

Trenco 818 Soundside Rd Edenton, NC 27932

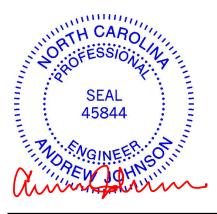
Re: 22-0799-A GARY ROBINSON-SUMMIT-LOT#1 ROOF

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Riverside Roof Truss.

Pages or sheets covered by this seal: I50154778 thru I50154813

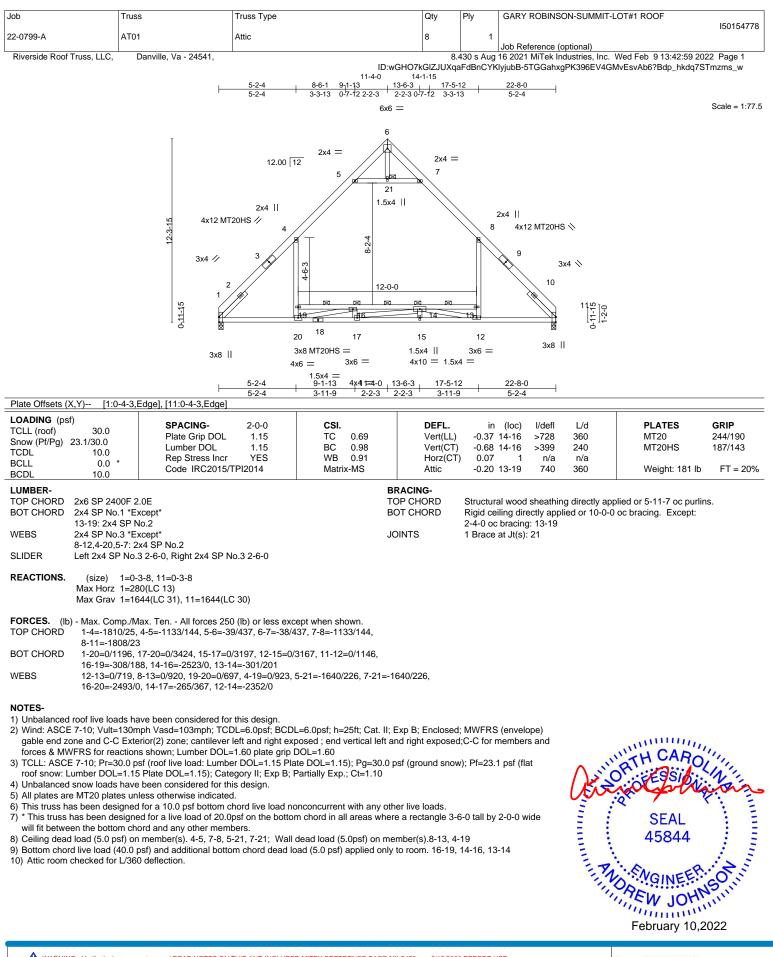
My license renewal date for the state of North Carolina is December 31, 2022.

North Carolina COA: C-0844



February 10,2022

Johnson, Andrew **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

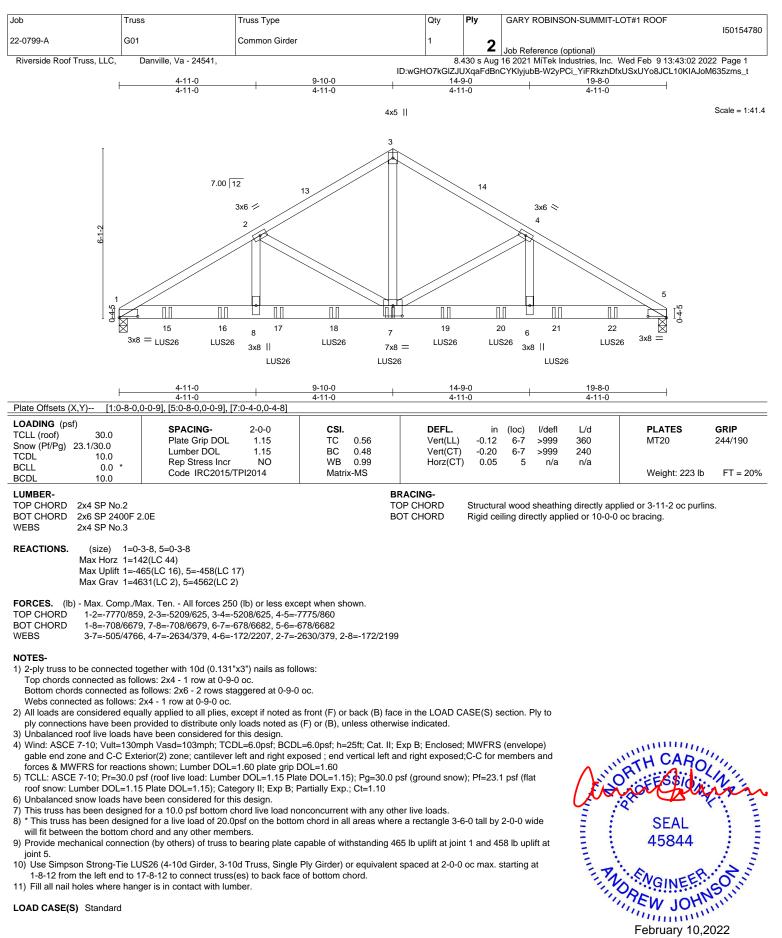


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

											
Job	Tru		Truss Type		Qty	Ply	GARY	ROBINSO	N-SUMMIT-LC	11#1 ROOF	150154779
22-0799-A		GE01	ATTIC		1	1		erence (or			
Riverside Roof T	russ, LLC,	Danville, Va - 24541,		1						/ed Feb 9 13:43:00 202 4QTP7jhBOXrYRytsUt0	
		- <u>1-0-0</u> 1-0-0			14-1-15 3-6-3 _{I I} 17-5-12	2	22-8-0				
		1-o-d	5-2-4 3-3-13 0-		-2-30-7-12 3-3-13 	I	5-2-4	I			Scale = 1:80.8
				6x6 3	_						00010 - 1.00.0
				7							
		Ţ	2x4 =		\						
			12.00 12 6		2x4 =						
			6								
			2x4	1.5x4	ш //		. .				
		မှ 5x12 MT ကို	20HS 1/ 5		/	9	2x4 4x12 M	MT20HS 🖄			
		12-0		8-2-4		R					
		3x4 🥢	4 m	ω			× ¹⁰	3x4	~		
			3 9 9	12-0-0)			11			
		<u>s</u> 2					Ŕ		12- ¹⁰ TO		
		0-11-15		di Z	15	14			0-11-15 12-0 1-2-0 1-1-15		
		0 / 8	21 ¹⁹	18	16	13		⊠ 3x8	0		
		3x8		= 3x6 =	1.5x4 4x10 = 1.5x4	3x6 = =		3X8			
			4x6 = 5-2-4 1 ⁹⁵ ¥413=	4x41 <u>-4</u> -0 1:			22-8-0				
	<u>)()</u> [0:0.0.4		5-2-4 3-11-9		-2-3 3-11-9		5-2-4				
LOADING (psf)		1,0-0-2], [12:0-4-3,Edge]									
TCLL (roof)	30.0	SPACING- Plate Grip DOL	2-0-0 CSI. 1.15 TC	0.99	DEFL. Vert(LL)		(loc) 15-17	l/defl >723	L/d 360	PLATES MT20	GRIP 244/190
Snow (Pf/Pg) 2 TCDL	3.1/30.0 10.0	Lumber DOL Rep Stress Incr	1.15 BC YES WB	0.98 0.91	Vert(CT) Horz(CT	-0.69	15-17	>396 n/a	240 n/a	MT20HS	187/143
BCLL BCDL	0.0 * 10.0	Code IRC2015/7			Attic		14-20	740	360	Weight: 184 lb	FT = 20%
LUMBER-				В	RACING-						
TOP CHORD	2x6 SP 2400F : 1-4: 2x6 SP No				OP CHORD OT CHORD				directly appli ad or 10-0-0 o	ed. c bracing. Except:	
BOT CHORD	2x4 SP No.1 *E	Except*				2-4-0 00	c bracing	: 14-20		3	
WEBS 2	14-20: 2x4 SP 2x4 SP No.3 *E	xcept*		JC	DINTS	I DIACE	e at Jt(s):	22			
	9-13,5-21,6-8: Left 2x4 SP No	2x4 SP No.2 .3 2-6-0, Right 2x4 SP N	o.3 2-6-0								
REACTIONS.	(size) 2-0	-3-8, 12=0-3-8									
	Max Horz 2=2	97(LC 13)									
	Max Grav 2=1	707(LC 31), 12=1643(LC	31)								
FORCES. (lb) TOP CHORD			(lb) or less except when sh)/439, 7-8=-38/440, 8-9=-113								
	9-12=-1806/2	3									
BOT CHORD		18-21=0/3426, 16-18=0 92, 15-17=-2527/0, 14-1	′3195, 13-16=0/3166, 12-13= 5=-301/204	=0/1144,							
WEBS		9-14=0/922, 20-21=0/69 0, 15-18=-265/368, 13-1	9, 5-20=0/925, 6-22=-1645/ 5=-2354/0	227, 8-22=-1	645/227,						
NOTES		0, 10 10 200,000, 10 1	200 // 0								
NOTES- 1) Unbalanced r	oof live loads h	ave been considered for	this design.								
			L=6.0psf; BCDL=6.0psf; h= eft and right exposed ; end							, mining and a second s	1.
forces & MWF	FRS for reaction	ns shown; Lumber DOL=	1.60 plate grip DOL=1.60		.				A	TH UARO	11 million
,			r DOL=1.15 Plate DOL=1.19 gory II; Exp B; Partially Exp.		ost (ground snow	/); Pt=23.	.1 pst (fla	it	1 Air	SOFE LAN	Kin
		e been considered for th	s design. ive load of 12.0 psf or 1.00 t	imes flat roof	load of 23.1 net	on over	ange		0000		
non-concurre	nt with other liv	e loads.		intes natioo	1000 01 20.1 poi	on oven	langs		E 1	SEAL	E E
		nless otherwise indicated d for a 10.0 psf bottom c	nord live load nonconcurrent	t with any oth	er live loads.					45844	E E
		ed for a live load of 20.0 nord and any other mem	osf on the bottom chord in al	Il areas wher	e a rectangle 3-6	6-0 tall by	/ 2-0-0 wi	ide	3.		1. 8
9) Ceiling dead I	load (5.0 psf) o	n member(s). 5-6, 8-9, 6	22, 8-22; Wall dead load (5						The second s	. NOINEER.	33
10) Bottom chor 11) Attic room cl			om chord dead load (5.0 psf	i) applied only	y to room. 17-20	, 15-17, <i>1</i>	14-15		and the	AFINICAL	Sam
•										NUMBER OF	<i></i>
										February 10,2	022
									I		
Design valid f	- Verify design para for use only with Mi	ameters and READ NOTES ON T Tek® connectors. This design i	HIS AND INCLUDED MITEK REFER	RENCE PAGE M	II-7473 rev. 5/19/2020 Individual building co	BEFORE I	USE. iot			ENGINEERING BY	10

nponent, not not the overall grithe BCSI Building Component BCSI Building Component BCSI Building Component



