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- EXTERIOR SHEATHING OR INSULATION BD SEE BUILDER'S SPECS.

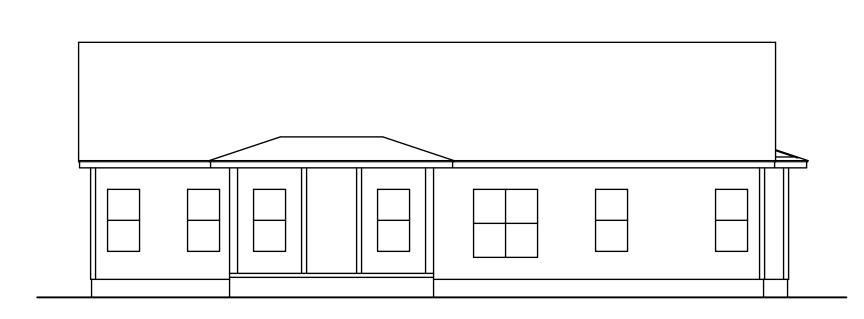
— EXTERIOR FINISH SEE BUILDER'S SPECS

- SOFFIT WITH VENTS

PROVIDE GLITTERS AND DOWNSPOLITS PER CODE

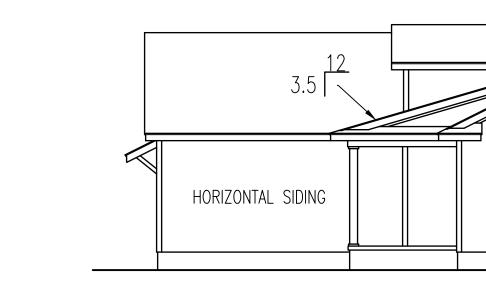
– TRUSS OR RAFTERS PER SPECIFICATION

— ASPHALT SHINGLES — #15 FELT (SEE SPEC.) — ROOF SHEATHING (SEE SPEC.)



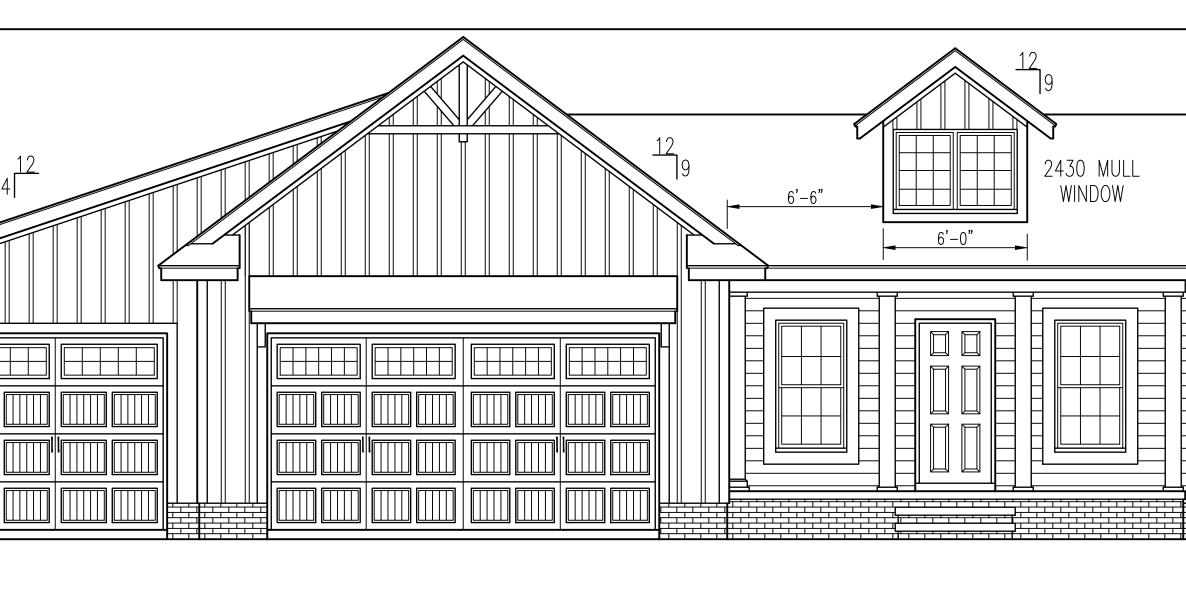
REAR ELEVATION

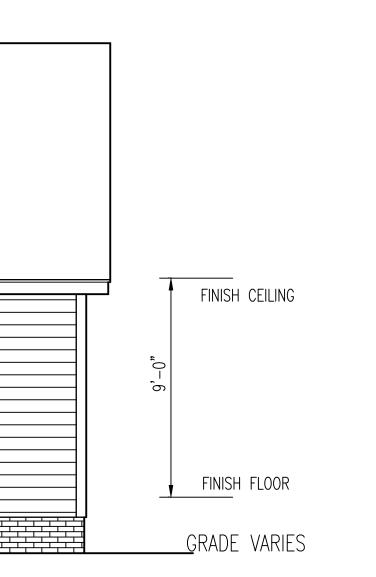
SCALE:1/8"=1'-0"

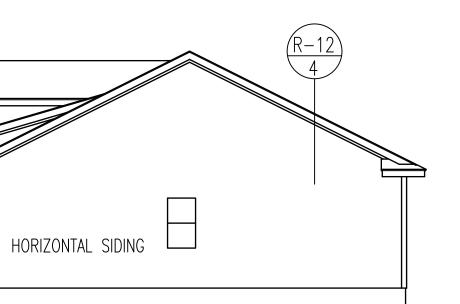


NOTICE TO CONTRACTOR All construction must comply with current NC Building Codes and is subject to field inspection and verification. APPROVED Winted building only review Permit holder responsible for full compliance with the code Mode/24/2021

