

Trenco 818 Soundside Rd Edenton, NC 27932

Re: B0419-1981 Jackson B

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E12959741 thru E12959761

My license renewal date for the state of North Carolina is December 31, 2019.

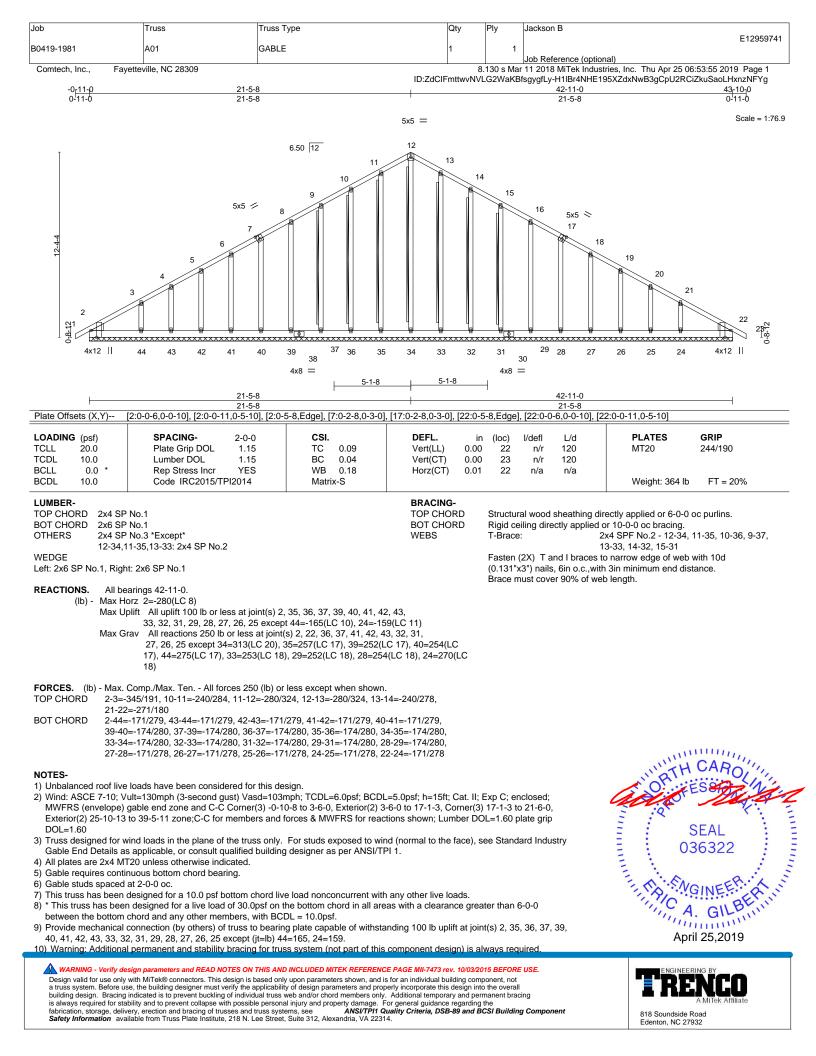
North Carolina COA: C-0844

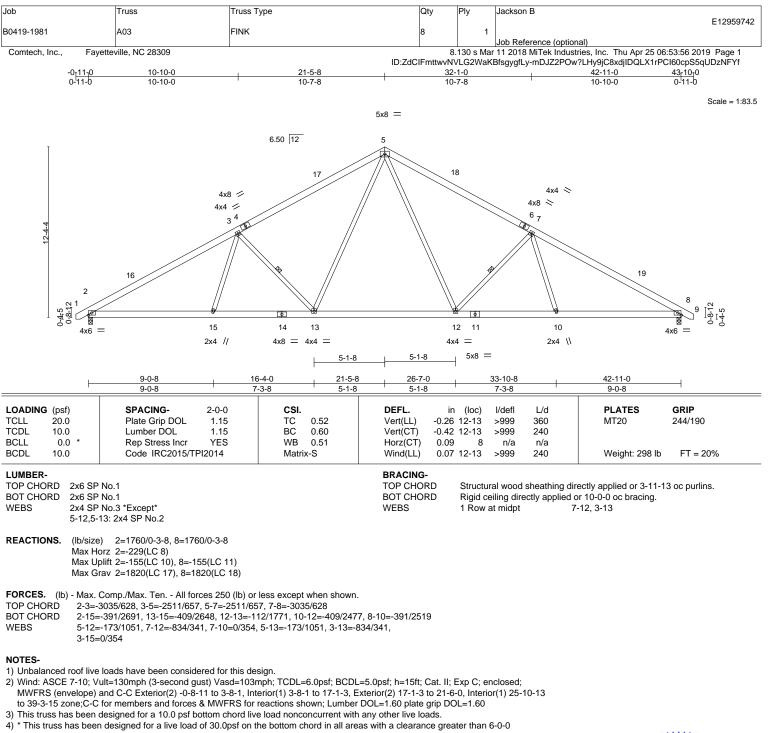


April 25,2019

Gilbert, Eric

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.