Continued on page 2

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Job		Truss	Truss Type	Qty	Ply	GARY ROBINSON-SUMMIT-LOT#1 ROOF
						I50154780
22-079	99-A	G01	Common Girder	1	2	
					_	Job Reference (optional)
Rive	rside Roof Truss, LLC,	Danville, Va - 24541,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Wed Feb 9 13:43:02 2022 Page 2
			ID:wGH	-107kGIZJI	JXqaFdBn	CYKlyjubB-W2yPCi_YiFRkzhDfxUSxUYo8JCL10KIAJoM635zms_t

LOAD CASE(S) Standard

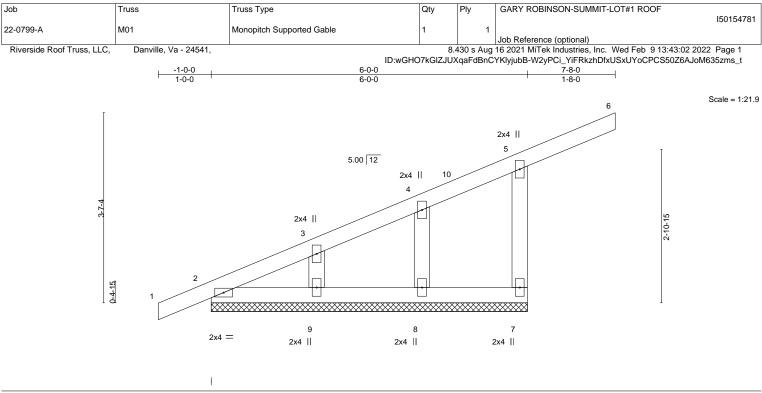
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-66, 3-5=-66, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-689(B) 15=-689(B) 16=-689(B) 17=-689(B) 18=-689(B) 19=-689(B) 20=-689(B) 21=-689(B) 22=-689(B)





Snow (Pf/Pg) 23.1/30.0 TCDL 10.0 BCU 0.0 * Rep S	ING- 2-0-0 Grip DOL 1.15 er DOL 1.15 itress Incr YES IRC2015/TPI2014	CSI. TC 0.30 BC 0.03 WB 0.04 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.00 0.00	(loc) 6 6 7	l/defl n/r n/r n/a	L/d 180 120 n/a	PLATES MT20 Weight: 30 lb	GRIP 244/190 FT = 20%
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LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 6-0-0.

(lb) - Max Horz 2=141(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 9, 8 except 7=-109(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 9, 8 except 7=315(LC 23)

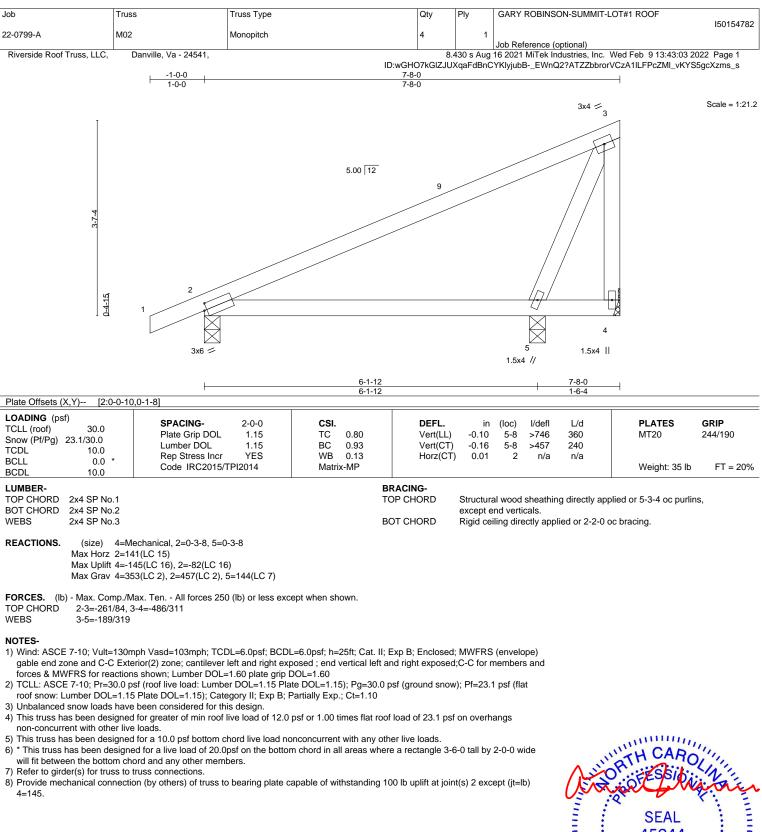
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 5-7=-300/216

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 9, 8 except (jt=lb) 7=109.





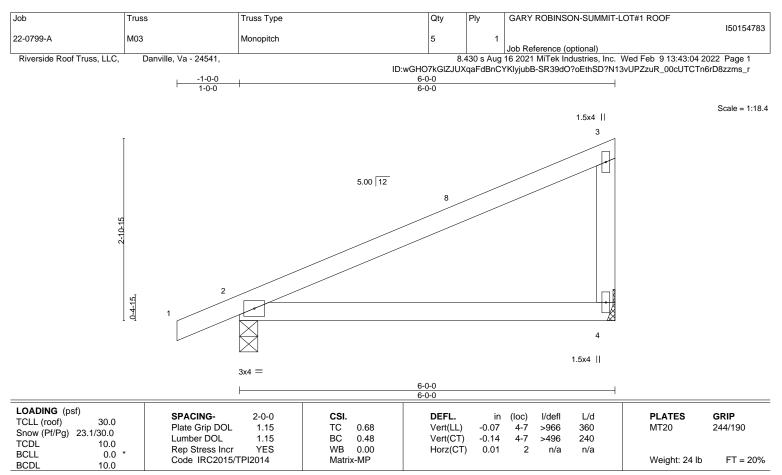




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LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-3-8 Max Horz 2=112(LC 15) Max Uplift 4=-52(LC 16), 2=-55(LC 16) Max Grav 4=286(LC 2), 2=380(LC 2)

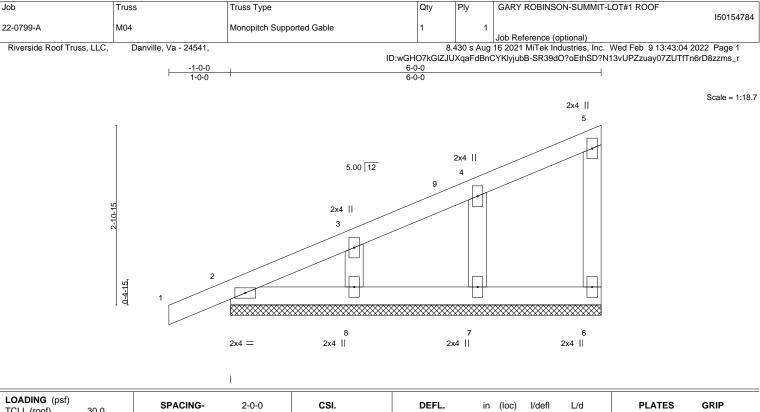
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.







LOADING (psf) TCLL (roof) 30.0 Snow (Pf/Pg) 23.1/30.0 TCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.11 BC 0.03 WB 0.04	DEFL. in (loc) l/defl L/d Vert(LL) 0.00 1 n/r 180 Vert(CT) -0.00 1 n/r 120 Horz(CT) 0.00 6 n/a n/a	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 27 lb FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 6-0-0.

(lb) - Max Horz 2=112(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 6, 2, 8, 7

Max Grav All reactions 250 lb or less at joint(s) 6, 2, 8, 7

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

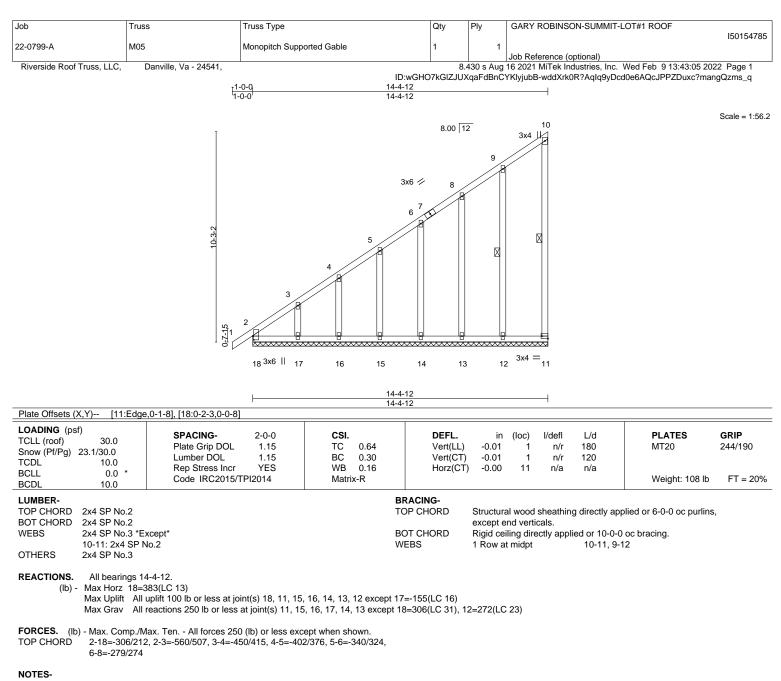
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 8, 7.



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- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 11, 15, 16, 14, 13, 12 except (jt=lb) 17=155.

