0 psf bottom been designed for a live load of 30				-6-0 tall by	2-0-0 wide	SAL SEE	
will fit between t	the bottom chord and any other men nical connection (by others) of truss	nbers.		0	,	at (it lb)		
1=423, 5=642.					.,		SE	AL 3
	ner connection device(s) shall be pr m chord. The design/selection of su				own and s	972 lb up at	086	822
LOAD CASE(S)	Standard							
<ol> <li>Dead + Roof Liv Uniform Loads (</li> </ol>	ve (balanced): Lumber Increase=1.7 (plf)	5, Plate Increase=1.15				n,	CALL CO	NEE
	3=-60, 3-6=-60, 1-5=-20						A.	GINE
	=-7520(B)						M	ay 11,2021
								• • -
	rify design parameters and READ NOTES ON the only with MiTek® connectors. This design is the only with MiTek® connectors.							
a truss system. Be building design. B	fore use, the building designer must verify the racing indicated is to prevent buckling of indi	e applicability of design parameters and p vidual truss web and/or chord members of	properly incorporate the only. Additional temport	is design in rary and p	nto the overa	11		ENEU
is always required	for stability and to prevent collapse with poss e, delivery, erection and bracing of trusses ar	ible personal injury and property damage		ce regardir	ig the	-	818 Soundsid	a Road

Is analys required to stability and to prevent contapse with possible personal injury and poperty damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see *Safety Information* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 818 Soundside Road Edenton, NC 27932



BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 19-11-0.

Max Horz 1=-186(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 19, 20, 21, 16, 15, 14, 11 except 22=-116(LC 12), 13=-103(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 18, 19, 20, 21, 22, 16, 15, 14, 13, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

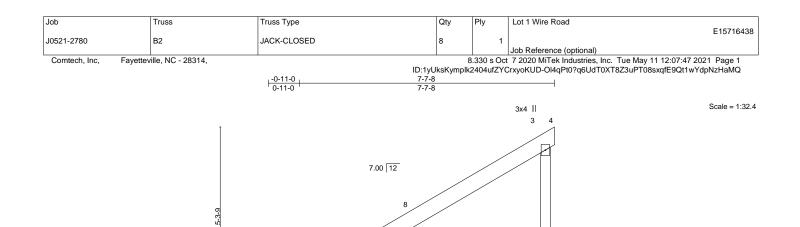
## NOTES-

- Unbalanced roof live loads have been considered for this design.
   Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 19, 20, 21, 16, 15, 14, 11 except (jt=lb) 22=116, 13=103.



🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This sknu include Der Die Der Die Der Die Der Die Dese Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			7-7-8
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.33	Vert(LL) -0.04 2-6 >999 360 MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.21	Vert(CT) -0.07 2-6 >999 240
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 n/a n/a
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)         0.00         2         ****         240         Weight: 48 lb         FT = 20%
LUMBER-	1		BRACING-

7-7-8

TOP CHORD

BOT CHORD

•

6

except end verticals.

5 3x4 Ш

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

## LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 BOT CHORD 2x4 SP No.2 WEBS

REACTIONS. (size) 6=Mechanical, 2=0-3-8 Max Horz 2=158(LC 12) Max Uplift 6=-82(LC 12) Max Grav 6=318(LC 19), 2=345(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-6=-288/220

0-4-0 0-10-3

3x4

## NOTES-

Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-0 to 3-7-13, Interior(1) 3-7-13 to 7-7-8 zone; C-C for members and forces & MWFRS for

- reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

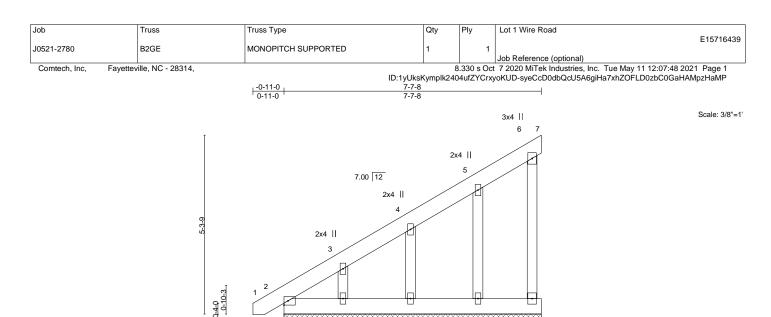
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.03 BC 0.01 WB 0.03 Matrix-P	DEFL.         in           Vert(LL)         -0.00           Vert(CT)         -0.00           Horz(CT)         -0.00	(loc) 1 1 7	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 57 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-			BRACING-					

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 WFBS OTHERS 2x4 SP No 2 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

8 3x4 ||

REACTIONS. All bearings 7-7-8.

(lb) - Max Horz 2=228(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 9, 10 except 11=-117(LC 12) Max Grav All reactions 250 lb or less at joint(s) 7, 8, 2, 9, 10, 11

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-269/227

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

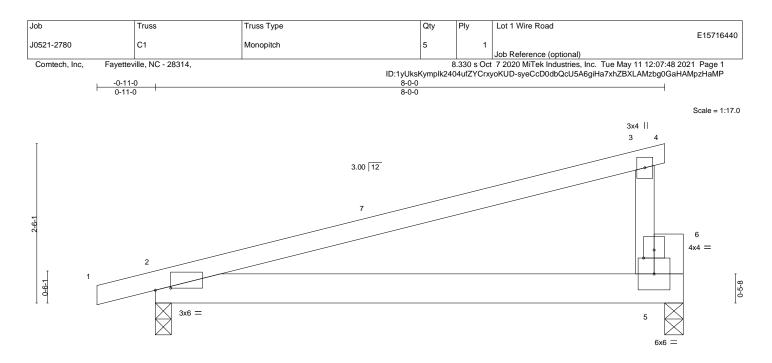
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 9, 10 except (it=lb) 11=117.

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el annual
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May 11,2021

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			0-3-0				
			8-3-8				
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]						
OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. i	n (loc) l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.84	Vert(LL) -0.0	5 2-5 >999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.24	Vert(CT) -0.10	0 2-5 >969	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00	0 5 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.10	0 2-5 >886	240	Weight: 37 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x4 SP	' No.1		TOP CHORD	Structural woo	od sheathing di	irectly applied or 5-3-4	oc purlins,
BOT CHORD 2x6 SP	' No.1			except end ve	rticals.		
WEBS 2x4 SP	No.2		BOT CHORD	Rigid ceiling d	irectly applied	or 10-0-0 oc bracing.	
OTHERS 2x6 SP	' No.1					-	

8-3-8

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



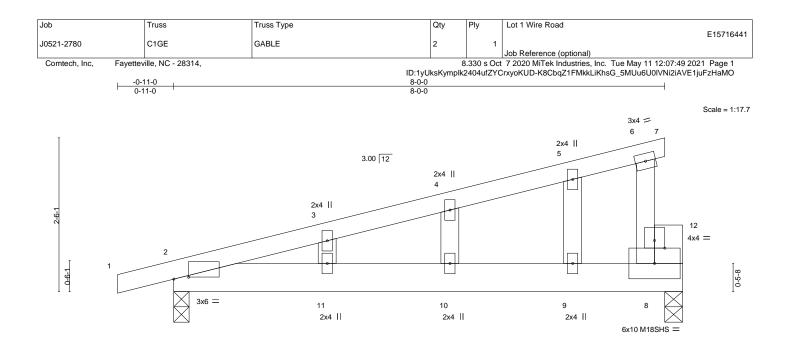


Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [12:0-2-0,0-1-8]		8-3-8 8-3-8			<u> </u>
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	DEFL.         in           Vert(LL)         0.09 1           Vert(CT)         -0.08 1           Horz(CT)         -0.00		PLATES MT20 M18SHS Weight: 41 lb	<b>GRIP</b> 244/190 244/190 FT = 20%
	' No.1		(	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	<i>y</i> 11	oc purlins,

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8) Max Uplift 2=-216(LC 8), 8=-188(LC 8)

Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.

4) Gable studs spaced at 2-0-0 oc.

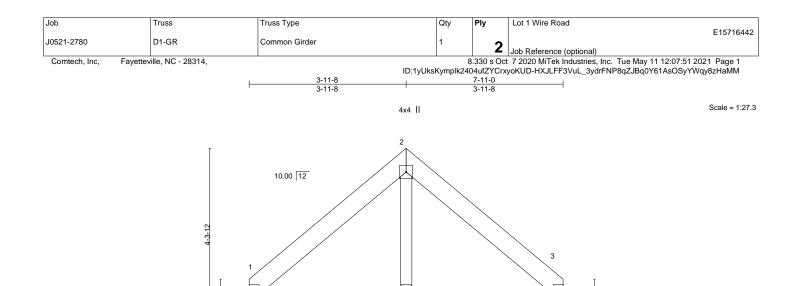
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216.8=188.



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7

4 3x10 ||

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

8

I/defl

>999

>999

>999

7-11-0

3-11-8

3-4

3-4

3-4

3 n/a

in (loc)

-0.02

-0.04

0.01

0.01

3x10 ||

L/d

360

240

n/a

240

6

3-11-8

3-11-8

0.38

0.57

CSI.

тс

вС

WB 0.39

Matrix-P



TCLL

TCDL

BCLL

BCDL

LOADING (psf)

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 WEDGE

20.0

10.0

0.0

10.0

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz	1=91(LC 24)
Max Uplift	1=-191(LC 8), 3=-180(LC 9)
Max Grav	1=2919(LC 1), 3=2779(LC 2)

FORCES. (lb)	- Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.
TOP CHORD	1-2=-2418/177, 2-3=-2418/177
BOT CHORD	1-4=-100/1678, 3-4=-100/1678
WEBS	2-4=-154/3142

-9 -9

Plate Offsets (X,Y)-- [1:0-0-4,0-0-5], [1:0-0-8,0-3-6], [3:0-0-4,0-0-5], [3:0-0-8,0-3-6]

Code IRC2015/TPI2014

SPACING-

Plate Grip DOL

Rep Stress Incr

Lumber DOL

3x10 ||

Н

2-0-0

1.15

1.15

NO

## NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 1-3=-20

## itinued on page

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1-0-3

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

PLATES

Weight: 100 lb

MT20

GRIP

244/190

FT = 20%



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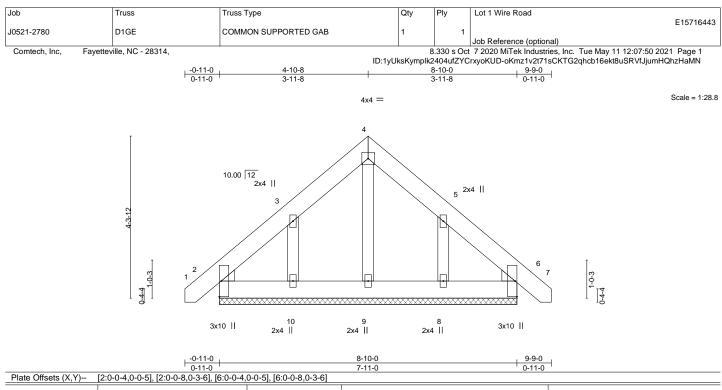
Job		Truss	Truss Type	Qty	Ply	Lot 1 Wire Road			
						E15716442			
J0521-	-2780	D1-GR	Common Girder	1	2				
					<b></b>	Job Reference (optional)			
Comt	tech, Inc, Fayette	ville, NC - 28314,			330 s Oct	7 2020 MiTek Industries, Inc. Tue May 11 12:07:51 2021 Page 2			
			ID:1yUksKymplk2404ufZYCrxyoKUD-HXJLFF3VuL_3ydrFNP8qZJBq0Y61AsOSyYWqy8zHaMM						

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)

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LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.02	Vert(LL)	0.00	6	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	0.00	6	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-P						Weight: 60 lb	FT = 20%
	<b>D</b>					BRACING						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No 2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. All bearings 7-11-0.

