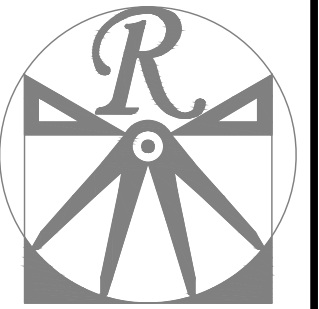


PLANS DESIGNED TO THE  
2018 NORTH CAROLINA STATE  
RESIDENTIAL BUILDING CODE.

GENERAL NOTES

1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
3. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED.
4. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL SITE REQUIREMENTS.



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**Lot 1 Ring Rosser Pittman  
5986 Rosser Pittman Rd  
Sanford, NC 27332**



STEPS PER GRADE AS REQ.

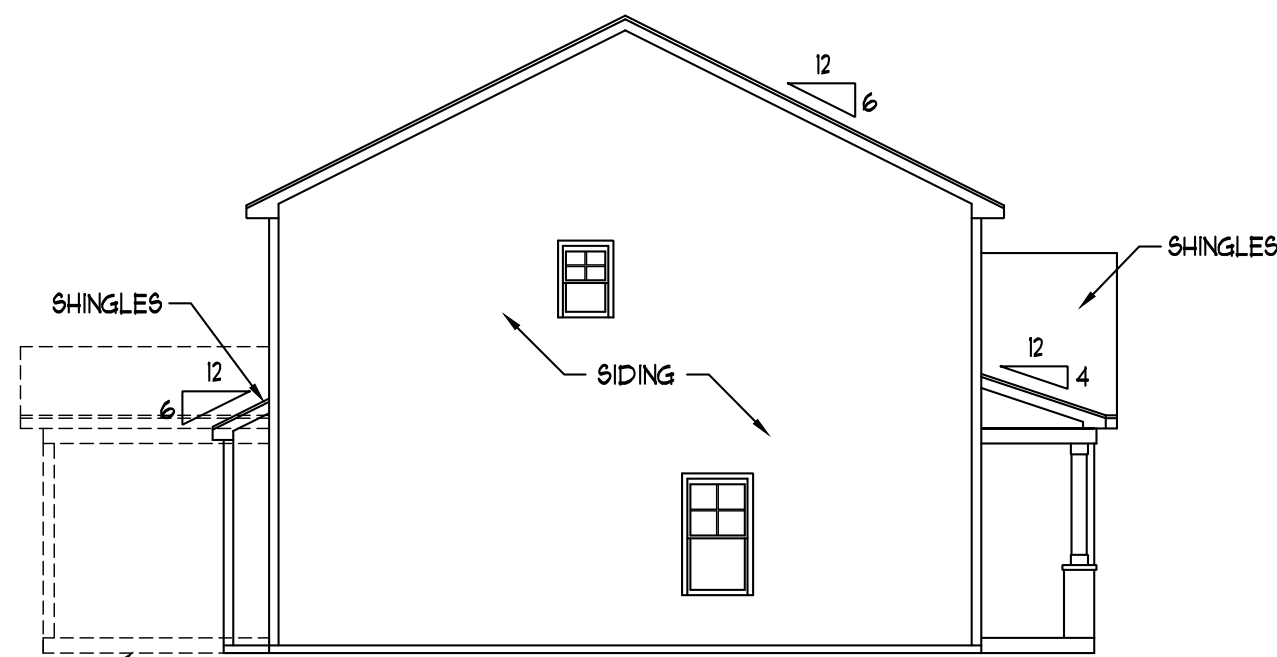
**FRONT ELEVATION-C**

SCALE: 1/4" = 1'-0"

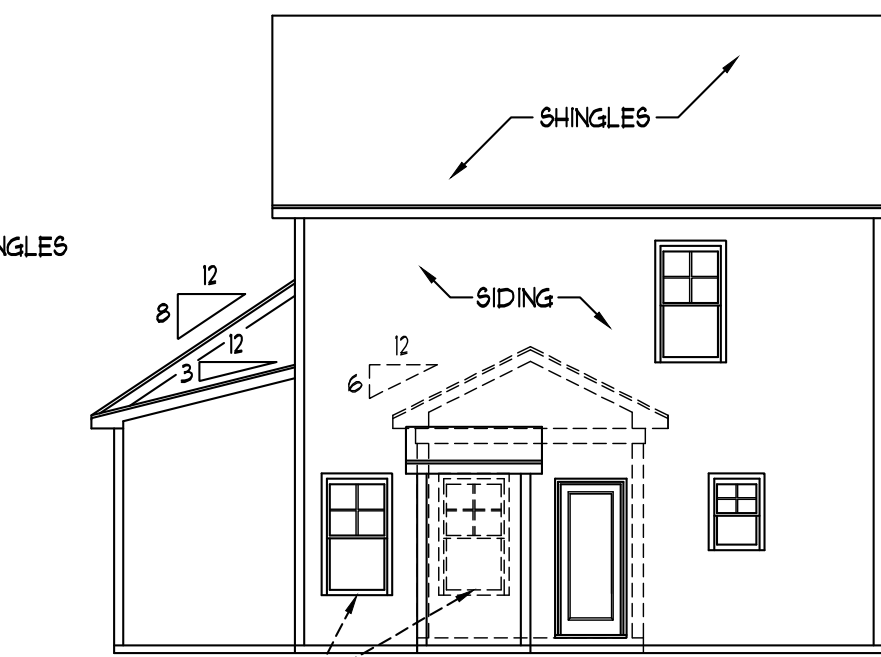
**NOTICE TO CONTRACTOR**  
All construction must comply with current NC Building Codes and is subject to field inspection and verification.

**APPROVED**  
Limited liability only review  
Permit holder responsible for full compliance with the code

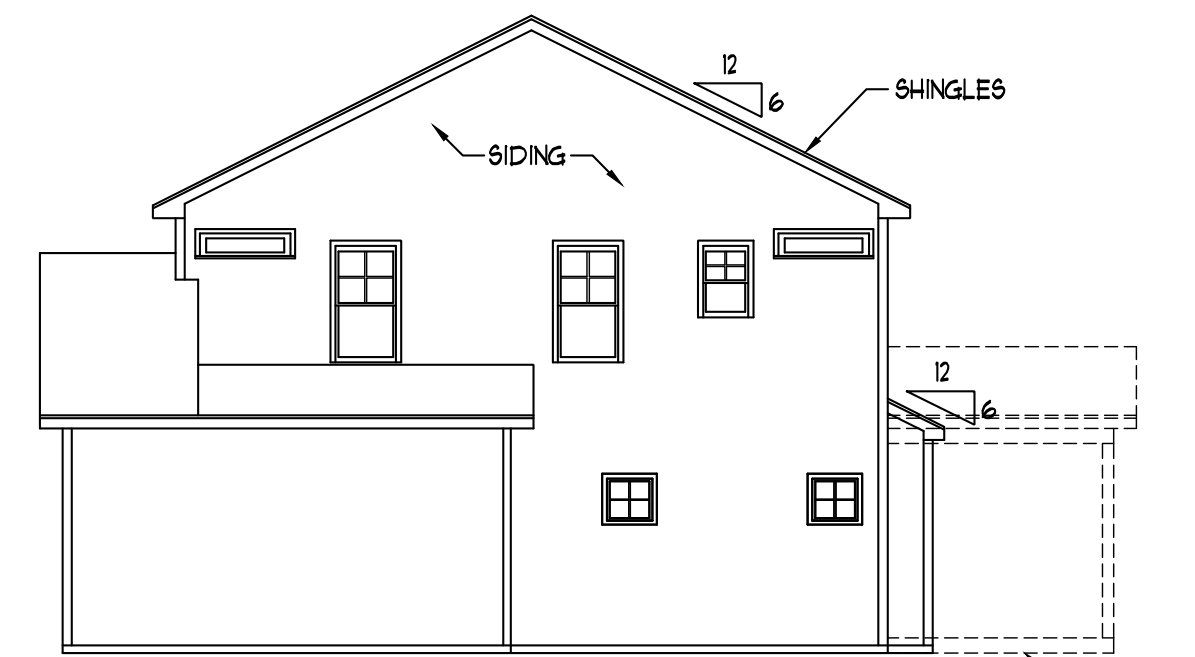
09/07/2021



**LEFT ELEVATION**  
SCALE: 1/8" = 1'-0"



**REAR ELEVATION**  
SCALE: 1/8" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/8" = 1'-0"

HVAC: Mainstream  
Electrical: Pioneer Electric  
Plumbing: Double J

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.  
**11x17 PRINTS ARE NOT TO SCALE**

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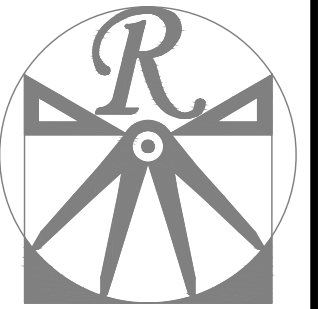
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CAROLINA COLLECTION  
MAGNOLIA-II

DATE: FEBRUARY 19, 2021  
REV.:  
SCALE: 1/4" = 1'-0"  
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ENGINEERED BY:  
REVIEWED BY:

C - ELEVATIONS

**A-3**





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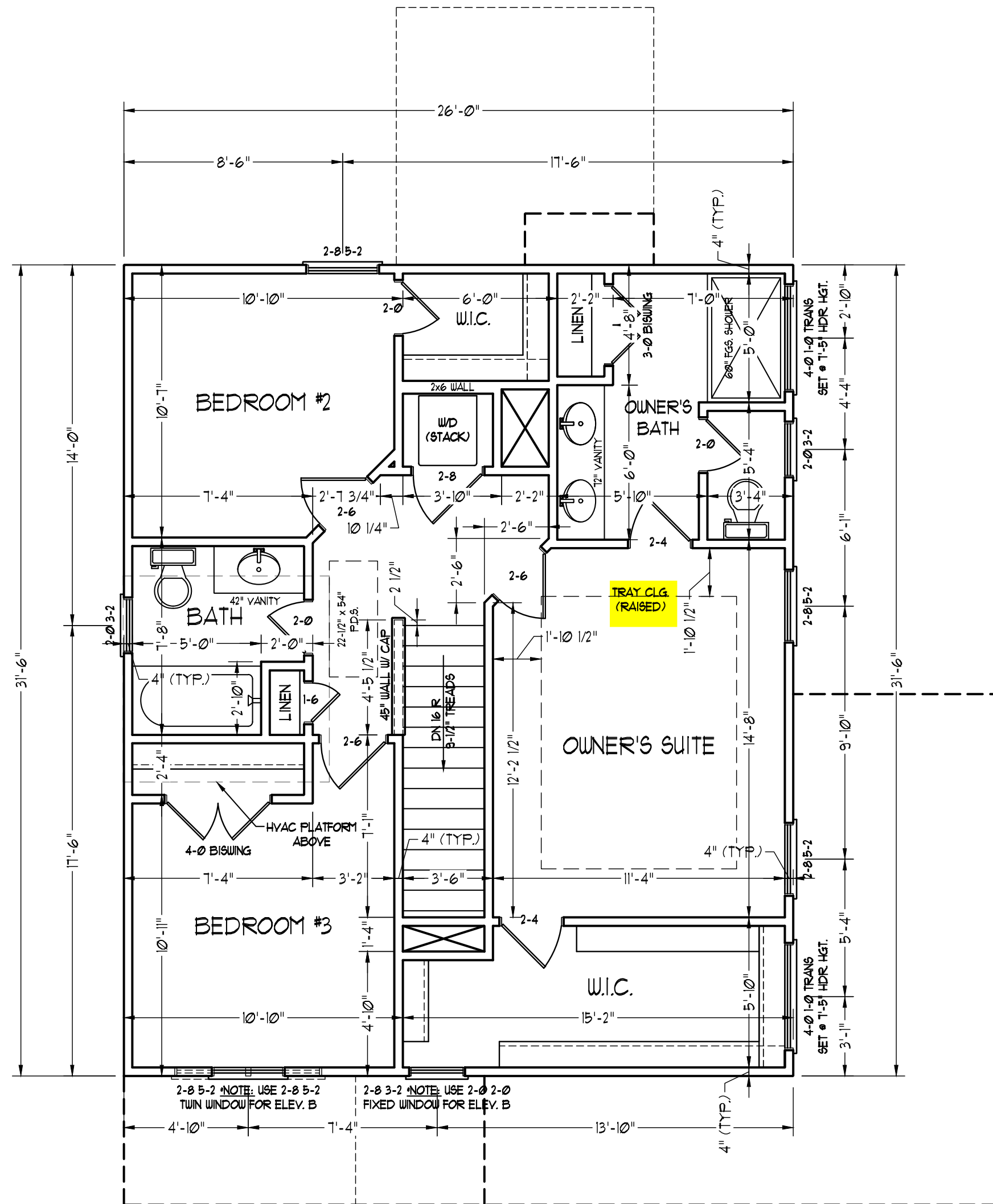
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DATE: FEBRUARY 19, 2021

REV.:

SCALE: 1/4" = 1'-0"

DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:

SECOND FLOOR PLAN

**A-5**

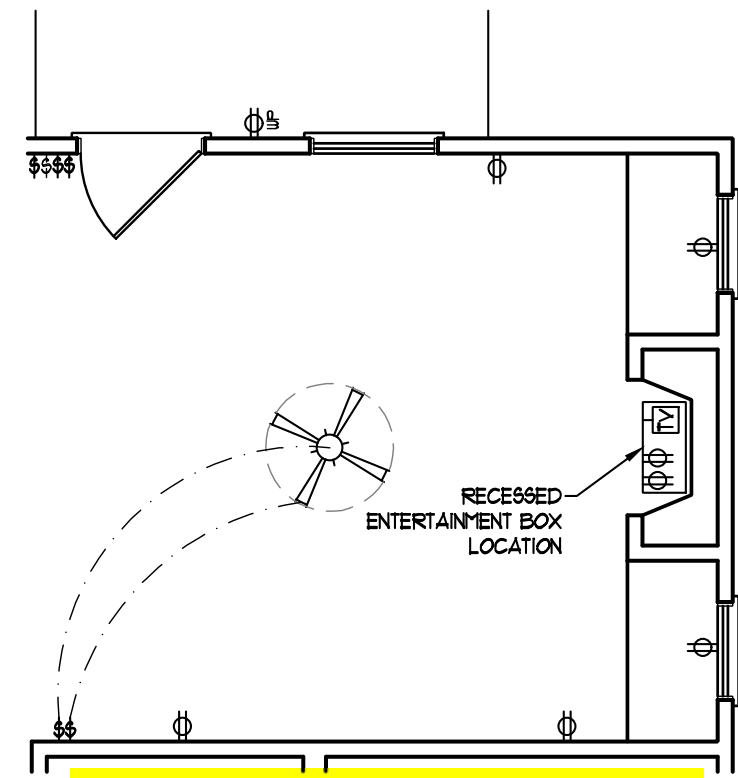
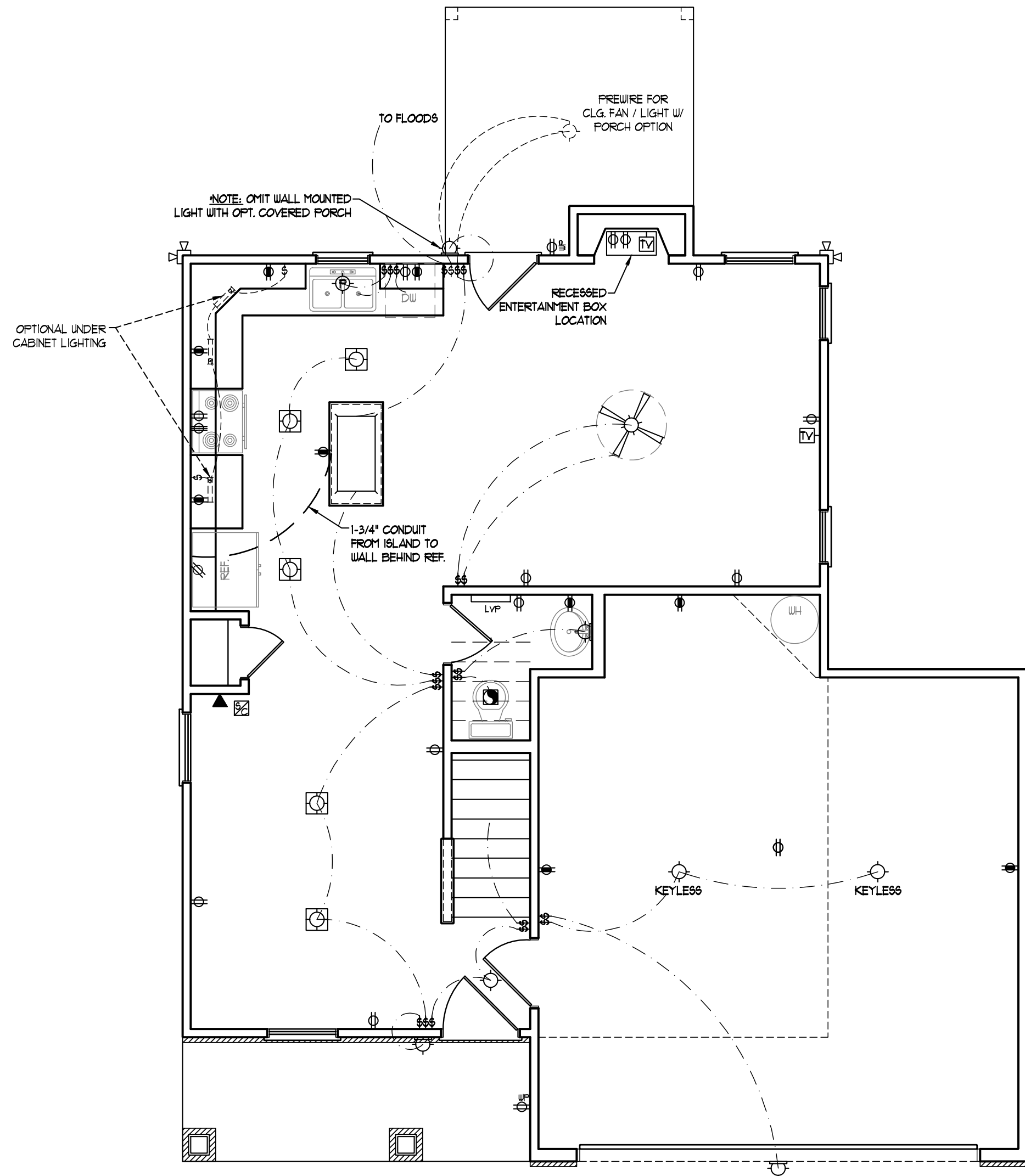


**ELECTRICAL LAYOUT NOTES:**

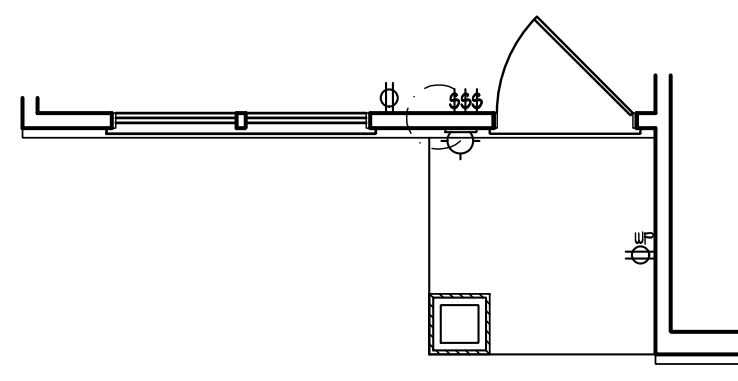
- 1) BLOCK AND WIRE FOR ALL CEILING FANS PER PLAN.
- 2) VANITY LIGHTS TO BE SET @ 90° AFF. (TYP.)
- 3) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4) PLACE SWITCHES 8" (MIN) FROM ROUGH OPENINGS.

**ELECTRICAL LEGEND**

- ⊕ 110 V OUTLET
- ⊕ 110 V GFI OUTLET
- ⊕ 110 V SWITCHED OUTLET
- bb ⊕ 110 V BASEBOARD OUTLET
- ⊕ 4-FLEX
- ⊕ COUNTER OR FLOOR MOUNTED
- ⊕ COUNTER OR FLOOR MOUNTED 110V GFI
- ⊕ WEATHERPROOF
- ⊕ 220 V OUTLET
- ⊕ 110 V DEDICATED CIRCUIT
- ⊕ 220 V DEDICATED CIRCUIT
- ⊕ SPECIAL PURPOSE (240 V, ETC.)
- ⊕ WALL MOUNT LIGHT
- ⊕ CEILING MOUNT LIGHT
- ⊕ PENDANT LIGHT
- ⊕ RECESSED CAN LIGHT
- ⊕ MINI CAN LIGHT
- ⊕ EYEBALL LIGHT
- ⊕ FLUORESCENT LIGHT
- ⊕ UNDERCABINET LIGHT
- ⊕ FLOOD LIGHT
- ⊕ SWITCH
- ⊕ DIMMER SWITCH
- ▲ TELEPHONE
- ▲ DATA
- ▲ TELEPHONE AND DATA
- TV TV CONNECTION
- TV TV / DATA
- CONDUIT FOR COMPONENT WIRING
- SPK SPEAKER
- SD 110 V SMOKE / CM DETECTOR
- SD 110 V SMOKE DETECTOR
- EXHAUST FAN
- LVP LOW VOLTAGE PANEL
- ALARM ALARM PANEL
- ⊕ CEILING FAN
- ⊕ CEILING FAN W/ LIGHT

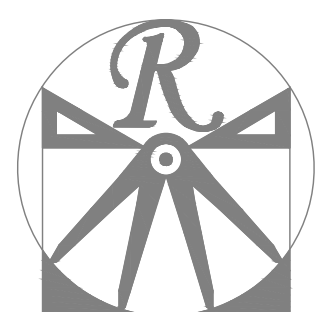


OPTIONAL FAMILY ROOM WITH ALTERNATE FIREPLACE LOCATION AND BUILT-INS



FRONT PORCH AND DINING ROOM WINDOW ELEVATION B

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REV.:  
SCALE: 1/4" = 1'-0"  
DRAWN BY: WG  
ENGINEERED BY:  
REVIEWED BY:

FIRST FLOOR  
ELECTRICAL  
PLAN

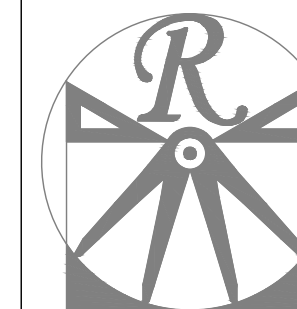
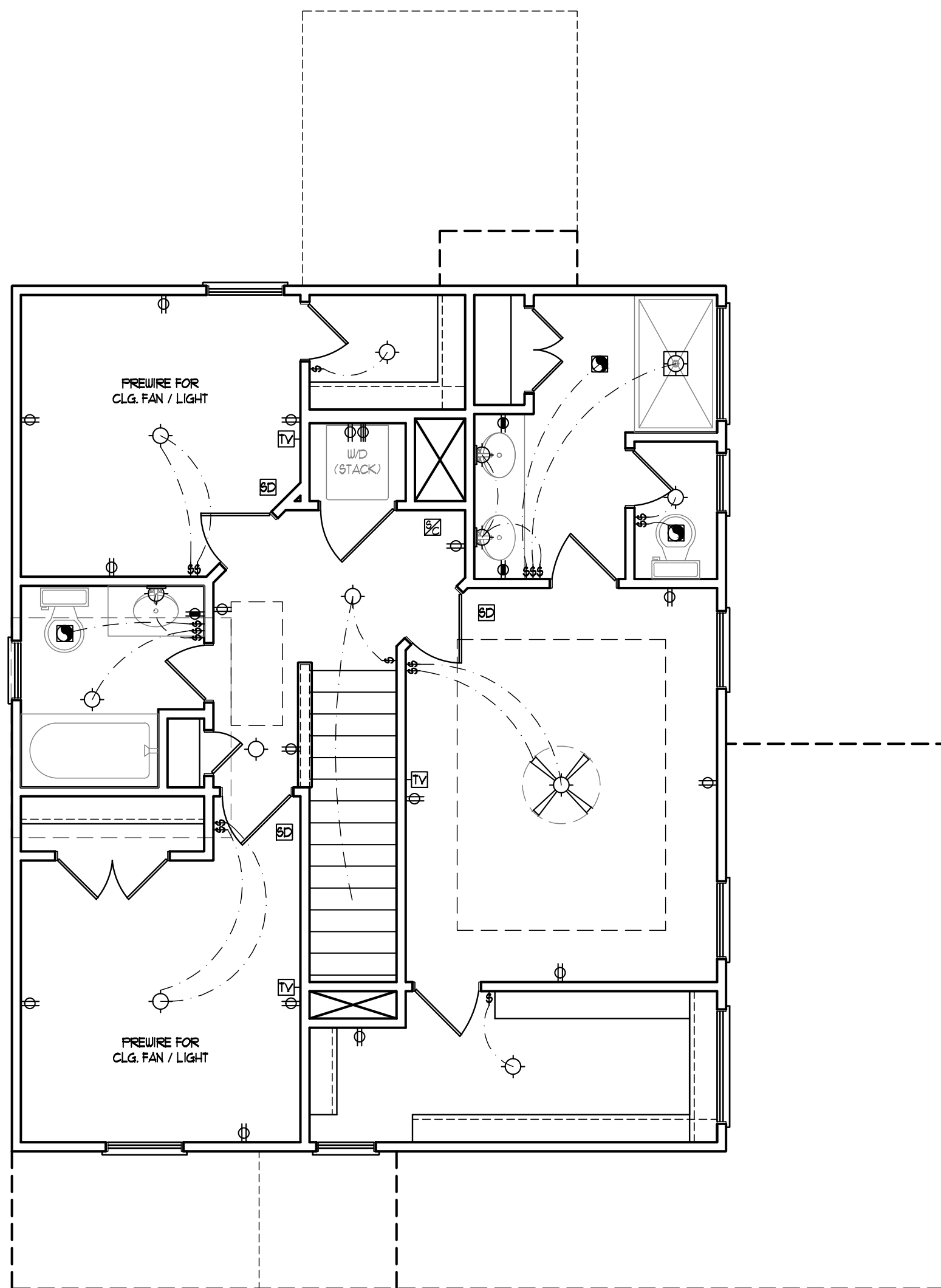
E-1

**ELECTRICAL LAYOUT NOTES:**

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- ▲ TELEPHONE
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- TV TV CONNECTION
- TV TV / DATA
- CD CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- SD 110 V SMOKE / CO DETECTOR
- SD 110 V SMOKE DETECTOR
- EF EXHAUST FAN
- LVP LOW VOLTAGE PANEL
- ALARM ALARM PANEL
- ⊕ CEILING FAN
- ⊕ CEILING FAN W/ LIGHT



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DATE: FEBRUARY 19, 2021

REV.:

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DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:

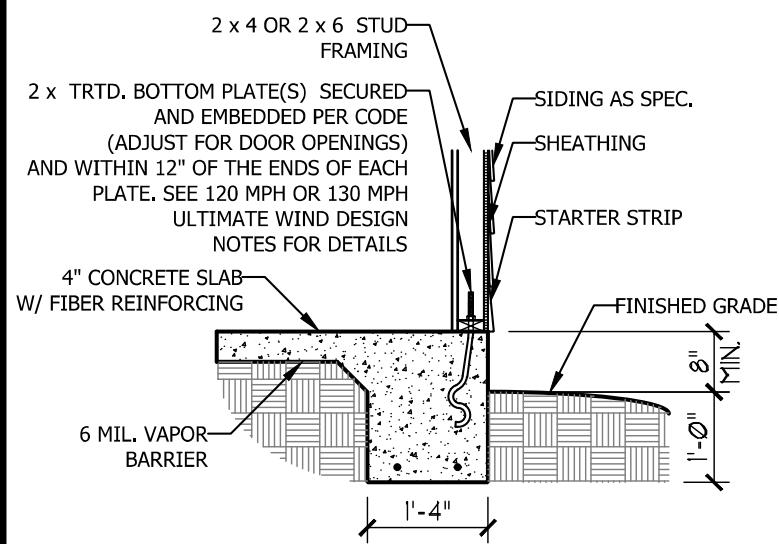
SECOND FLOOR  
ELECTRICAL  
PLAN

E-2

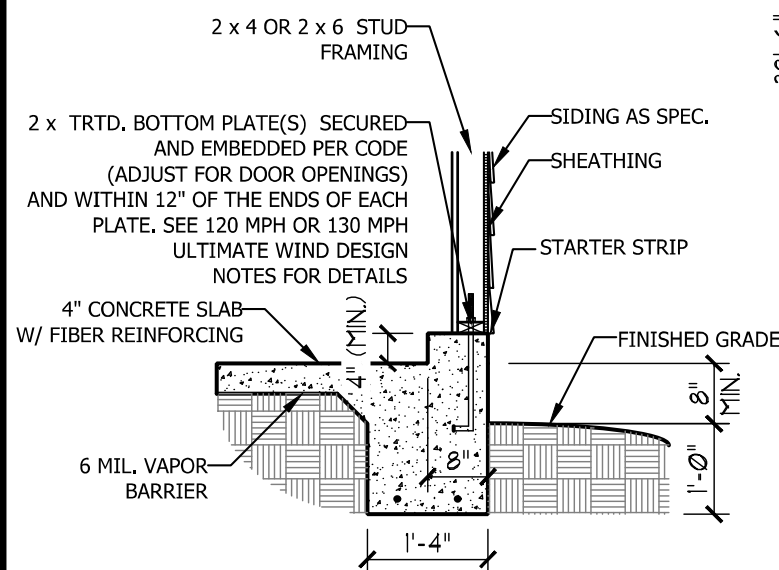
SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.  
**11x17 PRINTS ARE NOT TO SCALE**

**ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:**

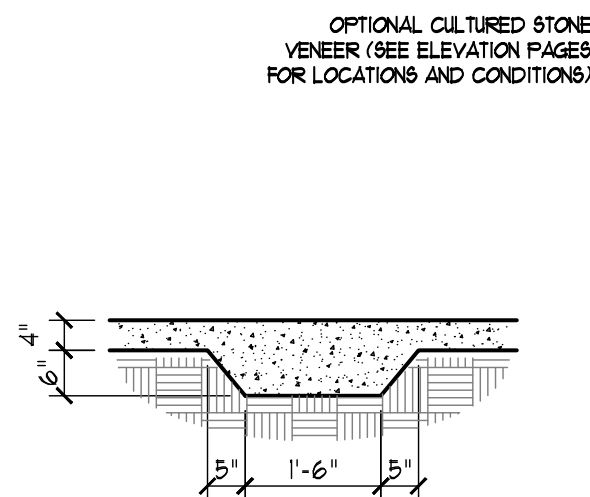
1. STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
2. FOR 120 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 6'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
3. FOR 130 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 4'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
4. MEAN ROOF HEIGHT IS LESS THAN 30 FEET.
5. EXTERIOR WALLS DESIGNED FOR 120 OR 130 MPH WINDS.
6. INSTALL 7/16" OSB SHEATHING ON ALL EXTERIOR WALLS OF ALL STORIES IN ACCORDANCE WITH SECTION R602.10.3 OF THE NRC, 2018 EDITION.
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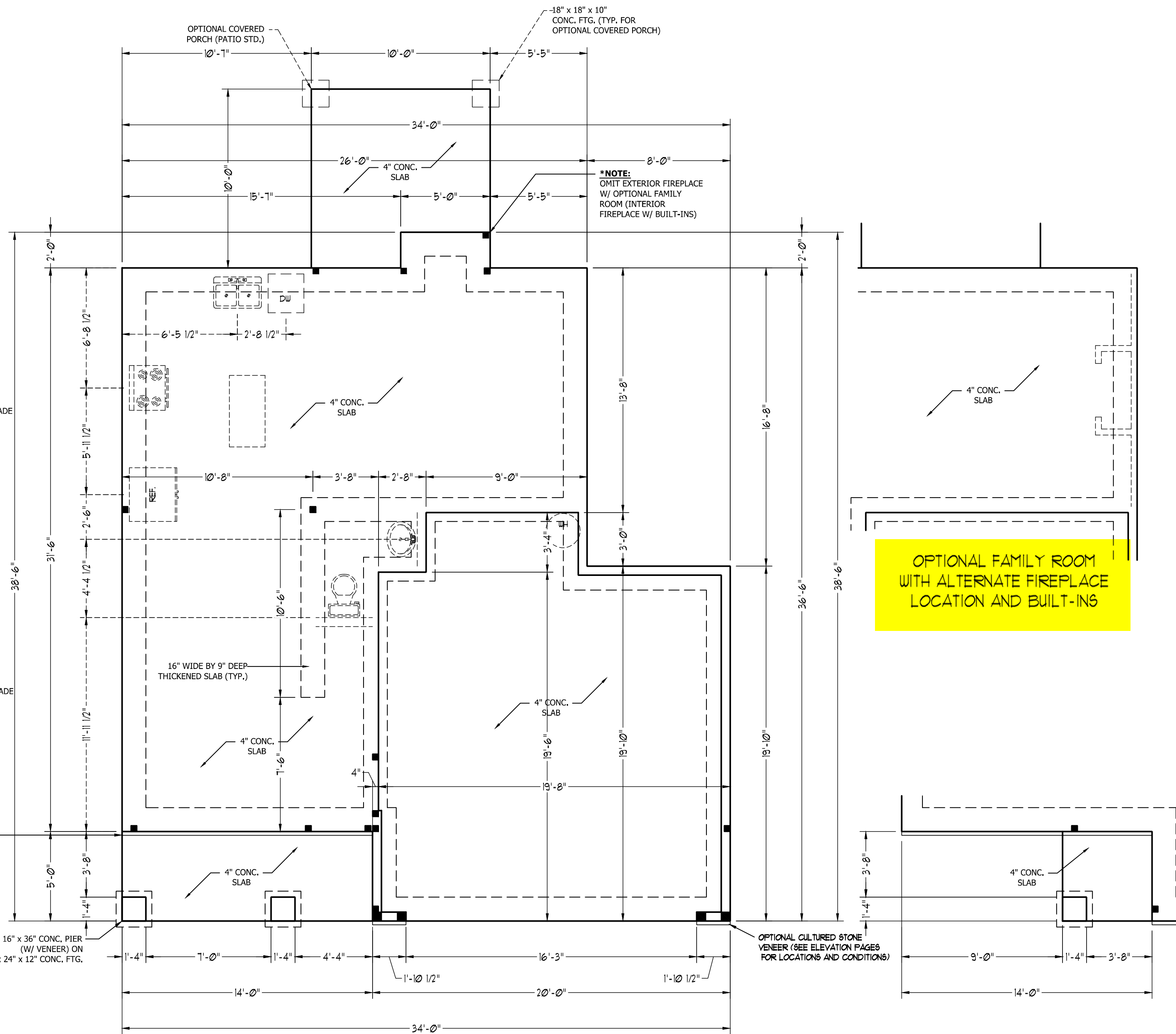
**TYPICAL SLAB DETAIL**



**GARAGE CURB DETAIL**

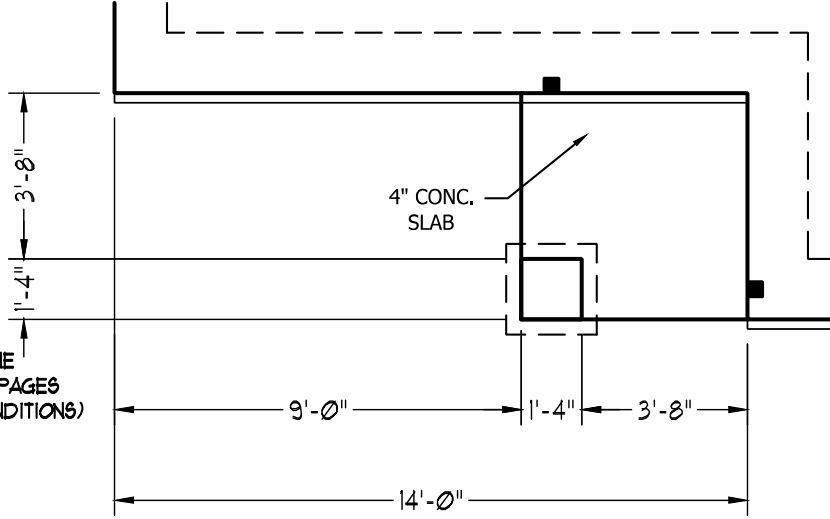


**THICKENED SLAB DETAIL**



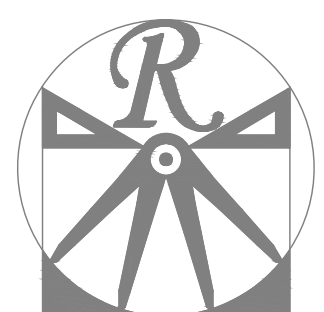
**\*NOTE:**  
OMIT EXTERIOR FIREPLACE W/ OPTIONAL FAMILY ROOM (INTERIOR FIREPLACE W/ BUILT-INS)

**OPTIONAL FAMILY ROOM WITH ALTERNATE FIREPLACE LOCATION AND BUILT-INS**



**FRONT PORCH ELEVATION B**

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DATE: FEBRUARY 19, 2021  
REV.:  
SCALE: 1/4" = 1'-0"  
DRAWN BY: WG  
ENGINEERED BY:  
REVIEWED BY:

MONO SLAB  
FOUNDATION  
PLAN

**S-1**



MASONRY STEM WALL SPECIFICATIONS				
WALL HEIGHT (FEET)	MASONRY WALL TYPE			
	8" CMU	4" BRICK AND 4" CMU	4" BRICK AND 8" CMU	12" CMU
2' OR LESS	UNGROUTED	GROUT SOLID	UNGROUTED	UNGROUTED
3'	UNGROUTED	GROUT SOLID	UNGROUTED	UNGROUTED
4'	GROUT SOLID	GROUT SOLID w/ #4 REBAR @ 48" O.C.	GROUT SOLID	GROUT SOLID w/ #4 REBAR @ 64" O.C.
5'	GROUT SOLID w/ #4 REBAR @ 36" O.C.	N/A	GROUT SOLID w/ #4 REBAR @ 36" O.C.	GROUT SOLID w/ #4 REBAR @ 64" O.C.
6'	GROUT SOLID w/ #4 REBAR @ 24" O.C.	N/A	GROUT SOLID w/ #4 REBAR @ 24" O.C.	GROUT SOLID w/ #4 REBAR @ 64" O.C.
7' OR MORE	ENGINEERED BASED ON SITE CONDITIONS			