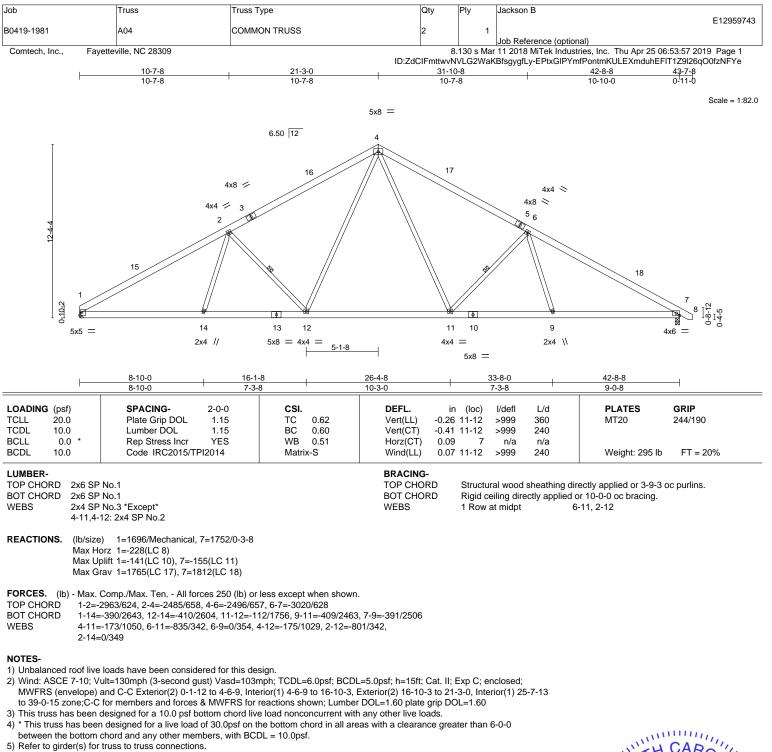
between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=155, 8=155.



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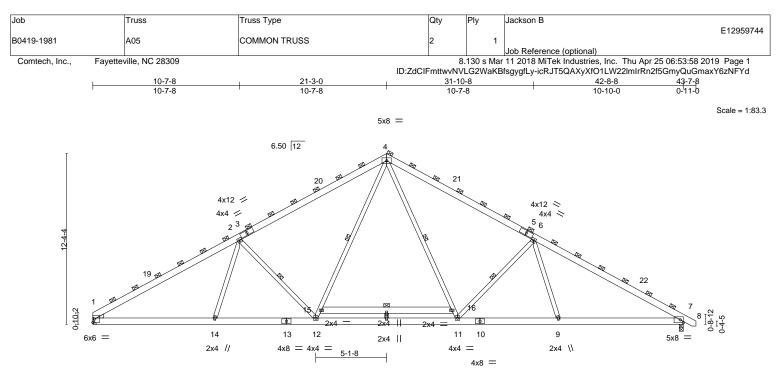


6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=141, 7=155.



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	<u>8-10-0</u> 8-10-0	16-1-8	21-3-0 5-1-8	26-4-8	33-8-0	42-8-8 9-0-8	-
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-1-8 1.15	CSI. TC 0.94	DEFL. Vert(LL) -0.4	in (loc) l/defl L/d 13 11-12 >999 360	PLATES MT20	GRIP 244/190
TCDL 10.0 BCLL 0.0 BCDL 10.0	* Lumber DOL * Rep Stress Incr Code IRC2015/TPI	1.15 NO 2014	BC 0.63 WB 0.77 Matrix-S	Horz(CT) 0.4	32 11-12 >999 240 11 7 n/a n/a 10 11-12 >999 240		FT = 20%

LUMBER-	
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LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.1	TOP CHORD	2-0-0 oc purlins (2-11-10 max.)
BOT CHORD	2x6 SP No.1		(Switched from sheeted: Spacing > 2-0-0).
WEBS	2x4 SP No.3 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
	4-11,4-12: 2x4 SP No.2, 15-16: 2x6 SP No.1	WEBS	1 Row at midpt 4-11, 6-11, 4-12, 2-12, 15-16
WEDGE			

Left: 2x4 SP No.3

REACTIONS. (lb/size) 1=2096/Mechanical, 7=2152/0-3-8 Max Horz 1=-243(LC 6) Max Uplift 1=-188(LC 10), 7=-203(LC 11)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 1-2=-3624/824, 2-4=-2982/862, 4-6=-2994/861, 6-7=-3658/828
- BOT CHORD 1-14=-554/3010, 12-14=-575/2952, 11-12=-280/2212, 9-11=-575/2987, 7-9=-556/3050
- WEBS 4-16=-270/1215, 11-16=-175/879, 6-11=-871/358, 6-9=0/375, 12-15=-176/853,
 - 4-15=-271/1188, 2-12=-830/357, 2-14=0/369

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-10-3, Exterior(2) 16-10-3 to 21-3-0, Interior(1) 25-7-13 to 39-0-15 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=188 7=203
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

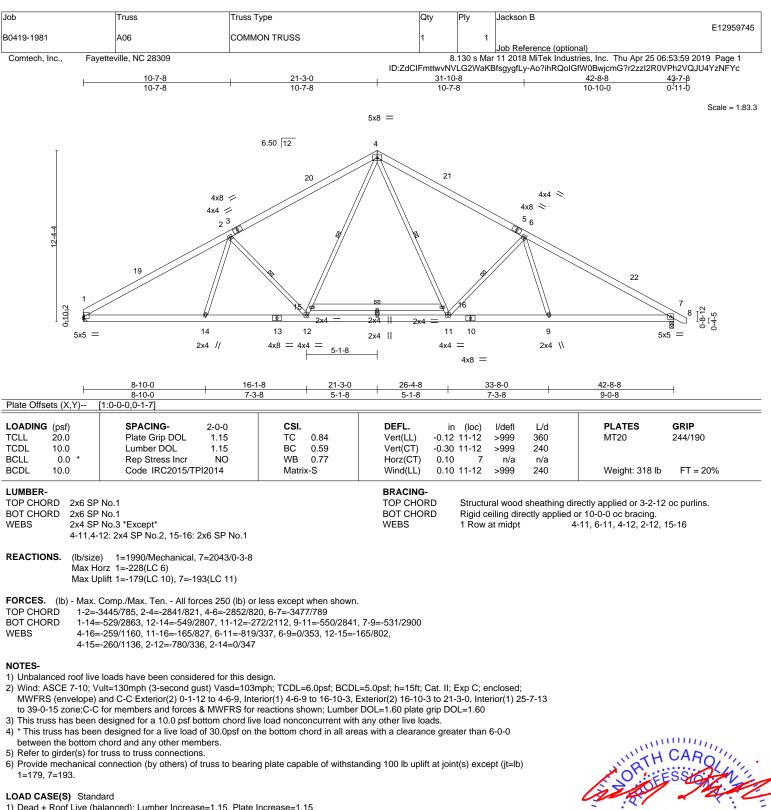
Uniform Loads (plf)