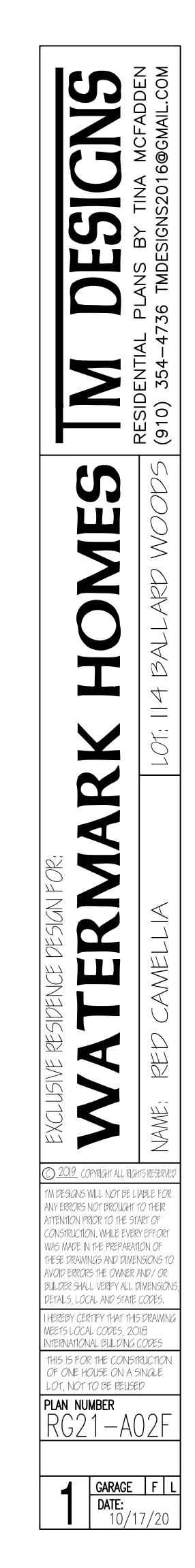
FRONTELEVATION SCALE:1/4"=1'-0"

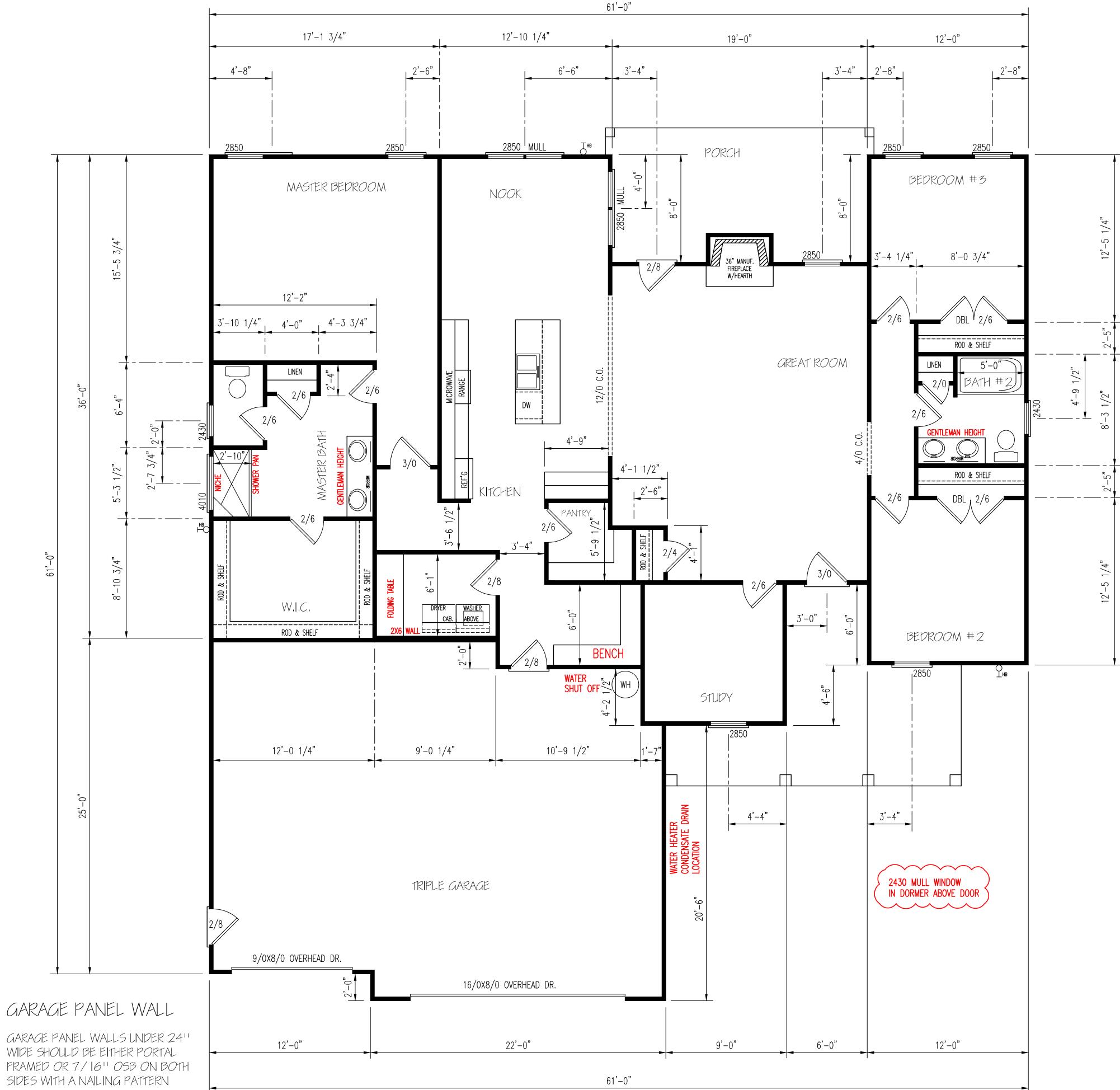






RIGHT ELEVATION



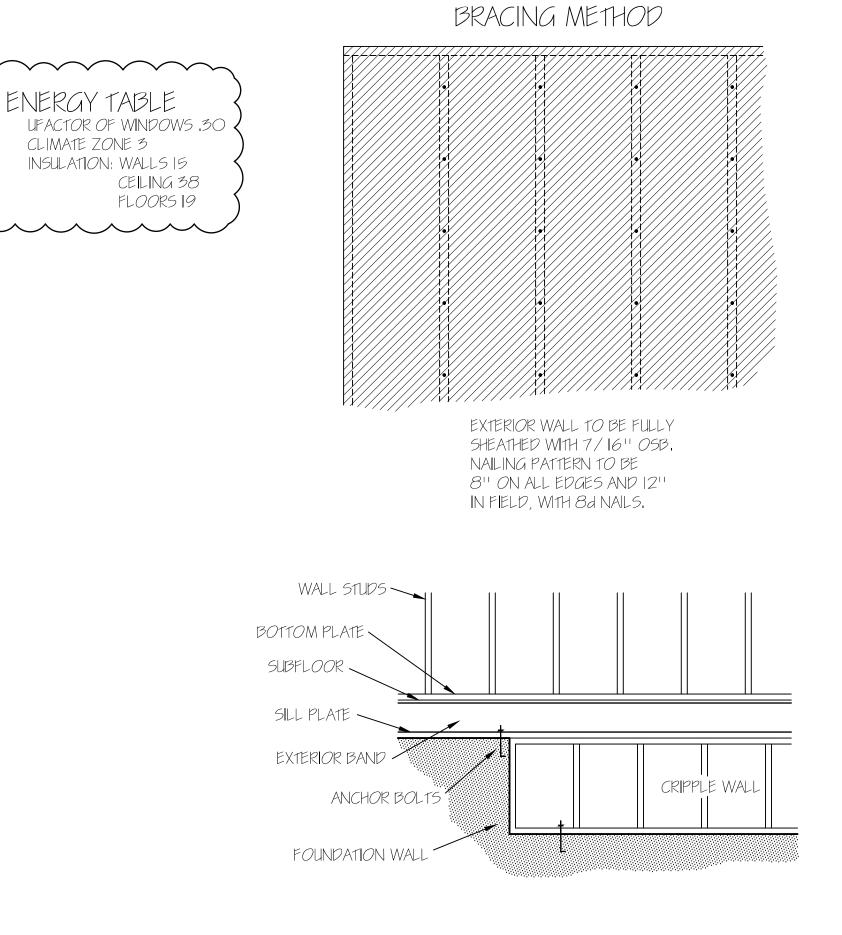


OF 311 ON ALL PANEL EDGES AND 6" IN THE FIELD,





GARAGE F.PORCH R.PORCH



XERIOR \	NALLS	
2X10 H	EADER	S
R SPAN		of studs
HEADER	JACKS	KINGS
R & C.O. F	1	1
R & C.O. '-11"	2	2
R & C.O. ABOVE	sized Engin	
SS NOTED	OTHER	WISE**

FLOOR PLAN SCALE:1/4''=1'-0''