**STRUCTURAL NOTES:**

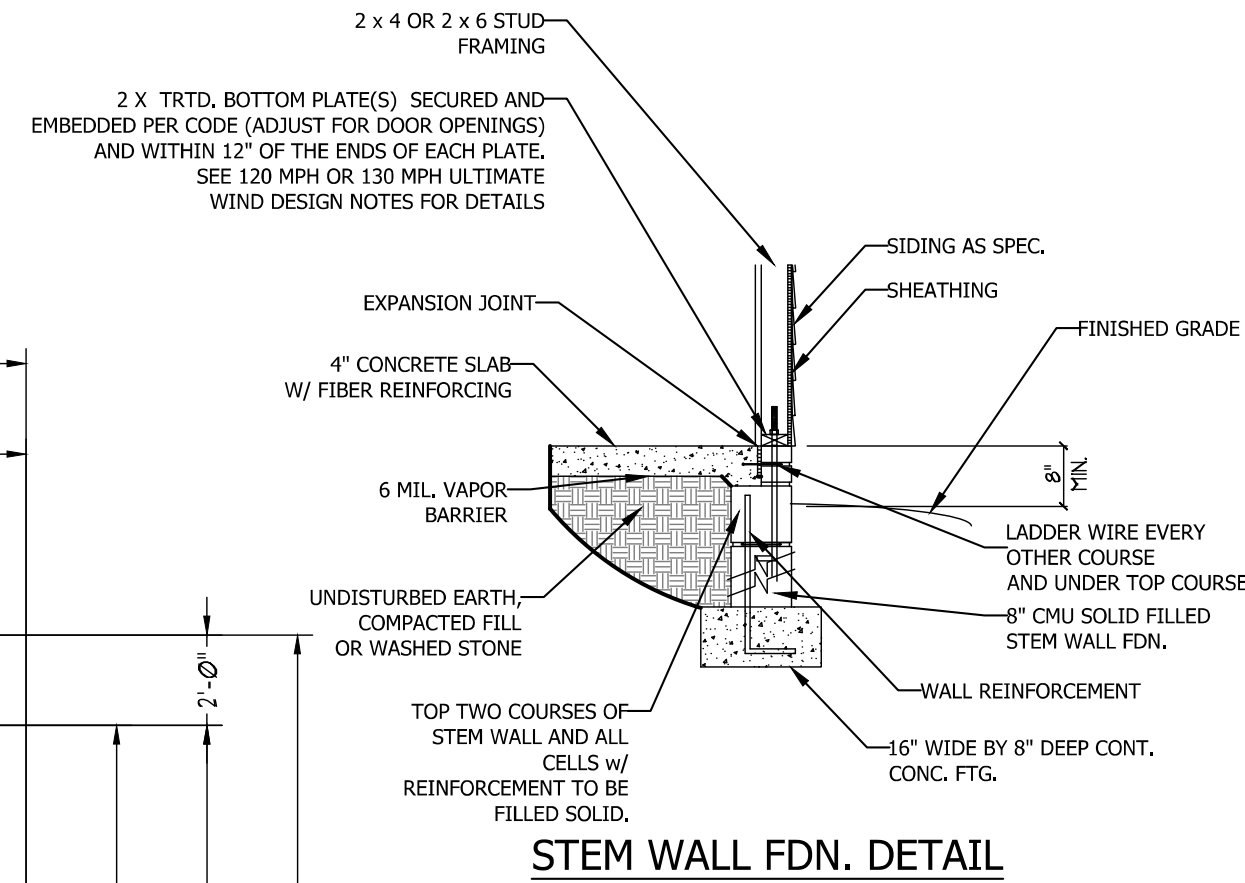
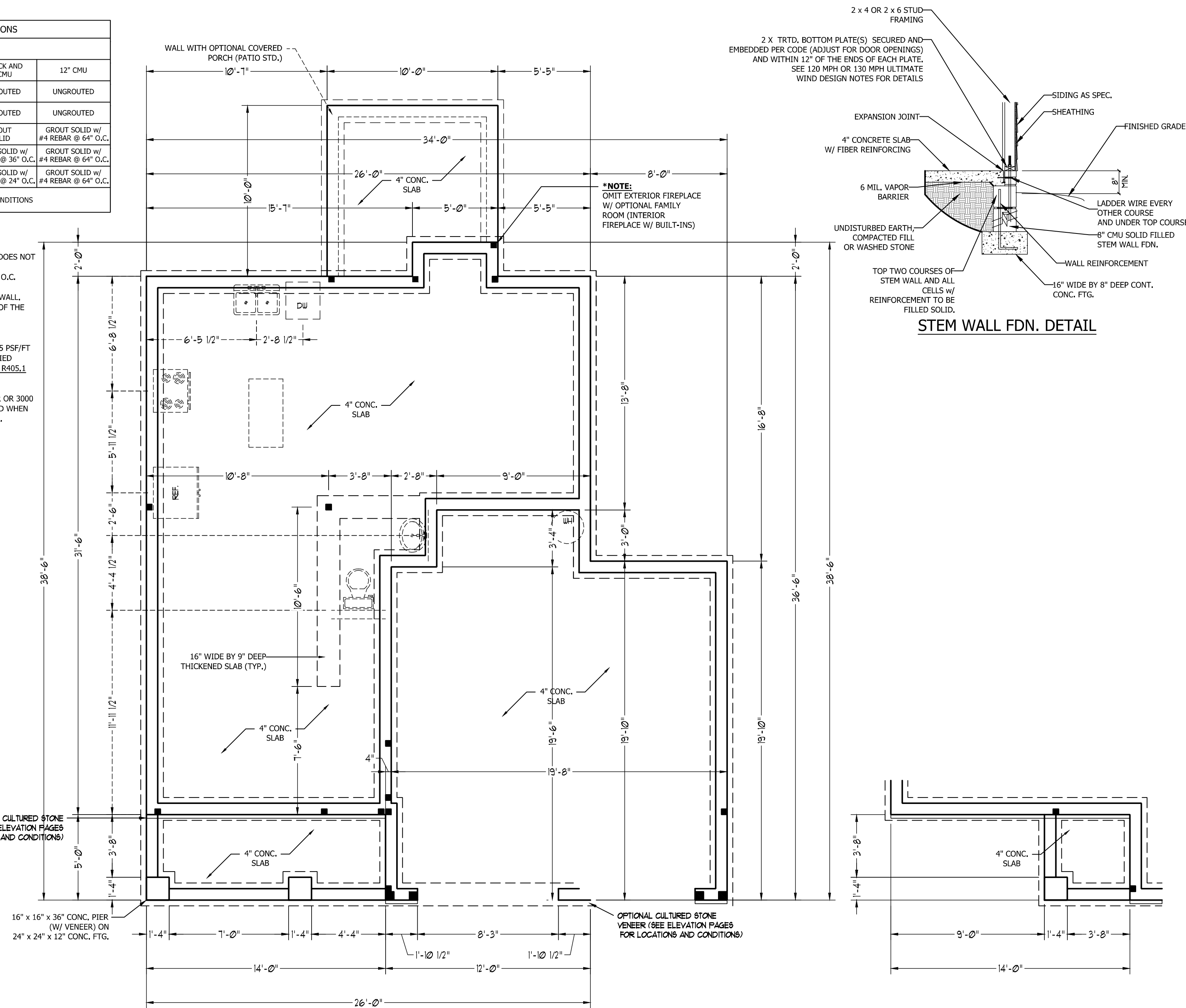
- TABLE ABOVE APPLIES TO HOUSE FOUNDATION ONLY. TABLE DOES NOT APPLY TO GARAGE FOUNDATION NOT COMMON TO HOUSE.
- TIE MULTIPLE WYTHES TOGETHER WITH LADDER WIRE @ 16" O.C. VERTICALLY.
- WALL HGT. IS MEASURED FROM TOP OF FOOTING TO TOP OF WALL.
- PREP SLAB PER R506.2.1 AND R505.2.2 BASE AND EXCEPTION OF THE 2018 NCRC
- MINIMUM 24" LAP SPlice LENGTH.
- BACKFILL OF CLEAN #57/ #67 WASHED STONE IS PERMITTED.
- BACKFILL OF WELL DRAINED SAND-GRAVEL MIXTURE SOILS (45 PSF/FT BELOW GRADE) CLASSIFIED AS GROUP 1 ACCORDING TO UNIFIED SOILS CLASSIFICATION SYSTEM IN ACCORDANCE WITH TABLE R405.1 OF THE 2018 NCRC ARE ALLOWABLE.
- LOCATE REBAR IN CENTER OF FOUNDATION WALL.
- WHERE REQUIRED, FILL BLOCK SOLID WITH TYPE "S" MORTAR OR 3000 PSI GROUT. USE OF "LOW LIFT GROUTING" METHOD REQUIRED WHEN FILLING WALLS WITH GROUT AT HEIGHTS OF 5' AND GREATER.

**ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:**

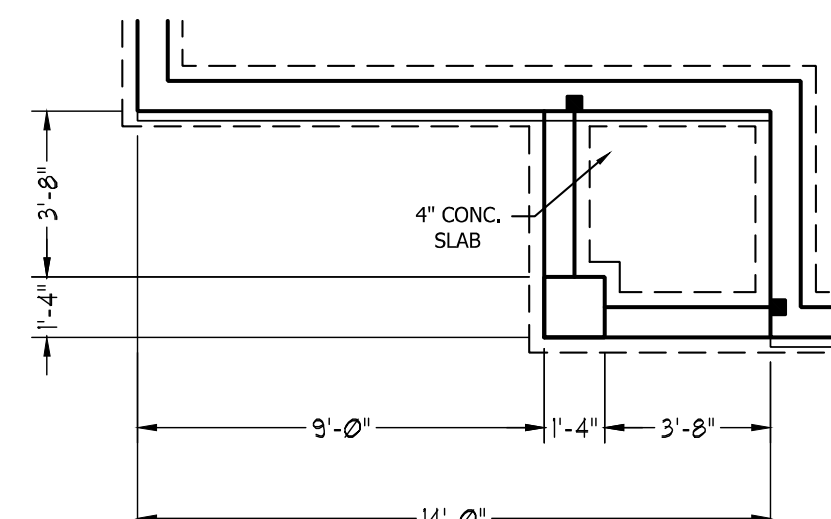
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ANCHOR SPACING AND EMBEDMENT		
WIND ZONE	120 MPH	130 MPH
SPACING	6'-0" O.C.	4'-0" O.C.
EMBEDMENT	7"	7" INTO CONCRETE 15" INTO MASONRY

NOTE: HORIZONTAL FOOTING REBAR REQUIRED IN HIGH WIND ZONES ONLY (140-150 MPH)

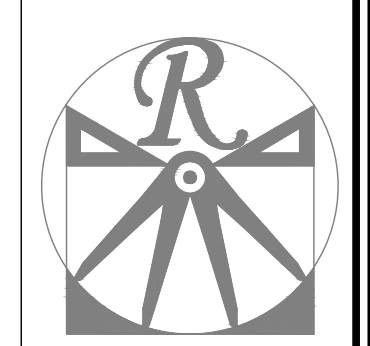


**STEM WALL FDN. DETAIL**



**FRONT PORCH ELEVATION B**

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.  
11x17 PRINTS ARE NOT TO SCALE



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**WEAVER HOMES**  
**CAROLINA COLLECTION**  
**MAGNOLIA-II**

DATE: FEBRUARY 19, 2021  
REV.:  
SCALE: 1/4" = 1'-0"  
DRAWN BY: WG  
ENGINEERED BY:  
REVIEWED BY:

STEMWALL SLAB  
FOUNDATION  
PLAN

**S-1**

**STRUCTURAL NOTES:**

1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO).
2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
3. INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
4. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.).
6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.).
7. FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

**BRACE WALL PANEL NOTES:**

**EXTERIOR WALLS:** ALL EXTERIOR WALLS TO BE SHEATHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

**REQUIRED LENGTH OF BRACING:** REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THEIR ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

**GYPSON:** ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSON INSTALLED. WHEN NOT USING METHOD GB GYPSON TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