Vert: 1-4=-64, 4-8=-64, 1-7=-21, 15-16=-60





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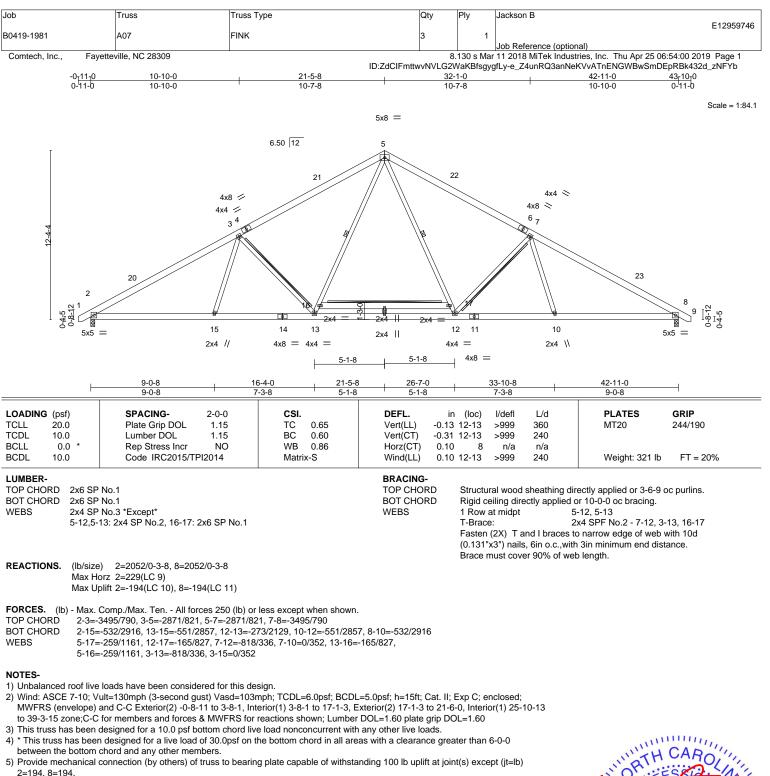
Uniform Loads (plf)

Vert: 1-4=-60, 4-8=-60, 1-7=-20, 15-16=-60



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6) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard

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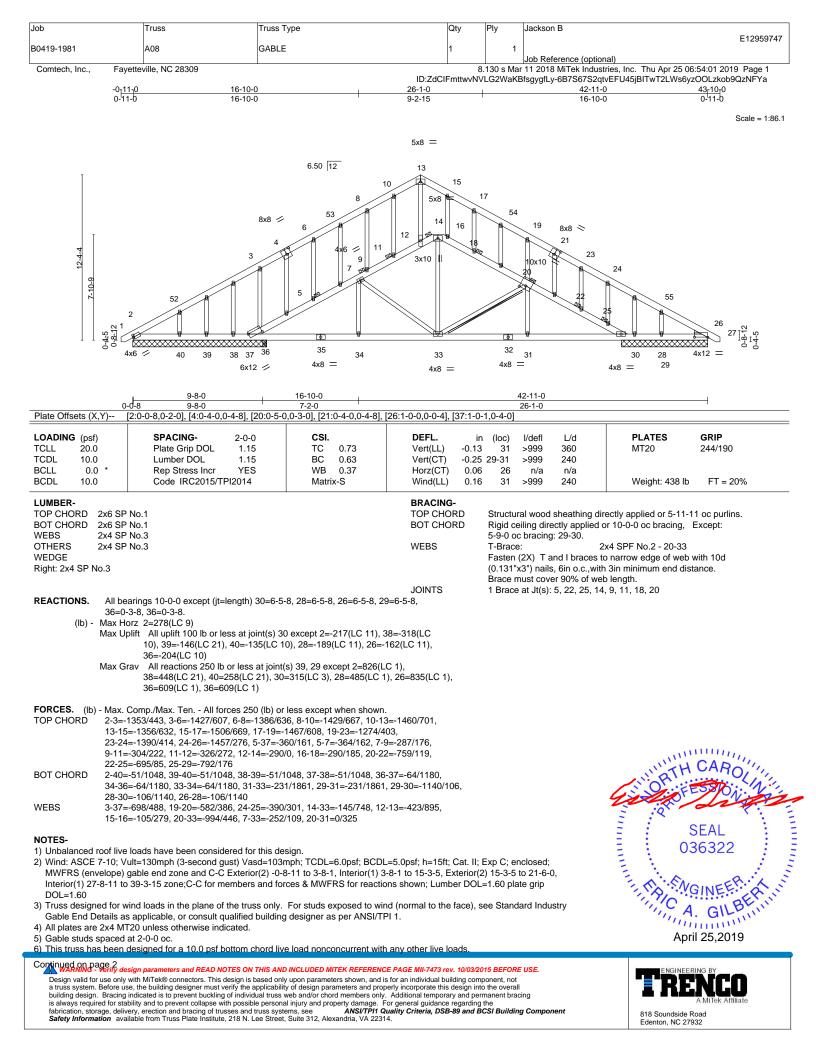
Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-60, 2-8=-20, 16-17=-60





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Job	Truss	Truss Type	Qty	Ply	Jackson B
B0419-1981	A08	GABLE	1	1	E12959747
					Job Reference (optional)
Comtech, Inc.,	Comtech, Inc., Fayetteville, NC 28309				11 2018 MiTek Industries, Inc. Thu Apr 25 06:54:02 2019 Page 2
			ID:ZdCIFmttwvN	VLG2WaKE	BfsgygfLy-aNhqJTTgbB15tefHHupiThbWGGSBireUBOY8htzNFYZ