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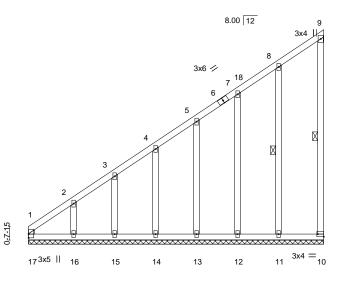
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ENGINEERING BY REPACT A MITEK Affili 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	GARY ROBINSON-SUMMIT-LOT#1 ROOF
					150154786
22-0799-A	M06	Monopitch Supported Gable	1	1	
					Job Reference (optional)
Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Wed Feb 9 13:43:06 2022 Page 1

ID:wGHO7kGIZJUXqaFdBnCYKlyjubB-OpBv2413mUy9SJXQAKXteOzn2ployLGmEQKKDszms_p 14-4-12 14-4-12



14-4-12

			17-7-	-						
Plate Offsets (2	X,Y) [10:Edge	,0-1-8]								
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	30.0 23.1/30.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.64 BC 0.30 WB 0.16	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a -0.00	(loc) - - 10	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 * 10.0	Code IRC2015/TPI2014	Matrix-R	. ,					Weight: 106 lb	FT = 20%
LUMBER-			В	RACING-						
TOP CHORD	2x4 SP No.2		т	OP CHORD	Structura	al wood	sheathin	g directly app	lied or 6-0-0 oc purling	5.
BOT CHORD	2x4 SP No.2				except e	nd verti	cals.	0 , 11		
WEBS	2x4 SP No.3 *Ex	cept*	B	OT CHORD	Rigid cei	ling dire	ectly app	ied or 10-0-0	oc bracing.	
	9-10: 2x4 SP No	.2	W		1 Row at			9-10, 8-11		
OTHERS	2x4 SP No.3							-, -		

REACTIONS. All bearings 14-4-12.

(Ib) - Max Horz 17=370(LC 13)
 Max Uplift All uplift 100 lb or less at joint(s) 10, 14, 15, 13, 12, 11 except 17=-117(LC 14), 16=-164(LC 16)
 Max Grav All reactions 250 lb or less at joint(s) 10, 14, 15, 13, 12 except 17=260(LC 13), 16=277(LC 29), 11=265(LC 22)

TOP CHORD 1-17=-322/266, 1-2=-556/508, 2-3=-451/414, 3-4=-402/376, 4-5=-340/324, 5-7=-279/274

10-3-2

NOTES-

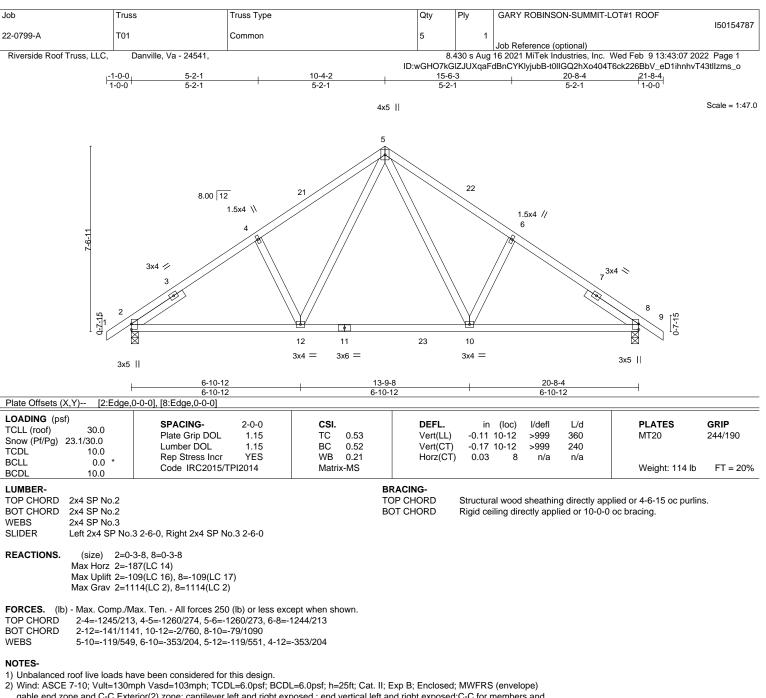
- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 14, 15, 13, 12, 11 except (jt=lb) 17=117, 16=164.



Scale = 1:56.2



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



2) Wind: ASCE 7-10; Vuit=130mph Vasd=103mph; ICDL=6.0pst; BCDL=6.0pst; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

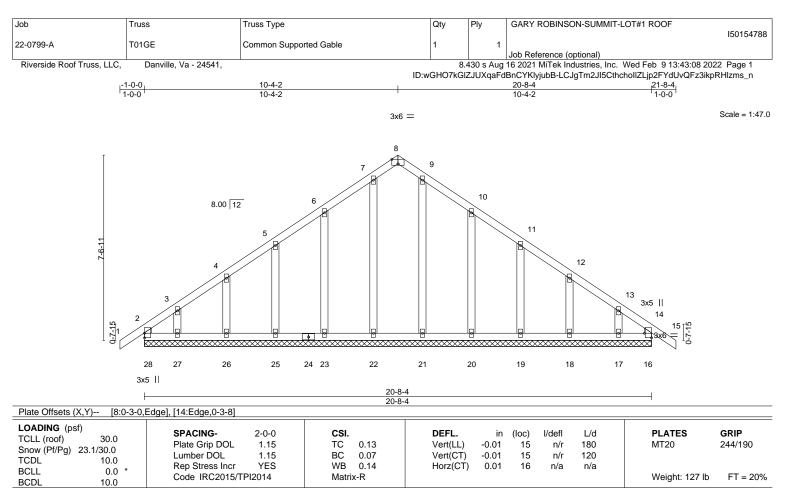
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=109, 8=109.





LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 20-8-4.

Max Horz 28=-203(LC 14) (lb) -

> Max Uplift All uplift 100 lb or less at joint(s) 28, 16, 23, 25, 26, 20, 19, 18 except 27=-122(LC 16), 17=-113(LC 17)

Max Grav All reactions 250 lb or less at joint(s) 28, 16, 22, 23, 25, 26, 27, 21, 20, 19, 18, 17

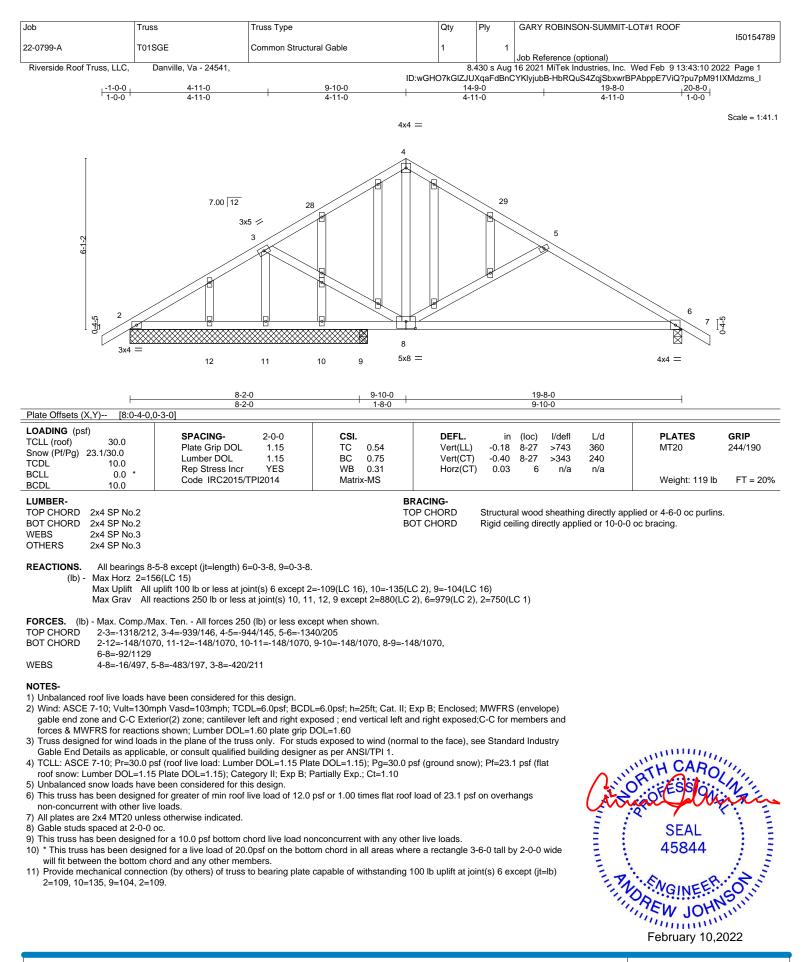
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 12) will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 16, 23, 25, 26, 20, 19, 18 except (jt=lb) 27=122, 17=113.

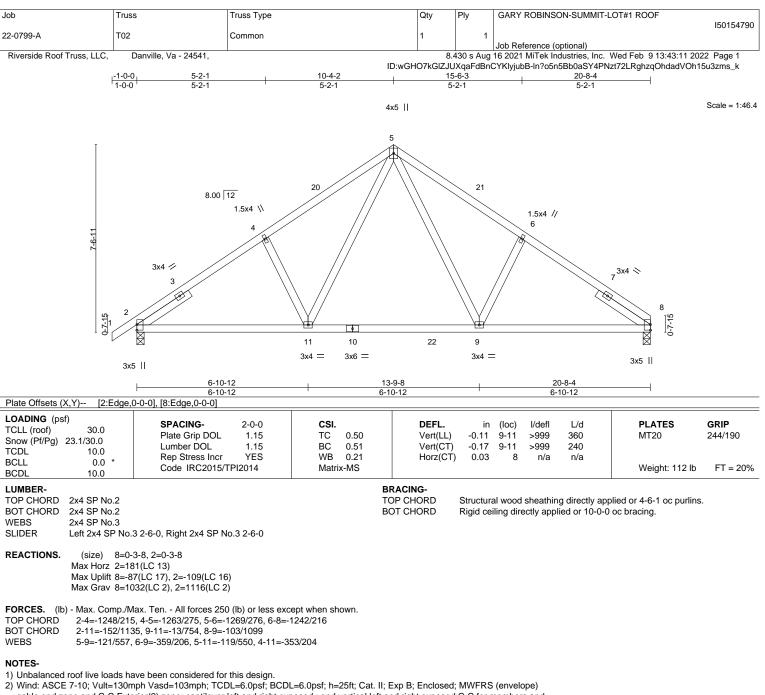








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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

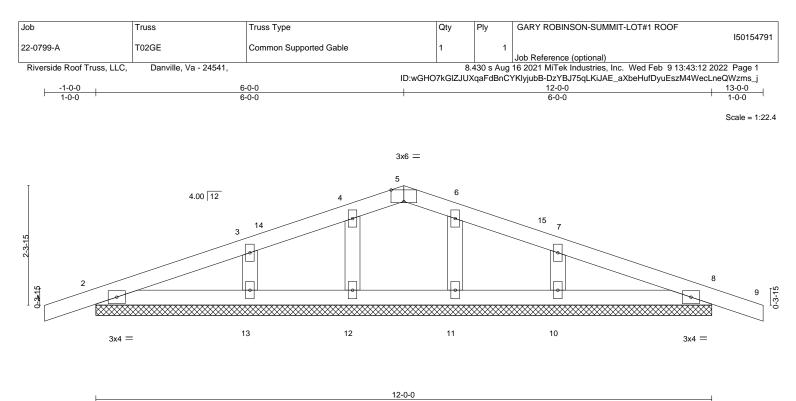
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 2=109.