(lb) - Max Horz 2=-118(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152.8=148.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



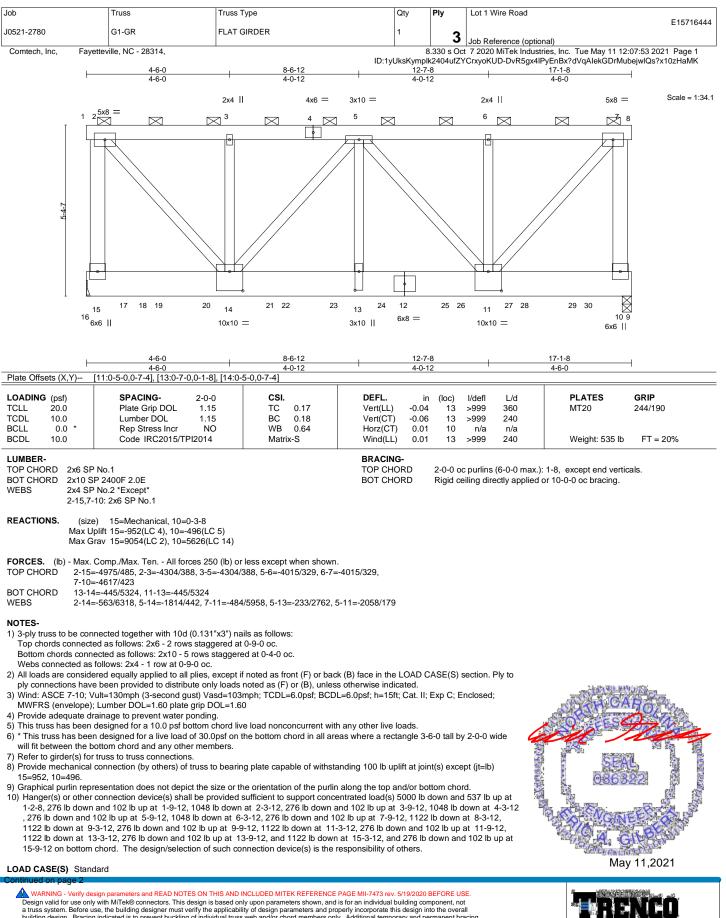
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 11,2021

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Design valid for use only with MTEk® connectors. This skill include a state of the skill include the state of the skill include the skill

A18 Soundside Road

lenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 1 Wire Road
J0521-2780	G1-GR	FLAT GIRDER	1		E15716444
50521-2760	GI-GK		'	3	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,			330 s Oct	7 2020 MiTek Industries, Inc. Tue May 11 12:07:53 2021 Page 2

ID:1yUksKymplk2404ufZYCrxyoKUD-DvR5gx4IPyEnBx?dVqAlekGDrMubejwlQs?x10zHaMK

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

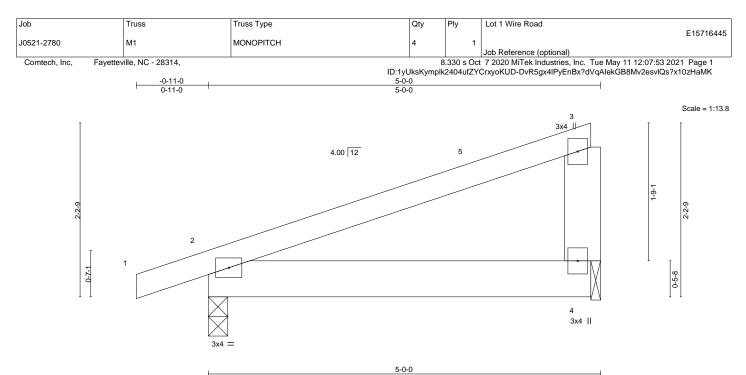
Vert: 1-2=-60, 2-7=-60, 7-8=-60, 9-16=-20

Concentrated Loads (lb)

Vert: 12=-276(F) 14=-259(B) 13=-291(B) 17=-5000(B) 18=-276(F) 19=-259(B) 20=-276(F) 21=-276(F) 22=-259(B) 23=-276(F) 24=-291(B) 25=-291(B) 26=-276(F) 27=-291(B) 28=-276(F) 29=-291(B) 30=-276(F) 21=-276(F) 21=-276(F) 22=-259(B) 23=-276(F) 24=-291(B) 25=-291(B) 26=-276(F) 27=-291(B) 26=-276(F) 21=-276(F) 22=-259(B) 23=-276(F) 24=-291(B) 25=-291(B) 26=-276(F) 21=-276(F) 22=-276(F) 23=-276(F) 23=-

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			1			5-0-0					1	
LOADING (p	sf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL 10	0.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	-0.01	2-4	>999	240		
BCLL 0	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL 10	0.0	Code IRC2015/TF	PI2014	Matri	x-P	Wind(LL)	0.01	2-4	>999	240	Weight: 24 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=63(LC 8) Max Uplift 2=-102(LC 8), 4=-79(LC 8)

Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

- capacity of bearing surface. 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

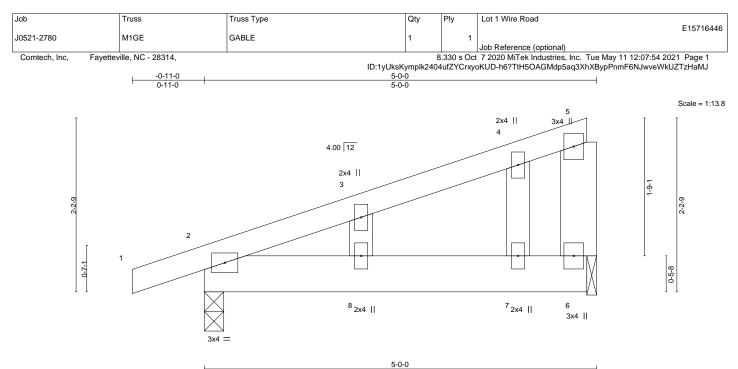
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

May 11,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		1	
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI. DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.09 Vert(LL) 0.01 8 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09 Vert(CT) -0.01 8 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02 Horz(CT) -0.00 6 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 27 lb FT = 20%

## LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD

 
 TOP CHORD
 Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8)

Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This trues has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

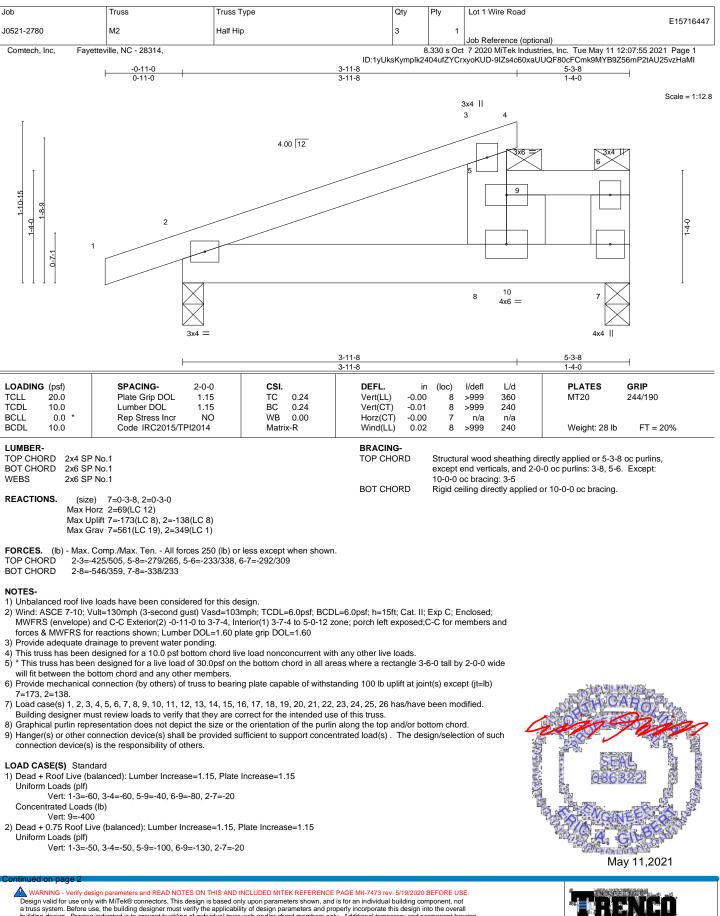
capacity of bearing surface.

- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147, 6=115.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Design valid for use only with MTerk® connectors. This solution that should be the Reference PAGE MICr473 fev. 519/2020 before USE. Design valid for use only with MTerk® connectors. This solution is a sea only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANS/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

1	dof	Truss	Truss Type	Qty	Ply	Lot 1 Wire Road
						E15716447
	J0521-2780	M2	Half Hip	3	1	
						Job Reference (optional)
	Comtech, Inc, Fayettevi	lle, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Tue May 11 12:07:55 2021 Page 2

8.330 s Oct 7 2020 MITek Industries, Inc. Tue May 11 12:07:55 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-9IZs4c60xaUUQF80cFCmk9MYB9Z56mP2tAU25vzHaMI