**HD:** 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

**METHODS:** PER TABLE R602.10.1

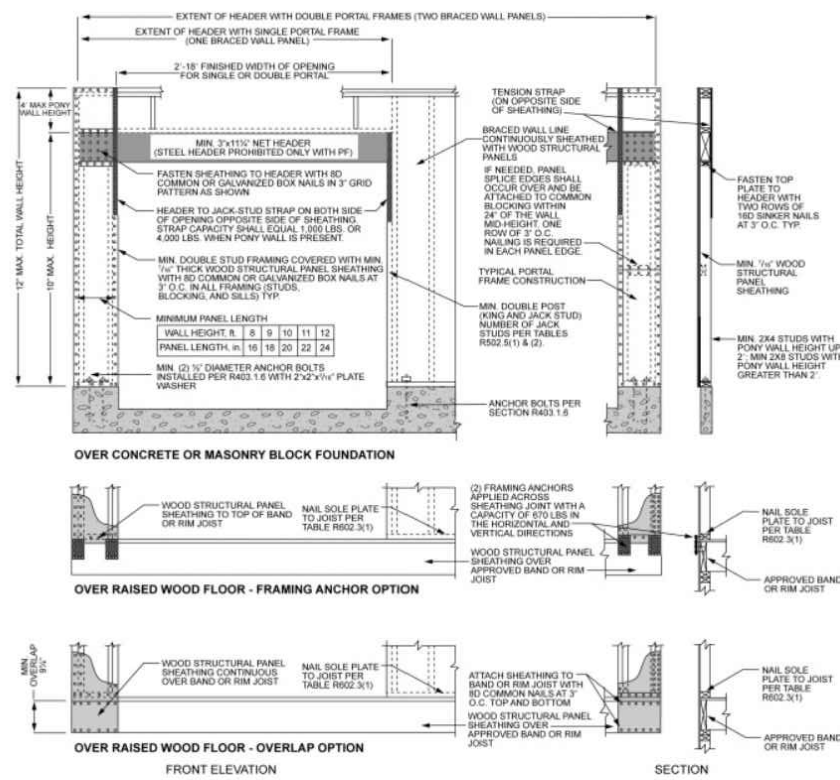
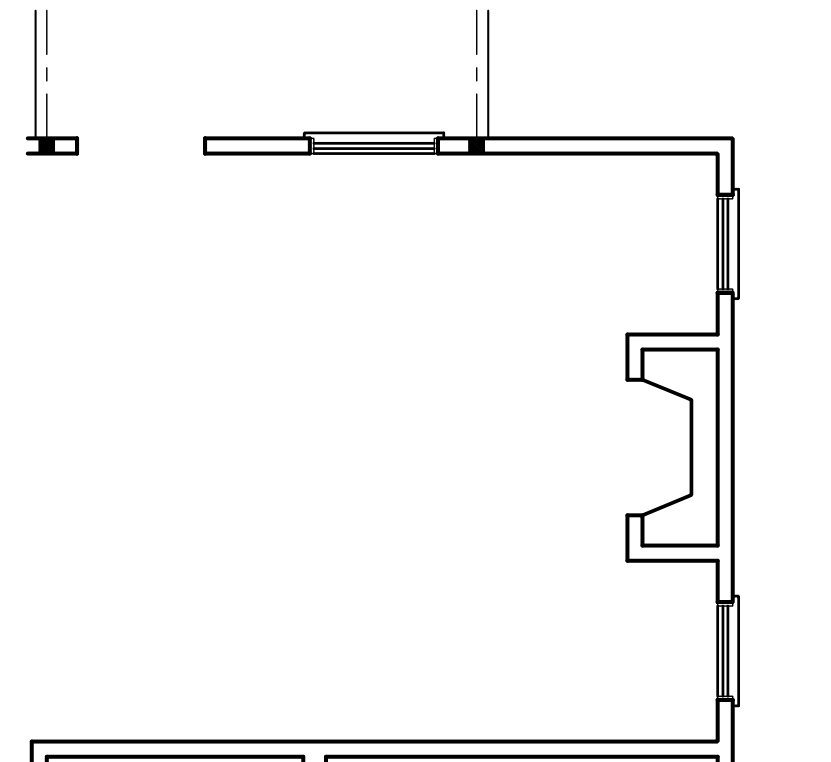
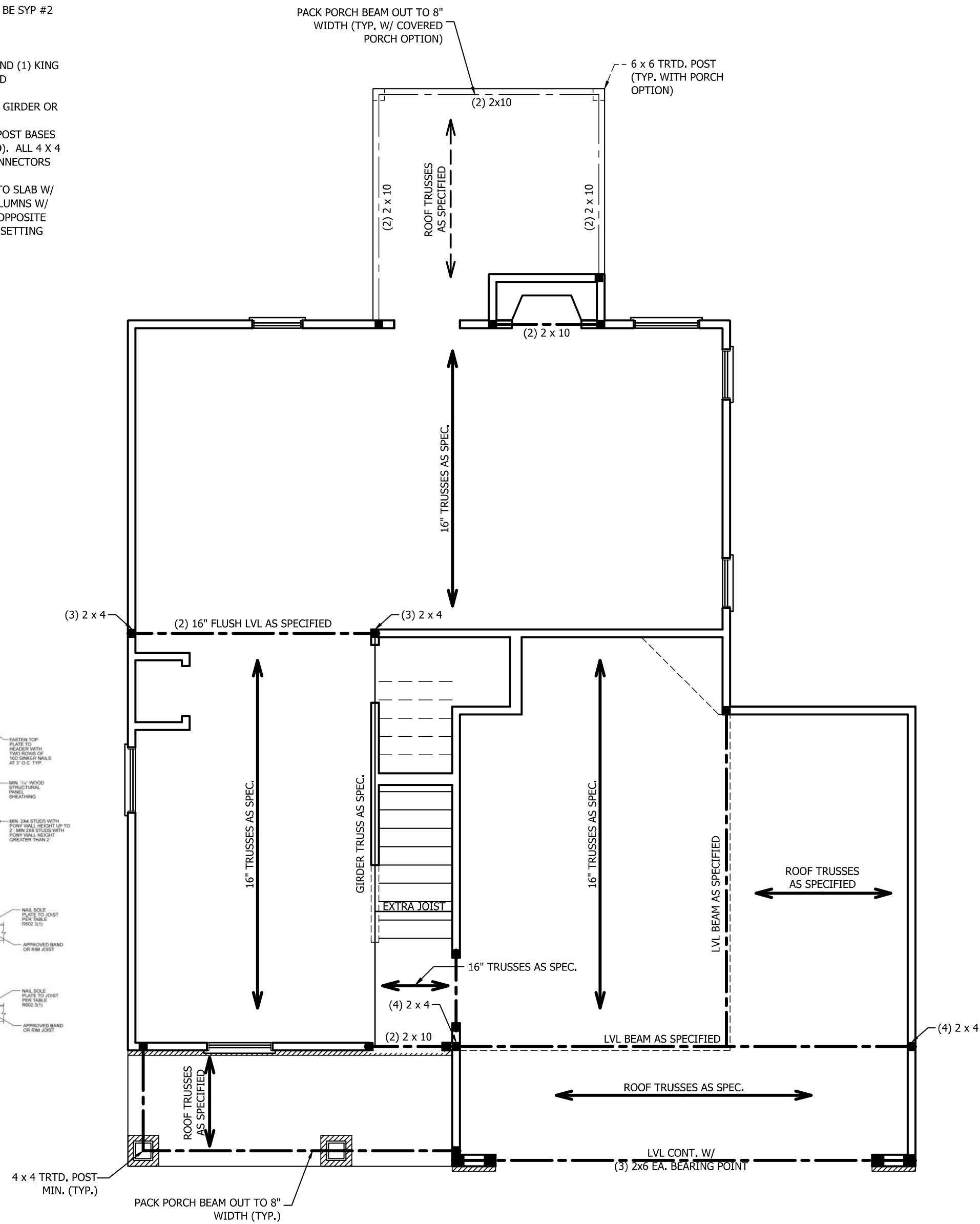
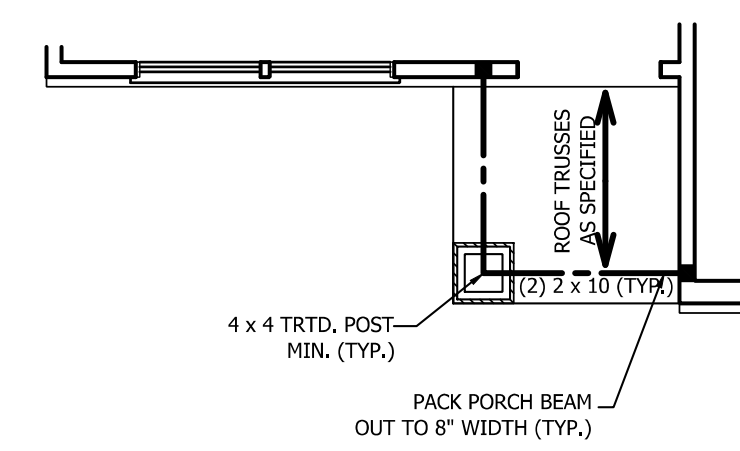


FIGURE R602.10.1  
METHOD PF--PORTAL FRAME CONSTRUCTION

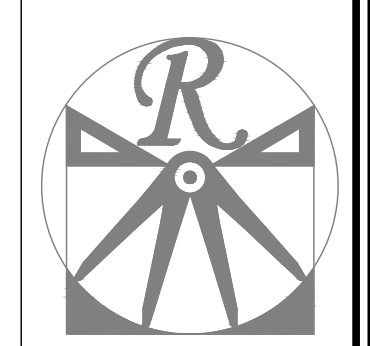


OPTIONAL FAMILY ROOM  
WITH ALTERNATE FIREPLACE  
LOCATION AND BUILT-INS



FRONT PORCH AND DINING  
ROOM WINDOW ELEVATION B

SCALE NOTE: 18x24 PRINTS ARE  
TO SCALE AS NOTED.  
11x17 PRINTS ARE NOT TO SCALE



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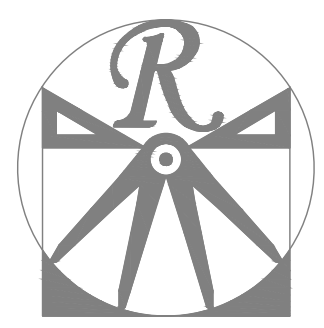
DATE: FEBRUARY 19, 2021  
REV.:  
SCALE: 1/4" = 1'-0"  
DRAWN BY: WG  
ENGINEERED BY:  
REVIEWED BY:

SECOND FLOOR  
FRAMING PLAN  
S-2



TABLE R602.7.5  
MINIMUM NUMBER OF FULL HEIGHT STUDS  
AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHES) (PER TABLE R602.3(5))	
	16	24
UP TO 3'	1	1
4'	2	1
8'	3	2
12'	5	3
16'	6	4

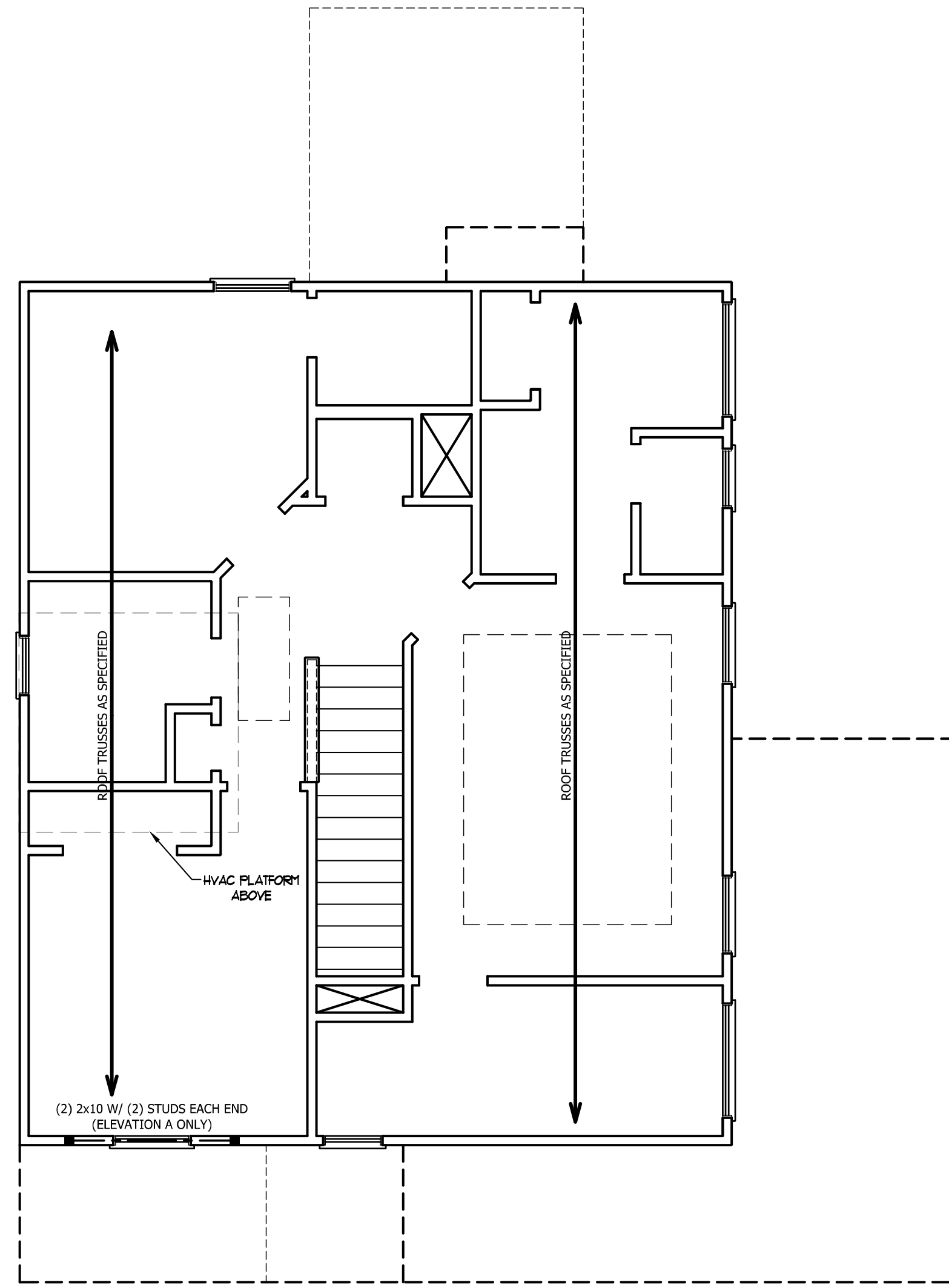


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**STRUCTURAL NOTES:**

1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO).
2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET  
TSP - TRIPLE STUD POCKET



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MAGNOLIA-II

DATE: FEBRUARY 19, 2021  
REV.:  
SCALE: 1/4" = 1'-0"  
DRAWN BY: WG  
ENGINEERED BY:  
REVIEWED BY:

ATTIC FLOOR  
FRAMING PLAN

S-3

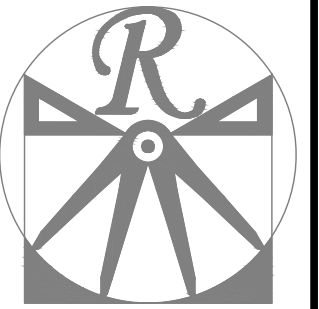
SCALE NOTE: 18x24 PRINTS ARE  
TO SCALE AS NOTED.  
**11x17 PRINTS ARE NOT TO SCALE**

**ATTIC VENT CALCULATION:**  
 100 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 0.7 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

**ATTIC VENT CALCULATION:**  
 1250 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 8.3 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

**STRUCTURAL NOTES:**

1. ALL FRAMING LUMBER TO BE #2 SPF (UNO).
2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
5. REFER TO SECTION R802.11 OF THE 2018 NCRRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