		12-0-0)				1	
Plate Offsets (X,Y) [5:0-3-0,E	Edge]							
LOADING (psf) TCLL (roof) 30.0 Snow (Pf/Pg) 23.1/30.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.11 BC 0.06 WB 0.04	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.00 8 0.00 9 0.00 8	8 n/r 9 n/r	L/d 180 120 n/a	PLATES MT20	GRIP 244/190
BCDL 0.0	Code IRC2015/TPI2014	Matrix-S					Weight: 47 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			ACING-	Structural woo	d sheathir	ng directly ap	plied or 6-0-0 oc purlir	15.

BOT CHORD

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

REACTIONS. All bearings 12-0-0.

(lb) - Max Horz 2=39(LC 20)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 12, 13, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 11 except 13=285(LC 34), 10=285(LC 35)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

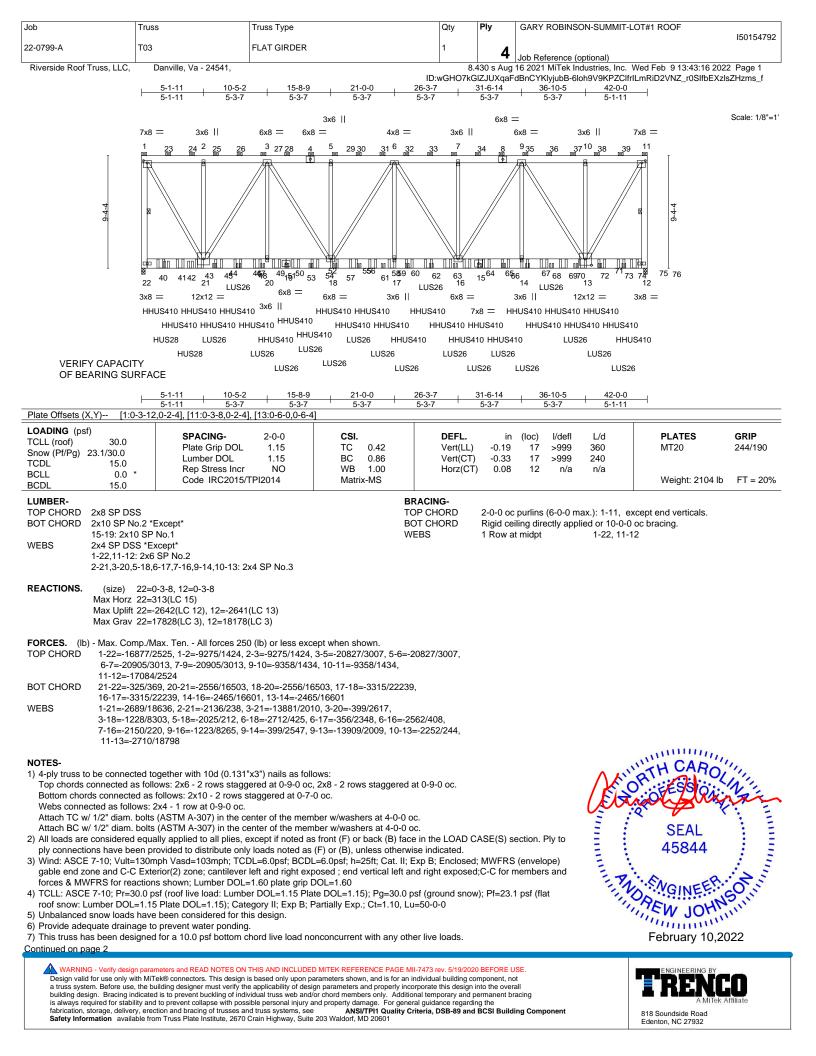
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 12, 13, 11,
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 12, 13, 11, 10.



Rigid ceiling directly applied or 10-0-0 oc bracing.





- [·	Job	Truss	Truss Type	Qty	Ply	GARY ROBINSON-SUMMIT-LOT#1 ROOF
						150154792
	22-0799-A	T03	FLAT GIRDER	1		
					4	Job Reference (optional)
	Riverside Roof Truss, LLC,	Danville, Va - 24541,		8.	430 s Aug	16 2021 MiTek Industries, Inc. Wed Feb 9 13:43:17 2022 Page 2
			ID:wGH	O7kGIZJU	XqaFdBnC	CYKlyjubB-axM4Mr9yAsKcG?tXJ8DSbiwkkFMh16rNmdUP5jzms_e

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 15.0psf.

- Bearing at joint(s) 22, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 22=2642, 12=2641.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

- 12) Use Simpson Strong-Tie HHUS410 (30-10d Girder, 10-10d Truss, Single Ply Girder) or equivalent spaced at 1-7-3 oc max. starting at 1-7-3 from the left end to 40-11-2 to connect truss(es) to front face of bottom chord.
- 13) Use Simpson Strong-Tie HUS28 (22-10d Girder, 4-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 4-0-12 to connect truss(es) to back face of bottom chord.
- 14) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 6-0-12 from the left end to 40-0-12 to connect truss(es) to back face of bottom chord.
- 15) Fill all nail holes where hanger is in contact with lumber.

16) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 173 lb down and 130 lb up at 0-2-12, 689 lb down and 106 lb up at 2-0-12, 689 lb down and 106 lb up at 4-0-12, 689 lb down and 106 lb up at 6-0-12, 689 lb down and 106 lb up at 8-0-12, 689 lb down and 106 lb up at 10-0-12, 689 lb down and 106 lb up at 12-0-12, 689 lb down and 106 lb up at 14-0-12, 689 lb down and 106 lb up at 16-0-12, 689 lb down and 106 lb up at 18-0-12, 689 lb down and 106 lb up at 20-0-12, 689 lb down and 106 lb up at 22-0-12, 733 lb down and 108 lb up at 24-0-12, 733 lb down and 108 lb up at 26-0-12, 733 lb down and 108 lb up at 28-0-12, 733 lb down and 108 lb up at 30-0-12, 733 lb down and 108 lb up at 32-0-12, 733 lb down and 108 lb up at 34-0-12, 733 lb down and 108 lb up at 36-0-12, 733 lb down and 108 lb Ib down and 108 lb up at 38-0-12, and 733 lb down and 108 lb up at 40-0-12, and 177 lb down and 103 lb up at 41-9-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

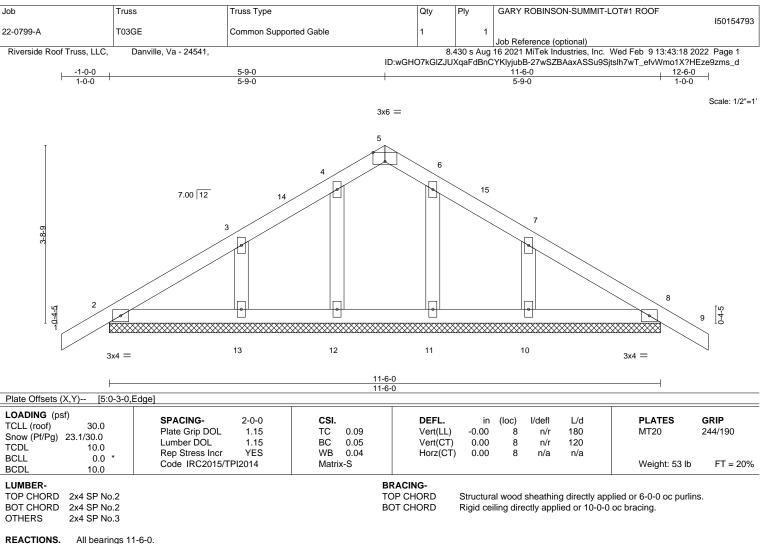
Uniform Loads (plf)

Vert 1-11=-76 12-22=-30

Concentrated Loads (lb)

Vert: 1=-3 4=-595 11=-10 19=-697(B) 18=-77(F) 17=-77(F) 7=-633 16=-697(B) 14=-77(F) 13=-77(F) 8=-633 15=-697(B) 23=-595 24=-595 25=-595 26=-595 27=-595 28=-595 29=-595 30=-595 31=-595 32=-595 33=-633 34=-633 35=-633 36=-633 37=-633 38=-633 39=-633 40=-77(F) 41=-697(B) 43=-77(F) 44=-697(B) 45=-77(F) 46=-697(B) 47=-77(F) 49=-775(F=-77, B=-697) 50=-77(F) 51=-697(B) 52=-77(F) 53=-697(B) 56=-77(F) 57=-697(B) 58=-77(F) 59=-697(B) 56=-77(F) 57=-697(B) 56=-77(F) 57=-77(F) 5 60=-77(F) 61=-697(B) 62=-766(F=-77, B=-689) 63=-766(F=-77, B=-689) 64=-77(F) 65=-77(F) 66=-77(F) 67=-766(F=-77, B=-689) 68=-689(B) 69=-77(F) 70=-689(B) 71=-77(F) 72=-689(B) 73=-766(F=-77, B=-689) 75=-766(F=-77, B=-689) 76=-77(F)





All bearings 11-6-0.