NOTES-

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

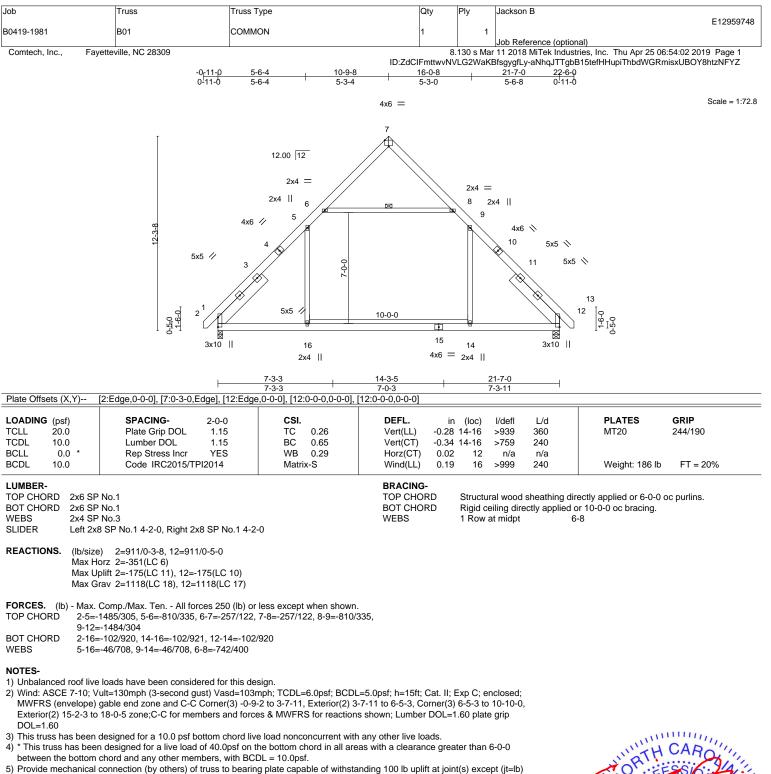
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 30 except (jt=lb) 2=217, 38=318, 39=146, 40=135, 28=189, 26=162, 36=204.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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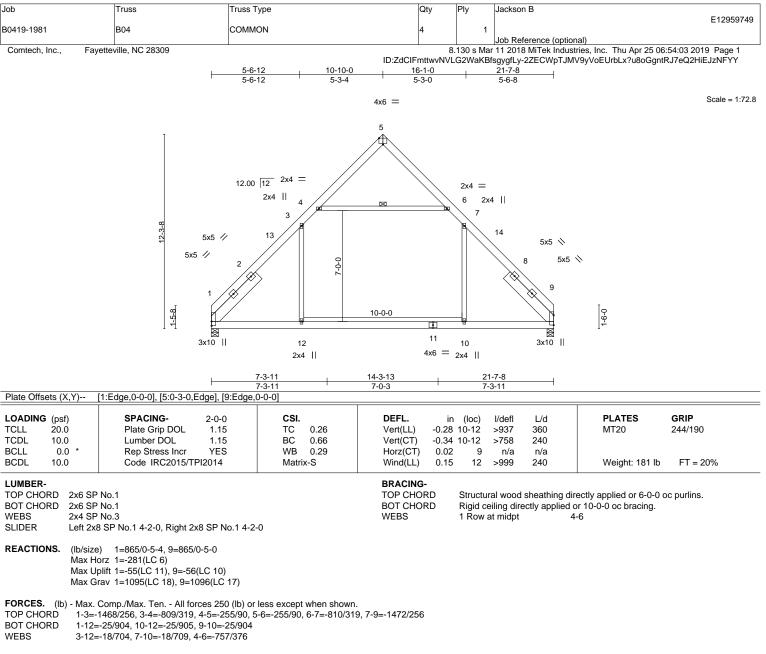


 Frovide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=175, 12=175.



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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 6-5-3, Exterior(2) 6-5-3 to 10-10-0, Interior(1) 15-2-3 to 17-2-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

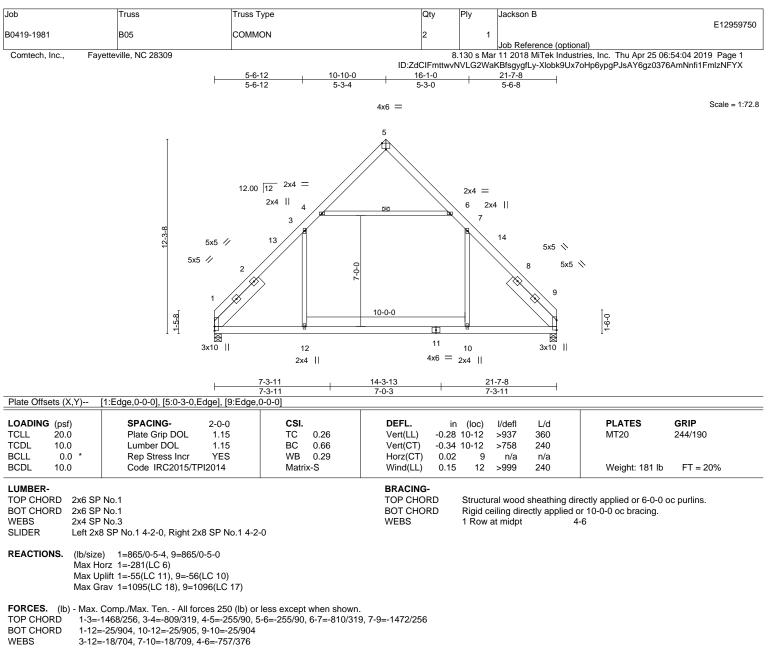
4) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