HEATED AREA

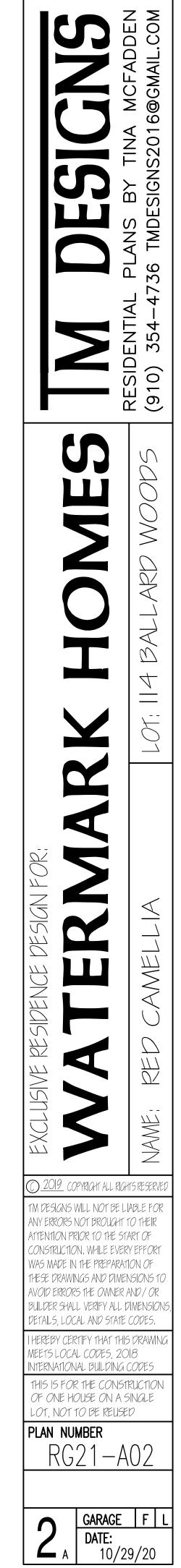
<u>2|46</u> SQ FT

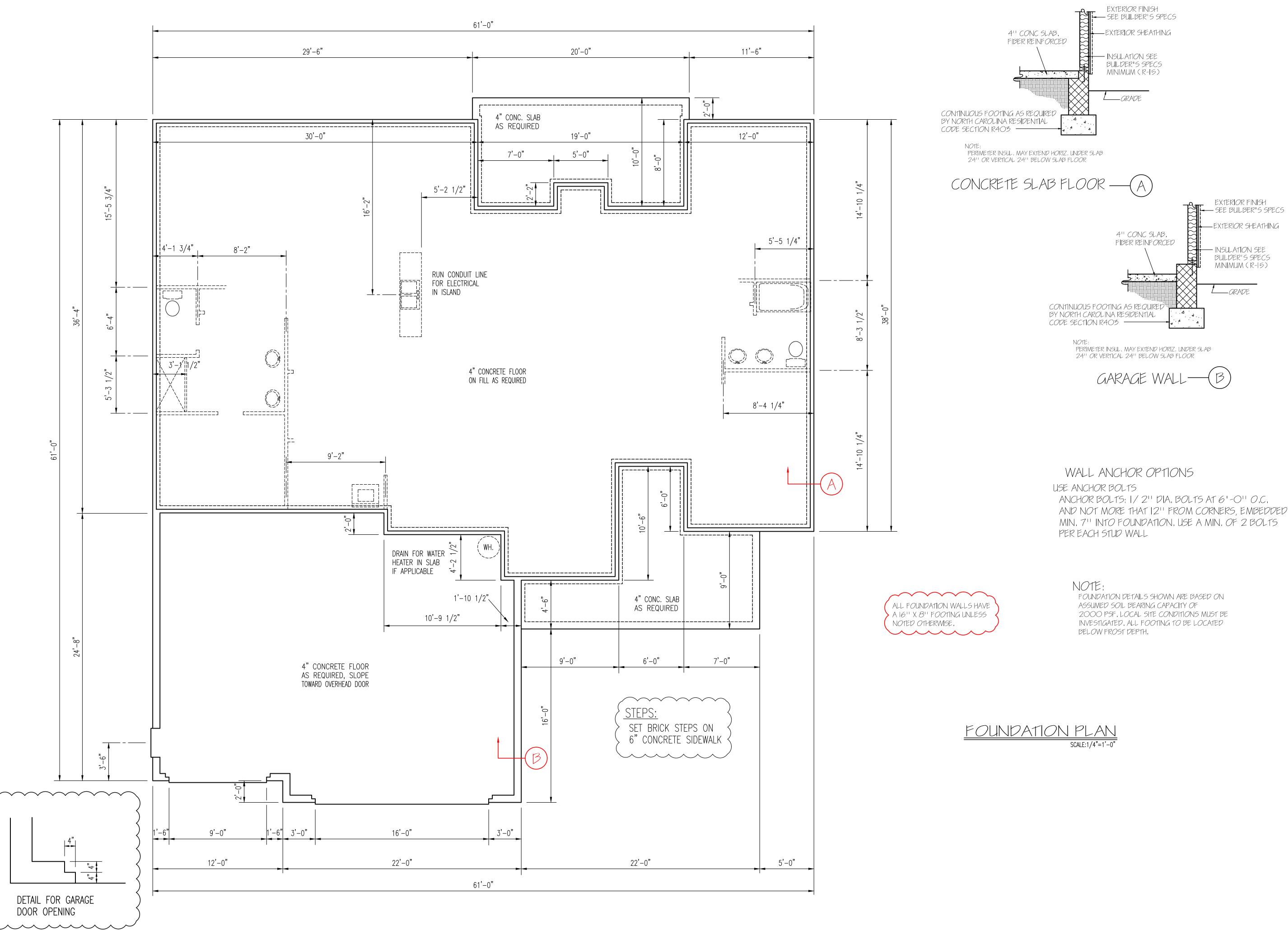
<u>OTHER AREAS</u>

85	SQ FT
194	SQ FT
92	SQ FT

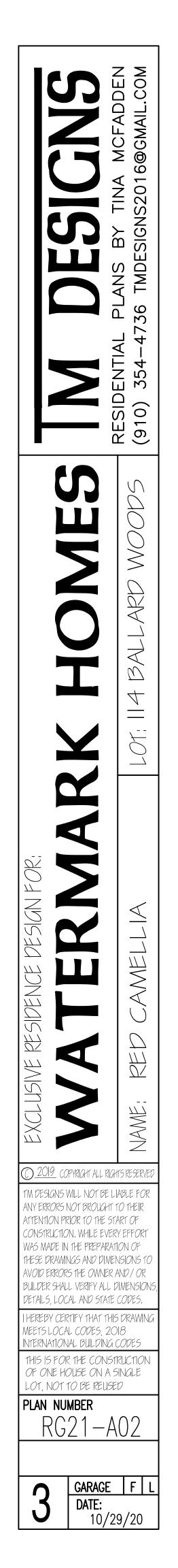
- FOUNDATION CRIPPLE WALLS SHALL BE FRAMED OF STUDS NOT SMALLER THAN THE STUDDING ABOVE. WHEN EXCEEDING 4 FT. IN HEIGHT, SUCH WALLS SHALL BE FRAMED OF STUDS HAVING THE SIZE REQUIRED FOR
- AN ADDITIONAL STORY. CRIPPLE WALLS WITH A STUD HEIGHT LESS THAN 14 INCHES SHALL BE CONTINUOUSLY SHEATHED ON ONE SIDE WITH WOOD STRUCTURAL PANELS FASTENED TO BOTH THE TOP AND BOTTOM PLATES IN ACCORDANCE WITH TABLE R602.3(1), OR CRIPPLE WALLS SHALL BE CONSTRUCTED OF SOLID BLOCKING.