(lb) -Max Horz 2=-97(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 12, 13, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 11 except 13=263(LC 34), 10=263(LC 31)

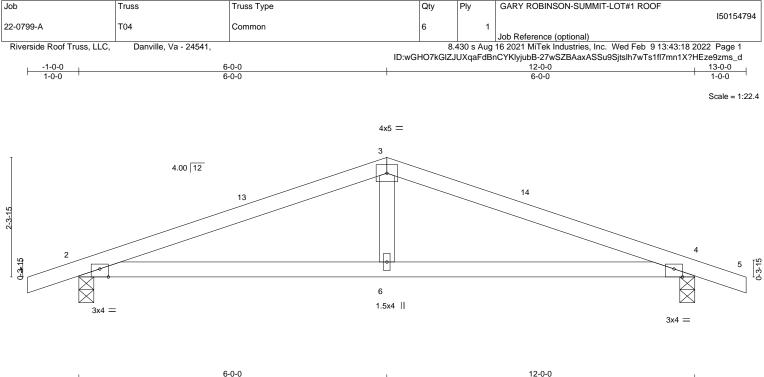
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 12, 13, 11,
- 10.



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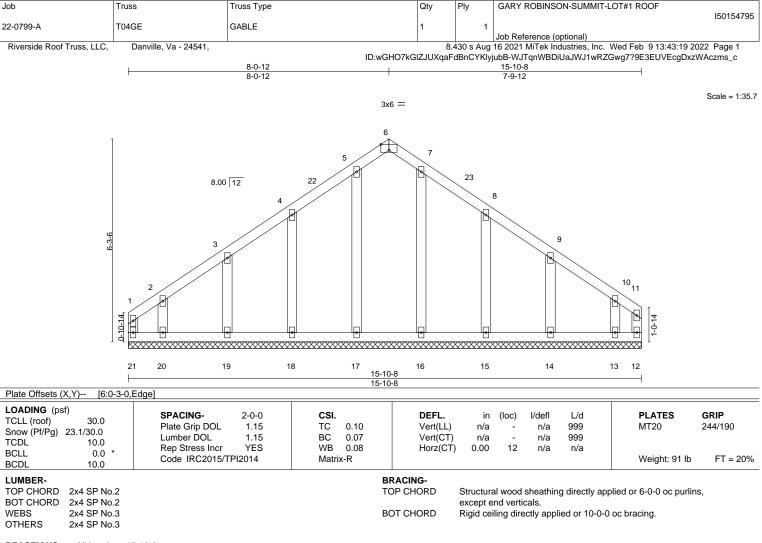
	6-0-0				12-0-0						
Plate Offsets (X,Y) [2:0-2-0,	6-0-0 Edge], [4:0-2-0,Edge]	· · ·			6-0-0						
LOADING (psf) TCLL (roof) 30.0 Snow (Pf/Pg) 23.1/30.0 TCDL TCDL 0.0 8 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.58 BC 0.65 WB 0.11 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.07	oc) l/defl 6-9 >999 6-9 >999 4 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 43 lb	GRIP 244/190 FT = 20%			
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	BRACING- DP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins. DT CHORD 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.										
REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=39(LC 20) Max Uplift 2=-108(LC 12), 4=-108(LC 13) Max Grav 2=680(LC 2), 4=680(LC 2)											
TOP CHORD 2-3=-1123/26	Max. Ten All forces 250 (lb) or less exc 1, 3-4≕1123/261 9, 4-6≕170/1009	ept when shown.									
 Wind: ASCE 7-10; Vult=130r gable end zone and C-C Ext forces & MWFRS for reaction TCLL: ASCE 7-10; Pr=30.0 p roof snow: Lumber DOL=1.1 Unbalanced snow loads have This truss has been designer non-concurrent with other liw This truss has been designer This truss has been designer This truss has been designer 	ave been considered for this design. nph Vasd=103mph; TCDL=6.0psf; BCDI erior(2) zone; cantilever left and right exp ns shown; Lumber DOL=1.60 plate grip f ssf (roof live load: Lumber DOL=1.15 Pla 5 Plate DOL=1.15); Category II; Exp B; F e been considered for this design. d for greater of min roof live load of 12.0 e loads. d for a live load of 20.0psf on the bottor ord and out out that members	bosed ; end vertical left a DOL=1.60 te DOL=1.15); Pg=30.0 p Partially Exp.; Ct=1.10 psf or 1.00 times flat root ponconcurrent with any oth	nd right exposed;(psf (ground snow); f load of 23.1 psf o ner live loads.	C-C for men ; Pf=23.1 ps on overhang	mbers and sf (flat gs	And	OR FEBSIO				

will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=108.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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REACTIONS. All bearings 15-10-8.

(lb) - Max Horz 21=157(LC 13)

 Max Uplift
 All uplift 100 b or less at joint(s) 18, 19, 15, 14 except 21=-111(LC 14), 12=-116(LC 15), 20=-147(LC 16), 13=-174(LC 17)

 Max Grav
 All reactions 250 lb or less at joint(s) 21, 12, 17, 18, 19, 20, 16, 15, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

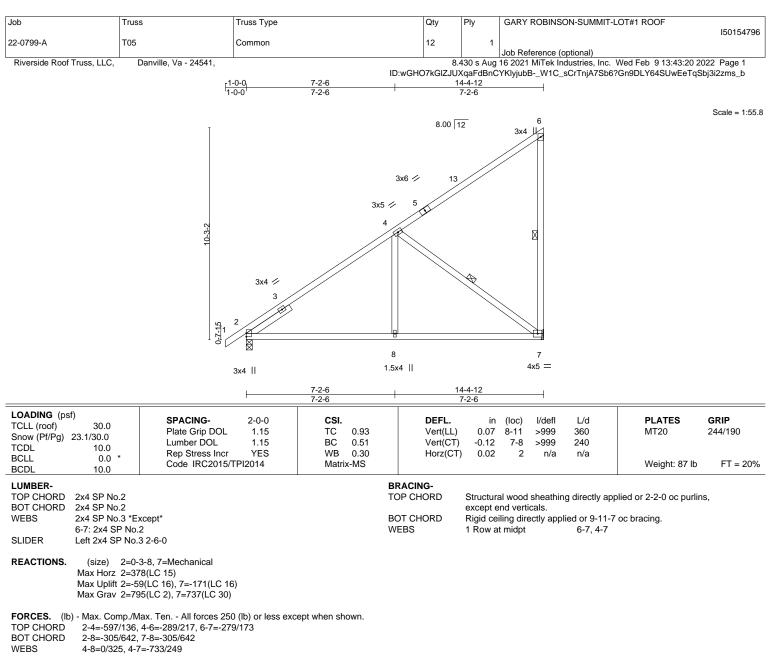
- 4) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

9) Gable studs spaced at 2-0-0 oc.

- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 19, 15, 14 except (jt=lb) 21=111, 12=116, 20=147, 13=174.



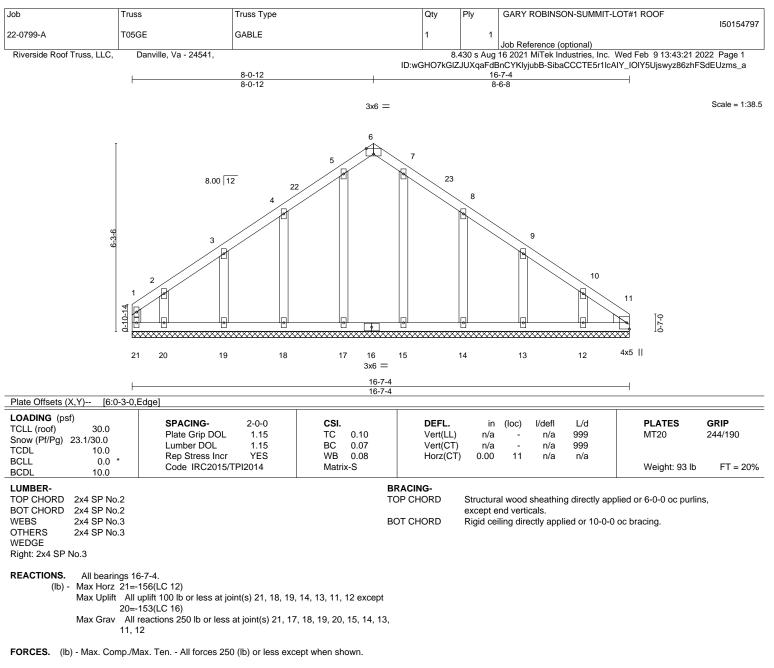




- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 23.1 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 7=171.





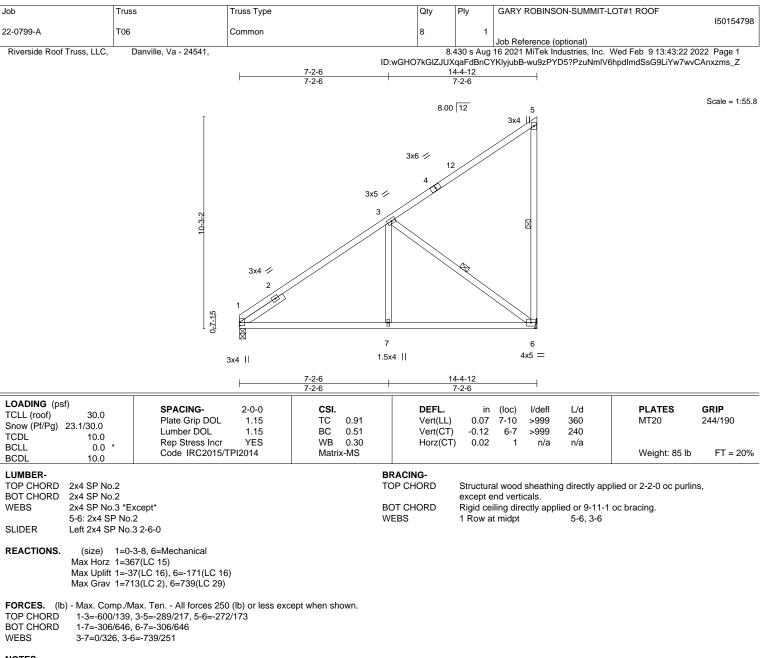


- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 18, 19, 14, 13, 11, 12 except (jt=lb) 20=153.