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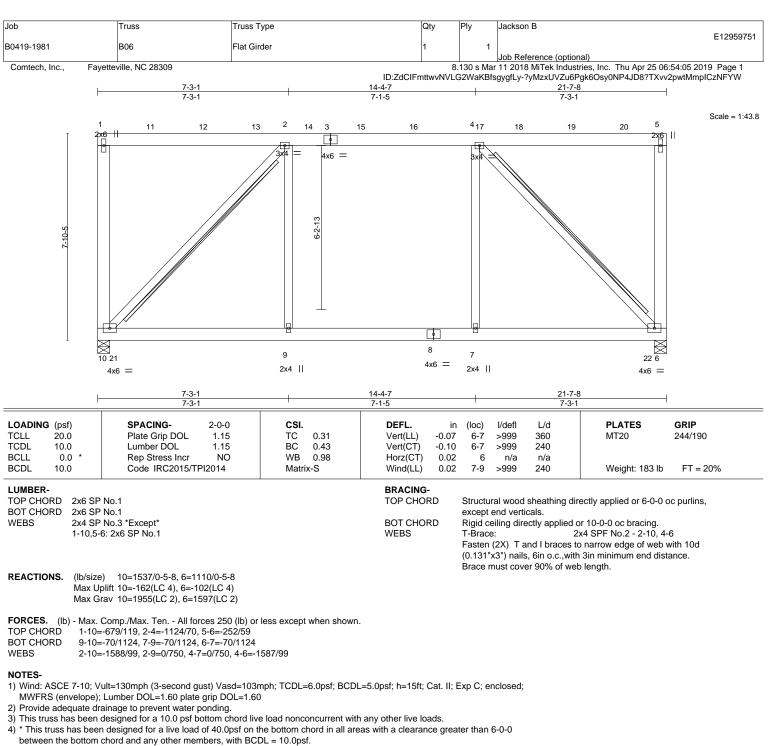
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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=162. 6=102.

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 439 lb down and 68 lb up at 0-2-12, 95 lb down and 20 lb up at 2-0-12, 95 lb down and 20 lb up at 4-0-12, 95 lb down and 20 lb up at 6-0-12, 95 lb down and 20 lb up at 4-0-12, 95 lb down and 20 lb up at 12-0-12,

7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-60, 6-10=-20

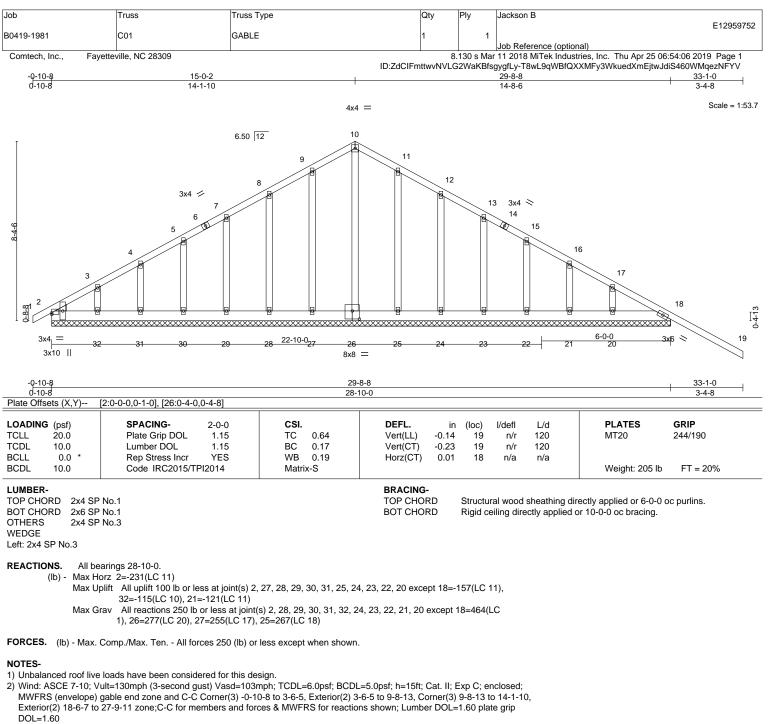
Concentrated Loads (lb)

Vert: 1=-439 11=-51(F) 12=-51(F) 13=-51(F) 14=-51(F) 15=-51(F) 16=-51(F) 17=-51(F) 18=-51(F) 19=-51(F) 20=-51(F)

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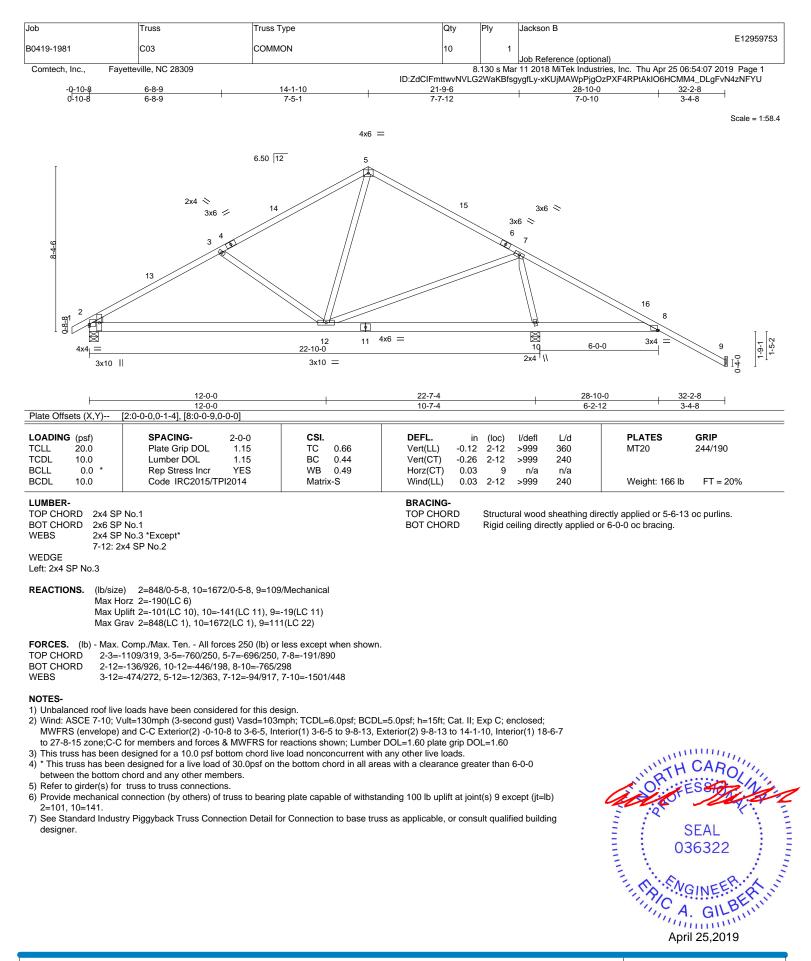


- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 27, 28, 29, 30, 31, 25, 24, 23, 22, 20 except (jt=lb) 18=157, 32=115, 21=121.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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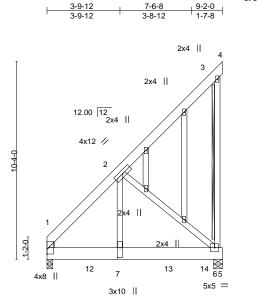






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Job	Truss	Truss Type	Qty	Ply	Jackson B	
					E129597	754
B0419-1981	D01	GABLE	1	2		
				_	Job Reference (optional)	
Comtech, Inc.,	Fayetteville, NC 28309		8	.130 s Mar	r 11 2018 MiTek Industries, Inc. Thu Apr 25 06:54:08 2019 Page 1	
			ID:ZdCIFmttwvNVL	G2WaKBf	fsgygfLy-PX25aWXRA1oFbZ6Re9w6iyrf9hRI5UUNaK?TvWzNFYT	





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Plate Offsets (X,Y)	[1:0-1-0,0-3-6],	[1:0-0-8,0-0-8],	[7:0-6-4,0-1-8]
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Plate Offsets (X,Y)	[1:0-1-0,0-3-6], [1:0-0-8,0-0-8], [7:0-6-4,	0-1-8]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.32 BC 0.84 WB 0.67 Matrix-S	DEFL. i Vert(LL) -0.04 Vert(CT) -0.03 Horz(CT) 0.01 Wind(LL) 0.03	36-7 16	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 201 lb	GRIP 244/190 FT = 20%
	No.1 No.3 *Except* ↓ SP No.2		BRACING- TOP CHORD BOT CHORD WEBS	except Rigid ce T-Brace Fasten (0.131";	end verticals. eiling directly applie e: (2X) T and I brace	directly applied or 6-0-0 d or 10-0-0 oc bracing. 2x4 SPF No.2 - 3-6 s to narrow edge of web vith 3in minimum end dis web length.	with 10d
Max Ho Max Up FORCES. (Ib) - Max. () 6=4921/0-5-8, 1=3585/0-3-8 brz 1=453(LC 8) blift 6=-798(LC 8), 1=-212(LC 4) Comp./Max. Ten All forces 250 (lb) or 1115/301	less except when shown					
WEBS 2-7=-4	463/2629, 6-7=-464/2648 416/5036, 2-6=-3458/605 nected together with 10d (0.131"x3") na	ils as follows:					
Top chords connecte Bottom chords conne Webs connected as f 2) All loads are conside ply connections have 3) Wind: ASCE 7-10; V MWFRS (envelope) (ad as follows: 2x6 - 2 rows staggered at acted as follows: 2x8 - 2 rows staggered follows: 2x4 - 1 row at 0-9-0 oc. red equally applied to all plies, except is been provided to distribute only loads ult=130mph (3-second gust) Vasd=103 gable end zone; Lumber DOL=1.60 plai	0-9-0 oc, 2x4 - 1 row at 0 d at 0-5-0 oc. i noted as front (F) or bac noted as (F) or (B), unles mph; TCDL=6.0psf; BCDI te grip DOL=1.60	k (B) face in the LOAD (s otherwise indicated. L=5.0psf; h=15ft; Cat. II;	Exp C; er	nclosed;	THUNNING TH	CARO 111
Gable End Details as 5) Gable studs spaced a 6) This truss has been of	designed for a 10.0 psf bottom chord liv	designer as per ANSI/TI e load nonconcurrent with	PI 1.		lard Industry	Contraction of the second	SEAL
between the bottom of 8) Provide mechanical of 6=798, 1=212.	a designed for a live load of 30.0psf on the chord and any other members. connection (by others) of truss to bearing the provided of the connection of the	g plate capable of withsta	anding 100 lb uplift at joi	nt(s) exce	ept (jt=lb)		36322
2-2-12, 2075 lb down on bottom chord. Th	onnection device(s) shall be provided so and 209 lb up at 4-1-4, and 2075 lb d e design/selection of such connection of permanent and stability bracing for true	own and 209 lb up at 6-4 levice(s) is the responsibi	-4, and 1970 lb down ar ility of others.	nd 199 lb u	up at 8-2-12 up at 8-2-12	ALL AND A	GINEEL PALIN

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard Continued on page 2

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-1473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



A. GI A. GIL

April 25,2019

Scale = 1:60.2

Job	Truss	Truss Type	Qty	Ply	Jackson B
					E12959754
B0419-1981	D01	GABLE	1	ົ	
				2	Job Reference (optional)
Comtech, Inc., Fayetter	/ille, NC 28309		8	130 s Mar	11 2018 MiTek Industries, Inc. Thu Apr 25 06:54:08 2019 Page 2

8.130 s Mar 11 2018 MiTek Industries, Inc. Thu Apr 25 06:54:08 2019 Page 2 ID:ZdCIFmttwvNVLG2WaKBfsgygfLy-PX25aWXRA1oFbZ6Re9w6iyrf9hRI5UUNaK?TvWzNFYT