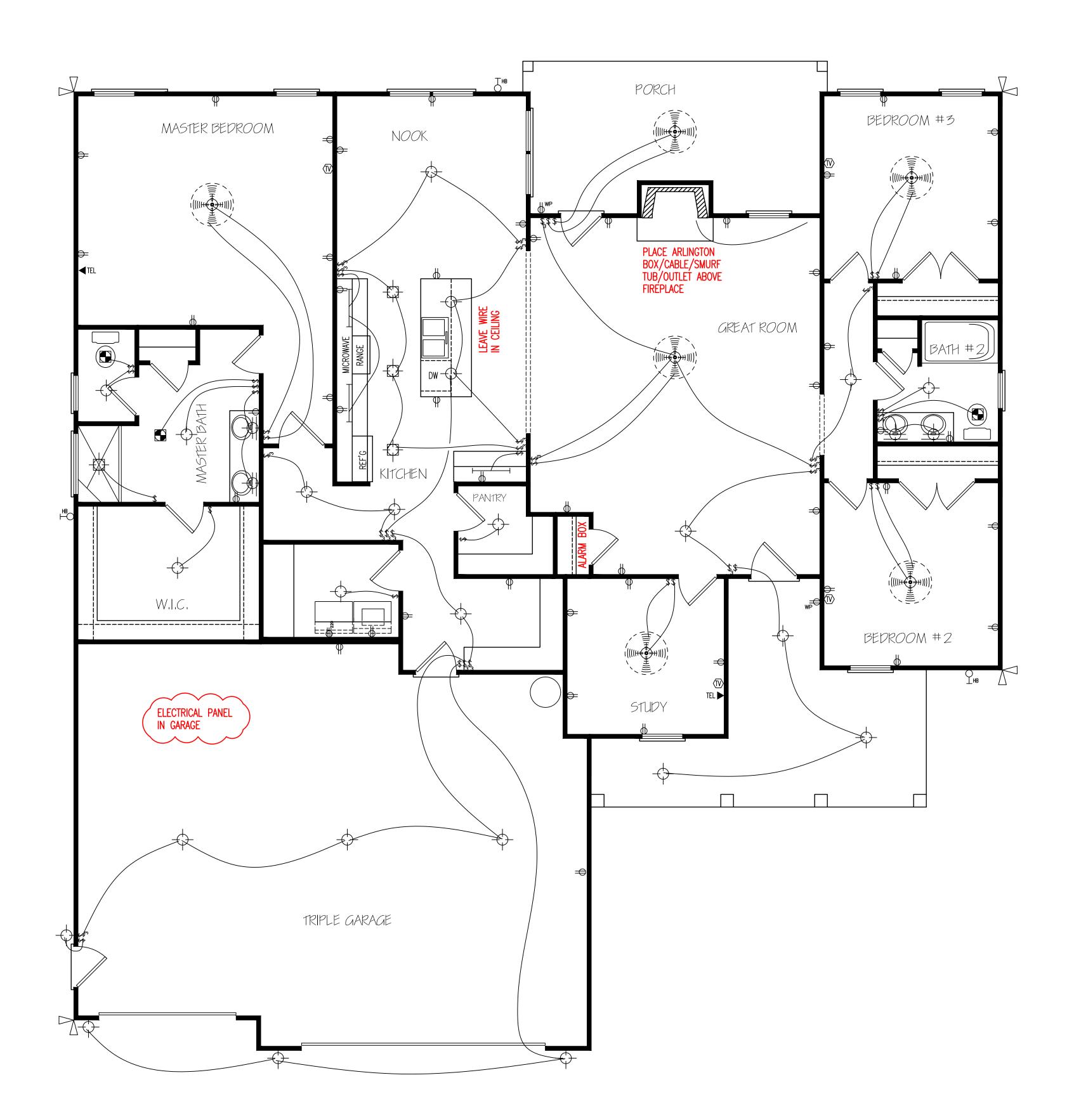
HERO PACKAGE



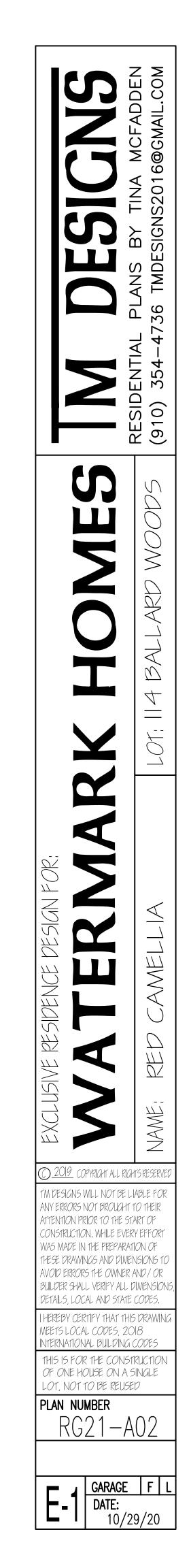


AND NOT MORE THAT 12" FROM CORNERS, EMBEDDED MIN, 7" INTO FOUNDATION, USE A MIN, OF 2 BOLTS





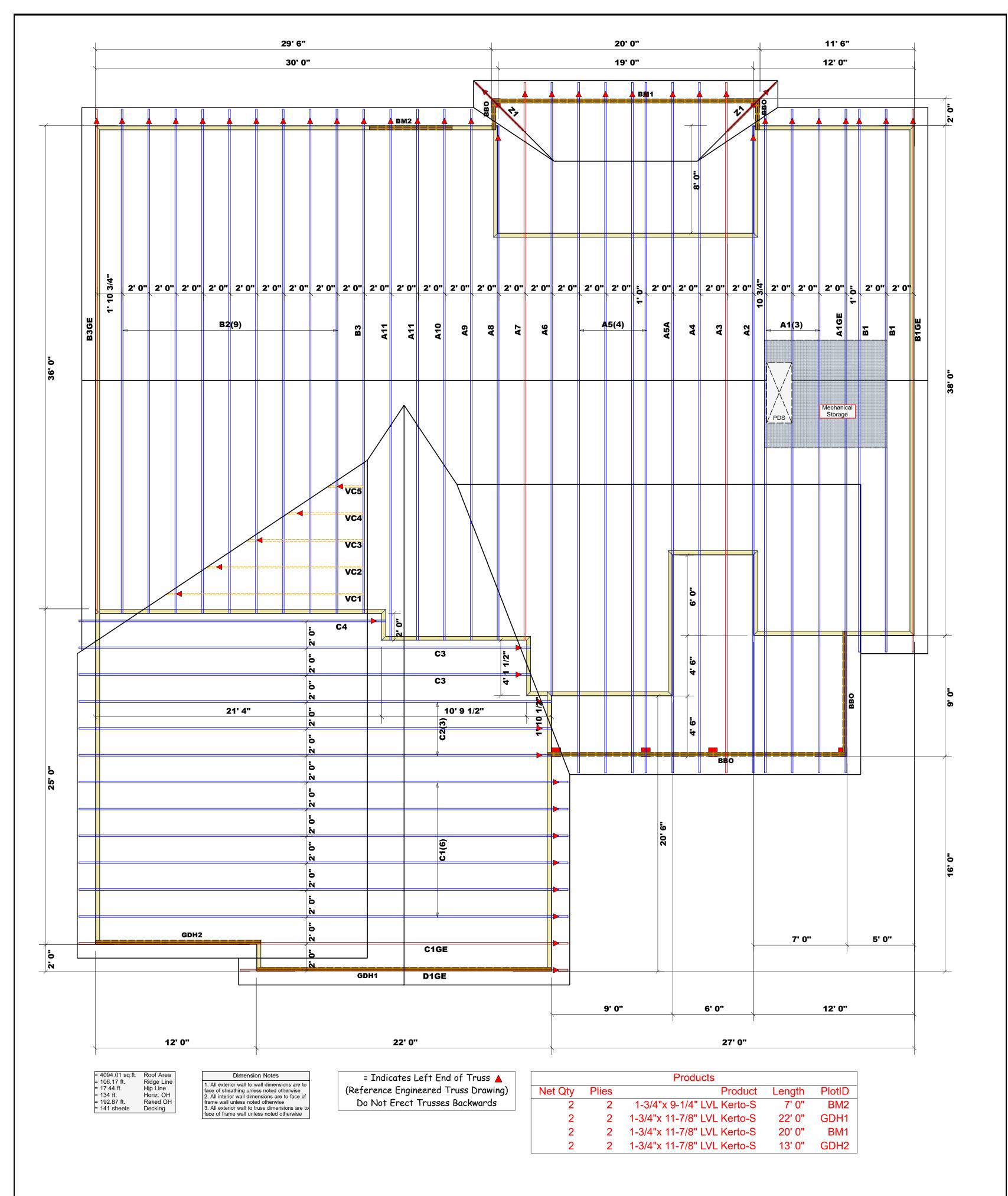




** UNDER CABINET LIGHTING NO SURROUND SOUND NO PREWIRE FOR CAMERA ~~~~~

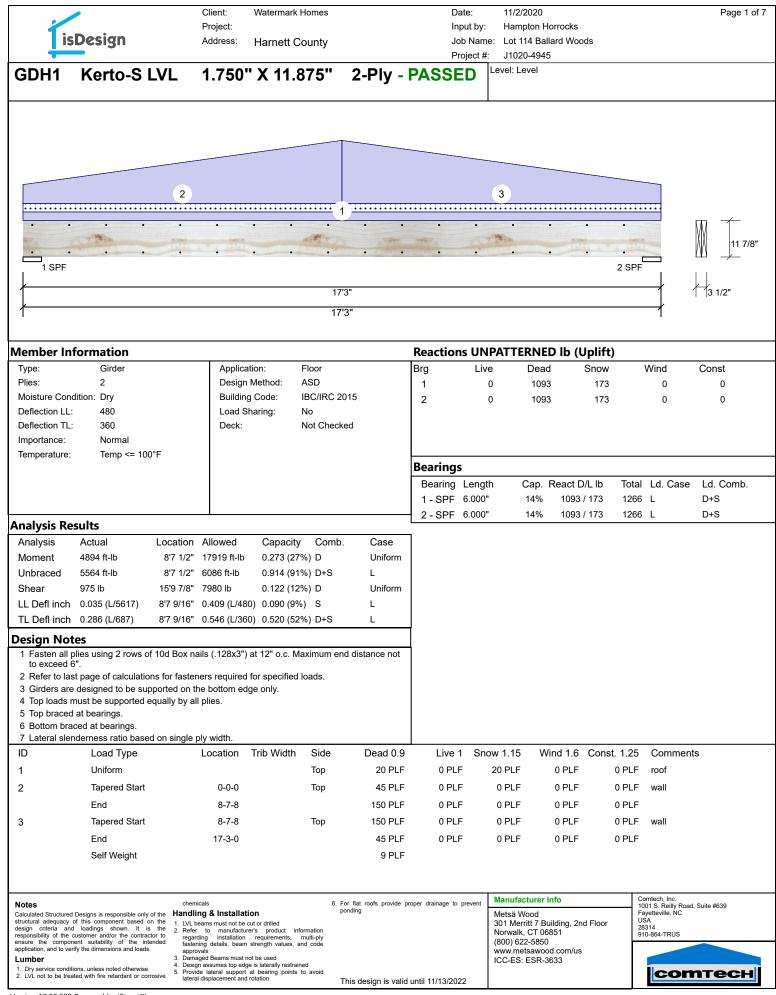
 \frown

HERO PACKAGE



Truss Placement Plan SCALE: NTS

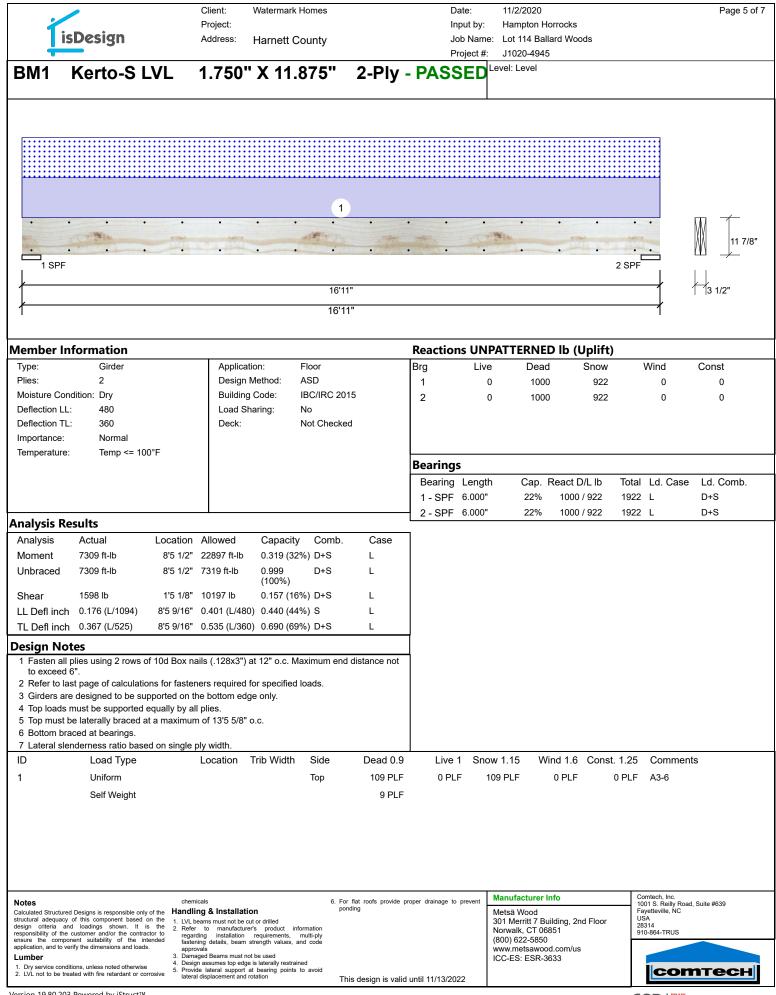
(BAS	HART FOR JAC ED ON TABLES R502.5(1) JACK STUDS REQUIRED	(& (b))	BUILDER	Watermark Homes	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is	
	HEADER/GIRDER	CTION 0) 0) 0) 0) 0) 0) 0) 0) 0) 00	JOB NAME	114 Ballard Woods	ADDRESS	Lot 114 Ballard Woods, Lillington, NC	responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	сотесн
END REACTION (UP TO) REQ D STUDS FOR (2) PLY HEADER	END REACTIG (UP TO) (REQ'D STUDS (3) PLY HEAI	END REAC (UP T (UP T) (4) PLY H	PLAN	Red Camellia GL	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6800 2 10200 3	SEAL DATE	10/17/20	DATE REV.	11/01/20	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 1500#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park
6800 4 8500 5 10200 6	102004127505153006	13600 4 17000 5	QUOTE #		DRAWN BY	Hampton Horrocks	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
119007136008153009			JOB #	J1020-4945	SALESMAN	Anthony Williams	signature Hampton Horrocks	Fax: (910) 864-4444