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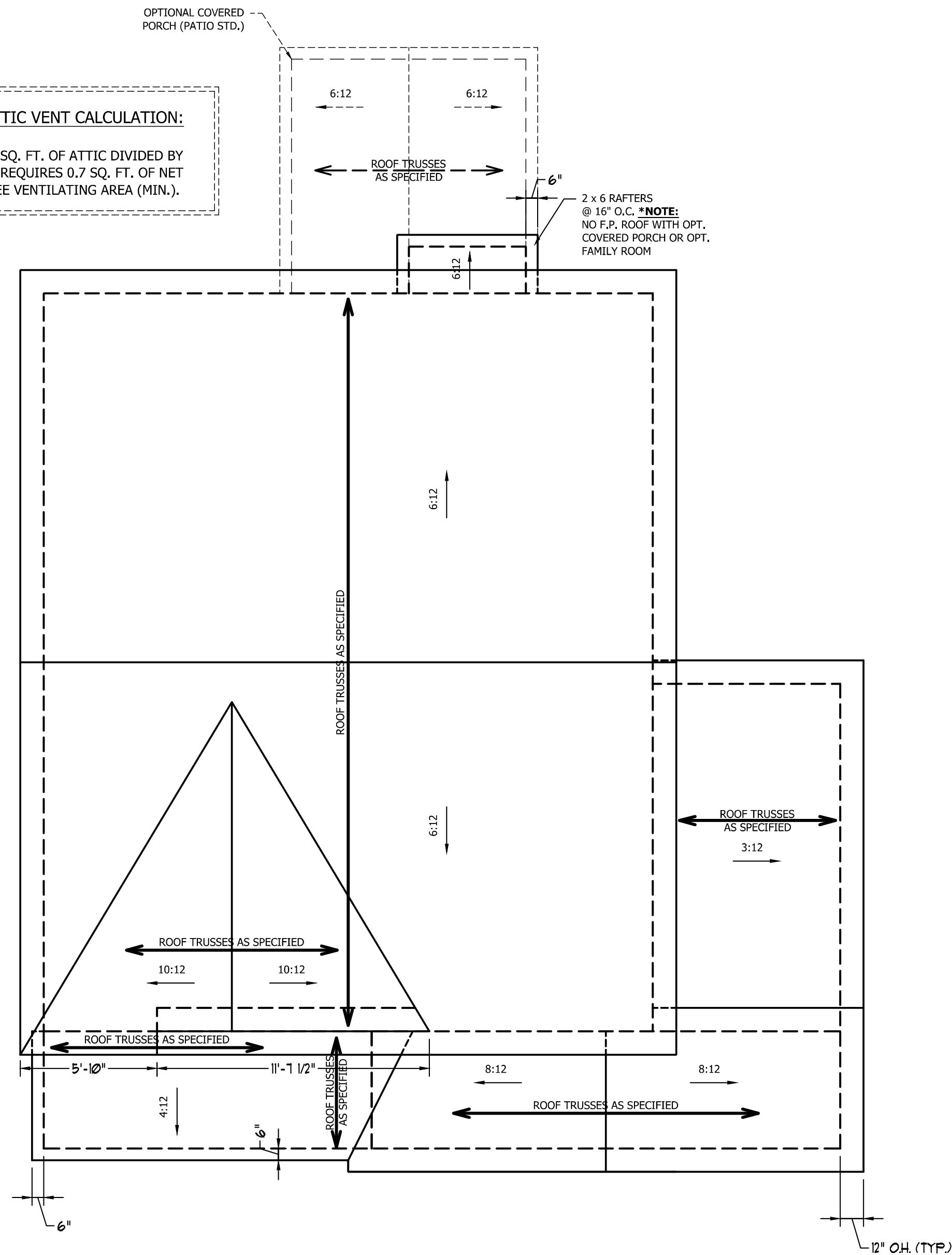
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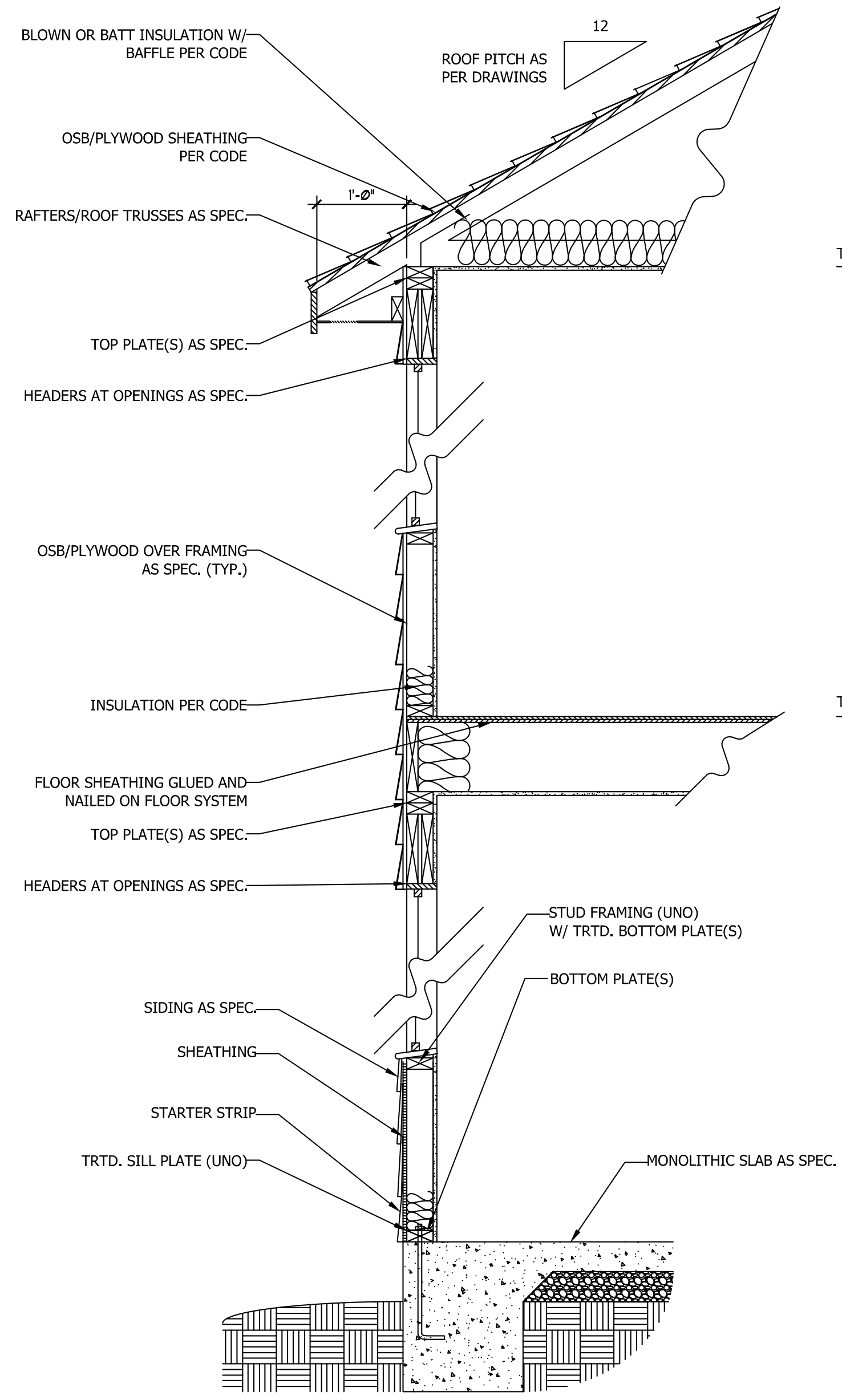
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DATE: FEBRUARY 19, 2021  
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 SCALE: 1/4" = 1'-0"  
 DRAWN BY: WG  
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 REVIEWED BY:

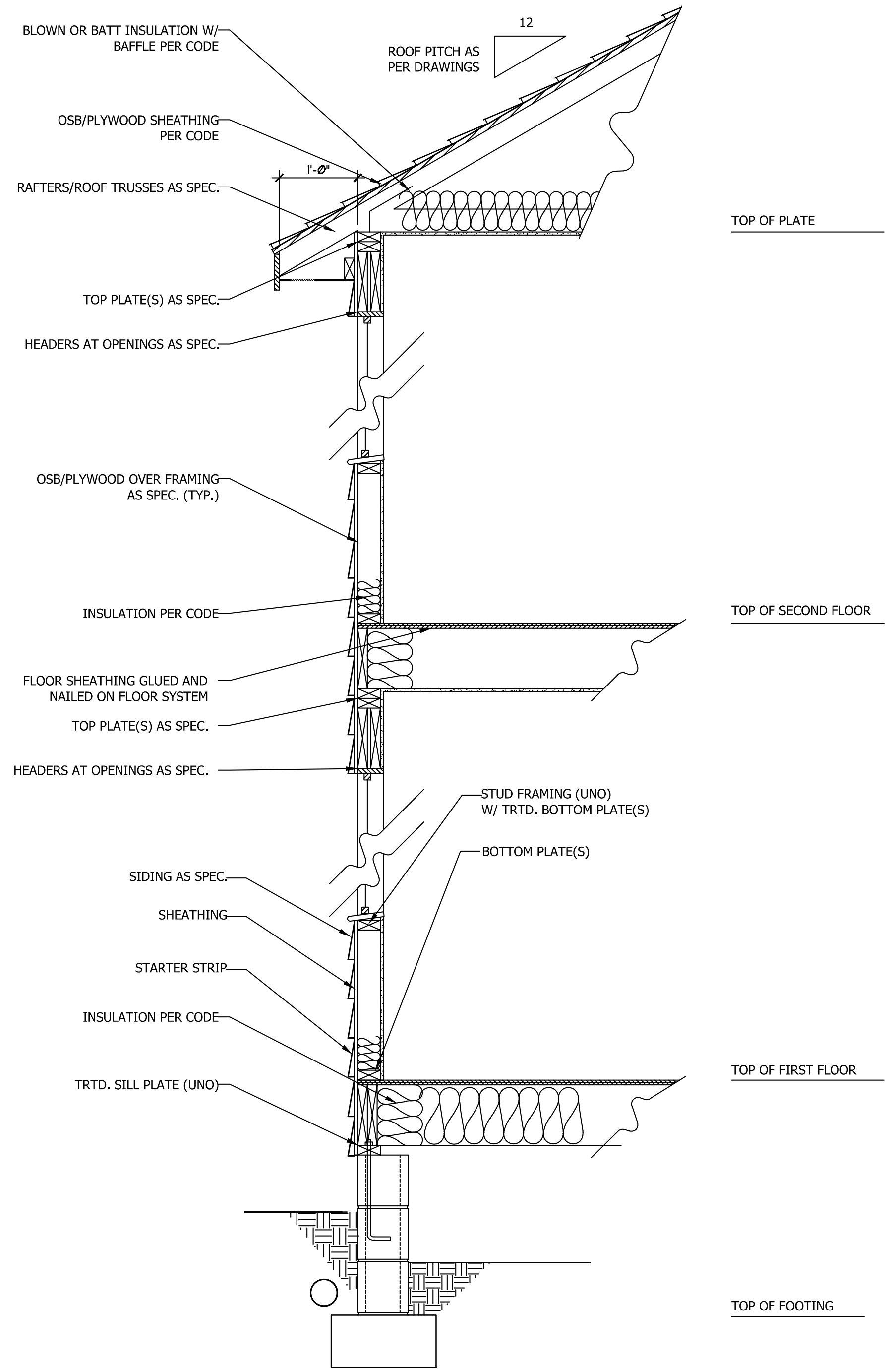
ROOF PLAN  
 ELEVATION - C  
**S-4**



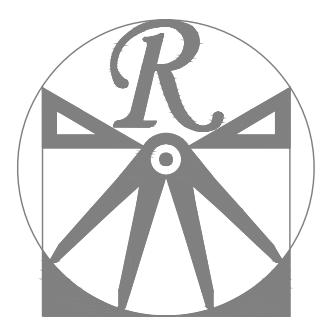
**ELEVATION C**



**WALL SECTION W/ SLAB  
W/ STD. SIDING SHOWN (NTS)**



**WALL SECTION W/ CRAWL SPACE  
W/ STD. SIDING SHOWN (NTS)**



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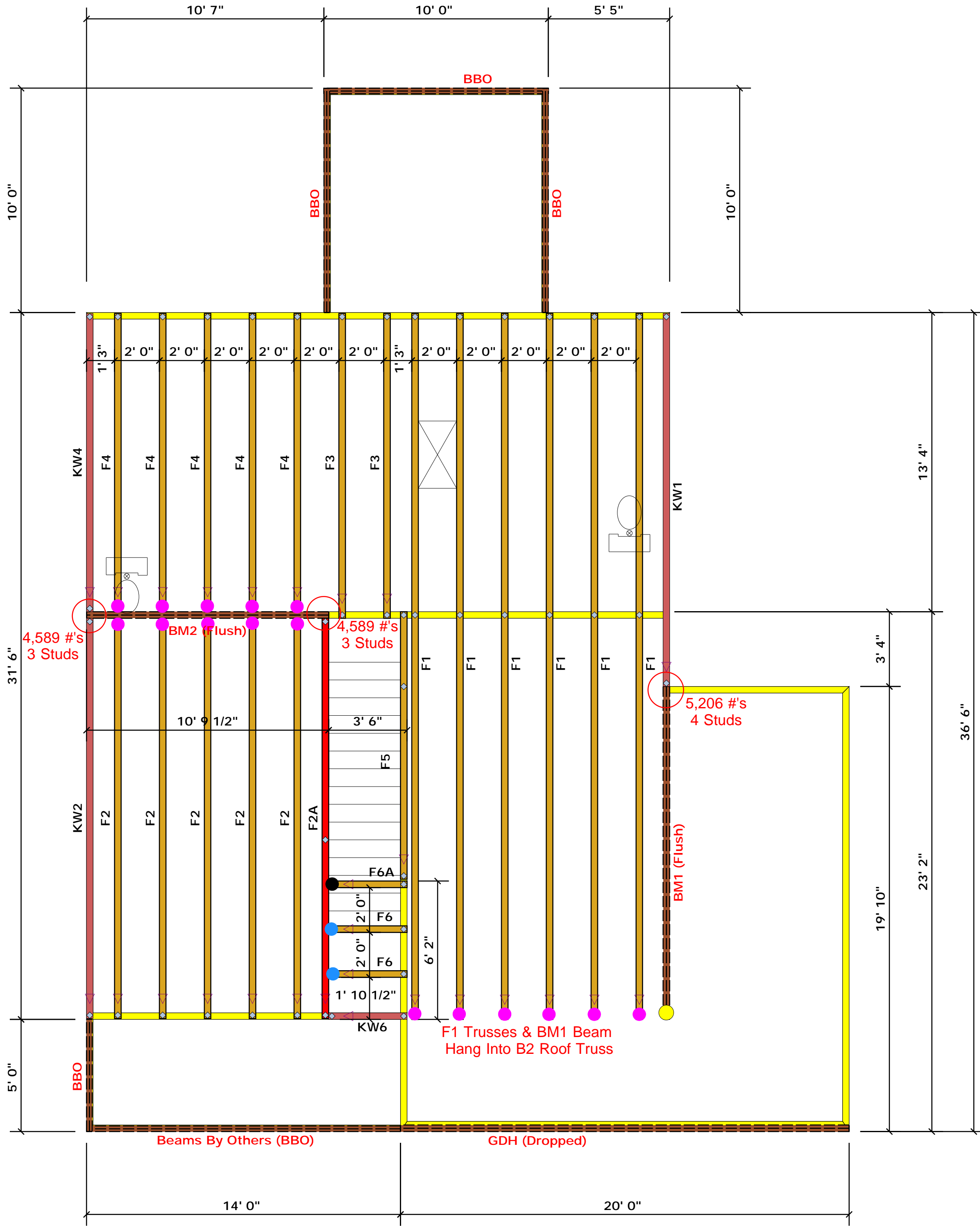
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**CAROLINA COLLECTION**  
**MAGNOLIA-II**