LOAD CASE(S) Standard	
Concentrated Loads (lb) Vert: 9=-350	
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25	
Uniform Loads (plf)	
Vert: 1-3=20, 3-4=-20, 5-6=-40, 2-7=-40	
Concentrated Loads (lb)	
Vert: 9=-300	
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=12, 2-8=52, 8-10=115, 7-10=52	
Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55 Concentrated Loads (lb)	
Vert: 9=548	
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52	
Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55	
Concentrated Loads (lb)	
<ol> <li>bead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)</li> </ol>	
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9	
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51	
Concentrated Loads (lb)	
Vert: 9=-420	
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9 Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51	
Concentrated Loads (lb)	
Vert: 9=-420	
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10	
Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7	
Concentrated Loads (lb) Vert: 9=154	
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Ver: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Ver: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Ver: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-334 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-48, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-33 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-6, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=9, 3-4=-2, 5-6=-1, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 2=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=9, 3-4=-2, 5-6=1, 2-7=-12	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=9, 3-4=-2, 5-6=-1, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=9, 3-4=-2, 5-6=-1, 2-7=-12 Horz: 1-2=-2, 2-3=9, 3-4=-2, 5-6=-1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (b) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (b) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (b) Vert: 9=-34 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-24, 2-3=-31, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 9=-34 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-42, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (plf) Vert: 1-2=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb)	
$\label{eq: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27} \\ \begin{tabular}{lllllllllllllllllllllllllllllllllll$	
$\begin{array}{l} \mbox{Horz: } 1.2 = -18, 2.3 = -24, 3.4 = -40, 3.5 = -27\\ \mbox{Concentrated Loads (b)}\\ \mbox{Vert: } 9 = 43\\ \end{tabular}$ 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1 - 2 = 0, 2.3 = -1, 3.4 = 6, 5-6 = -33, 2.8 = 2, 8-10 = 25, 7-10 = 2 Horz: 1 - 2 = -26, 2.3 = -19, 3.4 = -26, 3-5 = 34\\ \mbox{Concentrated Loads (b)}\\ \mbox{Vert: 9 = -339}\\ \end{tabular} 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1 - 2 = -2, 2.3 = -9, 3.4 = -2, 5-6 = -21, 2.7 = -20 Horz: 1 - 2 = -18, 2.3 = -11, 3-4 = -18, 3-5 = -0\\ \mbox{Concentrated Loads (b)}\\ \mbox{Vert: 9 = -234}\\ \end{tabular} 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1 - 2 = -26, 2.3 = -33, 3-4 = -26, 3-5 = -39\\ \mbox{Concentrated Loads (b)}\\ \mbox{Vert: 9 = -43}\\ \end{tabular} 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1 - 2 = -26, 2.3 = -33, 3-4 = -26, 3-5 = -39\\ \mbox{Concentrated Loads (b)}\\ \mbox{Vert: 9 = -43}\\ \end{tabular} 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60\\ \mbox{Vert: 9 = -43}\\ \end{tabular} 15) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60\\ \mbox{Vert: 9 = -43}\\ \end{tabular} 16) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60\\ \mbox{Vert: 9 = -43}\\ \end{tabular} 17) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60\\ \mbox{Vert: 9 = -43}\\ \end{tabular} 18) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60\\ \mbox{Vert: 9 = -43}\\ \end{tabular}	
$\label{eq: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27} \\ \begin{tabular}{lllllllllllllllllllllllllllllllllll$	
Horz: 1:218, 2:324, 3:4=-40, 3:5=-27         Concentrated Loads (b)         Vert: 9=43         10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60         Uniform Loads (pf)         Vert: 1:22, 2:3=-1, 3:4=6, 5:6=-33, 2:8=2, 8:10=25, 7:10=2         Horz: 1:22, 2:3=-1, 3:4=-6, 5:6=-33, 2:8=2, 8:10=25, 7:10=2         Horz: 1:22, 2:3=-1, 3:4=-6, 5:6=-33, 2:8=2, 8:10=25, 7:10=2         Horz: 1:22, 2:3=-1, 3:4=-2, 5:56=-21, 2:7=-20         Vert: 9=-339         11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60         Uniform Loads (pf)         Vert: 1:2-2-18, 2:3=-11, 3:4=-18, 3:5=-0         Concentrated Loads (lb)         Vert: 9=-234         12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60         Uniform Loads (pf)         Vert: 1:2=14, 2:3=21, 3:4=14, 5:6=-11, 2:7=-12         Horz: 1:2=-26, 2:3=-33, 3:4=-26, 3:5=-38         Concentrated Loads (lb)         Vert: 9=43         13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60         Uniform Loads (pl)         Vert: 1:2=2, 2:3=9, 3:4=2, 5:6=1, 2:7=-12         Horz: 1:2=-14, 2:3=-21, 3:4=-14, 3:5=-27         Concentrated Loads (lb)         Vert: 9=43         14)	
Hor: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Ver: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Ver: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Hor:: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-34 Concentrated Loads (lb) Ver: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Ver: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Hor:: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Ver: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Ver: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Ver: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Ver: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Ver: 1-2=-2, 2-3=-9, 3-4=2, 5-6=-1, 2-7=-12 Hor:: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Ver: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Ver: 1-2=-14, 2-3=-21, 3-4=-14, 5-5=-27 Concentrated Loads (lb) Concentrated Loads (lb)	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (b) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pf) Vert: 1-2=-6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (b) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (b) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pf) Vert: 1-2=-14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-14, 2-3=-3, 3-4=-2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-2, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-14, 2-3=21, 3-4=-14, 3-5=-27 Concentrated Loads (b) Vert: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-26, 2-3=-21, 3-4=-14, 2-5=-27 Concentrated Loads (b) Vert: 1-2=-26, 2-3=-21, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-26, 2-3=-21, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-26, 2-3=-21, 3-4=-26, 3-5=-39 Concentrated Loads (b) Vert: 1-2=-26, 2	

WARNIG - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



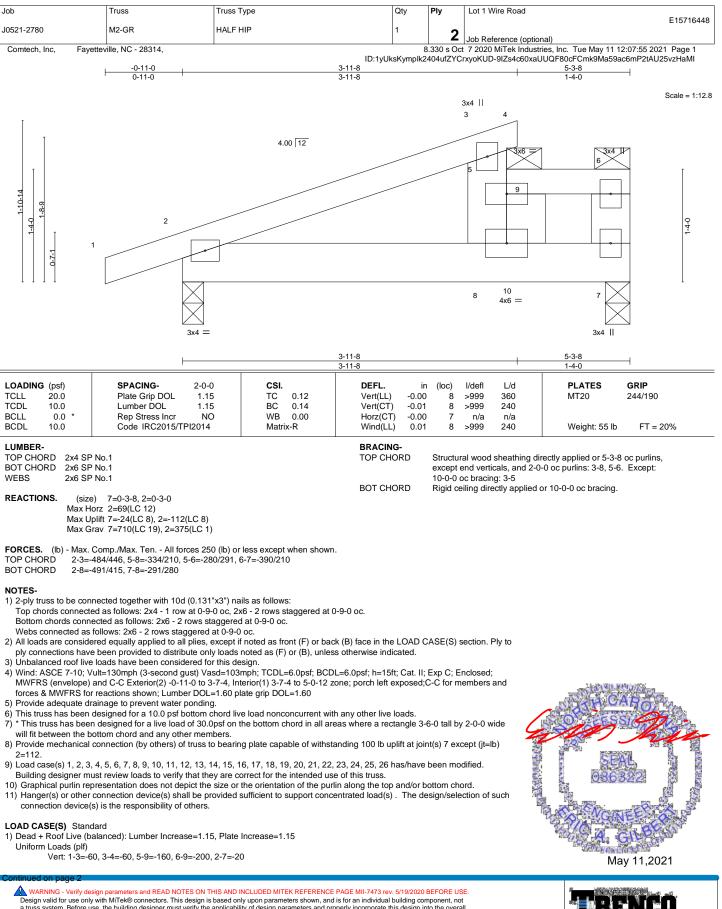
dof	Truss	Truss Type	Qty	Ply	Lot 1 Wire Road
					E15716447
J0521-2780	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayettevi	lle, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Tue May 11 12:07:55 2021 Page 3

8.330 s Oct 7 2020 MITek Industries, Inc. Tue May 11 12:07:55 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-9IZs4c60xaUUQF80cFCmk9MYB9Z56mP2tAU25vzHaMI

	ID:1yUksKympik2404ut2YCrxyoKUD-9I2s4c60xaUUQF80c
1.04	AD CASE(S) Standard
207	Uniform Loads (pif)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
	Concentrated Loads (lb)
40)	Vert: 9=43
	Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
	Vert: 1-2-6, 2-3=-1, 3-4=6, 5-6=-33, 2-7=-20
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
	Concentrated Loads (lb)
	Vert: 9=-234
17)	Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20
	Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 Concentrated Loads (lb)
	Vert: 9-234
18)	Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20
	Concentrated Loads (lb)
40)	
19)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3
	Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
	Concentrated Loads (lb)
	Vert: 9=-454
	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf) Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20
	Horz 1-2-13, 2-3-8, 3-4-13, 3-5-0
	Concentrated Loads (lb)
	Vert: 9=-375 ´
	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
	noi2. 172-19, 273-14, 374-19, 375-9 Concentrated Loads (Ib)
	Vert: 9=-375
22)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20
	Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0 Concentrated Loads (lb)
	Vert: 9375
23)	1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
,	Uniform Loads (plf)
	Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20
	Concentrated Loads (lb)
21)	Vert: 9=-400 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (pif)
	Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20
	Concentrated Loads (lb)
	Vert: 9=-400
	3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-6=-100, 2-7=-20
	Concentrated Loads (lb)
	Vert: 9=-350
	4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20
	Concentrated Loads (lb) Vert: 9=-350

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818 Soundside Road Edenton, NC 27932

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Job		Truss	Truss Type	Qty	Ply	Lot 1 Wire Road
						E15716448
J0521-2780		M2-GR	HALF HIP	1	2	
					<b>_</b>	Job Reference (optional)
Comtech, Inc,	Fayettev	ille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Tue May 11 12:07:55 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Tue May 11 12:07:55 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-9IZs4c60xaUUQF80cFCmk9Ma59ac6mP2tAU25vzHaMI

2)	Dead + 0.75 Root Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20
	Concentrated Loads (Ib)
	Vert: 9=-350
3)	Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40
	Concentrated Loads (lb)
	Vert: 9=-300
4)	Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
	Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52
	Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
	Concentrated Loads (lb)
	Vert: 9=548
5)	Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52
	Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
	Concentrated Loads (lb) Vert: 9=566
6)	Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
-,	Uniform Loads (plf)
	Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
	Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
	Concentrated Loads (lb)
	Vert: 9=-420
()	Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
	Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
	Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
	Concentrated Loads (lb)
	Vert: 9=-420
8)	Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-131, 2-8=10, 8-10=33, 7-10=10 Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
	Concentrated Loads (lb)
	Vert: 9=154
9)	Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12
	Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb)
	Vert: 9=43
10	) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
	Concentrated Loads (lb)
11	Vert: 9=-339 ) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20
	Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
	Concentrated Loads (lb)
	Vert: 9=-234
12	2) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
	Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
	Concentrated Loads (lb)
	Vert: 9=43
13	B) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
14	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb)

2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-400

WARNUG - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ocliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses sand truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



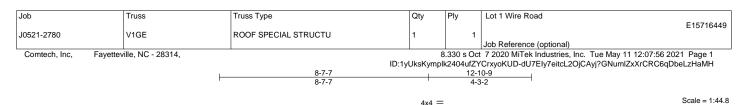
Job	Truss	Truss Type	Qty	Ply	Lot 1 Wire Road
					E15716448
J0521-2780	M2-GR	HALF HIP	1	2	
				<b>_</b>	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		. 8	.330 s Oct	7 2020 MiTek Industries, Inc. Tue May 11 12:07:55 2021 Page 3

8.330 s Oct 7 2020 MiTek Industries, Inc. Tue May 11 12:07:55 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-9IZs4c60xaUUQF80cFCmk9Ma59ac6mP2tAU25vzHaMI