isDesign	Client: Watermark Home: Project: Address: Harnett County		Date: Input by: Job Name:	11/2/2020 Hampton Horrocks Lot 114 Ballard Woods	Page 2 of 7
			Project #:	J1020-4945	
GDH1 Kerto-S LVL	1.750" X 11.875	" 2-Ply - PASS		evel: Level	
· · · · ·	• • •	• • • • •	•	• • • • •	
			•		<u> </u>
1 SPF				2	
/ /		17'3"			3 1/2"
f		17'3"			
Multi-Ply Analysis Fasten all plies using 2 rows of 10d	Box nails (128x3") at 12'	oc Maximum end dist	ance not	to exceed 6"	
Capacity 0.0 %			ance not		
Load 0.0 PLF Yield Limit per Foot 163.7 PL					
Yield Limit per Fastener81.9 lb.Yield ModeIV					
Edge Distance 1 1/2"					
Min. End Distance 3" Load Combination					
Duration Factor 1.00					
Notes cher	micals	6. For flat roofs provide proper drainage	to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the 1. LVL	ling & Installation beams must not be cut or drilled	ponding	N 3	/letsä Wood 801 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA 28314
design criteria and loadings shown. It is the 2. Reference of the customer and/or the contractor to regge ensure the component suitability of the intended fast	er to manufacturer's product information arding installation requirements, multi-ply ening details, beam strength values, and code		N (1	Norwalk, CT 06851 800) 622-5850	28314 910-864-TRUS
application, and to verify the dimensions and loads. app Lumber 3. Dan 4. Dan	rovals naged Beams must not be used sign assumes top edge is laterally restrained		Ŷ	vww.metsawood.com/us CC-ES: ESR-3633	
1. Dry service conditions, unless noted otherwise 5. Prov	vide lateral support at bearing points to avoid ral displacement and rotation	This design is valid until 11/13/2	2022		соттесн

is	Design	Client: Project: Address:	Watermark Hor Harnett Cou			Date: Input by: Job Name: Project #:	11/2/2020 Hampton Horr Lot 114 Ballar J1020-4945				Page 3 o
GDH2	Kerto-S LV	L 1.750	" X 11.87	5" 2-PI	y - PASS		evel: Level				
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	C. P. Mar		at	fin m			With I		1000	Ă Ă	11
•						•				V V V	
1 SPI	F							2 SPF			
ĺ.				10'3"							1/2"
ſ				10'3"					1		
ember Inf	formation				Reacti	ons UNP	ATTERNED	lb (Uplift)	1		
ype: Plies:	Girder 2	Applic	ation: Flo n Method: AS		Brg 1	Live 0	Dead 411	Snow 103	Wi		st 0
Noisture Cond		-		/IRC 2015	2	0	314	103			0
Deflection LL:			Sharing: No								
eflection TL: mportance:	360 Normal	Deck:	Not	Checked							
emperature:	Temp <= 100°F										
	-				Bearin	gs					
						g Length		eact D/L lb	Total Lo		Comb.
						F 6.000" F 6.000"	6% 5%	411 / 103 314 / 103	514 L 416 L	D+S D+S	
nalysis Re											
Analysis <i>N</i> oment	Actual Loo 999 ft-lb	cation Allowed 4'11" 22897 ft-lb		Comb. Ca D+S L	se						
Jnbraced	999 ft-lb	4'11 22697 It-IL 4'11" 9857 ft-Ib	0.101 (10%)								
Shear	278 lb 1	'5 1/8" 7980 lb	0.035 (3%)		iform						
L Defl inch	0.004 5 (L/26994)	5'1 1/2" 0.234 (L/4	80) 0.020 (2%)	S L							
L Defl inch	. ,	11/16" 0.312 (L/3	60) 0.060 (6%)	D+S L							
esign Not	es										
	lies using 2 rows of 10d	l Box nails (.128x3') at 12" o.c. Maxir	num end distanc	e not						
2 Refer to las	t page of calculations fo			ds.							
	designed to be support nust be supported equal		dge only.								
5 Top braced	-										
	ced at bearings. Iderness ratio based on	single ply width.									
C	Load Type	Location	Trib Width	Side Dea	d 0.9 Liv	e 1 Snov	v 1.15 Wind	d 1.6 Const	. 1.25 C	comments	
	Uniform									oof	
	Tapered Start	0-0-0	Ţ							all	
	End Self Weight	10-3-0			5 PLF 0 I 9 PLF	PLF	0 PLF 0	PLF	0 PLF		
	Con Worght										
		at and a la		0 F == f =t === t			Manufacturer Info)		ch, Inc.	
	Designs is responsible only of the of this component based on the			 For flat roofs ponding 	provide proper drainage	to prevent	Metsä Wood		1001 S	5. Reilly Road, Suite #6 eville, NC	39
sign criteria and	loadings shown. It is the sustomer and/or the contractor to	1. LVL beams must not be 2. Refer to manufact regarding installation	urer's product informa n requirements, multi	ply		1	301 Merritt 7 Build Norwalk, CT 0685 (800) 622 5850		28314	4-TRUS	
auma th	ent suitability of the intended	fastening details, bear	m strength values, and c	ode			(800) 622-5850				
sure the compon plication, and to veri Imber	ify the dimensions and loads.	approvals 3. Damaged Beams must	-			,	www.metsawood.c				