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- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

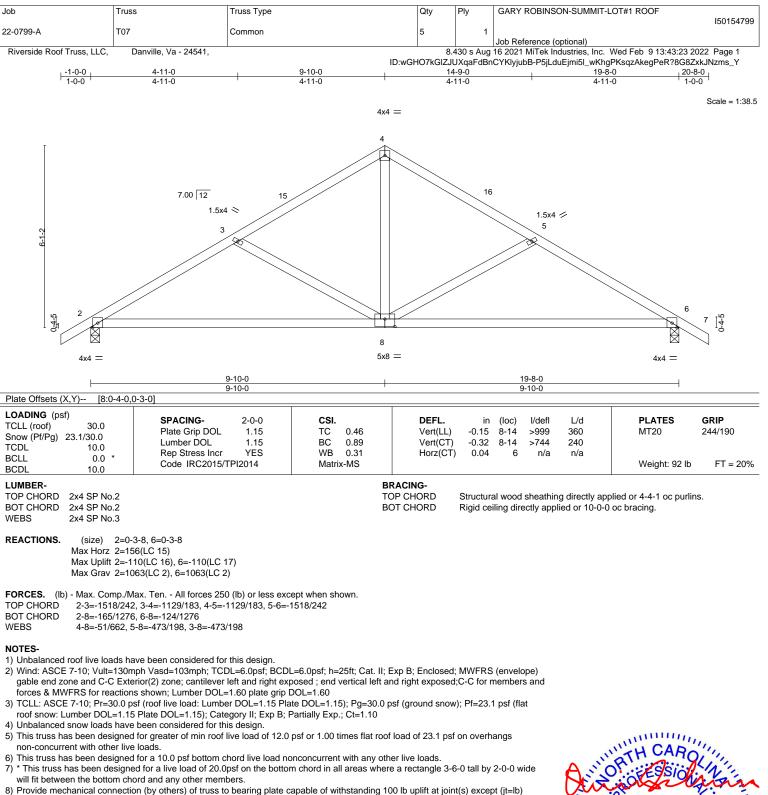
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 6=171.







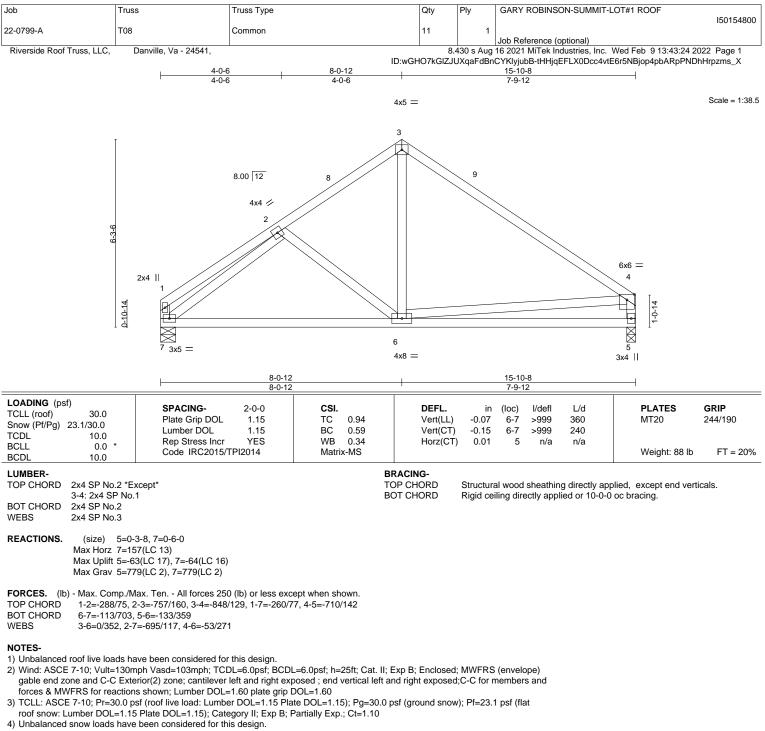


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2=110, 6=110.

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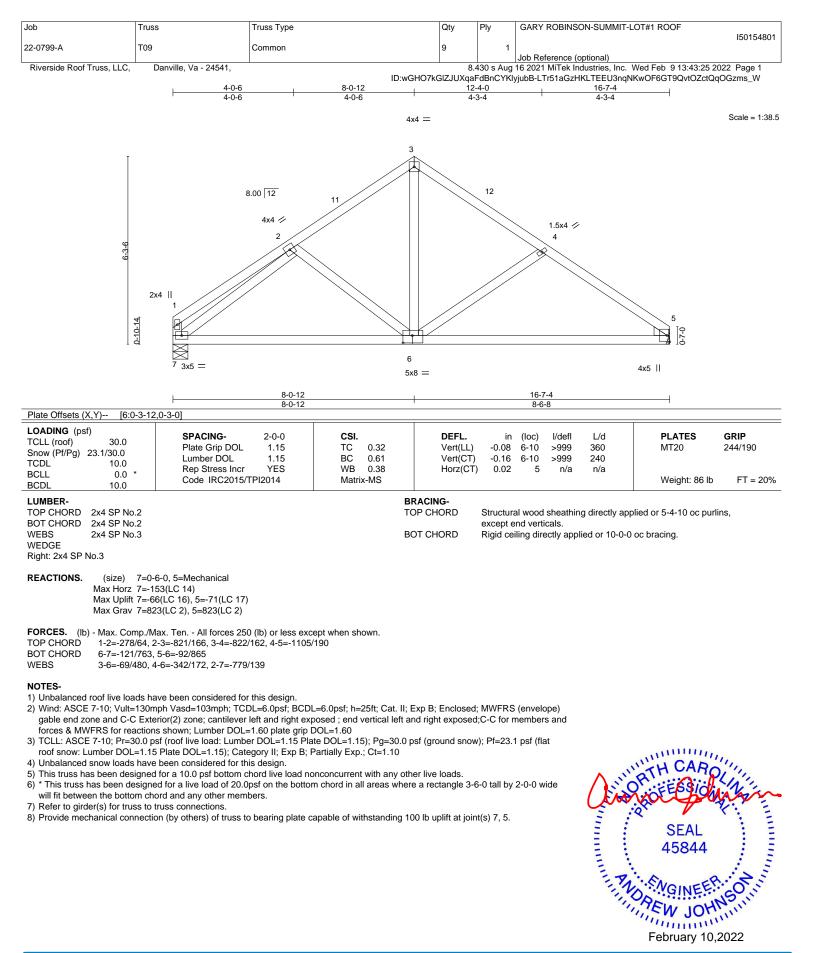
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 7.



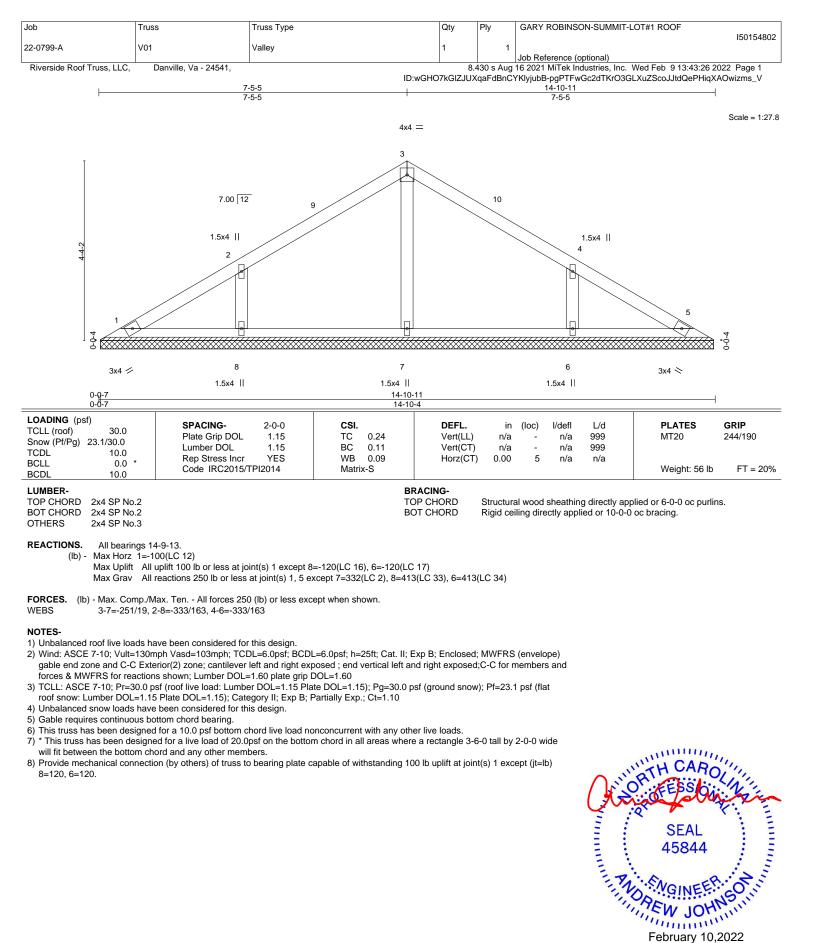
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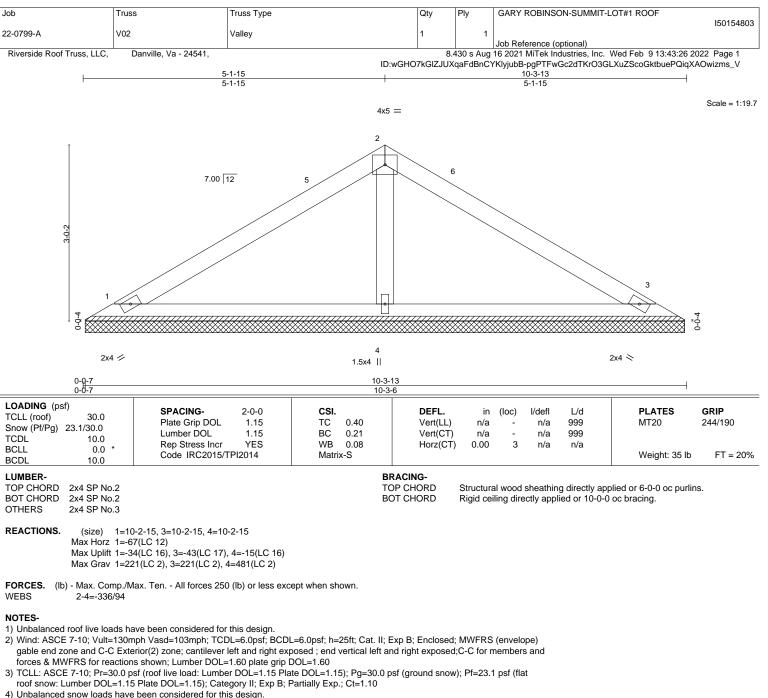
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Gable requires continuous bottom chord bearing.