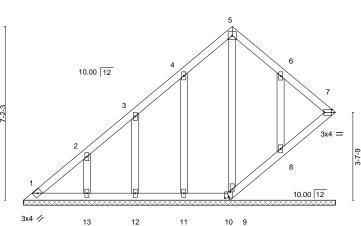
		· · · <b>,</b> · · · · · · · · · ·
LOAD CA	CASE(S) Standard	
	form Loads (plf)	
	Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12	
	Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39	
Conce	ncentrated Loads (lb)	
	Vert: 9=43	
,	ad + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60	
Unifoi	form Loads (plf)	
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12	
Conc	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 icentrated Loads (lb)	
COLIC	Vert: 9=43	
16) Dead	ad + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60	
	form Loads (plf)	
	Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20	
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12	
Conce	ncentrated Loads (lb)	
	Vert: 9=-234	
,	ad + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60	
Unitor	form Loads (plf)	
	Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20	
Conce	Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 ncentrated Loads (lb)	
Conco	Vert: 9=-234	
18) Dead	ad: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90	
,	form Loads (plf)	
	Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20	
Conce	ncentrated Loads (lb)	
	Vert: 9=-200	
	ad + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase	=1.60, Plate Increase=1.60
Unifoi	form Loads (plf)	
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26	
Conce	ncentrated Loads (lb)	
00110	Vert: 9=-454	
20) Dead	ad + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increas	e=1.60, Plate Increase=1.60
Unifo	form Loads (plf)	
	Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20	
	Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0	
Conce	ncentrated Loads (lb)	
	Vert: 9=-375	
,	ad + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber In form Loads (plf)	crease=1.60, Plate Increase=1.60
Unito	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20	
	Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9	
Conce	ncentrated Loads (Ib)	
	Vert: 9=-375	
22) Dead	ad + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber I	ncrease=1.60, Plate Increase=1.60
Unifo	form Loads (plf)	
	Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20	
0	Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0	
Conce	centrated Loads (lb) Vert: 9=-375	
23) 1st D	Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15	
	form Loads (plf)	
	Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20	
Conce	ncentrated Loads (lb)	
	Vert: 9=-400	
,	Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15	
Unifo	form Loads (plf)	
0	Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20	
Conce	vert: 9=-400	
25) 3rd D	Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15	
	form Loads (plf)	
	Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20	
Conce	ncentrated Loads (Ib)	
	Vert: 9=-350	
	Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15	
Unifo	form Loads (plf)	
<b>C 1</b>	Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20	
Conce	vertrated Loads (lb) Vert: 9=-350	

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4x4 =







TOP CHORD

BOT CHORD

3x4 =

OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.06	Vert(LL) n/a - n/a 999	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) n/a - n/a 999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.08	Horz(CT) 0.00 7 n/a n/a	
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 75 lb FT = 20%

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No 2

REACTIONS. All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12), 8=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-295/189

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.



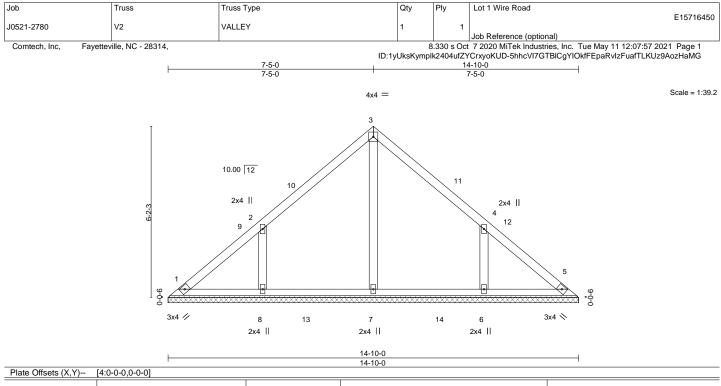
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 11,2021

🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTerk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses systems, see **ANS/TPI Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TCDL         10.0           BCLL         0.0 *           BCDL         10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.14 BC 0.15 WB 0.09 Matrix-S	Vert(CT) n/a Horz(CT) 0.00	- - 5	n/a 999 n/a 999 n/a n/a	Weight: 64 lb FT = 20%
LOADING (psf)	SPACING- 2-0-0	<b>CSI.</b>	<b>DEFL.</b> in	(loc) l	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.14	Vert(LL) n/a		n/a 999	MT20 244/190

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 14-10-0.

(lb) - Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)

WEBS 2-8=-338/247, 4-6=-338/247

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135, 6=135,



Structural wood sheathing directly applied or 6-0-0 oc purlins.

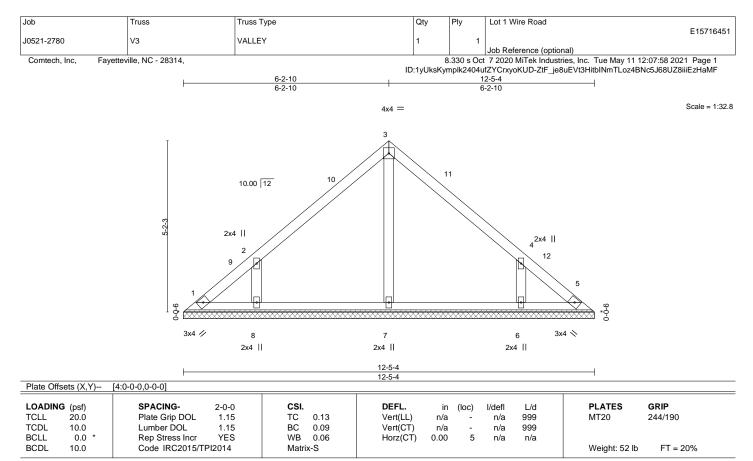
Rigid ceiling directly applied or 10-0-0 oc bracing.

May 11,2021

MARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **AVSUPTI Quality Criteria, DSB-89 and BCSI Building Compor Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

**REACTIONS.** All bearings 12-5-4.

(lb) - Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-312/241, 4-6=-312/241

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.

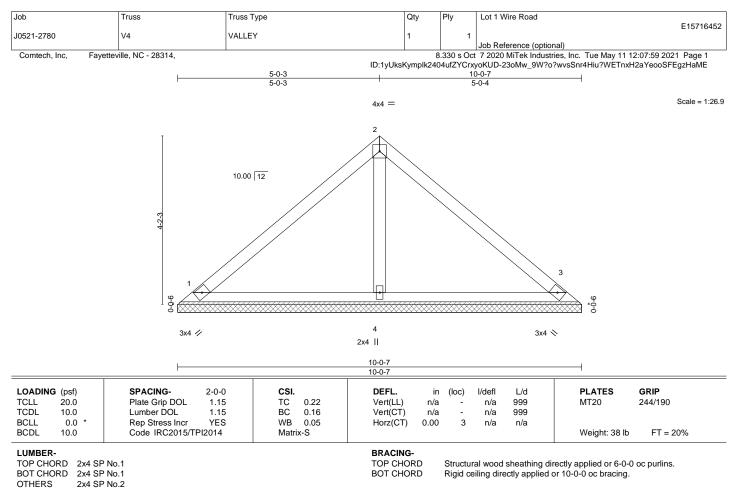


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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**REACTIONS.** (size) 1=10-0-7, 3=10-0-7, 4=10-0-7

Max Horz 1=-92(LC 8)

Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

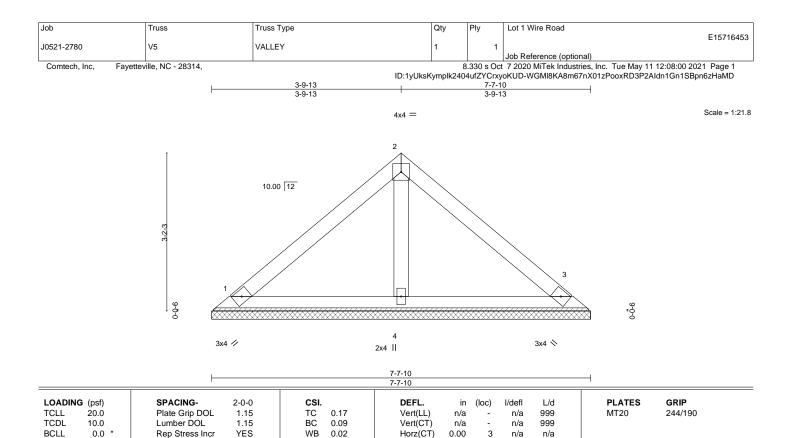
will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

n/a

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 28 lb

FT = 20%

n/a

LUMBER-
TOP CHORD
BOT CHORD

BCDL

2x4 SP No.1 2x4 SP No.1 RD OTHERS 2x4 SP No.2

10.0

REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10

Max Horz 1=68(LC 9)

Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

Matrix-P

arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

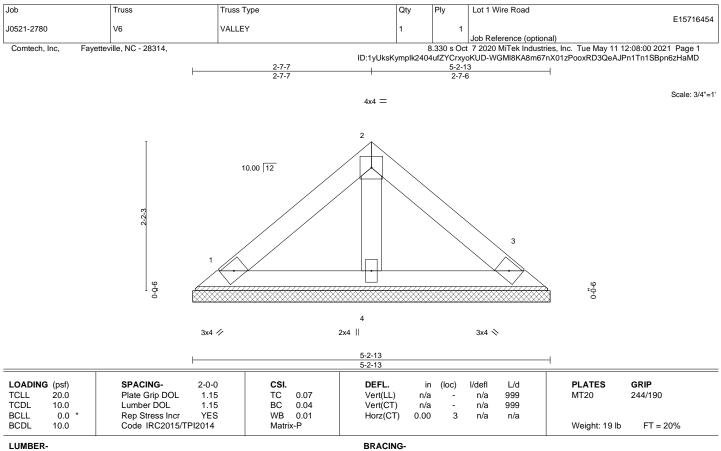
will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13

Max Horz 1=-44(LC 8)

Max Uplift 1=-15(LC 13), 3=-19(LC 13) Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

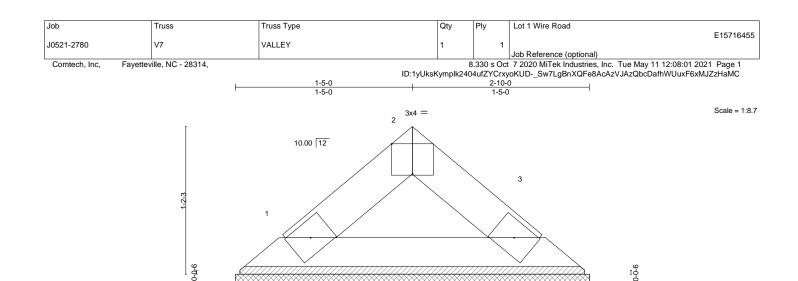


Structural wood sheathing directly applied or 5-2-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3x4 🥢

3x4 📎

		Ι			2-10-0						
Plate Offsets (X,Y) [2	2:0-2-0,Edge]				1					1	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.01	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TP	912014	Matrix	(-P						Weight: 8 lb	FT = 20%