New Control Market in the Decision of the Base Volume in the B		/		Client:	Watermark Homes	i	[Date:	11/2/2020		Page 4 of 7
Openant 1000-645 GDH2 Kerto-S LVL 1.750" X 11.875" 2-Piy - PASSED Law Low Image: Status of the status				Project:			I	nput by:	Hampton Horrocks		
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ODI/2 Keito-5 LvL 1.730 X 11.0/3 Z-Fig - PASSED Image: State of the state of					-		F	Project #:	J1020-4945		
Net Number of the second of the	GDH2	Kerto-S	IVI	1 750"	' X 11 875'	" 2-Plv	- PASS	FD ^I	Level: Level		
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Multi-Ply Analysis Fasten all plies using 2 rows of 10d Box nails (128x3") at 12" o.c. Maximum end distance not to exceed 6" Capacity 0.0% Multi busings Foot 103" Yead Kundings Foot 103" Weld Lind per Foot 103.7 PLF Yead Kundings Foot 10.7 PLF Weld Lind per Foot 10.7 PLF Weld Lind per Foot 10.0 Weld Stote 7 Joint Store 3" Joint Store 3" Joint Store 3" Joint Store 3" Min. End Octance 3" Joint Store 1.00 Network memory and the st	1 SF	۶F							2 SPF	/,	
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Notes Chemicals c. For hat rols provide proper drainage to prevent ponding Metsä Wood 1001 S. Relity Road, Suite #639 Calculated Structured Designs is responsible only of the structural adequacy of this component based on the responsibility of the culstomer and/or the contractor to ensure the component suitability of the culstomer and/or the contractor to ensure the component suitability of the culstomer and/or the contractor to ensure the component suitability of the culstomer and/or the equirements, multi-ply fastening details, beam strength values, and code application, and to verify the dimensions and loads. Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 USA 1. Dry service conditions, unless noted otherwise 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrined 9. Wing the far endergrad regression 0. Cremicals Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 USA 2. UV, not to be tracted with fire regarding the regression and to verify the dimensions and loads. 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrined 1. CC-ES: ESR-3633 Work metsawood.com/us ICC-ES: ESR-3633	Duration Factor		1.00								
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							Bearings	5					
							Bearing	Length	Cap.	React D/L lb	Total Ld. (Case Ld. Com	ıb.
							1 - SPF	-	54%		2788 L	D+S	
							2 - SPF		54%	1405 / 1383	2788 L	D+S	
nalysis R													
Analysis	Actual	Location		Capacity		Case							
Noment	3567 ft-lb		14423 ft-lb	0.247 (259	,	L							
Jnbraced	3567 ft-lb		11027 ft-lb	0.323 (329		L							
Shear	1858 lb		7943 lb	0.234 (239		L							
	h 0.027 (L/2419)) 0.200 (209	,	L							
L Defl inc	h 0.055 (L/1200)	3'	0.277 (L/240) 0.200 (209	%) D+S	L	-						
esign No							1						
	re designed to be sup blies must be fastened		-	-									
	s must be supported e	-			5.								
-	ed at bearings.												
	raced at bearings.	t on cinalo n	v width										
D Laterar si	enderness ratio based Load Type		-	Trib Width	Side	Dead 0.9	_ILive ^	1 Snov	w 1.15 V	Vind 1.6 Cons	125 Com	ments	
1	Uniform	'			Тор	461 PLF			61 PLF	0 PLF	0 PLF A10-		
1					104			40			SILI AIU-		
	Self Weight					7 PLF							
		chemica				r flat roofs provide	proper drainage to	prevent	Manufacture	Info	Comtech, Ir 1001 S. Re	illy Road, Suite #639	
lotes		of the Handlin	g & Installatio		ро	nding			Metsä Wood 301 Merritt 7 F	Building, 2nd Floor	Fayetteville USA	, NC	
alculated Structur tructural adequad	red Designs is responsible only only of this component based or	the 1. IVI bea	ams must not be out	it or drilled					SO I MOUTHLE / E	- ananing, 2110 1 1001	28314		
alculated Structur tructural adequad esign criteria a esponsibility of th	cy of this component based or and loadings shown. It is a customer and/or the contract	the 1. LVL beauthe 2. Refer	ams must not be cu to manufacturer ng installation	r's product info	rmation nulti-plv			1	Norwalk, CT 0	6851	28314 910-864-TF	RUS	
alculated Structur tructural adequad esign criteria a esponsibility of th nsure the comp	y of this component based or and loadings shown. It is	the 1. LVL beautries the 2. Refer	to manufacturer ng installation ng details, beam s		nulti-ply			1	Norwalk, CT 0 (800) 622-585	6851 0		RUS	
alculated Structur ructural adequac esign criteria a sponsibility of th sure the comp oplication, and to umber	cy of this component based or and loadings shown. It is a customer and/or the contract conent suitability of the inte	n the the or to nded 1. LVL bea 2. Refer regardir fastenir approva 3. Damag 4. Design	to manufactured ng installation ng details, beam s als ed Beams must noi assumes top edge	r's product info requirements, r strength values, ar	nulti-ply id code ed				Norwalk, CT 0	6851 0 od.com/us	910-864-TF		