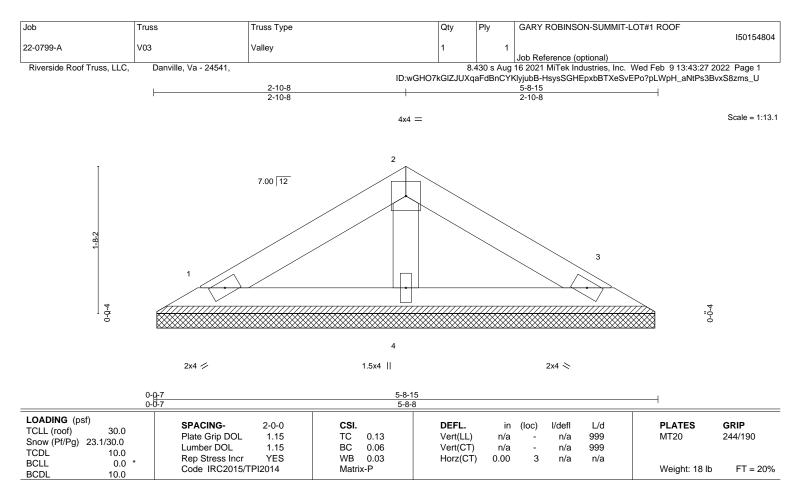
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.







BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD

2x4 SP No.2 OTHERS 2x4 SP No.3

REACTIONS. 1=5-8-2, 3=5-8-2, 4=5-8-2 (size) Max Horz 1=-34(LC 12) Max Uplift 1=-22(LC 16), 3=-27(LC 17) Max Grav 1=124(LC 2), 3=124(LC 2), 4=218(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

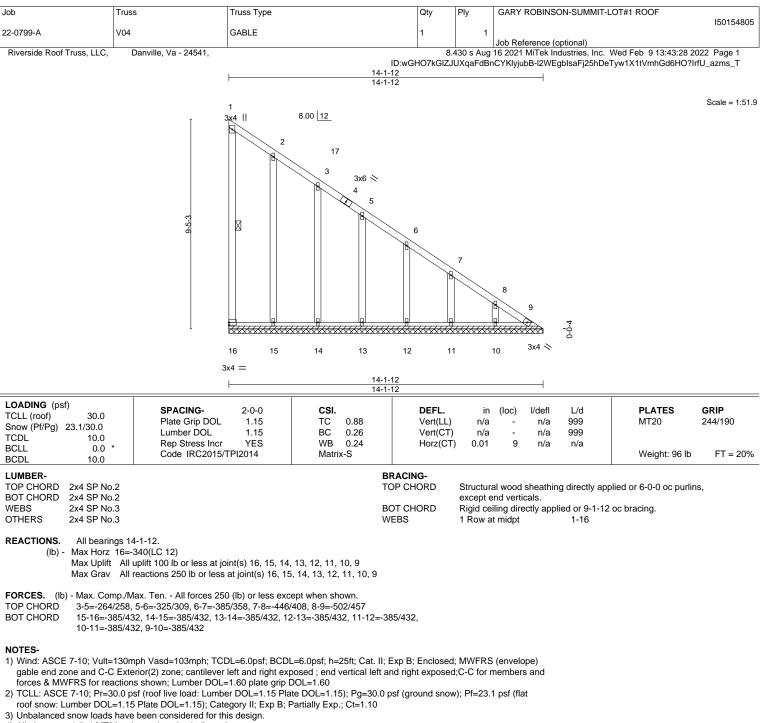
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 5-8-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





All plates are 1.5x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

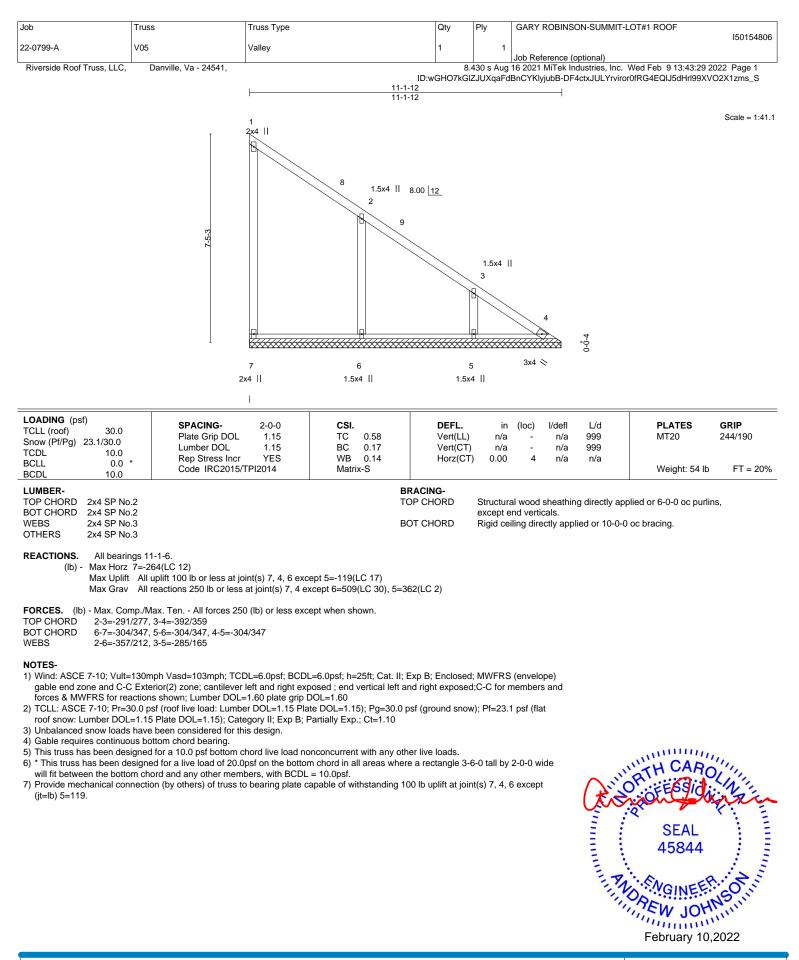
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 15, 14, 13, 12, 11, 10, 9.



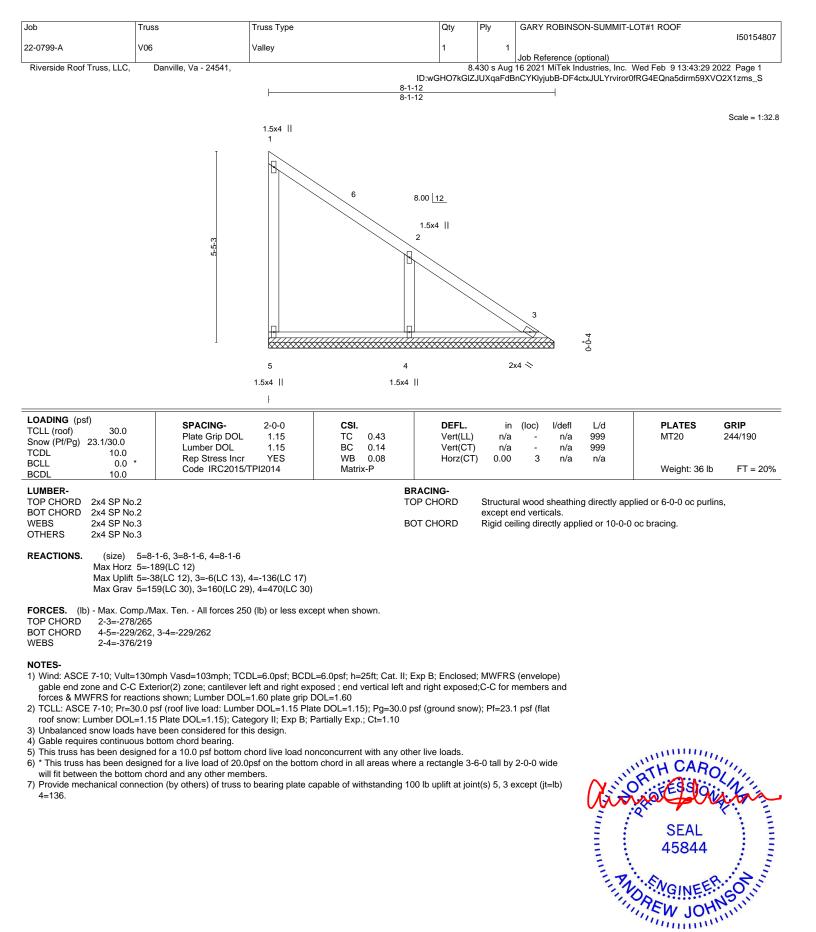




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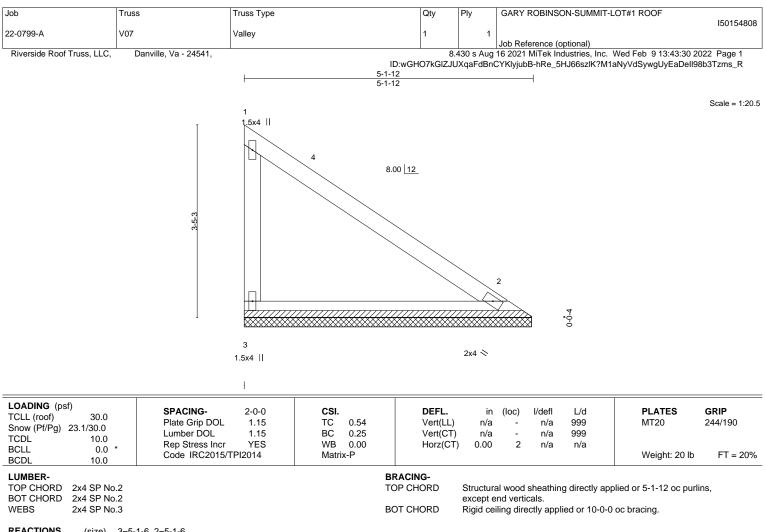


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February 10,2022





REACTIONS. (size) 3=5-1-6, 2=5-1-6 Max Horz 3=-114(LC 12) Max Uplift 3=-53(LC 17), 2=-13(LC 17) Max Grav 3=233(LC 30), 2=226(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

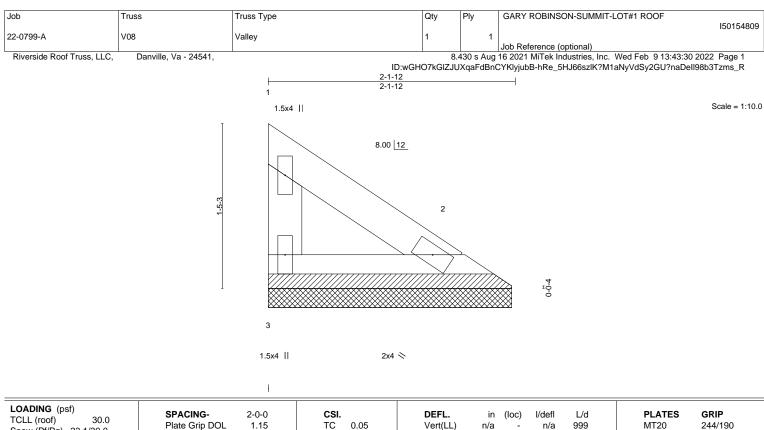
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.







LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	30.0 23.1/30.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.05 BC 0.02 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 2	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 * 10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 7 lb	FT = 20%
			-	DACING						

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

 Structural wood sheathing directly applied or 2-1-12 oc purlins, except end verticals.
 Braid exiling directly applied or 10.0.0 oc bracing

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=2-1-6, 2=2-1-6 Max Horz 3=-38(LC 12) Max Uplift 3=-18(LC 17), 2=-4(LC 17) Max Grav 3=78(LC 30), 2=76(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

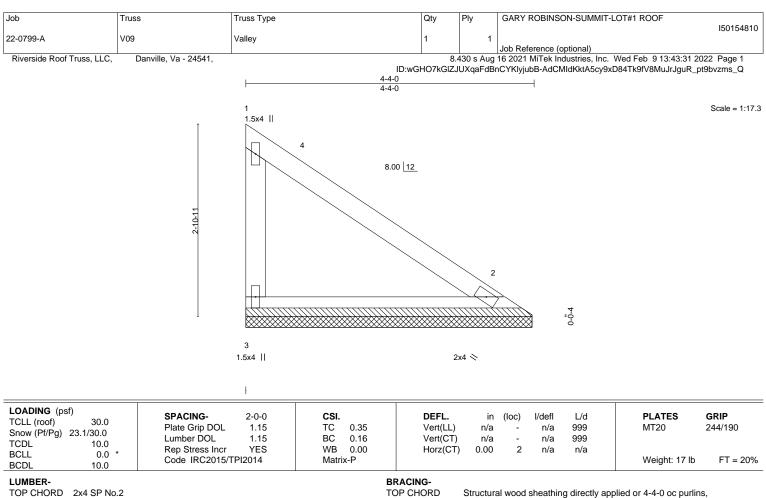
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.







BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

WEBS 2x4 SP No.3 **REACTIONS.** (size) 3=4-3-10, 2=4-3-10 Max Horz 3=-93(LC 12)

Max Uplift 3=-43(LC 17), 2=-11(LC 17)

Max Grav 3=191(LC 30), 2=185(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



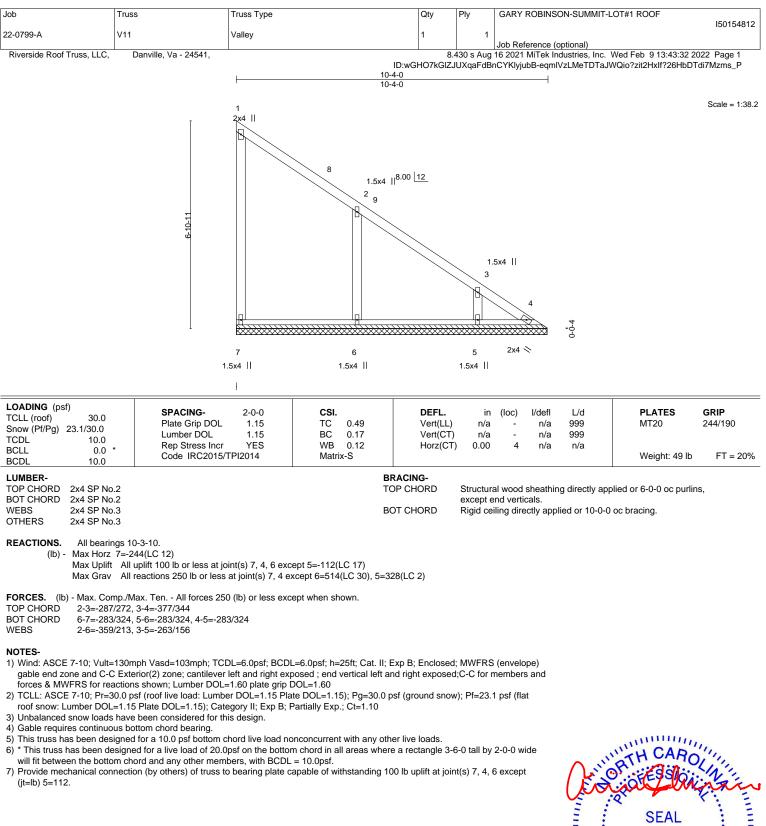


Job	Truss	Truss Type		Qty	Ply	GARY R	OBINSON-SUMM	IIT-LOT#1 ROOF	150454044
2-0799-A	V10	Valley		1	1		<i>.</i>		I50154811
Riverside Roof Truss,	LLC, Danville, Va - 2454	 1,				16 2021 N		nc. Wed Feb 9 13:43:32	
		L	7-4-(0	JUXqaFdE	BnCYKlyjuł	bB-eqmlVzLMeTD ⊣	TaJWQio?zit2JEIgl261bl	DTdi7Mzms_P
		,	7-4-(0			1		
		1.5x4							Scale = 1:29
	Ţ	1							
		6	7 .						
			8	.00 12					
				1.5x4					
	-10-11			2					
	4								
					\diagdown	3			
				l,		À			
	l						0-0- 4-0-0-		
		5 1.5x4	1.5x4	4	2x	4 📎			
LOADING (psf) TCLL (roof) 3	0.0 SPACING		SI.	DEFL.	in	(loc)	l/defl L/d	PLATES	GRIP
Snow (Pf/Pg) 23.1/3	0.0 Plate Grip		C 0.34 BC 0.12	Vert(LL) Vert(CT)	n/a) n/a	-	n/a 999 n/a 999	MT20	244/190
BCLL	0.0 * Rep Stress		VB 0.07 Aatrix-P	Horz(CT) 0.00	3	n/a n/a	Weight: 32 I	b FT = 20%
BCDL 1 LUMBER-	0.0			ACING-					
TOP CHORD 2x4 S	P No.2 P No.2			P CHORD				applied or 6-0-0 oc pur	lins,
WEBS 2x4 S	P No.3		BO	T CHORD		end vertica iling direc	als. tly applied or 10-	-0-0 oc bracing.	
	P No.3								
	ze) 5=7-3-10, 3=7-3-10, 4 Horz 5=-169(LC 12)	! =7-3-10							
	Uplift 5=-35(LC 12), 3=-13(l Grav 5=164(LC 30), 3=120								
		ces 250 (lb) or less except whe	n shown						
OP CHORD 2-3=	-260/246		n shown.						
	=-341/202								
NOTES- 1) Wind: ASCE 7-10;	Vult=130mph Vasd=103mp	h; TCDL=6.0psf; BCDL=6.0psf	f; h=25ft; Cat. II; E>	xp B; Enclosed	; MWFR	6 (envelop	be)		
		tilever left and right exposed ; e r DOL=1.60 plate grip DOL=1.6		d right exposed	d;C-C for	members	and		
		Lumber DOL=1.15 Plate DOL= i); Category II; Exp B; Partially		f (ground snov	w); Pf=23.	1 psf (flat			
3) Unbalanced snow	loads have been considered	for this design.	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,						
5) This truss has beer	n designed for a 10.0 psf bo	ottom chord live load nonconcu				2.0.0	4.0		
will fit between the	bottom chord and any other			0				WH CAR	1111
 Provide mechanica 4=131. 	al connection (by others) of t	truss to bearing plate capable of	of withstanding 100) Ib uplift at joir	nt(s) 5, 3 (except (jt=	•lb)	OR	LINI
							U	winter for	M.M.
								SEAL	
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							Contraction of the second	30+4	
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								February 10	0.2022

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February 10,2022

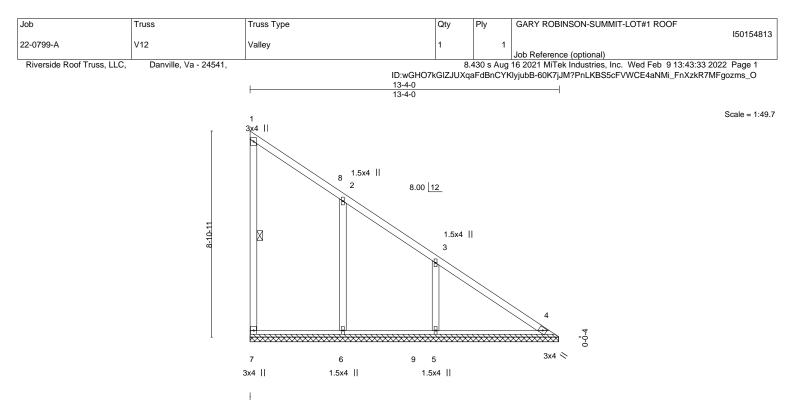




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A MITEK Affilia 818 Soundside Road

Edenton, NC 27932



LOADING (psf) TCLL (roof) 30.0 Snow (Pf/Pg) 23.1/30.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.83 BC 0.23 WB 0.22 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.01	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 68 lb	GRIP 244/190 FT = 20%
LUMBER-			RACING-				н а		

TOP CHORD	2X4 SP N0.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 9-6-13 oc bracing. 1 Row at midpt 1-7

REACTIONS. All bearings 13-3-10.

(lb) -Max Horz 7=-319(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 7, 4 except 6=-126(LC 17), 5=-163(LC 17)

Max Grav All reactions 250 lb or less at joint(s) 7, 4 except 6=530(LC 30), 5=542(LC 30)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-295/283, 3-4=-439/412
- BOT CHORD 6-7=-362/409, 5-6=-362/409, 4-5=-362/409
- WFBS 2-6=-330/198. 3-5=-395/217

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=30.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=30.0 psf (ground snow); Pf=23.1 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 4 except (jt=lb)
- 6=126, 5=163.



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