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

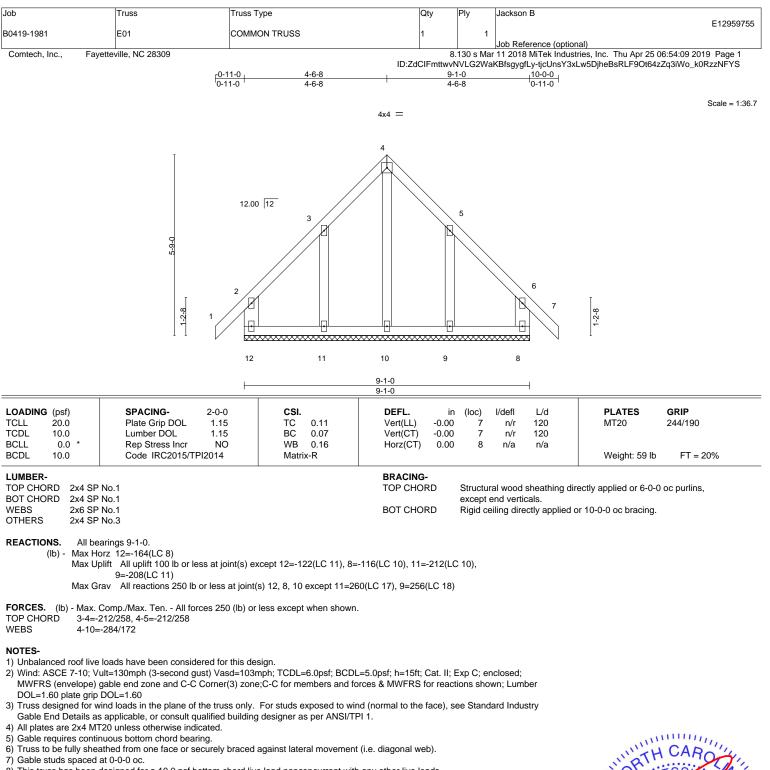
Uniform Loads (plf) Vert: 1-3=-60, 3-4=-20, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-2075(B) 12=-1676(B) 13=-2075(B) 14=-1970(B)

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- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 12, 116 lb uplift at joint 8, 212 lb uplift at joint 11 and 208 lb uplift at joint 9.



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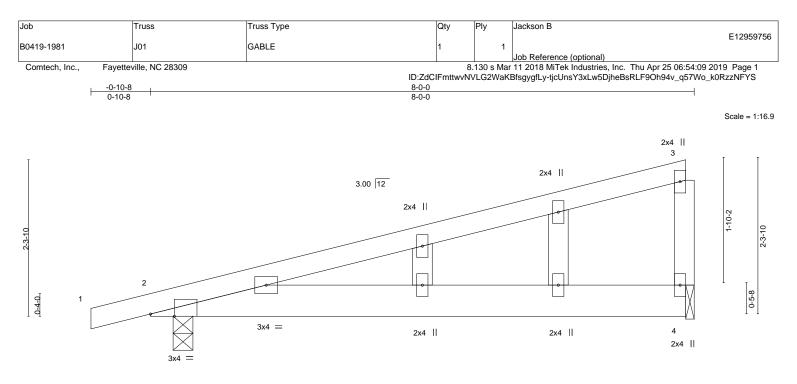


Plate Offsets (X,Y)	0-4-0 ' [2:0-4-3,Edge]		7-8-0						1
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
rcll 20.0	Plate Grip DOL 1.15	TC 0.88	Vert(LL)	-0.05	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.36	Vert(CT)	-0.10	2-4	>969	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00	4	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.00	2	****	240	Weight: 37 lb	FT = 20%

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD BOT CHORD 2x6 SP No.1

2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS

REACTIONS. (lb/size) 2=374/0-3-8, 4=303/0-1-8 Max Horz 2=106(LC 6)

Max Uplift 2=-140(LC 6), 4=-104(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber
- DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 2 and 104 lb uplift at ioint 4.



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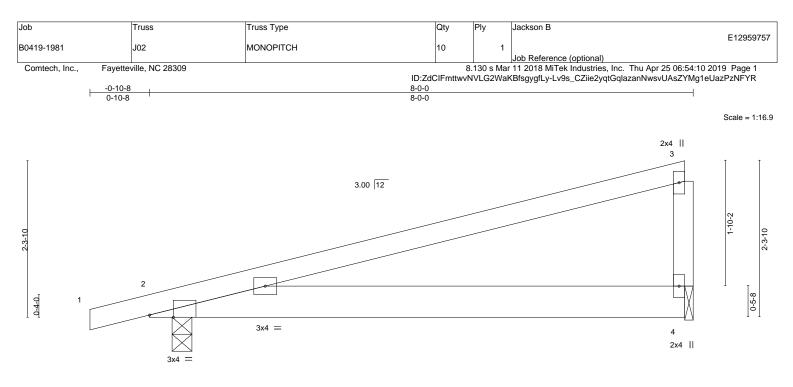


Plate Offsets (X,Y)	0-4-0 0-4-0 [2:0-4-3,Edge]								
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
rcll 20.0	Plate Grip DOL 1.15	TC 0.88	Vert(LL)	-0.05	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.64	Vert(CT)	-0.10	2-4	>969	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00	4	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.10	2-4	>886	240	Weight: 34 lb	FT = 20%

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=374/0-3-8, 4=303/0-1-8 Max Horz 2=75(LC 6)

Max Uplift 2=-159(LC 6), 4=-132(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

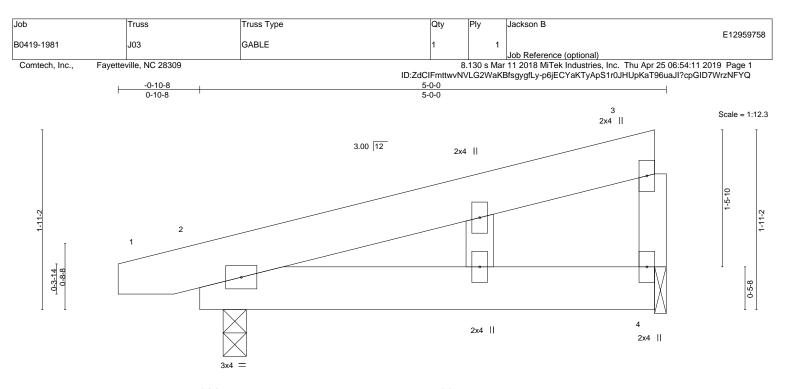
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 2 and 132 lb uplift at joint 4.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REPERENCE PAGE MIL-14's rev. Invozens Derrore USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