2-10-0

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

**REACTIONS.** (size) 1=2-10-0, 3=2-10-0

Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

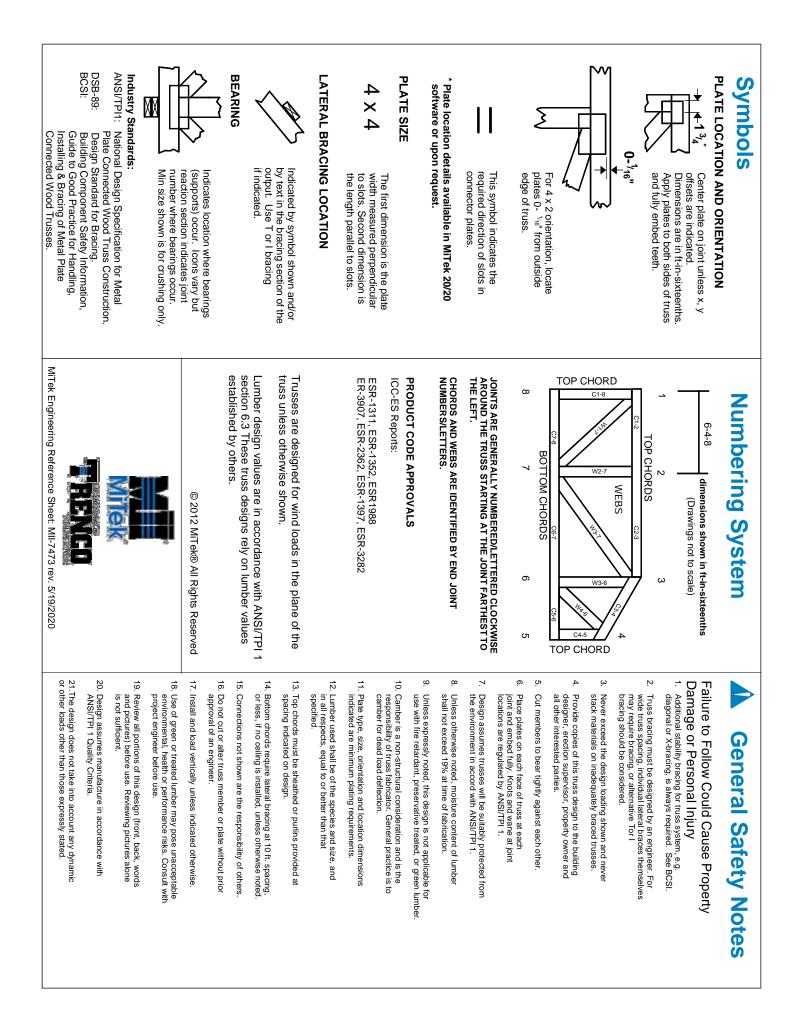


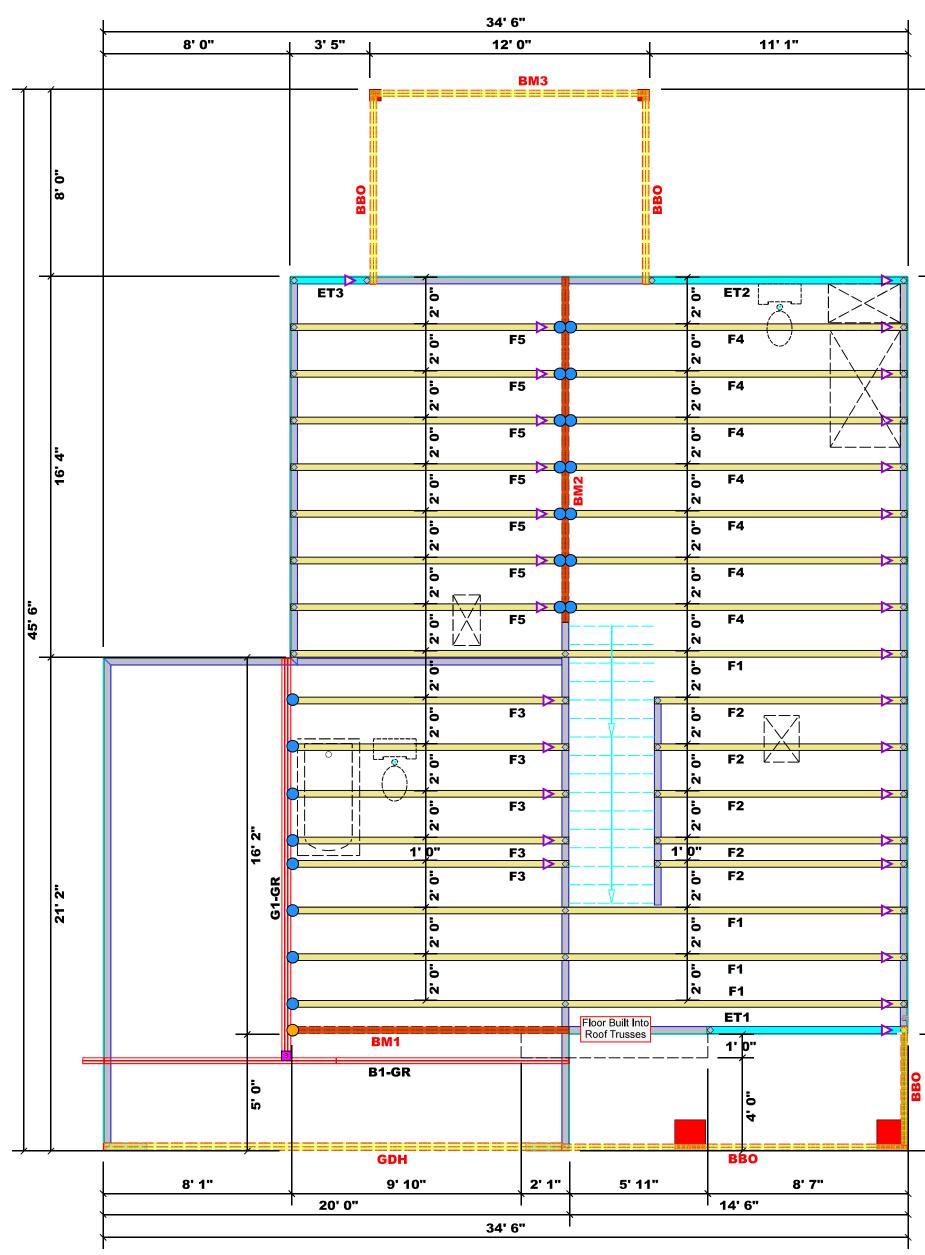
Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







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T				OOF EAN							
	Fayet		, N.C.	ial Par 28309 -8787							
Bearing	Fax	: (910)	864-4	1444	are #						
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.											
Signature David Landry David Landry											
LOA		ART FO	OR JAG	CK STL	IDS						
(BASED ON TABLES RESUZE(I) & (b); NUMBER OF LACK STUDS REQUIRED & EALEND OF PEADEX/STRDER NO 100 LLS ALE STUDS REQUIRED & EALEND OF READEX/STRDER NO 100 LLS ALE STUDS ALE											
1700         1         2550         1         3400         1           3400         2         5100         2         6800         2           5100         3         7650         3         10200         3           6800         4         10200         4         13600         4											
8500         5         12750         5         17000         5           10200         6         15300         6         1											
15300 9											
CITY / CO. Harnett / Harnett	Lot 1 Wire Road	Floor	DATE REV. 05/11/21	DRAWN BY David Landry	SALES REP. Lenny Norris						
CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP						
Weaver Development Co. Inc.	Lot 1 Wire Road	Hickory "A"	Seal Date	Quote #	J0521-2781						
BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #						

Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes 1. Plumbing drop locations shown are NOT exact. 2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses. 3. Adjust spacing as needed not to exceed 24"oc.

	Conne	ion	Nail Info	ormation		
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS410	USP	22	NA	16d/3-1/2"	16d/3-1/2"
	THDH412	USP	1	NA	16d /3-1/2"	16d /3-1/2"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	12' 0"	2x10 SPF No.2	2	2
GDH	20' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

Truss Placement Plan Scale: 1/4"=1'

# Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

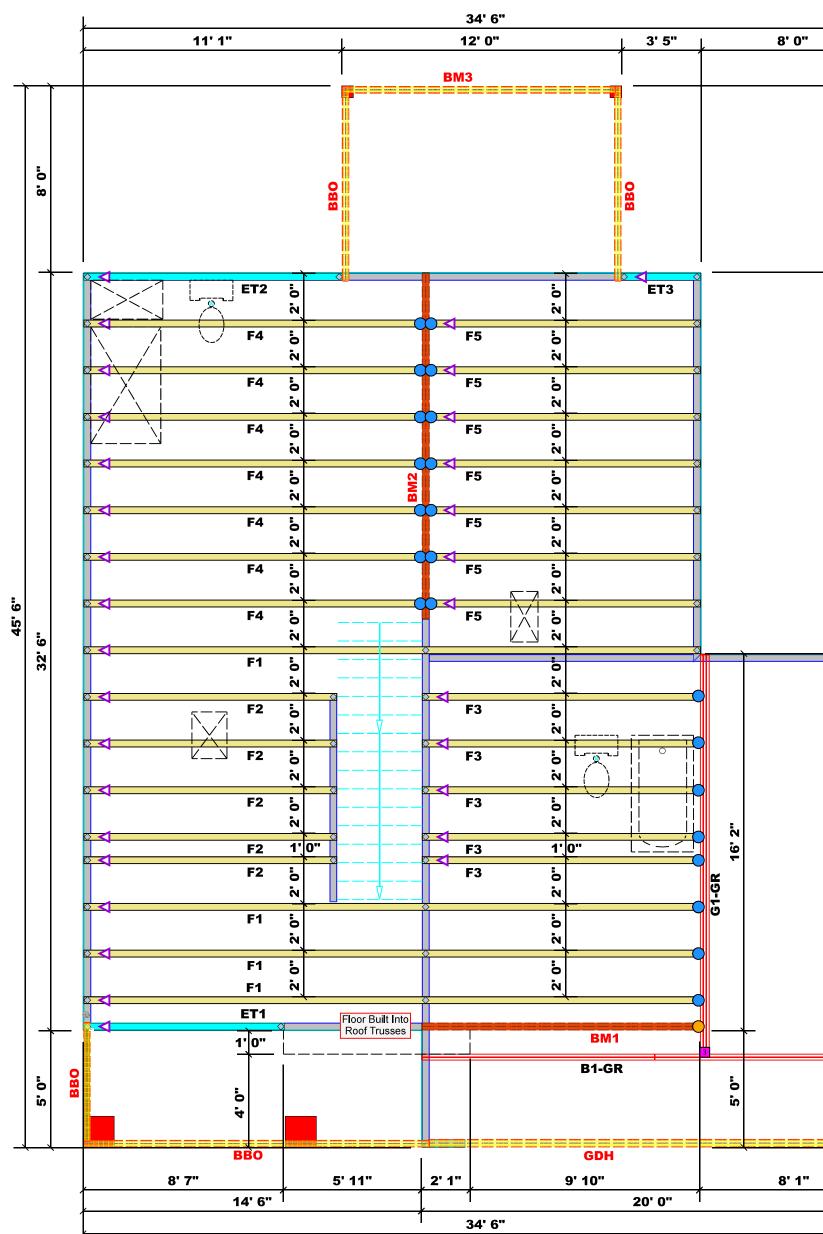
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