DATE: FEBRUARY 19, 2021  
REV.:  
SCALE: 1/4" = 1'-0"  
DRAWN BY: WG  
ENGINEERED BY:  
REVIEWED BY:

TYPICAL WALL SECTIONS

D-1





### Truss Placement Plan SCALE: NTS

- = HUS410 (Qty. 16)
- = MSH422 (Qty. 2)
- = MSH422IF (Qty. 1)
- = THD410 (Qty. 1)

▲ = Denotes Left End of Truss  
(Reference Engineered Truss Drawing)

*All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.*

○ -- Denotes Reaction Greater than 3,000 lbs.  
Reaction / # of Studs

Products				
PlotID	Length	Product	Plies	Net Qty
GDH (Dropped)	20' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
BM1 (Flush)	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2 (Flush)	11' 0"	1-3/4"x 16" LVL Kerto-S	2	2

LOAD CHART FOR JACK STUDS

REACTION (LBS)	SPACING (IN)	NO. OF STUDS
1700	1	2550
3400	2	5100
5100	3	7650
6800	4	10200
8500	5	12750
10200	6	15300
11900	7	
13600	8	
15300	9	

BUILDER	Weaver Development	CITY / CO.	Broadway / Harnett
JOB NAME	Lot 1 Ring-Rosser Pittman Rd.	ADDRESS	Lot 1 Ring-Rosser Pittman Rd.
PLAN	Magnolia-II "C"	MODEL	Floor
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Christine Shivy
JOB #	J0521-3377	SALES REP.	Lenny Norris

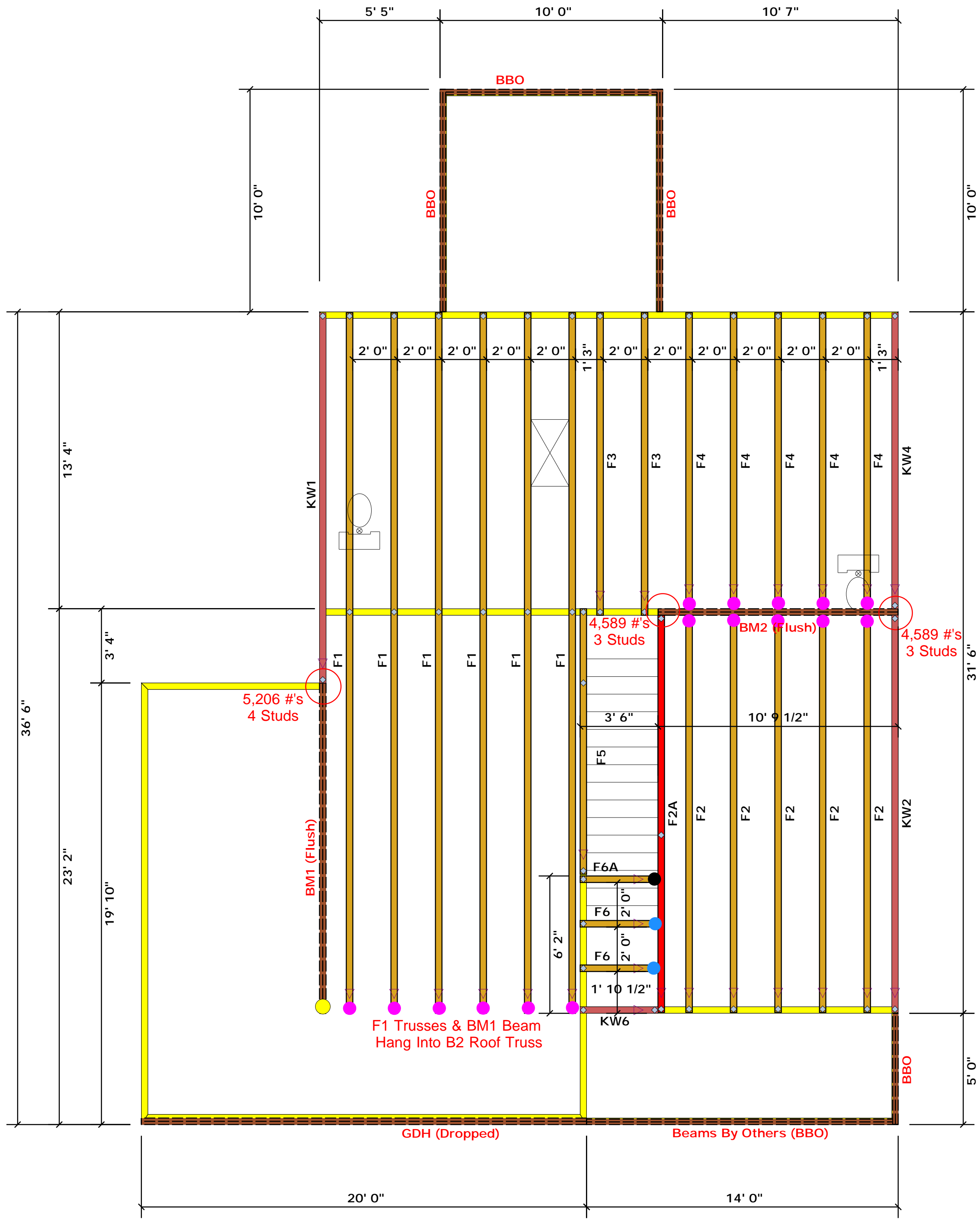
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSH-B1 and BCSH-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: Christine Shivy  
Christine Shivy

**ROOF & FLOOR TRUSSES & BEAMS**

Reilly Road Industrial Park  
Fayetteville, N.C. 28309  
Phone: (910) 864-8787  
Fax: (910) 864-4444



### Truss Placement Plan SCALE: NTS

▲ = Denotes Left End of Truss  
(Reference Engineered Truss Drawing)

*All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.*

○ -- Denotes Reaction Greater than 3,000 lbs.  
*Reaction / # of Studs*

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- = MSH422 (Qty. 2)
- = MSH422IF (Qty. 1)
- = THD410 (Qty. 1)

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PlotID	Length	Product	Plies	Net Qty
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BM1 (Flush)	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2 (Flush)	11' 0"	1-3/4"x 16" LVL Kerto-S	2	2

**LOAD CHART FOR JACK STUDS**

REACTION (LBS)	SPACING (IN)	NO. OF JACK STUDS
1700	1	2550
3400	2	5100
5100	3	7650
6800	4	10200
8500	5	12750
10200	6	15300
11900	7	
13600	8	
15300	9	

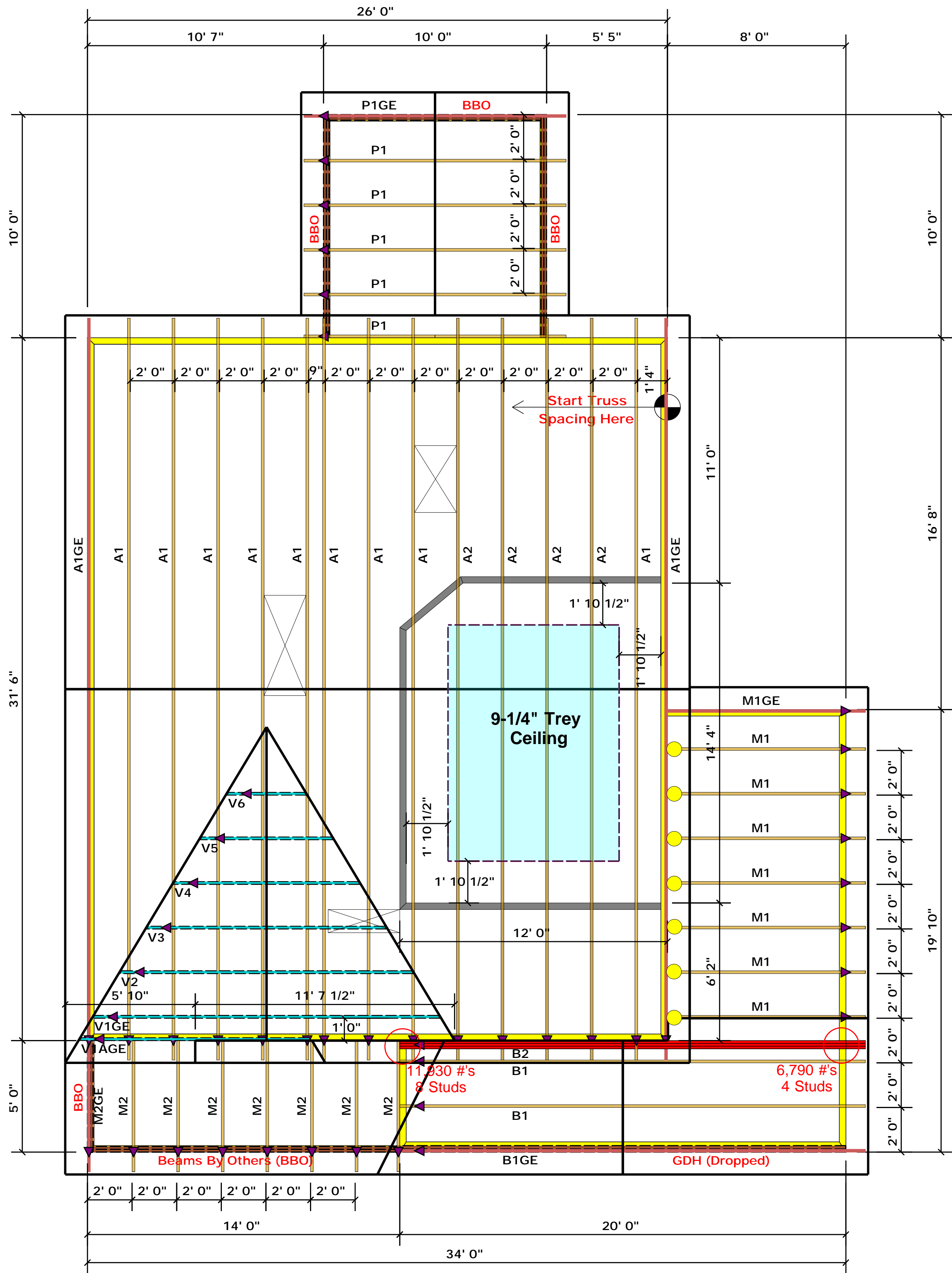
BUILDER	Weaver Development	CITY / CO.	Broadway / Harnett
JOB NAME	Lot 1 Ring-Rosser Pittman Rd.	ADDRESS	Lot 1 Ring-Rosser Pittman Rd.
PLAN	Magnolia-II "C"	MODEL	Floor
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Christine Shivy
JOB #	J0521-3377	SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSH-B1 and BCSH-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

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Christine Shivy

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Fayetteville, N.C. 28309  
Phone: (910) 864-8787  
Fax: (910) 864-4444



● = JUS24 (Qty. 7)

▲ = Denotes Left End of Truss  
(Reference Engineered Truss Drawing)

### Truss Placement Plan SCALE: NTS

○ -- Denotes Reaction Greater than 3,000 lbs.  
Reaction / # of Studs

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

LOAD CHART FOR JACK STUDS			
BASED ON THE FOLLOWING ASSUMPTIONS:			
1. JACK STUDS ARE SPACED AT 24" ON CENTER			
2. JACK STUDS ARE SUPPORTED BY A CONCRETE SLAB			
3. JACK STUDS ARE SUPPORTED BY A CONCRETE SLAB			
4. JACK STUDS ARE SUPPORTED BY A CONCRETE SLAB			
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		

BUILDER	Weaver Development	CITY / CO.	Broadway / Harnett
JOB NAME	Lot 1 Ring-Rosser Pittman Rd.	ADDRESS	Lot 1 Ring-Rosser Pittman Rd.
PLAN	Magnolia-II "C"	MODEL	Roof
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Christine Shivy
JOB #	J0521-3376	SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