		0-	3-0			4	-9-0				1	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL :	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.43	Vert(CT)	-0.01	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-P	Wind(LL)	0.02	2-4	>999	240	Weight: 28 lb	FT = 20%

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 4=184/0-1-8, 2=234/0-3-0 Max Horz 2=64(LC 10)

Max Uplift 4=-118(LC 6), 2=-139(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

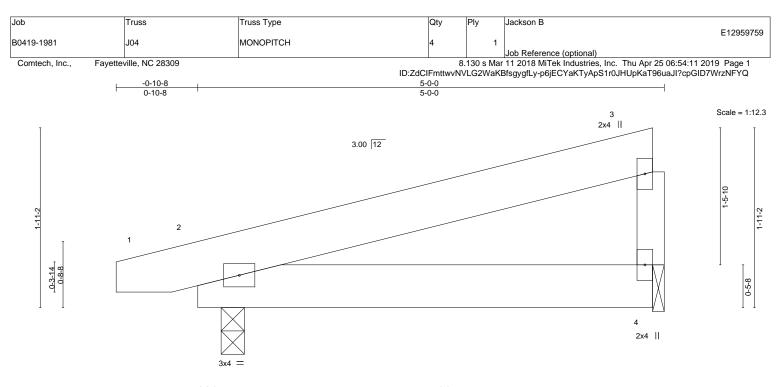
3) Gable studs spaced at 2-0-0 oc.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 4 and 139 lb uplift at joint 2.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSTPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Pitat Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





0-3-0				5-0-0 4-9-0						1	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	тс	0.34	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.43	Vert(CT)	-0.01	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2	014	Matrix	(-P	Wind(LL)	0.02	2-4	>999	240	Weight: 27 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 4=184/0-1-8, 2=234/0-3-0 Max Horz 2=45(LC 10) Max Uplift 4=-82(LC 6), 2=-98(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 4 and 98 lb uplift at joint 2.



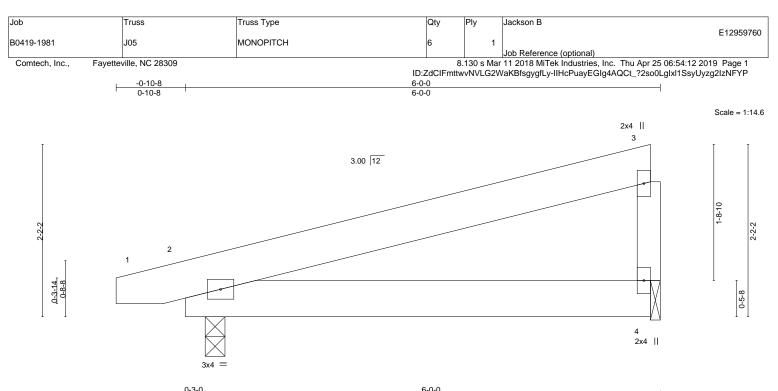
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.



	0-3-0							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	TC 0.22 Ve BC 0.29 Ve WB 0.00 Ho	in rt(LL) -0.01 rt(CT) -0.03 rz(CT) 0.00 nd(LL) 0.00	(loc) 2-4 2-4 4 2	l/defl >999 >999 n/a ****	L/d 360 240 n/a 240	PLATES MT20 Weight: 32 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.3

BRACING-TOP CHORD

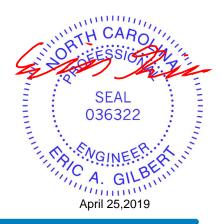
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=274/0-3-0, 4=225/0-1-8 Max Horz 2=53(LC 10) Max Uplift 2=-49(LC 6), 4=-37(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

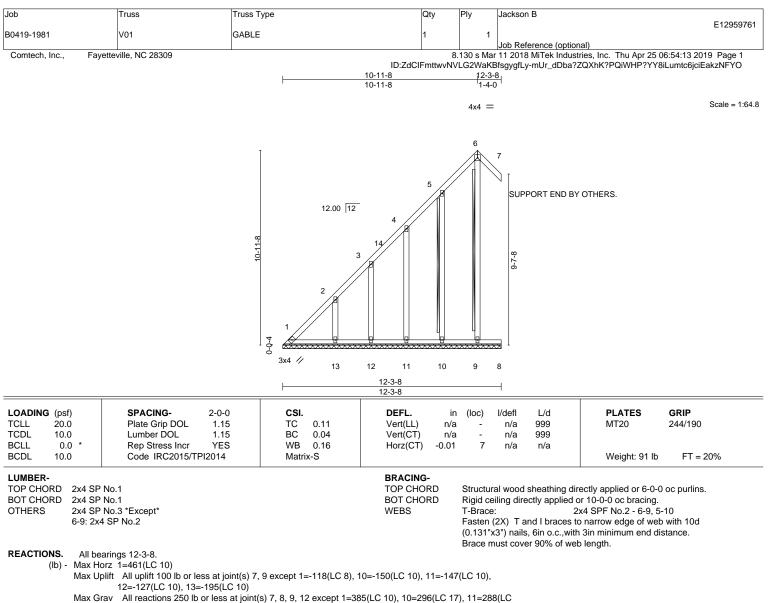
NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 2 and 37 lb uplift at joint 4.



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17), 13=255(LC 17)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-584/455, 2-3=-414/307, 3-4=-291/213

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-4 to 4-11-8, Interior(1) 4-11-8 to 6-6-11, Exterior(2) 6-6-11 to 10-11-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 9 except (jt=lb) 1=118, 10=150, 11=147, 12=127, 13=195.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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