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 Ļ			Reilly F Fayet Phor	Road Ir teville	ndusti (, N.C. 0) 864	<b>BEAN</b> rial Par 28309 I-8787 4444	rk )
 .0		deemed require attache size an reaction 15000# retained Tables, retained	I to comp ments. Th d Tables ments ) to d number as greated A registo d to desig n that exo A registo d to desig to desig s that exo	ly with the e contract derived f of determin of wood s than 3000 red desig n the sup red desig red desig n the sup ceed 1500	e prescrip tor shall rom the p e the min studs req 0# but no n profess port syste e specific n profess port syste 0#.	I to 3000# otive Code refer to th prescriptiv nimum fou juired to s sional sha em for any ed in the a sional sha em for all	e ie undation upport than II be y attached II be
8.		LO	AD CH (base mber of c	ART FC d on tabu ack studs	DR JA: Es 8502.5(	2.000 ndry CK STU D & EA END	JDS
. 4"		об (д. а.) 1700 3400 5100 6800 8500 10200 11900 13600 15300	2 3 4 5 6 7 8	종 1197월 2550 7650 1020 12751 1530	2 3 24 25	680 1020 1360	(01 40) 20 00 00 5
16	Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise	±					
	All Walls Shown Are Considered Load Bearing	Harnett / Harnett	Lot 1 Wire Road	Floor	05/11/21	David Landry	Lenny Norris
1. 2"	2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.         3. Adjust spacing as needed not to exceed 24"oc.         Connector Information         Sym       Product         Manuf       Qty         Supported Member       Header	CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
21	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	Weaver Development Co. Inc.	t 1 Wire Road	Hickory "A"	al Date	Quote #	J0521-2781
 , ,		BUILDER We	JOB NAME Lot	PLAN Hid	SEAL DATE Seal	QUOTE # Qu	<b>JOB #</b> J0
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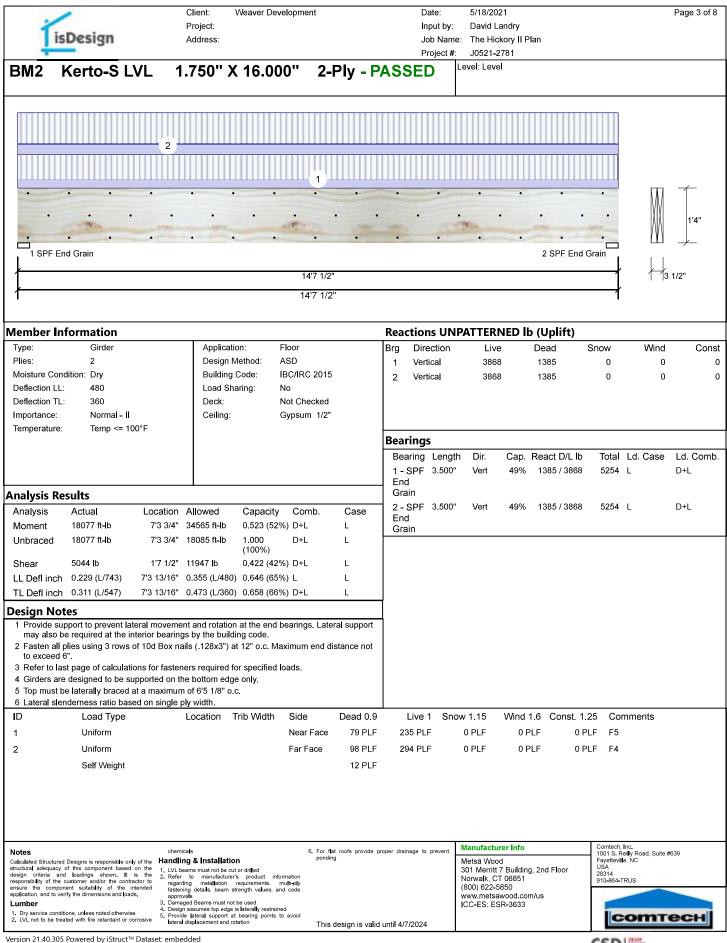
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

Design	Pi	lient: V roject: ddress:	Veaver Devel	opment			out by:	5/18/2021 David Land The Hickory	•			Page 1 of
Kerto-S LV	/L 1.7	750'' X	16.000	)'' 2-F	Ply - P		bject #:	J0521-2781 evel: Level	<u> </u>			
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			11'11"						┥			
		A 11 11							• •	•		
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Temp <= 100	°F					Rearings						
						Bearing 1 - SPF	Length 3.500"	Vert 9	95% 2869 /	2079 4948	L	Ld. Con D+S
sults						2 - SPF	3.500"	Vert 9	95% 2869 /	2079 4948	L	D+S
Actual 13679 ft-lb 13679 ft-lb 3615 lb 0.069 (L/2000)	5'11 1/2" 39 5'11 1/2" 13 1'7 1/2" 13 5'11 1/2" 0.	9750 ft-lb 3699 ft-lb 3739 lb 287 (L/480)	0.999 (100%) 0.263 (26%) 0.240 (24%)	D+S 0 D+S 0 S	Case L L L							
	5'11 1/2" 0.	383 (L/360)	0.428 (43%)	D+S	L	ļ						
<b>tes</b> pport to prevent later required at the inte	erior bearings l	by the buildir	ig code. 12" o.c. Max	imum end dis								
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6". st page of calculatior e designed to be sup must be supported e	ported on the l qually by all pl a maximum of d on single ply	bottom edge ies f 8'8 3/8" o.c. width	only.	Side	Dead 0.9	Live 1	Snow	1.15 W	find 1.6 Cor	nst. 1.25 Cc	omments	
	formation           Girder           2           dition: Dry           480           360           Normal - II           Temp <= 100	2 formation Girder 2 dition: Dry 480 360 Normal - II Temp <= 100°F sults Actual Location A 13679 ft-lb 5'11 1/2" 13 13679 ft-lb 5'11 1/2" 13 13615 lb 1'7 1/2" 13 0.069 (L/2000) 5'11 1/2" 0.	Image: solution of the second seco	2         1           11/11"         11/11"           11/11"         11/11"           formation         II/11"           Girder         Application: FI           2         Building Code: IB           Load Sharing: Net         Deck: Net           360         Deck: Net           Normal - II         Temp <= 100"F	2         1           1         1	Image: constraint of the second sec	Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED           Image: Solution of the state of the s	Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED           Image: Strain Strai	Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           2         1         2         1         2         3 <t< td=""><td>Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           2         1         2         1         2         1         2         <t< td=""><td>Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           2         1         <t< td=""><td>Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           Image: Construction of the state of the state</td></t<></td></t<></td></t<>	Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           2         1         2         1         2         1         2 <t< td=""><td>Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           2         1         <t< td=""><td>Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           Image: Construction of the state of the state</td></t<></td></t<>	Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           2         1 <t< td=""><td>Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           Image: Construction of the state of the state</td></t<>	Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level: Level           Image: Construction of the state

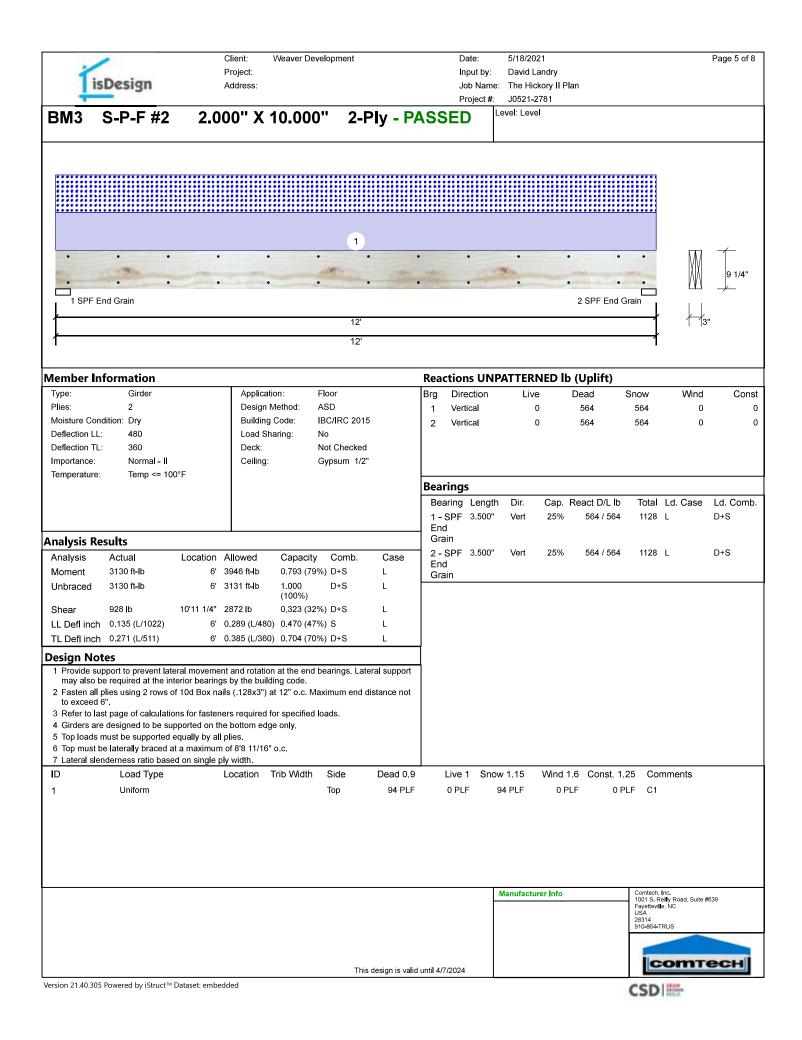
	/		Client:	Weaver Developr	nent	Date			Page 2 of 8
1	isDesign		Project:			Input			
- +	ISDESIGI		Address:			Proje	Name: The Hickon ect #: J0521-278		
BM1	Kerto-S		1 750"	X 16.000"	2_Dlv		Level: Level	<u> </u>	
	Nerto-5		1.7 50	X 10.000	<b>Z-</b> F Ty	-FASSED			
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/				11'11"				$\rightarrow$	
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-	y Analysis			(100 0)) (10				1.61	
Fasten a <b>ll</b> Capacity	plies using 3 i	rows of 0.0		s (.128x3") at 12	" o.c Maxim	num end distanc	e not to excee	d 6".	
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Yield Limit pe			.6 PLF						
Yield Limit p∉ Yield Mode	er Fastener	81.9 IV	JID.						
Edge Distand		1 1/3	2"						
Min. End Dis Load Combir		3"							
Duration Fac		1.00	)						
							Manufacturer	Info	Comtech, Inc.
Notes Calculated Struct	tured Designs is responsible	e on ly of the 🖡	chemicals Handling & Installa	ation	<ol><li>For flat roofs pro ponding</li></ol>	ovide proper drainage to prev	Metsä Wood		1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequ	acy of this component ba and loadings shown the customer and/or the c	sed on the	1. LVL beams must not b 2. Refer to manufac	e cut or dri∎ed turer's product information				Building, 2nd Floor 6851	USA 28314 910-864-TRUS
ensure the co	mponent suitability of the to verify the dimensions and	e intended loads.	fastening details, bea approvals	n requirements, multi-ply m strength values, and code			(800) 622-585 www.metsawo	0	
	conditions, unless noted othe	rwise	<ol> <li>Damaged Beams mus</li> <li>Design assumes top e</li> </ol>	t not be used dge is laterally restrained ort at bearing points to avoid			ICC-ES: ESR-		loomer and
	e treated with fire retardant		lateral displacement a	nd rotation		s valid until 4/7/2024			соттесн
Varcian 21.40	305 Powered by iStru	ATH DATA A	. omboddod						and the second second