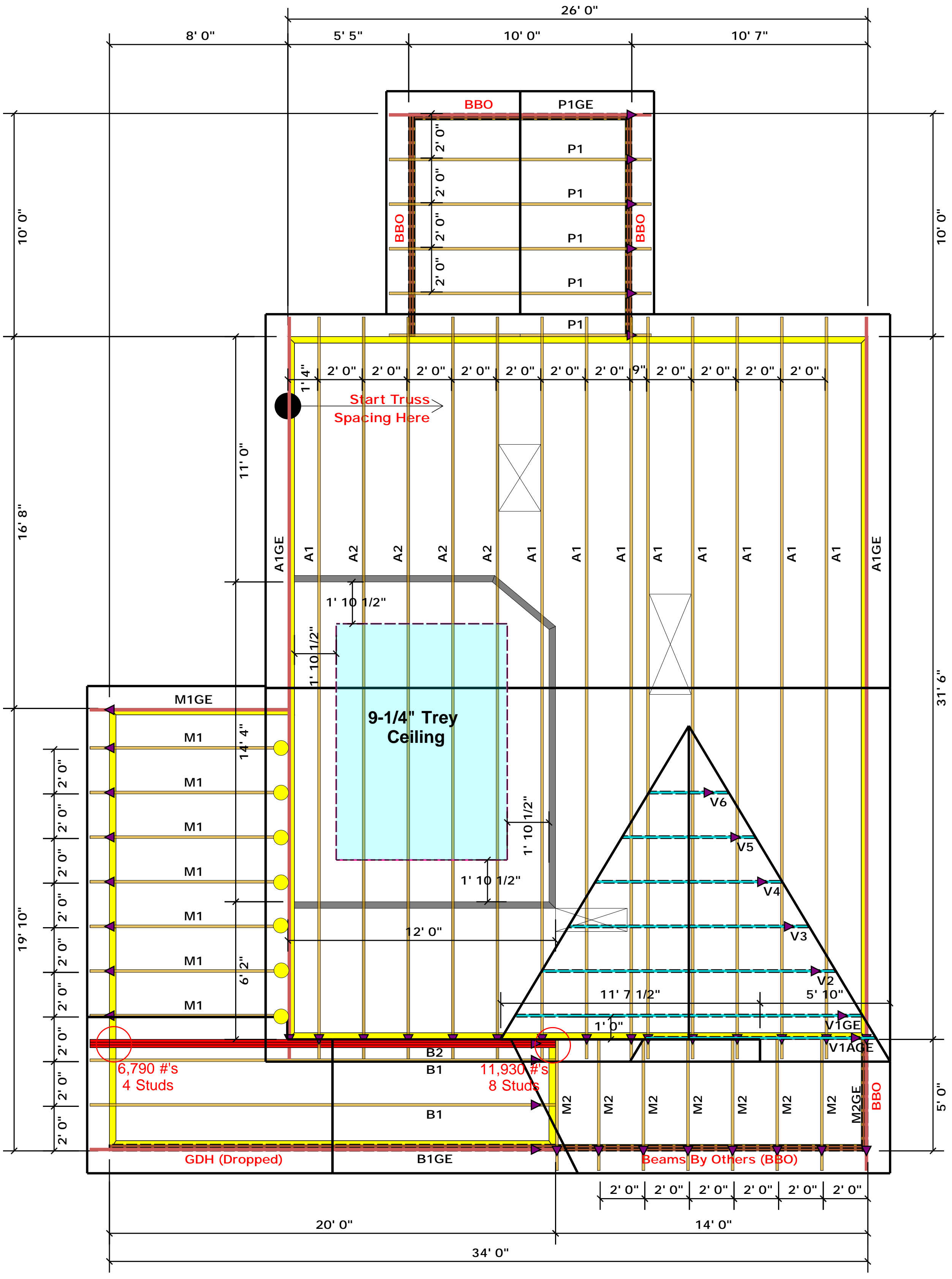
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: Christine Shivy  
Christine Shivy

**ROOF & FLOOR TRUSSES & BEAMS**

Reilly Road Industrial Park  
Fayetteville, N.C. 28309  
Phone: (910) 864-8787  
Fax: (910) 864-4444





● = JUS24 (Qty. 7)

▲ = Denotes Left End of Truss  
(Reference Engineered Truss Drawing)

### Truss Placement Plan SCALE: NTS

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.  
○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

LOAD CHART FOR JACK STUDS		
MEMBER SIZE (IN)	SPACING (IN)	MAXIMUM LOAD (LBS)
1700	1	2550
1700	2	5100
1700	3	7650
1700	4	10200
1700	5	12750
1700	6	15300
1700	7	17850
1700	8	20400
1700	9	22950

BUILDER	Weaver Development
JOB NAME	Lot 1 Ring-Rosser Pittman Rd.
PLAN	Magnolia-II "C"
SEAL DATE	Seal Date
QUOTE #	Quote #
JOB #	J0521-3376

CITY / CO.	Broadway / Harnett
ADDRESS	Lot 1 Ring-Rosser Pittman Rd.
MODEL	Roof
DATE REV.	//
DRAWN BY	Christine Shivy
SALES REP.	Lenny Norris

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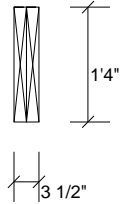
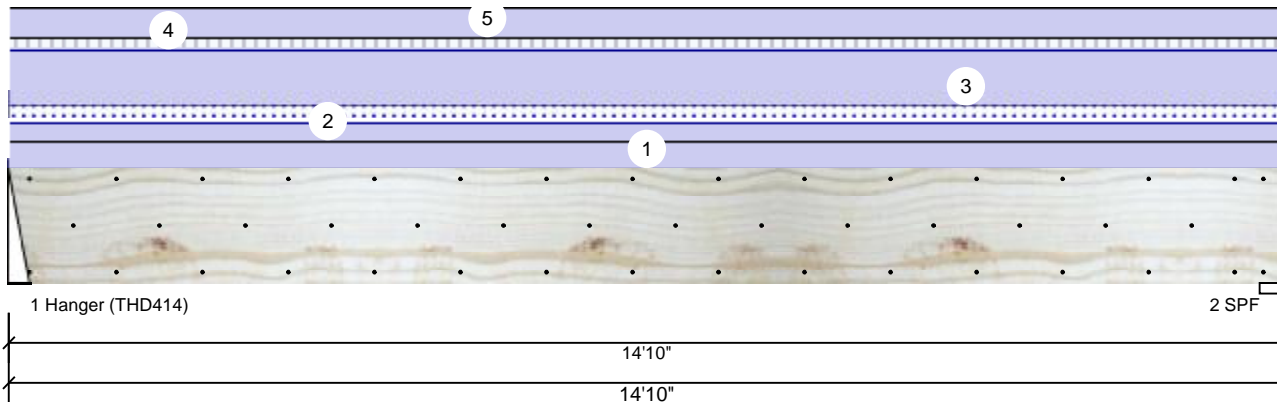


Client: Weaver Homes  
 Project: Magnolia II "C"  
 Address: Magnolia II "C"

Date: 5/5/2021  
 Input by: Christine Shivy  
 Job Name: Magnolia II "C"  
 Project #:

**BM1 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED**

Level: Level



**Member Information**

Type: Girder	Application: Floor
Plies: 2	Design Method: ASD
Moisture Condition: Dry	Building Code: IBC/IRC 2015
Deflection LL: 480	Load Sharing: No
Deflection TL: 360	Deck: Not Checked
Importance: Normal	
Temperature: Temp <= 100°F	

**Reactions UNPATTERNED Ib (Uplift)**

Brg	Live	Dead	Snow	Wind	Const
1	296	4522	577	0	0
2	298	4548	580	0	0

**Bearings**

Bearing	Length	Cap. React D/L Ib	Total Ld. Case	Ld. Comb.
1 - Hanger	3.000"	57%	4522 / 655	5177 L D+0.75(L+S)
2 - SPF	3.500"	100%	4548 / 658	5206 L D+0.75(L+S)

**Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	15931 ft-lb	7'4 3/4"	31109 ft-lb	0.512 (51%)	D	Uniform
Unbraced	18237 ft-lb	7'4 3/4"	18282 ft-lb	0.998 (100%)	D+0.75(L+S) L	
Shear	3723 lb	1'6 1/8"	10752 lb	0.346 (35%)	D	Uniform
LL Defl inch	0.041 (L/4231)	7'4 13/16"	0.361 (L/480)	0.110 (11%)	0.75(L+S) L	
TL Defl inch	0.324 (L/535)	7'4 13/16"	0.481 (L/360)	0.670 (67%)	D+0.75(L+S) L	

**Design Notes**

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 6'4 7/8" o.c.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Wall
2	Uniform			Near Face	78 PLF	0 PLF	78 PLF	0 PLF	0 PLF	M1
3	Uniform			Top	251 PLF	0 PLF	0 PLF	0 PLF	0 PLF	A1GE
4	Uniform			Far Face	15 PLF	40 PLF	0 PLF	0 PLF	0 PLF	Floor Load
5	Uniform			Top	130 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Load
	Self Weight				12 PLF					

**Notes**

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

**Manufacturer Info**

Metsä Wood  
 301 Merritt 7 Building, 2nd Floor  
 Norwalk, CT 06851  
 (800) 622-5850  
[www.metsawood.com/us](http://www.metsawood.com/us)  
 ICC-ES: ESR-3633

Comtech, Inc.  
 1001 S. Reilly Road, Suite #639  
 Fayetteville, NC  
 USA  
 28314  
 910-864-TRUS



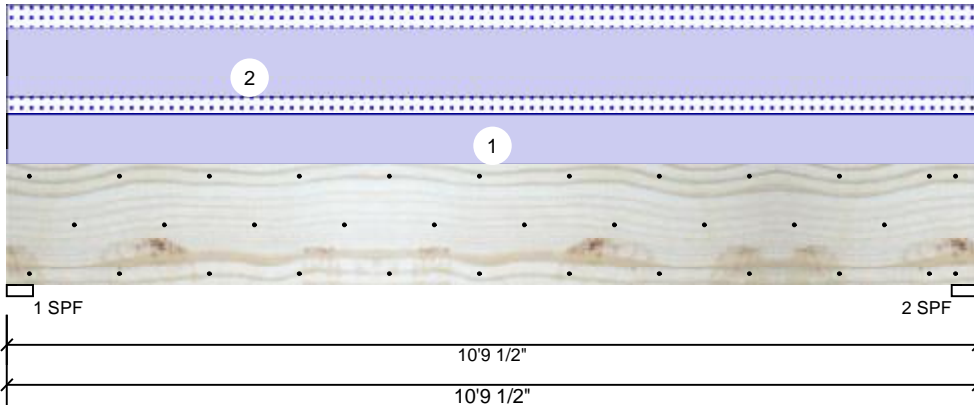


Client: Weaver Homes  
 Project: Magnolia II "C"  
 Address: Magnolia II "C"

Date: 5/5/2021  
 Input by: Christine Shivy  
 Job Name: Magnolia II "C"  
 Project #:

**BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED**

Level: Level



**Member Information**

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

**Reactions UNPATTERNED Ib (Uplift)**

Brg	Live	Dead	Snow	Wind	Const
1	0	3456	1133	0	0
2	0	3456	1133	0	0

**Bearings**

Bearing	Length	Cap.	React D/L	Ib	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	88%	3456 / 1133	4589	L	D+S	
2 - SPF	3.500"	88%	3456 / 1133	4589	L	D+S	

**Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	11397 ft-lb	5'4 3/4"	39750 ft-lb	0.287 (29%)	D+S	L
Unbraced	11397 ft-lb	5'4 3/4"	11799 ft-lb	0.966 (97%)	D+S	L
Shear	4386 lb	1'6 5/8"	13739 lb	0.319 (32%)	D+S	L
LL Defl inch	0.029 (L/4357)	5'4 3/4"	0.259 (L/480)	0.110 (11%)	S	L
TL Defl inch	0.115 (L/1076)	5'4 3/4"	0.345 (L/360)	0.330 (33%)	D+S	L

**Design Notes**

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	267 PLF	0 PLF	89 PLF	0 PLF	0 PLF	F4
2	Uniform			Near Face	361 PLF	0 PLF	121 PLF	0 PLF	0 PLF	F2
	Self Weight				12 PLF					

**Notes**

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**Lumber**

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

**Manufacturer Info**

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 301 Merritt 7 Building, 2nd Floor  
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[www.metsawood.com/us](http://www.metsawood.com/us)  
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 1001 S. Reilly Road, Suite #639  
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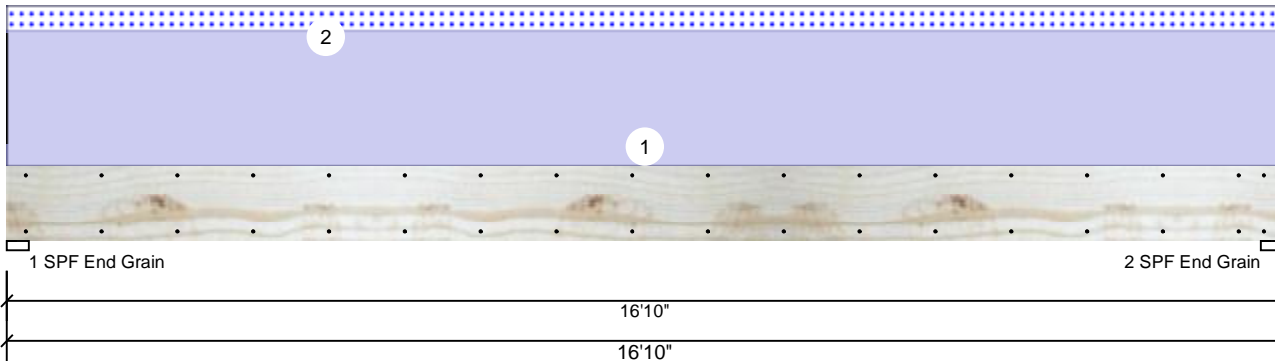


Client: Weaver Homes  
 Project: Magnolia II "C"  
 Address: Magnolia II "C"

Date: 5/5/2021  
 Input by: Christine Shivy  
 Job Name: Magnolia II "C"  
 Project #:

**GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED**

Level: Level



**Member Information**

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

**Reactions UNPATTERNED Ib (Uplift)**

Brg	Live	Dead	Snow	Wind	Const
1	0	1887	337	0	0
2	0	1887	337	0	0

**Bearings**

Bearing	Length	Cap. React	D/L Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	21%	1887 / 337	2224	L	D+S
2 - SPF End Grain	3.500"	21%	1887 / 337	2224	L	D+S

**Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7516 ft-lb	8'5"	17919 ft-lb	0.419 (42%)	D	Uniform
Unbraced	8857 ft-lb	8'5"	8875 ft-lb	0.998 (100%)	D+S	L
Shear	1614 lb	15'7 3/8"	7980 lb	0.202 (20%)	D	Uniform
LL Defl inch	0.070 (L/2809)	8'5 1/16"	0.409 (L/480)	0.170 (17%)	S	L
TL Defl inch	0.462 (L/425)	8'5 1/16"	0.546 (L/360)	0.850 (85%)	D+S	L

**Design Notes**

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 10'8 1/4" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	175 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Loads
2	Uniform			Top	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	2'-0" Gable End
	Self Weight				9 PLF					

**Notes**

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chemicals

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