Version 21.40.305 Powered by iStruct™ Dataset: embedded



	/		Client:	Weaver Developm	ient	Da		5/18/2021	Page 4 of 8
1	isDesign		Project: Address:				ut by: Name:	David Landry The Hickory II Plan	
+						Pro	oject #:	J0521-2781	
BM2	Kerto-S	LVL	1.750"	X 16.000"	2-Ply	- PASSED		evel: Level	
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	End Grain							2 SPF End	
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1				Î	4'7 1/2"				1
Multi-Di	y Analysis								
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Capacity	<u>p</u>	79.8 %		(					
Load Yield Limit p	er Foot	196.0 F 245.6 F							
Yield Limit p Yield Mode	er Fastener	81.9 lb. IV							
Edge Distan	се	1 1/2"							
Min. End Dis Load Combi		3" D+L							
Duration Fac		1.00							
Nete					Q East first an off	uide seeses dooloo oo '		Manufacturer Info	Comtech, Inc.
Notes Calculated Struct structural adequ	ctured Designs is responsib Jacy of this component b	le only of the Han	nemicals dling & Installa /L beams must not be		<ol> <li>For flat roofs pro ponding</li> </ol>	wide proper drainage to p	Jevent	Metsä Wood	<ul> <li>1001 S. Reilly Road, Suite #639</li> <li>Fayetteville, NC</li> <li>USA</li> </ul>
design criteria responsibility of	and loadings shown. the customer and/or the omponent suitability of t	t is the 2 Re contractor to re	efer to manufactu garding insta <b>ll</b> ation	urer's product information requirements, multi-ply				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	28314 910-864-TRUS
application, and	to verify the dimensions and	d loads. ap 3. Da	provals amaged Beams must	n strength values, and code not be used				www.metsawood.com/us ICC-ES: ESR-3633	
<ol> <li>Dry service of 2. LVL not to b</li> </ol>	conditions, unless noted oth e treated with fire retardan	erwise 5 Pr	esign assumes top ed rovide lateral suppor teral displacement an	lge is laterally restrained t at bearing points to avoid d rotation	This design is	valid until 4/7/2024			соттесн
L	205 D 11 10								

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	1	Clien		ver Developn	nent		Date:	5/18/2021	Page 6 of 8
1	isDesign	Proje					Input by:	David Landry	
- 4	Ispesign	Addr	ess:				Job Name: Project #:	The Hickory II Plan J0521-2781	
BM3	S-P-F #2	2.000	" X 10	000"	2-Plv	- PASSE		evel: Level	
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Capacity Load		0.0 % 0.0 PLF							
Yield Limit p		157.4 PLF							
Yield Limit p Yield Mode	er Fastener	78.7 lb. IV							
Edge Distan		1 1/2"							
Min. End Dis		3"							
Load Combin Duration Fac		1.00							
								Manufacturer Info	Comtech Inc
							H	Mandidulurer IIIO	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
									USA 28314
									910-864-TRUS
					This design	is valid until 4/7/20	024		соттесн
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is	Design	Client: Project: Address:	Weaver Developm	ent	Job	e: 5/18/20 it by: David I Name: The Hi ect #: J0521-	₋andry ckory II Plan			Page 7 of
GDH I	Kerto-S LVL	1.750"	X 11.875"	2-Ply - P	ASSED	Level: Lev	el			
*******		2	******			3		*****		
		-		1						
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lember Inf	formation				Reactions	UNPATTER	NED lb (Uplif	t)		
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Plies: Moisture Conc Deflection LL: Deflection TL: Importance:	480		haring: No Not Ch		1 Vertica 2 Vertica		0 1509 0 1509	168 168	0	
Temperature:	Temp <= 100°F		- )		<u> </u>					
					Bearings Bearing L 1 - SPF 3 End	-	Cap. React D/ 16% 1509 /		Ld. Case L	Ld. Con D+S
nalysis Re					Grain 2 - SPF 3	.500" Vert	16% 1509 /	168 1677	1	D+S
	6008 ft-lb 6678 ft-lb 1288 lb 1 0.035 (L/5617) 8 <sup>15</sup>	cation Allowed 8'5" 17919 ft-lb 8'5" 6684 ft-lb '3 3/8" 7980 lb 5 1/16" 0.409 (L/48	0.335 (34%) D 0.999 D+ (100%) 0.161 (16%) D 0) 0.085 (9%) S	Uniform L	End Grain					
		5 1/16" 0.546 (L/36	0) 0.638 (64%) D+	S L	4					
may also be 2 Fasten all p to exceed 6 3 Refer to las 4 Girders are 5 Top loads n 6 Top must be	port to prevent lateral n e required at the interior lies using 2 rows of 10d	bearings by the buil Box nails (.128x3") In fasteners required ed on the bottom ed ly by all plies. aximum of 14'10 7/1	ding code. at 12" o.c. Maximur for specified loads. ge only.							
ID 1	Load Type Uniform	Location	Trib Width Side	e Dead 0.9 60 PLF	Live 1 0 PLF	Snow 1.15 0 PLF	Wind 1.6 Cons 0 PLF	st. 1.25 Cc 0 PLF Wa	mments	
1 2	Uniform		Тор Тор	60 PLF 90 PLF	0 PLF 0 PLF	0 PLF 0 PLF	0 PLF	0 PLF Wa		
3	Tie-In Self Weight	0-0-0 to 16-10-0	1–0–0 Top	20 PSF 9 PLF	0 PSF	20 PSF	0 PSF	0 PSF Ro	of Load	
tructural adequacy of esign criteria and esponsibility of the co nsure the compon pplication, and to veri .umber . Dry service conditi	Designs is responsible only of the of this component based on the loadings shown. It is the ustomer and/or the contractor to ent suitability of the intended fy the dimensions and loads. ons, unless noted otherwise ted with fire retardant or corrosive	fastening details, beam approvals 3. Damaged Beams must r 4. Design assumes top edo	ut or drilled ar's product information requirements, multi-ply strength values, and code ot be used a is laterally restrained at bearing points to avoid	6. For flat roofs provide p ponding This design is valid		Metsä Wo 301 Merrit Norwalk, C (800) 622-	od t 7 Building, 2nd Floor CT 06851 5850 awood.com/us	Fayettev USA 28314 910-864-	Reilly Road, Suite a lle, NC	

-	/		Client: Project:	Weaver Developn	nent	Date: Input I	5/18/2021 by: David Landry	Page 8 of 8
1	isDesign		Address:				ame: The Hickory II Plan	
GDH	Kerto-S	LVL	1.750"	X 11.875"	2-Ply -	- PASSED	Level: Level	
	· ·	•	• •		• •	• •	· · · ·	···· ·· · · · · · · · · · · · · · · ·
1 SPF	End Grain						2 \$	
					16'10"			3 1/2"
1					16'10"			1
Multi-Ply	/ Analysis							
Fasten a <b>ll</b> Capacity	p <b>l</b> ies using 2	rows of		s (.128x3") at 12	' o.c Maxim	um end distance	not to exceed 6".	
Load Yield Limit pe	er Foot	0.0						
Yield Limit pe Yield Mode	er Fastener	81.9 IV	) lb.					
Edge Distand		1 1/3	2"					
Min. End Dis Load Combin		3"						
Duration Fac	tor	1.00	)					
Notes			chemicals		6. For flat roofs pro- ponding	vide proper drainage to preve		Comtech, Inc. 1001 S. Reilly Road, Suite #639
structural adequa design criteria	tured Designs is responsib acy of this component ba and loadings shown	ased on the the the	Handling & Installa 1. LVL beams must not b 2. Refer to manufac	e cutor dri∎ed turer's product information			Metsä Wood 301 Merritt 7 Building, 2nd F Norwalk, CT 06851	26314
ensure the cor application, and t	the customer and/or the mponent suitab∎ity of ti o verify the dimensions and	contractor to ne intended I loads.	regarding installatio fastening details, bea approvals	n requirements, multi-ply m strength values, and code			(800) 622-5850 www.metsawood.com/us	910-864-TRUS
	onditions, unless noted othe	erwise	<ol> <li>Damaged Beams mus</li> <li>Design assumes top e</li> <li>Provide lateral suppo</li> </ol>	dge is laterally restrained rt at bearing points to avoid			ICC-ES: ESR-3633	соттесн
	20E Doworod by iStr		lateral displacement a		This design is	valid until 4/7/2024		

Version 21.40.305 Powered by iStruct<sup>™</sup> Dataset: embedded



**Trenco** 818 Soundside Rd Edenton, NC 27932

Re: J0521-2781 Lot 1 Wire Road

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15739897 thru E15739904

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 18,2021

Gilbert, Eric **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

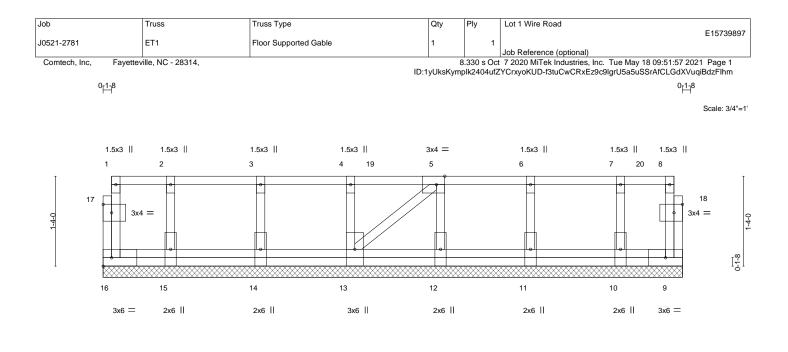


Plate Offsets (X,Y)	[5:0-1-8,Edge], [17:0-1-8,0-1-8], [18:0-1	-8 0-1-81	8-7-0 8-7-0			I
LOADING (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.00 WB 0.05 Matrix-P	<b>DEFL.</b> ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	a - n/a 999	PLATES MT20 Weight: 54 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SP	No.1(flat) No.1(flat) No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	, ,,	oc purlins,

REACTIONS. All bearings 8-7-0.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

OTHERS

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

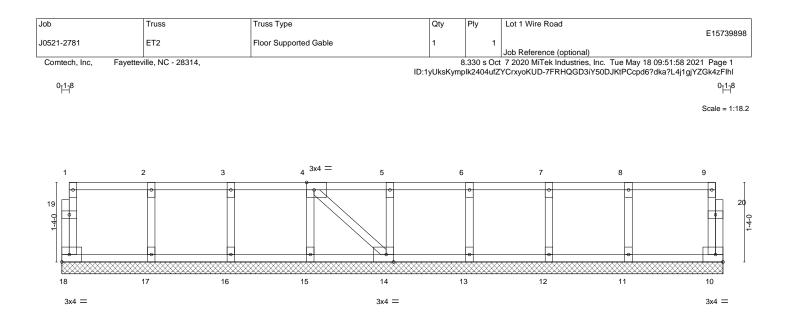
Uniform Loads (plf) Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb) Vert: 3=-71 6=-71 19=-71 20=-77



May 18,2021



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



			<u>11-1-0</u> 11-1-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [14:0-1-8,Edge]					
LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.07 BC 0.01 WB 0.04 Matrix-S	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	a - n/a 999 a - n/a 999	<b>PLATES</b> MT20 Weight: 52 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER-           TOP CHORD         2x4 SP No.1(flat)           BOT CHORD         2x4 SP No.1(flat)           WEBS         2x4 SP No.3(flat)           OTHERS         2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or	, ,,	) oc purlins,	

REACTIONS.

DNS. All bearings 11-1-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

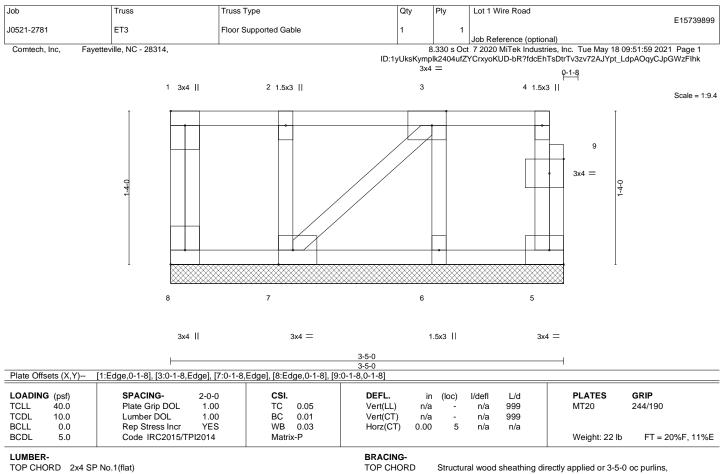
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss evaluat and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPI 1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD

2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WFBS 2x4 SP No.3(flat) OTHERS

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

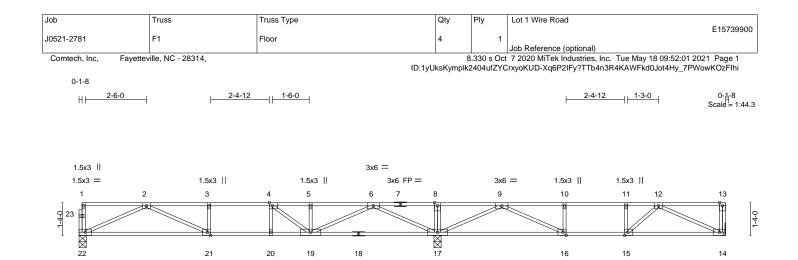
6) CAUTION, Do not erect truss backwards.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3x10 =

H	<u> </u>			26-5		
Plate Offsets (X,Y)	[4:0-1-8,Edge], [15:0-1-8,Edge], [16:0-1	-8,Edge], [21:0-1-8,Edge]		11-3	-4	
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.63 BC 0.65 WB 0.53 Matrix-S	Vert(LL) -0.17	n (loc) I/defl L/d 7 21-22 >999 480 5 21-22 >697 360 3 14 n/a n/a	PLATES MT20 Weight: 129 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 S WEBS 2x4 S REACTIONS. (siz	P No.1(flat) P No.1(flat) P No.3(flat) ze) 22=0-3-8, 17=0-3-8, 14=Mechanica Grav 22=727(LC 10), 17=1669(LC 1), 14		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied ( 6-0-0 oc bracing: 17-19,16-1	or 10-0-0 oc bracing, I	•
FORCES.         (lb) - Max           TOP CHORD         13-1           8-9           BOT CHORD         21-2           15-1           WEBS         8-17	. Comp./Max. Ten All forces 250 (lb) or 4=-611/0, 2-3=-1987/0, 3-4=-1987/0, 4-5 c0/1282, 9-10=-1182/0, 10-11=-1182/0, 1 22=0/1314, 20-21=0/1987, 19-20=0/1987 6=0/1182, 14-15=0/957 '=-284/0, 2-22=-1440/0, 2-21=0/744, 3-2 )=-647/0, 9-17=-1466/0, 9-16=0/918, 10-	less except when shown. 5=-1739/0, 5-6=-1739/0, 6- 1-12=-1182/0 7, 17-19=-192/817, 16-17=- 1=-260/0, 6-17=-1781/0, 6-	-367/573, -19=0/1122,			
2) All plates are 3x4 M	ve loads have been considered for this d IT20 unless otherwise indicated.	0				

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

1.5x3 ||

3x10 =

3x6 FP =

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

3x6 =

LOAD CASE(S) Standard 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 14-22=-10, 1-13=-100

Concentrated Loads (lb) Vert: 13=-500

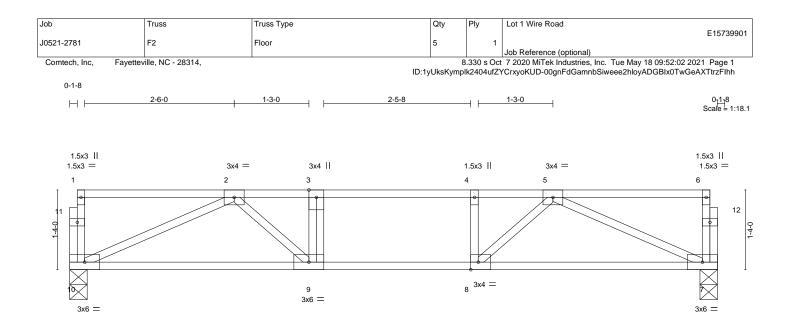


May 18,2021

3x6 =

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ocliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses sand truss systems, see **AVSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			10-10-0 10-10-0			
Plate Offsets (X,Y)	[8:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.42 BC 0.35 WB 0.30 Matrix-S	DEFL.         i           Vert(LL)         -0.03           Vert(CT)         -0.11           Horz(CT)         0.02	1 9-10 >999 360	PLATES MT20 Weight: 56 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end verticals.			

(size) 10=0-3-8, 7=0-3-8 REACTIONS. Max Grav 10=576(LC 1), 7=576(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1234/0, 3-4=-1234/0, 4-5=-1234/0

BOT CHORD 9-10=0/981, 8-9=0/1234, 7-8=0/982 WEBS 2-10=-1073/0, 5-7=-1075/0, 5-8=0/485, 2-9=0/478, 3-9=-255/0, 4-8=-266/0

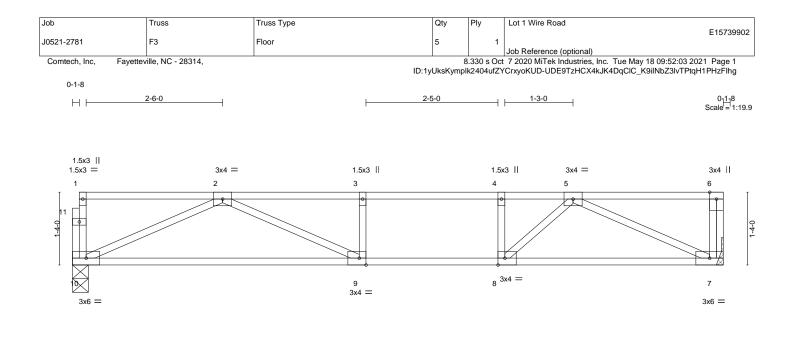
NOTES-

 Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			<u>11-11-0</u> 11-11-0			
Plate Offsets (X,Y)	[8:0-1-8,Edge], [9:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrNOCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.78 BC 0.61 WB 0.34 Matrix-S	DEFL. ir Vert(LL) -0.19 Vert(CT) -0.29 Horz(CT) 0.02	9-10 >490 360	PLATES MT20 Weight: 59 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER-           TOP CHORD         2x4 SP No.1(flat)           BOT CHORD         2x4 SP No.1(flat)           WEBS         2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.		

# **REACTIONS.** (size) 10=0-3-8, 7=Mechanical

Max Grav 10=635(LC 1), 7=1142(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 6-7=-616/0, 2-3=-1508/0, 3-4=-1508/0, 4-5=-1508/0

BOT CHORD 9-10=0/1112, 8-9=0/1508, 7-8=0/1124

## WEBS 2-10=-1219/0, 2-9=0/558, 5-7=-1238/0, 5-8=0/654, 4-8=-352/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

## Uniform Loads (plf)

Vert: 7-10=-10, 1-6=-100

Concentrated Loads (lb)

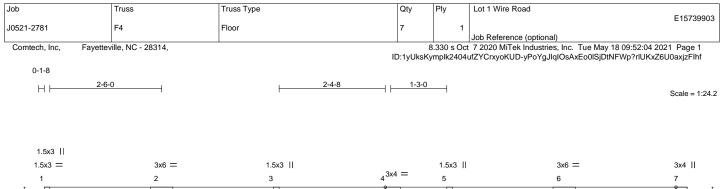
Vert: 6=-500





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BIB Soundside Road Edenton, NC 27932



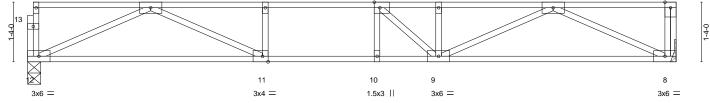


Plate Offsets (X,Y)	[4:0-1-8,Edge], [11:0-1-8,Edge]		1		1	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. i	n (loc) l/defl L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.61	Vert(LL) -0.20	9-10 >838 480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.84	Vert(CT) -0.2	5 9-10 >684 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.46	Horz(CT) 0.03	3 8 n/a n/a		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 73 lb	FT = 20%F, 11%E
LUMBER-		•	BRACING-			
	PNo.1(flat) PNo.1(flat)		TOP CHORD	Structural wood sheathing di except end verticals.	ectly applied or 6-0-0	oc purlins,
	No.3(flat)		BOT CHORD	Rigid ceiling directly applied	or 10-0-0 oc bracing.	
REACTIONS. (size	e) 12=0-3-8, 8=Mechanical					
May	rav 12=778(LC 1), 8=784(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2243/0, 3-4=-2243/0, 4-5=-2186/0, 5-6=-2186/0

BOT CHORD 11-12=0/1424, 10-11=0/2243, 9-10=0/2243, 8-9=0/1429 WEBS

2-12=-1561/0, 2-11=0/958, 3-11=-303/0, 6-8=-1573/0, 6-9=0/836, 5-9=-271/41, 4-9=-428/186

## NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

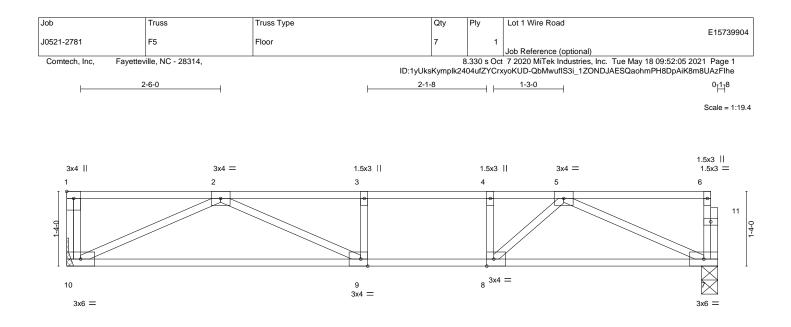
5) CAUTION, Do not erect truss backwards.





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			<u>11-7-8</u> 11-7-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8	,Edge]				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.60 BC 0.51 WB 0.33 Matrix-S	DEFL.         in           Vert(LL)         -0.16           Vert(CT)         -0.25           Horz(CT)         0.02	9-10 >846 480 9-10 >540 360	PLATES MT20 Weight: 58 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER-           TOP CHORD         2x4 SP No.1(flat)           BOT CHORD         2x4 SP No.1(flat)           WEBS         2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	<i>y</i> 11	) oc purlins,	
REACTIONS. (size Max G	e) 10=Mechanical, 7=0-3-8 Grav 10=626(LC 1), 7=619(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1441/0, 3-4=-1441/0, 4-5=-1441/0

BOT CHORD 9-10=0/1081, 8-9=0/1441, 7-8=0/1087

WEBS 2-10=-1190/0, 2-9=0/515, 5-7=-1190/0, 5-8=0/606, 4-8=-323/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



May 18,2021

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